Subsurface Conditions Data Report Phase 5 Fill and Subgrade Improvement Third Runway Embankment Sea-Tac International Airport SeaTac, Washington



Prepared for HNTB

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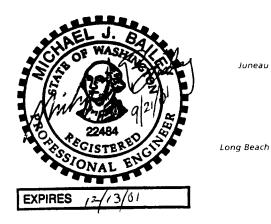


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Prepared by Hart Crowser, Inc.



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SUBSURFACE CONDITIONS DATA REPORT PHASE 5 FILL AND SUBGRADE IMPROVEMENT THIRD RUNWAY EMBANKMENT SEA-TAC INTERNATIONAL AIRPORT SEATAC, WASHINGTON

INTRODUCTION

This data report compiles information on subsurface conditions, to support the Phase 5 construction for the Third Runway Embankment Project at the Sea-Tac International Airport. A list of documents, which include original presentation of the geotechnical and hydrogeologic field and laboratory testing data, is provided in the References section.

The site is located at the Sea-Tac International Airport (STIA), in SeaTac, Washington. A vicinity map presented on Figure 1 shows the general area; Figures 2 and 3, Site and Exploration Plan - Phase 5 Work Area West and Work Areas South, show the proposed work areas and existing exploration locations.

We have organized this report into several sections.

- The main text begins with a summary of the geologic units followed by a more detailed discussion of subsurface conditions.
- This is followed by a discussion of the groundwater conditions and available hydrogeologic information.
- Appendices A and B follow the main text and present results of Hart Crowser's subsurface explorations and laboratory testing, respectively.
- Appendix C presents exploration and test results reported by others.
- Appendix D provides specific information on peat soils in the area.

Purpose and Scope of this Report

This report provides information on subsurface soil and groundwater conditions for the planned Phase 5 construction for the Third Runway. We anticipate Phase 5 construction will include placement of compacted embankment fill and subgrade improvement (overexcavation and replacement with compacted fill, in the area west of the airfield between areas between South 154th and South 188th Street. Sources of the fill include on-site stockpiles and some excavation on the airfield. Soils in Borrow Area 4, located south of the airfield, are described in a separate report (Hart Crowser 2001). Additional fill sources off site may also be used in accordance with the contract documents. The information presented herein provides the basis for geotechnical engineering aspects of design and should be made available to prospective bidders prior to construction.

Information presented herein was obtained in general accordance with Tasks 1.03, 1.13, and 1.3.5 in Modification No. 8 to Hart Crowser's contract with HNTB dated May 1, 1998.

GENERALIZED GEOLOGIC DESCRIPTION AND SUBSURFACE SOIL CONDITIONS

This section provides a description of the geologic and subsurface soil conditions within the Phase 5 Fill areas, shown on Figures 2 and 3, based on explorations at the site by Hart Crowser and others.

Generalized Geologic Conditions

Various existing reports (AGI 1998 and Hart Crowser 1999a, 1999b, 2000a, 2000b, 2000c, 2000d, and 2000e) describe the generalized geologic conditions for areas along the proposed runway, including parts of the areas covered by this report. In summary, the following geologic units have been identified at the Third Runway project site:

- Fill (loose to medium dense, locally dense, variably graded silt, sand, and gravel);
- Alluvium (primarily soft to stiff, peat, clay, and silt; and very loose to medium dense, fine to medium non-silty to silty sand);
- Recessional Outwash (primarily medium dense to dense, silty sand and gravel, and/or soft to medium stiff, sandy silt and/or sandy clay);
- Glacial Till (dense to very dense, silty sand and gravel); and
- Advance Outwash (dense to very dense, non-silty to silty sand and gravel and very stiff to hard silt and clay).

Subsurface Conditions

Subsurface soil conditions interpreted from materials encountered in explorations at the site and soil properties inferred from laboratory tests formed the basis for the information contained in this report. Variations between explorations occur due to the variability in gradation, moisture content, and density/consistency of soils at the site. The nature and extent of these variations may not become evident until construction. If variations become evident, it will be necessary to re-evaluate our interpretation of the soil conditions at the site, as well as any recommendations based on those interpretations.

This report discusses the subsurface conditions in three main work areas: Work Area West, Work Area South 1, and Work Area South 2 as shown on the 60 percent draft Phase 5 Construction Plans and the enclosed Figures 2 and 3. Subsurface conditions in these three areas are addressed separately.

Subsurface conditions are also described in the vicinity of stormwater detention ponds D and G, which may be modified as part of Phase 5 construction.

In this report, "pre-construction fill" refers to fills placed prior to about 1997 for which no information is available other than indicated by exploration and test results presented herein. Fill placed since 1997 were constructed as part of the Third Runway project by the Port of Seattle.

Work Area West

This is the largest of the five Phase 5 Fill work areas. Work in this area will include:

- Cuts and fill placement;
- Embankment construction; and
- Overexcavation and replacement of subgrade soils.

The following soils were encountered in Work Area West:

Topsoil. This soil was not consistently encountered in our explorations. Typically, this soil consists of a loose mixture of silt and sand with roots and other organic material. Topsoil is generally 1/2 to 1 foot thick where encountered. **Pre-Construction Fill, consisting of loose to medium dense, variable mixture of silt or clay, sand, and gravel.** Fill soils encountered in Work Area West are typically associated with prior site use including paved streets and residential housing. Buried utilities and other features such as possible abandoned wells or underground storage tanks could be encountered. Fill is generally absent in the low-lying portions of the site, adjacent to wetlands. Most of the fill is less than 1 foot thick. The density and granular nature of the fill materials resembles the recessional outwash deposits and the fill is sometimes difficult to distinguish from the outwash.

Other fill in the area was placed as compacted embankment or stockpiled fill soils placed by STIA contractors since 1997. Typically the embankment and stockpiled fill consists of dense, non-silty to silty, sand and gravel with cobbles.

Alluvial deposits consisting of interlayered silt, clay, sand, and peat. These soils occur mainly in the low-lying areas to depths of up to about 15 feet.

The consistencies of the clay and silt deposits vary widely from soft to stiff. The clays and silts generally contain sand fractions ranging from slightly sandy to sandy. The existing alluvial sands are generally loose to medium dense.

Peat was encountered in the wetlands in the west central part of Work Area West, around Runway Station 180+00. Both surficial and shallow buried peat deposits were encountered in this area. Buried deposits tend to be medium stiff to stiff, whereas the surficial peat exhibited consistencies in the very soft to soft range. Buried peat deposits were encountered at depths ranging from 3.5 to 9.5 feet and varied in thickness between 1.5 and 5.5 feet. Peat deposits near the ground surface varied in thickness between a few inches and about 2 feet. A detailed discussion of the extent and characteristics of the peat deposits in Work Area West (and in the North Safety Area, which is not part of Phase 5) is included in Appendix D.

Colluvium and Recessional Outwash consisting of medium dense to dense, slightly silty to silty, slightly gravelly to gravelly sand. Recessional outwash overlies the glacial till, or advance outwash where the glacial till has been eroded. Thickness of the colluvium and recessional deposits varies over the site, but is generally less than 20 feet. These deposits are generally intermittent or may be absent where alluvial materials are located and dense to very dense glacial till or advance outwash sand and gravel underlies the alluvium.

Glacial Till consisting of dense to very dense, slightly gravelly to gravelly, silty to very silty sand. In general, Glacial Till differs from the overlying recessional

soils by having a higher silt content and much higher density. The top of the glacial soils is generally within 10 to 20 feet of the ground surface.

Glacial till is generally encountered near the surface of the west facing slope on the east side of Work Area West. The till is absent, downslope to the west, where the advance outwash soils are exposed. Springs and seeps occur along the western edge of the till due to both perched water and interflow above the till horizon as well as seepage through the underlying advance sands.

Figure 4 presents an elevation contour map of the highest measured groundwater levels in this area. Groundwater conditions are discussed later in this report.

Advance Outwash consisting of dense to very dense, slightly silty, slightly gravelly to gravelly sand. In general, the advance outwash can typically be distinguished from the glacial till by lower silt content. However, observations at the site suggest that some areas of advance outwash are silty.

Portions of the advance outwash include hard silt and clay soils. Below a depth of about 30 feet these soils have been reported to be part of the Lawton Silt and Clay or "Pre-Vashon Deposition." These hard soils may be laminated or contain planes of separation (partings). Furthermore, these deposits are typically reported to be relatively plastic and are often slickensided (i.e., showing evidence of previous failure planes).

Work Area South 1

The majority of the work in this area will consist of excavating an existing fill stockpile and cutting to proposed ground elevations. Figures 5 and 6 show the location of subsurface explorations in Work Area South as well as initial estimates of the depth of excavation (refer to contract documents for specifics). Available information on soils to be excavated in Work Areas South 1 and 2 is summarized in Tables 4 and 5. The subsurface conditions in this area are generally as follows:

Topsoil. Typically, topsoil up to about 1 to 1.5 feet thick was reported in some previous explorations in this area, but was typically not described in the logs.

Pre-construction fill consisting of very loose to very dense, silty sand with asphalt, and occasional gravel and organics. Pre-construction fill consisting of silty sand with gravel and some asphalt concrete and organic debris was encountered in only two borings. Boring AT97-B14 was drilled 5 feet away from boring AT97-B13 after the initial boring was abandoned due to the presence of

debris. The borings are located at the very north end of this work area, adjacent to an access road. The fill depth at this location was 22 feet.

No other information is available on the pre-construction fill in this area.

Recessional Outwash consisting of medium dense, slightly silty sand. An approximately 3-foot-thick layer of Recessional Outwash was encountered in test pit AT94b-TP5. This was the only exploration within this work area that encountered Recessional Outwash.

Glacial Till consisting of very dense, silty sand with occasional gravel. This soil unit generally was encountered at very shallow depths in the majority of the explorations within this work area. However, borings AT97-B8 and AT97-B33 encountered Advance Outwash without encountering Till. Some borings contained up to 13 feet of very dense Weathered Till overlying very dense unweathered Till.

Advance Outwash consisting of dense to very dense sand with minor amounts of gravel and silt. This soil unit typically underlies the Glacial Till.

Work Area South 2

Construction in Work Area South 2 is anticipated to consist of removing existing stockpiled fill for use in the embankment and excavation of existing native soils to grade. Location of the stockpiles, available explorations and anticipated cut depths are shown on Figure 3.

The following soil units were encountered during explorations performed by AGI Technologies in 1994 and 1997. The recent fill stockpiles were placed after AGI's field exploration in this area and were therefore not encountered in their explorations.

Pre-construction fill consisting of very loose to very dense, non-silty to silty sand. This layer was encountered in <u>all</u> of the explorations in this work area. The fill thickness ranges from about 10 to 48 feet (some explorations were not advanced deep enough to encounter natural soil deposits). The fill material appears to be predominantly glacial till type soils, likely obtained during grading of portions of the airfield in 1961 or 1962 (AGI 1999). Occasional organics, and wood and concrete debris were also encountered.

Glacial Till consisting of dense to very dense, silty sand with some gravel and occasional cobbles. This layer was generally encountered in the borings that were advanced deep enough to encounter natural soils. In some areas, the

Glacial Till layer might be thin or non-existent. In boring AT97-B15 for example, no glacial till was encountered below the recent fill, which was underlain by Advance Outwash.

Advance Outwash consisting of dense to very dense, non-silty to slightly silty sand. Advance Outwash was encountered in borings AT94b-B2 and AT97-B15. This soil unit is generally overlain by Glacial Till except where the Glacial Till was removed, e.g., by erosion.

Stockpiles 1 and 2

Eight test pits were completed to assess soils in Stockpile Nos. 1 and 2 located within Work Area South 2. Table 6 presents a summary of soils observed. Conversations with the field inspector who observed stockpile construction for the Port of Seattle indicates that Stockpile No. 1 consists mostly of non-silty gravely sand, with a separate area of silty sand and gravel (Till fill), while Stockpile No. 2 consists of a mixture of silty sand and gravel (Till fill). Visual classification of the test pit soils and soil gradation test results concur with this description. Gradation tests for the non-silty portion of Stockpile No. 1 indicate this material would be suitable for use within the embankment underdrain.

Pond D

Subsurface conditions in this area appear to be variable Alluvial and Colluvial soils over Advance Outwash soils. The following soil units have been identified:

- Loose to dense, non-silty to very silty, non-gravelly to very gravelly sand with interbedded occasional hard silt layers.
- Hard SILT in HC00-TP138 (from the ground surface to the bottom of the test pit at 13 feet) and in HC00-B147 (below 23 feet).
- Very dense, silty, gravelly sand.

Pond F

Subsurface conditions generally consist of sandy pre-construction fill overlying Glacial Till or Advance Outwash soils. Test pit HC00-TP318 encountered a layer of stiff, very sandy silt between depths of 6 and 12.5 feet.

Loose to medium dense, silty, gravely sand fill with various amounts of organic material and concrete (1 to 5 feet thick).

- Medium dense to dense, slightly silty to silty, slightly gravelly to gravelly sand (2 to 9 feet thick).
- Dense to very dense, silty, slightly gravelly to gravelly sand.

GENERALIZED HYDROGEOLOGIC CONDITIONS IN WORK AREAS

Groundwater is encountered in discontinuous zones perched on the Glacial Till and the Advance Outwash soils. The Advance Outwash, also known as the Shallow Regional Aquifer, discharges to Miller and Des Moines Creeks, and via underflow to Puget Sound and the Green River valley (AGI 1996).

The following sections summarize water level data and hydraulic conductivity data collected in the three Phase 5 Work Areas and at Ponds D and F. Water levels observed in open borings at the time of drilling (ATD) and seepage observed in test pits are shown on the exploration logs (Appendices A and F). Water levels observed in monitoring wells are presented in Tables 1 and 2.

Work Area West

Table 1 presents water level measurements in 24 wells in Work Area West. Elevation contours for the highest measured groundwater levels are shown on Figure 4.

Typically up to about 3 feet of seasonal fluctuation has been observed in wells close to Miller Creek, with less groundwater fluctuation in wells located in upland areas east of the creek.

Dewatering will be required during excavation of unsuitable subgrade soils, to enable removal of all soft or loose soils within the designated area, and backfill with compacted fill. Magnitude and rate of flow will vary locally due to changes in gradation and density of the soils.

Seepage and wet soils are typically observed at the surface in wetlands in some areas where fill placement or subgrade improvement (overexcavation and replacement) is anticipated. East of this area (east of the former 12th Avenue South) artesian conditions were observed in two wells (AT94A-B3 and AT96-B4). Artesian pressures are likely sustained by recharge occurring in higher elevation areas of the existing airport area to the east.

Hydraulic Conductivity Testing

Slug testing was performed in four wells (HC99-B37 through HC99-B40) within or adjacent to Work Area West. The mean hydraulic conductivity was 1.1×10^{-4} cm/sec. Hydraulic conductivity values are presented in Table 2, along with soil material type observed within the screened interval.

Work Area South 1

Water level data from wells AT97-B8 and AT97-B14 are presented in Table 3. AGI and Hart Crowser advanced several other explorations (borings and test pits) in this area to depths ranging from 8 to 19.5 feet. Only a few of these explorations encountered groundwater. At the time of drilling/excavation, groundwater was encountered at elevations ranging from 356 to 367.5 feet, generally at least 8 feet below the final proposed ground surface. Bottom elevations of the explorations that did not encounter groundwater were generally at least 7 feet below the final proposed ground surface.

Based on available data, cuts within most of Work Area South 1 will likely not encounter groundwater. However, groundwater might be encountered near the wetland area near boring/well AT97-B8, where high water levels measured in this well are near the final proposed ground surface elevation.

Note that water levels vary with time and may rise above elevations reported herein.

Work Area South 2

There are no monitoring wells located in Work Area South 2. Therefore, no long-term water level readings are available for this area.

Several borings in this area were advanced to depths ranging from 14.5 to 34.5 feet. Only the deepest boring (AT97-B18) encountered groundwater at the time of drilling at elevation 329 feet. Bottom of boring elevations range from 301 to 359.5 feet and are generally below proposed final ground surface elevations. Water level observations at the time of drilling may not accurately represent water table conditions, and may vary over time.

Test pit AT94b-TP16 encountered water at elevation 363 feet, approximately 3 feet above the proposed final ground surface elevation at this location. Based on observation in the other explorations in this area, this seepage probably represents a local perched water zone of limited extent.

Pond D

There are no monitoring wells in this area. Some seepage was observed during excavation of some of the test pits between about 341 and 350 feet in elevation. These likely represent local perched water zones of limited extent. No water was encountered during drilling of boring HC00-B147, which was advanced to a depth of 39 feet.

One infiltration test in a test pit at Pond D indicated good infiltration or seepage potential, but no infiltration was observed in other tests attempted at Pond D (Hart Crowser 2000f).

Pond F

No groundwater was encountered during drilling and excavation of the explorations. Borings HC01-B401, HC01-B402, and HC01-B403 were advanced to depths of 24.3, 24.4, and 28.5 feet, respectively. Some seepage was observed in test pit HC00-TP202. We believe that the seepage was caused by perched water in this area. No monitoring wells are located in this area.

USE OF THIS REPORT

This report has been prepared for the exclusive use of HNTB Corporation and the Port of Seattle for the site and project described herein. Hart Crowser's work has been accomplished and report prepared, in accordance with generally accepted geotechnical engineering practices for the nature and conditions of the work completed in the same or similar localities at the time the work was completed. No other warranty, express or implied, is made.

Hart Crowser appreciates the opportunity to provide this information. Please call if you have any questions.

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	AT94	AT94A-B3	AT96	6-B4	AT97-B69	-B69	HC99-B37	-837	HC99-B38	-B38	HC99-B39	-B39	HC99-B40	B40
	Depth*	ш	Depth*	Elevation	Depth*	Elevation	Depth*	Elevation	Depth*	Elevation	Depth*	Elevation	Depth*	Elevation
	in Feet	in Feet	in Feet	in Feet	in Feet	in Feet	in Feet	in Feet	in Feet	in Feet	in Feet	in Feet	in Feet	in Feet
Measuring Point	00.0	2	00.0	5	0.00	337.2	0.00	237.65	00.0	230.88	0.00	230.80	0.00	250.63
Ground Level*	1.4		-0.3	280	3.2	334	3.1	234.6	3.3	227.6	-0.3	231.1	2.0	248.6
Top of Screen*	23.4		36.7	243.0	27.7	310	9.1	228.6	12.3	218.6	4.7	226.1	14.0	236.6
Bottom of Screen*	33.4	240.0	46.7	233.0	29.7	308	19.1	218.6	22.3	208.6	14.7	216.1	24.0	226.6
							-							
Date: 3/8/1999					ł	ł	3.52	234.13	4.40	226.48	0.69	230.11	4.88	245.75
3/10/1999					6.18	331.0								
4/5/1999					6.59	330.6	3.58	234.07	4.41	226.47	0.74	230.06	5.26	245.37
5/4/1999					7.43	329.8	3.82	233.83	4.60	226.28	0.86	229.94	5.75	244.88
5/15/1999					1	;								
6/14/1999					8.08	329.1	5.12	232.53	5.90	224.98	1.68	229.12	6.89	243.74
7/13/1999					8.41	328.8	4.72	232.93	5.93	224.95	2.05	228.75	7.18	243.45
8/13/1999					8.83	328.4	5.70	231.95	6.08	224.80	2.18	228.62	7.13	243.50
9/14/1999					9.16	328.0	6.47	231.18	6.48	224.40	2.51	228.29	7.67	242.96
10/13/1999					9.12	328.1	4.50	233.15	5.98	224.90	2.09	228.71	7.32	243.31
11/11/1999					8.13	329.1	3.22	234.43	4.25	226.63	2.90	227.90	5.80	244.83
12/9/1999					6.80	330.4	3.27	234.38	4.38	226.50	0.27	230.53	5.00	245.63
1/13/2000					6.48	330.7	3.20	234.45	4.35	226.53	0.54	230.26	4.86	245.77
2/14/2000	i				6.54	330.7	3.12	234.53	4.33	226.55	0.59	230.21	4.49	246.14
3/9/2000	Flowing	>272	Flowing	>280	6.82	330.4	3.17	234.48	4.43	226.45	0.61	230.19	5.57	245.06
4/11/2000		>272	Flowing	>280	7.45	329.8	3.35	234.30	4.60	226.28	0.88	229.92	5.08	245.55
5/10/2000	Flowing	>272	Flowing	>280	7.78	329.4	3.19	234.46	4.32	226.56	0.88	229.92	5.14	245.49
6/19/2000	-7.15	280.6	Flowing	>280	8.40	328.8	3.76	233.89	4.91	225.97	1.15	229.65	6.01	244.62
7/10/2000	-6.00	279.4	-1.73	281.4	8.84	328.4	3.96	233.69	5.72	225.16	1.61	229.19	6.50	244.13
10/10/2000	-1.38	274.8	-0.81	280.5	9.90	327.3	3.84	233.81	5.99	224.89	2.17	228.63	8.39	242.24
1/22/2001	-8.30	281.7	-2.35	282.1	7.82	329.4	3.30	234.35	4.42	226.46	0.79	230.01	5.25	245.38
5/4/2001	-8.54	281.9	1.04	278.7	8.93	328.3	3.31	234.34	4.58	226.30	1.05	229.75	5.44	245.19

Table 1 - Water Level Data - Work Area West

Depth* All depths are below measuring point (NOT below the ground surface) -- Indicates data not available.

Italics = Estimated

Sheet 2 of 4

125	Elevation	In reet	0.102	258.2	254.2	249.2																254.06	253.91	253.01	261 41	250.07	252 14	252.73	-
HC00-B125				-0.4	3.6	8.6																3.74	3.89	4 79	6.30	6.83	5.66	5.07	
3123	Elevation	IN F 661	10.00	234.7	223.7	213.7				<u></u>										-		235.58	235.74	235.39	235.03	235.05	235.74	235.64	-
HC00-B123			0.0	6.Z	14.0	24.0																2.06	1.90	2.25	2.61	2.59	1 90	2.00	
B121	Elevation	111 FEEL		229.1	225.0	215.0																230.98	231.16	230.69	230.25	230.25	231.26	231.22	-
HC00-B121			•	Z.1	6.8	16.8																0.80	0.62	1.09	1.53	1.53	0.52	0.56	
B120	Elevation	111 FUEL	00.00-	234.0	219.3	214.3	 															231.83	232.01	231.41	230.83	230.70	232.06	231.96	•
HC00-B120	Depth* in Eact			N .N	17.6	22.6																5.1	4.92	5.52	6.1	6.23	4.87	4.97	
B118	Elevation	298.61	2077	231.1	291.7	286.7														291.98	291.90	290.77	290.97	290.09	289.49	288.76	292.24	290.86	
HC00-B118	Depth* in East	000		-	7.0	12.0														6.63	6.71	7.84	7.64	8.52	9.12	9.85	6.37	7.75	
B111	Elevation	286.06	285 2	0.002	276.3	266.3														279.26	279.12	277.72	277.47	276.89	276.11	275.23	279.49	278.29	
HC00-B111	Depth* in East	0.00	80	0.0	9.8	19.8														6.80	6.94	8.34	8.59	9.17	9.95	10.83	6.57	7.77	
B106	Elevation	315.81	314 1		304.1	294.1														310.32	310.31	309.60	309.43	308.77	308.34	307.29	308.79	308.12	
HC00-B106	Depth* in Feet	0.00	17		11.7	21.7														5.49	5.50	6.21	6.38	7.04	7.47	8.52	7.02	7.69	
		Measuring Point	Ground Level*		1 op of Screen*	Bottom of Screen*	1 AIE 2/0/ 1999	3/10/1999	4/5/1999	5/4/1999	5/15/1999	6/14/1999	7/13/1999	8/13/1999	9/14/1999	10/13/1999	11/11/1999	12/9/1999	1/13/2000	2/14/2000	3/9/2000	4/11/2000	5/10/2000	6/19/2000	7/10/2000	10/10/2000	1/22/2001	5/4/2001	

Table 1 - Water Level Data - Work Area West

Italics = Estimated Depth* All depths are below measuring point (NOT below the ground surface) -- Indicates data not available. Hart Crowser 497828\Tables1&2&3WaterLevels&slugtesting.xis - Table 1-Work Area West

_	tion	3et	258.64	257.8	245.8	235.8								-						250.70	249.73	248.43	247.89	247.14	246.76	245.65	249.21	248.32
HC00-B141	Elevation	in Feet																										
HC00	Depth*	in Feet	00.0	0.0	12.9	22.9														7.94	8.91	10.21	10.75	11.50	11.88	12.99	9.43	10.32
B137	Elevation	in Feet	267.21	264.5	257.5	249.5																		264.67	263.09			
HC00-B137	Depth*	in Feet	0.00	2.8	9.8	17.8																		2.54	4.12			
B133	Elevation	in Feet	243.47	241.1	236.1	231.1	·													240.96	241.03	240.94	240.86	239.50	238.46	237.53	240.76	240.71
HC00-B133	Depth*	in Feet	0.00	2.4	7.4	12.4														2.51	2.44	2.53	2.61	3.97	5.01	5.94	2.71	2.76
B132	Elevation	in Feet	229.96	227.4	216.4	211.4											<u> </u>				<u> </u>	227.31	227.41	227.05	226.72	226.67	227.38	227.33
HC00-B132	Depth*	in Feet	0.00	2.6	13.6	18.6																2.65	2.55	2.91	3.24	3.29	2.58	2.63
B130	Elevation	in Feet	225.46	223.1	218.1	214.1																223.73	223.79	223.54	223.32	223.58	223.99	223.90
HC00-B130	Depth*	in Feet	0.00	2.3	7.3	11.4																1.73	1.67	1.92	2.14	1.88	1.47	1.56
B129	Elevation	in Feet	245.83	243.2	236.2	231.2														242.76	242.68	242.24	242.23	241.27	240.82	239.23	241.08	241.89
HC00-B129	Depth*	in Feet	0.00	2.6	9.6	14.6														3.07	3.15	3.59	3.6	4.56	5.01	6.6	4.75	3.94
B126	Elevation	in Feet	251.56	250.2	243.2	238.2														249.54	249.74	249.59	249.49	249.02	248.88	246.41	249.16	249.37
HC00-B126	Depth*	in Feet	0.00	1.4	8.4	13.4														2.02	1.82	1.97	2.07	2.54	2.68	5.15	2.40	2.19
			Measuring Point	Ground Level*	Top of Screen*	Bottom of Screen*	Date: 3/8/1999	3/10/1999	4/5/1999	5/4/1999	5/15/1999	6/14/1999	7/13/1999	8/13/1999	9/14/1999	10/13/1999	11/11/1999	12/9/1999	1/13/2000	2/14/2000	3/9/2000	4/11/2000	5/10/2000	6/19/2000	7/10/2000	10/10/2000	1/22/2001	5/4/2001

Table 1 - Water Level Data - Work Area West

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Italics = Estimated Depth* All depths are below measuring point (NOT below the ground surface) -- Indicates data not available. Hart Crowser 497828\Tables1&2&3WaterLevels&slugtesting.xls - Table 1-Work Area West

Sheet 3 of 4

	HC00	HC00-B142	HC00	HC00-B143	HC00	HC00-B144	HC00-	HC00-B145	HC00	HC00-B146
	Depth*	Elevation								
	in Feet	in Feet								
Measuring Point	0.00	272.72	0.00	242.27	0.00	248.99	0.00	265.11	0.00	263.55
Ground Level*	2.7	270.1	3.2	239.1	2.4	246.6	2.3	262.8	2.9	260.7
Top of Screen*	17.2	255.6	7.2	235.1	8.9	240.1	12.3	252.8	11.9	251.7
Bottom of Screen*	22.2	250.6	12.2	230.1	13.9	235.1	17.3	247.8	16.9	246.7
		-								
Date: 3/8/1999										
3/10/1999				•						
4/5/1999										
5/4/1999										
5/15/1999										
6/14/1999										
7/13/1999										
8/13/1999										
9/14/1999										
10/13/1999										
11/11/1999										
12/9/1999										
1/13/2000										
2/14/2000							2.94	262.17		
3/9/2000	3.27	269.45			2.98	246.01	3.14	261.97	3.81	259.74
4/11/2000	3.53	269.19			3.17	245.82	3.51	261.60	4.02	259.53
5/10/2000	3.58	269.14			3.00	245.99	3.18	261.93	3.79	259.76
6/19/2000	4.57	268.15	4.23	238.04	4.11	244.88	3.95	261.16	4.56	258.99
7/10/2000	5.17	267.55	5.96	236.31	5.76	243.23	4.59	260.52	5.92	257.63
10/10/2000	6.19	266.53			7.07	241.92	5.84	259.27	6.26	257.29
1/22/2001	3.85	268.87	3.61	238.66	3.11	245.88	3.38	261.73	3.63	259.92
5/4/2001	3.94	268.78	3.64	238.63	3.35	245.64	3.59	261.52	3.94	259.61

Table 1 - Water Level Data - Work Area West

Sheet 4 of 4

Depth* All depths are below measuring point (NOT below the ground surface)

-- Indicates data not available.

Italics = Estimated

Table 2 - Hydraulio	Conductivity	- West Wall	Area
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Location	Soil Types in Screen Interval	Hydraulic Conductivity in cm/sec
Work Area West		
НС99-В37	Dense, very silty, fine to medium SAND and sandy, silty PEAT	9.5 x 10 ⁻⁵
HC99-B38	Soft, slightly sandy SILT and very dense, slightly gravelly to gravelly, silty SAND	7.0 x 10 ⁻⁵
нс99-вз9	Medium dense to very dense, slightly gravelly, silty, fine SAND	1.5 x 10 ⁻⁴
HC99-B40	Medium dense to very dense, slightly gravelly, silty SAND	<u>1.3 x 10⁻⁴</u>
	Mean:	1.1 x 10 ⁻⁴

Hart Crowser 497828\Tables1&2&3WaterLevels&slugtesting.xls - Table 2-Slug Test Data

Table 3 - Work Area South Water Level Data

		AT9	7-B8	AT9	7-B14
		Depth*	Elevation	Depth*	Elevation
		in Feet	in Feet	in Feet	in Feet
Measurin	g Point	0.00	379.2		
Ground L	evel*	2.2	377	N/A	386
Top of Sc	reen*	15.2	364.0		
Bottom of	Screen*	20.2	359.0		
Date:	3/10/2000	4.65	374.6		
	4/11/2000	5.50	373.7		
	5/10/2000	5.83	373.4		
	6/20/2000	7.02	372.2		
	7/10/2000	7.88	371.3		
	10/10/2000	10.39	368.8		
	1/22/2001			19.97	
	1/23/2001	6.26	372.9		
	5/3/2001	5.72	373.5		
	5/10/2001			16.97	

Italics = Estimated

- Depth* All depths are below measuring point (NOT below the ground surface).
 - Indicates data not available.

Hart Crowser 497828\Tables1&2&3WaterLevels&slugtesting.xls - Table 3-Work Area South

Table 4 - Soil Conditions by Area (500 ft x 500 ft Squares) - Work Area South 1

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									Observed Optimum	
				Cut Death		Estimated Fines Content Range	Molsture	Moisture Content (MC) of	Moisture Content (OMC) Range for Prev Filling	Generalized
New Area I.D.	Grid Coordinates	Cut Depth In Feet	Borings and Test Pits	at Exploration Location in Feet	Comments / Soli Description	in Percent Based on Description	Sensitvity (low, med., high)	Representative Samples In Percent	and Similar Fines Content Range in Percent	Comparison of Average OMC and MC
	N15000, E 11000		HC00-TP223*	(1)	Wet, (med dense), slightty gravelty, sitty SAND	12 to 50	med to high	21	3 to 13	Above OMC
17	N14500, E 11000	0 to 4			*Located outside Phase 5 Area		,			
	N15000, E 11500				Very little cut in this area; mostly fill					
-	N14500, E 11500						_			
_	N15000, E11500									
18	N14500, E11500	2 to 7	A197-B27•	(4)	Moist, dense SAND (Adv. Outwash), occasional gravel	0 to 5	low	9.6	2 to 7	Above OMC
	N15000, E12000				*Located outside Phase 5 work area					
T	N14500 , E12000									
	N15500, E11000		AT97-B10	Fill area	12" of topsoil + dry, very dense, silty SAND (Weathered Till)	12 to 50	med to high	,	3 to 13	
19	N15000, E11000	0 to 4	AT97-B11	Fill area	12" of topsoil + dry, very dense, sity SAND (Weathered Till)	12 to 50	med to high	8.9	3 to 13	Near avg. OMC
	N15500 E11500				* Located in fill area; not in cut.					
	N15000, E11500				Stock pile located along east edge of Area 19					
	N15500, E11500									
20	N15000, E11500	1 to 5	A197-B9	1	Moist, very dense, sitty SAND, some gravel (Weathered Till)	12 to 50	med to high	7.5	3 to 13	Near avg. OMC
	N15500, E12000									
	N15000 E12000						-			
	N16000, E11000		A194B-TP5*		Moist to wet, med. dense SAND with some silt (Rec. Outwash)					
21	N15500, E11000	0 to 14	AT97-B5	3	Moist, very dense, silty SAND (Weathered Tilt)	12 to 50	med to high	58	3 to 13	Below avg. OMC
	N16000, E11500				*16 feet of new fill was placed here; cut depth is only 6 feet.					
T	N15500, E11500				Stockpile located along east edge of Area 21					
	N16000, E11500		AT97-B8*	ε	1' of topsod + Moist, dense SAND (Adv Outwash)	0 to 5	łow	4	2 to 7	Above OMC
52	N15500, E11500	6 to 26			boring terminated at 19.5' below ground surface		_			
	N15000, E12000				* Located outside of Phase 5 work area					
	N15500, E12000				Large portions of Area 22 are occupied by stockpile.				•	
	N16500, E11000		AT97-B4*	Fill area	Sifty sand FILL + SAND (Rec. Outwash) + sifty SAND	0 to 50	med to high	19.9	2 to 13	Above OMC
53	N16000, E11000	0 to 13			* Located in fill area, not in cut.					
	N16500, E11500									
	N16000 E11500									
	N16500, E11500		AT97-B13/-B14*	7	Moist, med. dense, sitty SAND Fit.L, some gravel, asphalt, organics	12 to 50	med to high	9.2	3 to 13	Slightly above avg. OMC
54	N16000, E11500	7 to 22								
	N16500, E12000				 Located just north of Phase 5 work area. 					
	N16000, E12000				Large portion of Area 24 occupied by stockpile					
	N16500, E12000	1								
55	N16000, E12000	0 to 16	none		Only west edge of Area 25 part of Phase 5					
	N16500, E12500									
	N16000, E12500									

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Hart Crowser 497828\Tables4&5_P5datareport xis - Table 4 (South 1)

Table 5 - Soil Conditions by Area (500 ft x 500 ft Squares) - Work Area South 2

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-	ł		Cut Depth		Estimated Fines			Observed OMC Range for	-
Grid Coordinates	Depth In Feet	borings and Test Pits	at Exploration Location in Feet	Comments / Soil Description	Content Kange In Percent Based on Description	Moisture Sensitvity (low, med., high)	Moisture Content of Representative Samples in Percent	Prev Filling and Similar Fines Content Range in Percent	Generalized Comparison of Average OMC and MC
N11500 E10500 N12000 E10500 N11500 E11000 N12000 E11000	14 to 17	AT97-B36*	16	Moist, med. dense, of dense, silly SAND FilLL with gravel and organics Only easterminest part of Area 1 is in Phase 5 Located weat of Phase 5 Area	12 to 50	med to high	14.4 and 18.2	3 to 13	Above OMC
N11500 , E11000 N12000 , E11000 N11500 , E11500 N12000 , E11500	11 to 28	AT97-B18 AT97-B19 AT97-B20*	115 12 14	Dy, medium dense, shy SAND FLL, erren coarter gravel Marting, danse for word verse, shy SAND FLL. Hendi, medium dense to dense, shy SAND FLL. Hendi, medium dense to dense, shy SAND FLL. Stoczeles of states for one - ratius of fit sub-unitrown. Stoczeles of reservit sha NE corner - ratius of	12 to 50 12 to 50 12 to 50	med to high med to high med to high	8.5 to 9.4 8.1 6.6 to 13.2	8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	Near ang OMC Near ang OMC Below and above ang OMC
N11500, E11500 N12000, E11500 N11500, E12000 N12000, E12000	9	none		Only northeast corner of Area 3 is in Phase 5 work area					
0000	10 to 14	A1948-82	12	Crris west edge of Aree 4 ks in Phase 5 work area Motel medium dense, sity SAVID Fill.	12 to 50	med to high	6.4 to 9.7	3 to 13	New or slightly above avg. OMC
	6 to 46	AT948-B2	12	Most, medium dense, sity SAND FiL. Stockpile of recent fill - nature of fill solis unknown	12 to 50	med to high	841097	3 to 13	Near or slightly above avg. OMC
N12500 E11500 N12000 E11500 N12000 E12000 N12500 E12000	4 to 47	A197 829* A197-830**	(6) + 21' di new fill	Moist, medium dense, sithy SAND FILL. Moist, medium dense, sithy SAND FILL. - Located visit autorethers 5 work area - Located visit stockpibe boundary.	12 to 50 12 to 50	med to high med to high	7.6	3 to 13 3 to 13	Skyrity below any OMC Skyrity below any OMC
N12500 E12000 N12000 E12000 N12500 E12500 N12000 E12500	9	HORE		Only northwest corner of Area 7 is in Phase 5 work area					
N13000 E11000 N12500 E11000 N13000 E11500 N12500 E11500	5 to 35	A1948-82 A197-831	12 7	Moist, medium dense, sitly SAND FiLL. Moist, medium dense, sitly SAND FiLL. Stockole of recent fill at SE corner - nature of fill solis unknown	12 to 50 12 to 50	med to high med to high	8.4 to 8.7 5.9	3 to 13 3 to 13	Near or slightly above avg OMC Below avg OMC
0000	6 to 45	none		Stockpile of recent fill covers most of this area - nature of fill solis unknown					
	3 to 20	POR		Only west edge of Area 10 is in Phase 5 work area Stockpie stope is located along west edge of Area 10					
N13500, E11000 N13000, E11500 N13000, E11500 N13000, E11500	2 to 11	A197-815	2	12" of topsoil + most, medium dense, sity SAND FILL some gravel Stockple along east edge of Area 11	12 to 50	med to high	80 1	3 to 13	Above avg. OMC
N13500, E11500 N13000, E11500 N13500, E12000 N13000, E12000	7 to 17	A197 B16 A1948 TP16*	10 (9) +9' of new fill	Motel, loose to medium dense, safy SAND FIL., some gravel Motel to wet, signify safty to sify SAND FIL. Insee organics *located within stockpie boundary	12 to 50 5 to 50	med to high med to high	97	3 tb 13 2 tb 13	Above avg CMC Above OMC
	4 10 80	AT97-823*	(3)	12" of togsoil + most, medkum dense, sifty SAND Fit.Lisome gravel 1-control outside Phase 5 work area Dity southwest conter of Aveas 13 in Phase 5 work area	12 to 50	med to high	96	3 to 13	Above avg CMC
N14000, E11000 N13500, E11000 N14000, E11500 N13500, E11500	9 to 13	AT97-B37+	(3)	Moet, medium dense, silly SAND FILL with gravel & organics "Located outside Phase 5 work area	13 (test results)	med	18.3	3 to 13	Above OMC
	6 to 32	A197-B17•	13	Most, locee, suity SAND FILL, some gravel, trace organics - Located within stockpile boundary Stockpile of recent fils covers parts of this area - nature of fil	12 lo 50	med to high	7 3 and 15 9	3 to 13	Below and above avg OMC
N14000, E12000 N13500, E12000 N14000, E12500 N13500, E12500	5 to 8	AT97-822*		Moret, med dense, sity SAND FiLL, trace gravel "Located outside Phase 5 Anna Conty wast exbe of Ana 16 in Phase 5	12 to 50	med to high	5 7	3 to 13	Below avg OMC

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Hart Crowser 49/82%1ables485_Pridatareport xis - 1able 5 (South 2)

Table 6 - Test Pit Summary for Stockpile Nos. 1 and 2 - Work Area South 2

Depth in Feet	Sample No.	Water Content in Percent	Lab Tests	Description
0 to 2	S1	2	GS	(Medium dense), damp, gray-brown, sandy GRAVEL.
2 to 5.5	S2	N/A	None	(Medium dense to dense), damp to moist, brown, gravelly, medium SAND with scattered cobbles

HC01-TP1

HC01-TP2

Depth in Feet	Sample No.	Water Content in Percent	Lab Tests	Description
0 to 5.5	S1	3	GS	(Medium dense to dense), damp, brown, very gravelly SAND with scattered cobbles.

HC01-TP3

Depth Feet	n San N		Water Content in Percent	Lab Tests	Description
0 to 5.	5 S	1	7	GS	(Medium dense), moist, brown, slightly gravelly, silty, medium to fine SAND.

HC01-TP4

Depth in Feet	Sample No.	Water Content in Percent	Lab Tests	Description
0 to 7	S1	10	GS	(Medium dense), moist, brown, slightly gravelly, silty SAND with concrete and wood debris, and scattered cobbles.
				- Gravelly sand layers throughout test pit.

HC01-TP5

Depth in Feet	Sample No.	Water Content in Percent	Lab Tests	Description
0 to 10	S1	N/A	None	(Medium dense), damp, brown, slightly silty to silty, slightly gravelly to gravelly SAND with scattered cobbles up to 10-inch diameter.
				- Wood debris at 3.5-foot depth. - Wood debris with creosote odor at 8-foot depth.

Sheet 1 of 2

Table 6 - Test Pit Summary for Stockpile Nos. 1 and 2 - Work Area South 2

Depth in Feet	Sample No.	Water Content in Percent	Lab Tests	Description
0 to 2	S1	N/A	None	(Medium dense), damp, brown, gravelly, very silty SAND.
2 to 10	S2	N/A	None	(Loose to medium dense), moist, gray, gravelly, slightly silty to silty SAND.

HC01-TP6

HC01-TP7

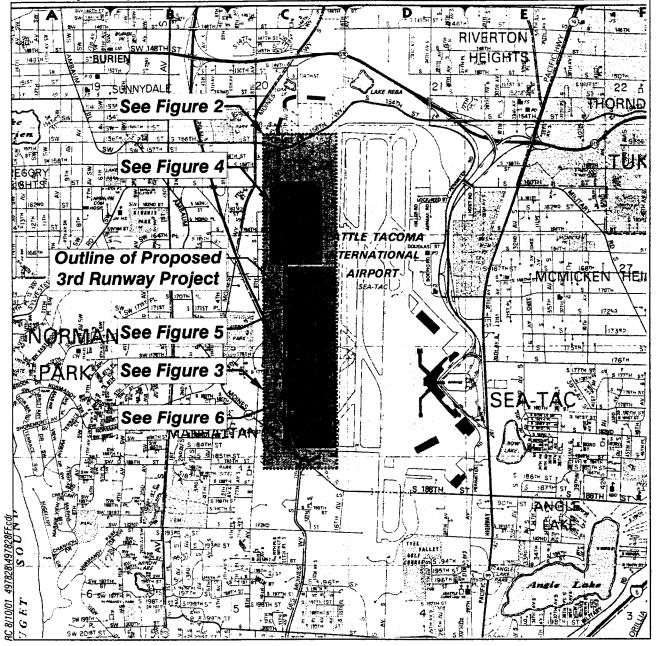
Depth in Feet	Sample No.	Water Content in Percent	Lab Tests	Description
0 to 9.5	S1	N/A	None	(Medium dense to dense), moist, gray-brown, gravelly, silty SAND with large concrete debris.
				- Wood debris at 8-foot depth.

HC01-TP8

Depth in Feet	Sample No.	Water Content in Percent	Lab Tests	Description
0 to 6	S1	3	GS	(Medium dense), damp to moist, brown, very sandy GRAVEL.

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Vicinity Map



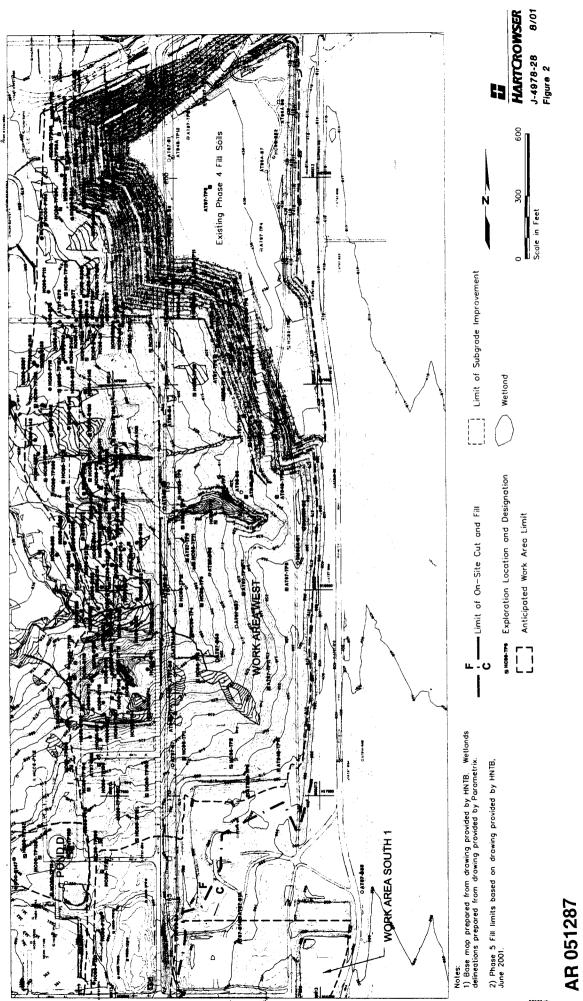
2000

4000

Scale in Feet

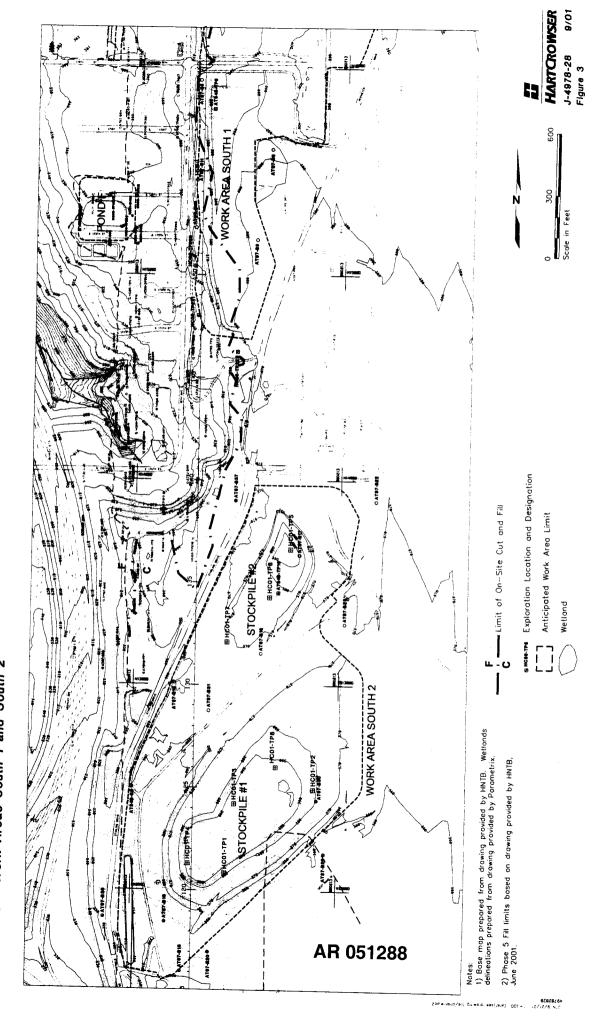
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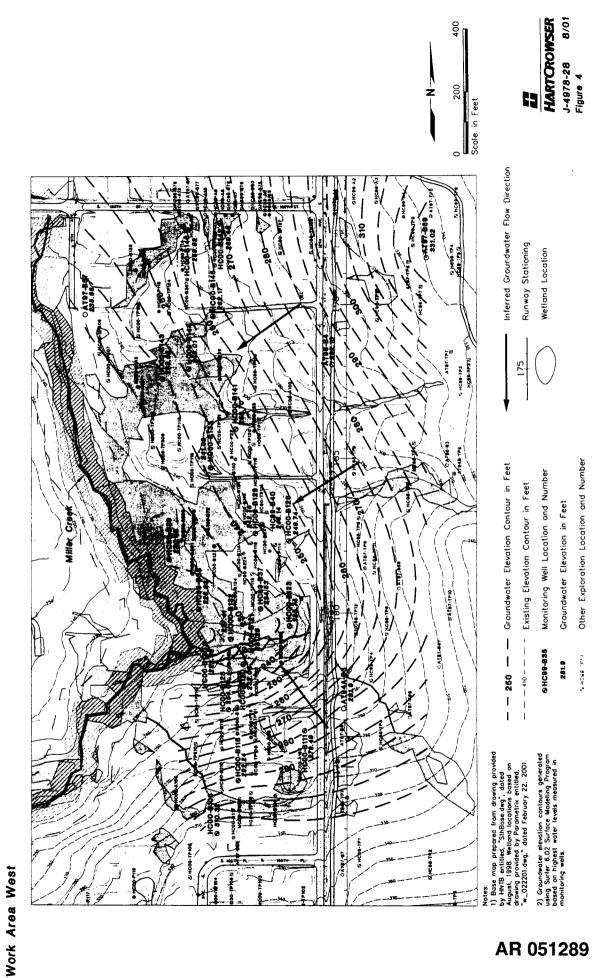




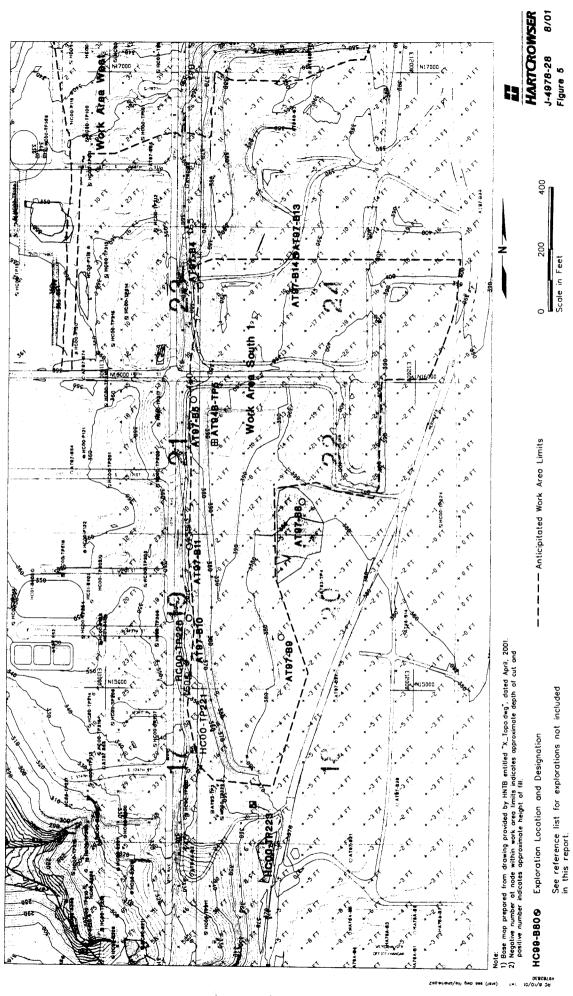
00. 8/14/01 1+300 (see)see orgaing file/chorlie.pc2



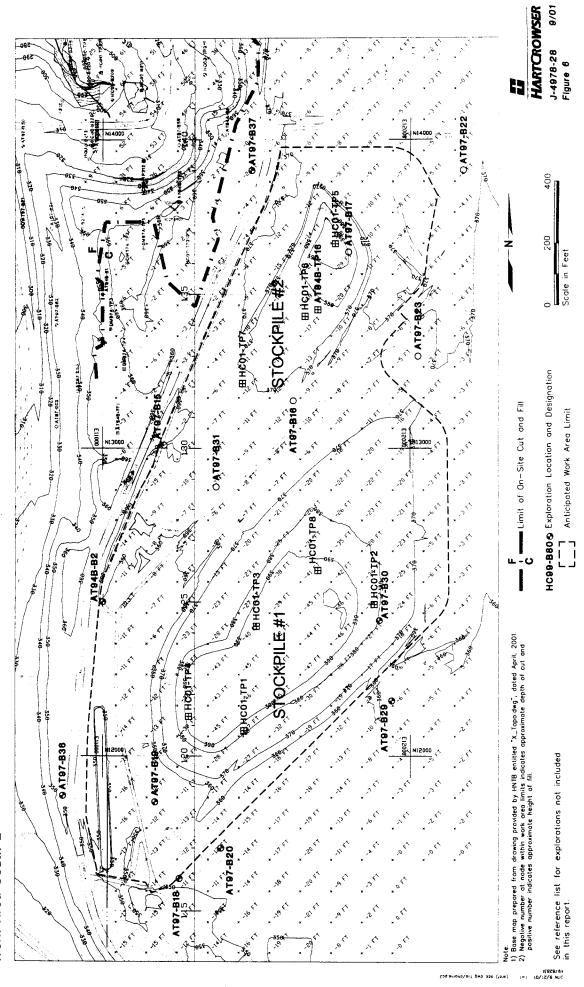
Site and Exploration Plan Phase 5 - Work Areas South 1 and South 2



Groundwater Elevation Contour Map



Anticipated Excavation Plan Work Area South 1



Anticipated Excavation Plan Work Area South 2

APPENDIX A FIELD EXPLORATIONS METHODS AND ANALYSIS

Hart Crowser 4978-28 September 21, 2001

This appendix documents the processes Hart Crowser used in determining the nature of the soils underlying the project site addressed by this report. The discussion includes information on the following subjects:

- Explorations and Their Location;
- The Use of Auger Borings;
- Standard Penetration Test (SPT) Procedures;
- Use of Shelby Tubes;
- Drilling of Hand-Auger Holes;
- Excavation of Test Pits;
- The Use of Cone Penetrometer Probes;
- Hydraulic Conductivity Testing (Slug Test); and
- Water Level Measurement.

Subsequent to the field work described in this appendix, Hart Crowser obtained additional undisturbed (shelby tube) samples of clay, silt, and peat soils, and accomplished field vane shear tests in peat. Typically the shelby tube samples were obtained by drilling near previous boring locations and the new samples were labeled by appending a letter (such as "A" or "B") to the exploration number. In cases such as this a new exploration log was not produced for the report. Results of the field vane shear tests on peat are presented in Appendix D.

Explorations and Their Location

This appendix presents exploration data collected by Hart Crowser. Exploration logs generated by Applied Geotechnology, Inc. (AGI) and Dames & Moore are presented in Appendix C. This appendix includes the following subsurface explorations:

Borings

HC99-B37 through HC99-B40, HC99-B75 through HC99-B80, HC00-B106 through HC00-B108, HC00-B110, HC00-B111, HC00-B113 through HC00-B134, HC00-B137 through HC00-B147, HC00-B221, HC00-B224, HC00-B300, HC00-B303, HC00-B305 through HC00-B307, HC01-B401 through HC01-B403.

Hand Auger Explorations

HC00-A100, HC00-A105, HC00-A109, HC00-A137, HC00-A143, HC00-A300, HC00-A301.

Test Pits

HC98-TP1 through HC98-TP12, HC99-TP1 through HC99-TP5, HC99-TP7, HC99-TP9, HC99-TP34, HC99-TP36, HC99-TP36A through HC99-TP36D, HC99-TP44, HC00-TP100 through HC00-TP108, HC00-TP110 through HC00-TP121, HC00-TP123 through HC00-TP129, HC00-TP133, HC00-TP134, HC00-TP137 through HC00-TP139, HC00-TP202, HC00-TP204, HC00-TP221, HC00-TP223, HC00-TP226, HC00-TP301 through HC00-TP305, HC00-TP308 through HC00-TP311, HC00-TP318, HC00-TP319.

Cone Penetrometer Test Probes

HC00-P22A, HC00-P22B, HC00-P23 through HC00-P26, HC00-P100, HC00-P104 through HC00-P108, HC00-P111 through HC00-P112, HC00-P114, HC00-P115, HC00-P118 through HC00-P120, HC00-P122, HC00-P123.

The exploration logs within Appendix A show our interpretation of the material encountered based on drilling (or excavation), sampling, and testing data. They indicate the depth where the soils change. Note that the change may be gradual. In the field, we classified the samples taken from the explorations according to the methods presented on Figure A-1 - Key to Exploration Logs. This figure also provides a legend explaining the symbols and abbreviations used in the logs.

Location of Explorations. Figures 2 and 3 show the location of explorations in Work Area West and Work Area South, respectively.

Locations for borings and test pits designated J-4978-06 were originally located by hand taping or pacing from existing physical features. The ground surface elevations at these locations were interpreted from the aerial survey topography provided by HNTB.

Locations for borings and test pits designated J-4978-16 were located using GPS survey by Hart Crowser on October 1, 1999. Port of Seattle surveyors performed x, y, z survey for all wells on October 13, 1999, which replaced the GPS locations. GPS coordinates were used for the test pits. The ground surface elevations of the test pits can be interpreted from the aerial survey topography.

Locations for borings and test pits designated J-4978-18, J-4978-21, J-4978-23, J-4978-26, J-4978-27, and J-4978-31 were located using a global positioning system (GPS) survey by Hart Crowser. Port of Seattle surveyors performed an x, y, z survey for the top of the casing elevations of the wells and ground surface elevations for piezocones, test pits, and some borings completed without wells. Where available, the Port's survey supersedes the GPS locations. Where Port survey data are not available, ground surface elevations were interpreted from aerial survey topography provided by HNTB.

The method used in the determination of their locations determines the accuracy of the location and elevation of the explorations.

The Use of Auger Borings

Hart Crowser performed 59 borings that are included in this report. The borings used a 3-3/8-inch inside diameter hollow-stem auger and were advanced with a truck-mounted drill rig subcontracted by Hart Crowser. The drilling was continuously observed by an engineering geologist from Hart Crowser. Detailed field logs were prepared of each boring. Using the Standard Penetration Test (SPT), we obtained samples at 2-1/2- to 5-foot-depth intervals.

Groundwater levels in the borings were noted at the time of drilling (ATD) and following installation and development of observation wells where noted on the boring logs and shown in Tables 1 and 3.

The borings logs are presented on Figures A-3 through A-61 at the end of this appendix.

Standard Penetration Test (SPT) Procedures

This test is an approximate measure of soil density and consistency. To be useful, the results must be used with engineering judgment in conjunction with other tests. The SPT (as described in ASTM D 1586) was used to obtain disturbed samples. This test employs a standard 2-inch outside diameter split-

spoon sampler. Using a 140-pound hammer, free falling 30 inches, the sampler is driven into the soil for 18 inches. The number of blows (N value) required to drive the sampler <u>the last 12 inches only</u> is the Standard Penetration Resistance. This resistance, or blow count, measures the relative density of granular soils and the consistency of cohesive soils. The blow counts are plotted on the boring logs at their respective sample depths.

Soil samples are recovered from the split-barrel sampler, field classified, and placed into watertight jars. They are then taken to Hart Crowser's laboratory for further testing.

In the Event of Hard Driving

Occasionally, very dense materials or the presence of gravel and/or cobbles prevented driving the total 18-inch sample. When this happens, the penetration resistance is entered on logs as follows:

Penetration less than six inches. The log indicates the total number of blows over the number of inches of penetration.

Penetration greater than six inches. The blow count noted on the log is the sum of the total number of blows completed <u>after</u> the first 6 inches of penetration. This sum is expressed over the number of inches driven that exceed the first 6 inches. The number of blows needed to drive the first 6 inches is not reported. For example, a blow count series of 12 blows for 6 inches, 30 blows for 6 inches, and 50 (the maximum number of blows counted within a 6-inch increment for SPT) for 3 inches would be recorded as 80/9.

Use of Shelby Tubes

To obtain a relatively undisturbed sample for classification and testing in finegrain soils, a 3-inch-diameter thin-walled steel (Shelby) tube sampler was pushed hydraulically below the auger in select borings. The tubes were sealed in the field and taken to our laboratory for extrusion and classification.

Drilling of Hand-Auger Holes

Hart Crowser engineering geologists drilled seven hand-auger borings using portable equipment. The geologists observed the soil in the hand-augered holes and reported the findings on a field log. Our geologists took representative samples of soil types for testing at Hart Crowser's laboratory. Groundwater levels during excavation of the holes were noted on the logs. The density/consistency of the soils (as presented parenthetically on the test pit logs

Page A-4

to indicate their having been estimated) is based on visual observation only as disturbed soils cannot be measured for in-place density in the laboratory.

The hand-auger logs are presented on Figures A-62 through A-65.

Excavation of Test Pits

Seventy-five test pits were excavated across the site with a tractor-mounted backhoe. The sides of these excavated pits offer direct observation of the subgrade soils. The test pits were located by and excavated under the direction of an engineering geologist from Hart Crowser. The geologist observed the soil exposed in the test pits and reported the findings on a field log. Our geologist took representative samples of soil types for testing at Hart Crowser's laboratory. Groundwater levels or seepage during excavation were noted on the logs. The density/ consistency of the soils (as presented parenthetically on the test pit logs to indicate their having been estimated) is based on visual observation only, as disturbed soils cannot be measured for in-place density.

The test pit logs are presented on Figures A-66 through A-111.

Cone Penetration Test Procedures

The electric piezocone penetrometer test procedure involves hydraulically pushing a series of cylindrical rods into the soil at a constant rate of two centimeters per second and subsequently monitoring soil and pore fluid response near the conical tip. The cylindrical rod at the bottom of the drill string houses the pressure transducer and load cells which, during probing, measure the parameters indicated above. The results are often used with engineering judgment in conjunction with other tests, preferably the SPT procedure, which allows soil sample collection for direct comparison purposes. Tests were performed in general accordance with procedures outlined in ASTM D 3441, Standard Method for Deep, Quasi-Static, Cone and Friction-Cone Penetration Tests of Soil.

The cone system is mounted on a truck or bulldozer to provide the necessary reaction for the applied loads. The cone tip has a surface area of about 10 square centimeters (cm²) and an angle of 30 degrees from the axis. The friction sleeve has a surface area of about 150 cm². Prior to testing, a plastic filter element, which has been saturated under vacuum in glycerin, is placed behind the cone tip. This filter element transmits pore pressures to the transducer. Load cells measure end resistance on the tip and frictional resistance on the friction sleeve. As the cone penetrates the soil, measurements are continuously recorded on a portable computer at depth increments of about 5 centimeters.

The classification method used to develop an interpreted soil profile is based on normalized parameters provided by the piezocone, as there are no soil samples collected with a penetrometer system of this type.

The relationship between the cone tip resistance and friction ratio, which has been normalized for soil overburden stresses, can be established to predict soil behavior (Jeffries and Davies 1991 and 1993). This relationship has been applied to the soil classification chart developed by Robertson as reported in Lunne et al. 1997 (refer to Figure A-2) according to the following equation:

$$I_{c} = \sqrt{\{3 - \log[Q \cdot (1 - B_{q})]\}^{2} + [1.5 + 1.3 \cdot \log(F)]^{2}}$$

Where,

Ic = Soil behavior index

$$Q = \frac{q_T - \sigma_{vo}}{\sigma'_{vo}}$$

q_T = Corrected cone tip resistance

 σ_{vo} = Total overburden stress

$$\sigma'_{vo}$$
 = Effective overburdens stress

$$B_q$$
 = Normalized pore pressure

$$B_q = \frac{\Delta u}{q_T - \sigma_{vo}}$$

F = Normalized friction ratio

$$R_f = \frac{f_s}{q_T - \sigma_{vo}} \cdot 100\%$$

 $f_{\rm S}$ = Sleeve friction

Hydraulic Conductivity Testing (Slug Testing)

Hydraulic conductivity testing was performed using the slug test method. In this method the water level (hydraulic head) in the well is rapidly raised or lowered, and the rate at which it returns to its initial state is used to calculate hydraulic conductivity for the formation surrounding the wellscreen. Data were collected using an Aquistar data logger in conjunction with a Instrumentation Northwest PSI9000 pressure transducer. Tests were conducted as follows:

- A transducer was set in the well and allowed to equilibrate with ambient conditions, and background water level data were collected.
- One or two slug rods (solid PVC rods) were rapidly introduced into the well (causing a near-instantaneous rise in water level), to initiate a falling head test. Water level data were collected in logarithmically increasing time increments using the data logger and transducer. For wells where depth to water was small, a falling head test was not attempted.
- Water level in the well was allowed to re-equilibrate.
- The slug rod or rods were rapidly pulled from the well (causing a nearinstantaneous drop in water level) to initiate a rising head test. Water level data were collected in logarithmically increasing time increments using the data logger and transducer.
- Most of the wells responded reasonably quickly, and therefore multiple slug tests were performed for most wells.

Data were pre-processed as described in Butler (1998), and hydraulic conductivity values were estimated using the method of Bouwer and Rice (1976) for unconfined aquifers. The estimated values are summarized in Table 2.

Water Level Measurement

Water levels were measured using a Solinst water level probe, graduated in 0.01foot increments. Depth to water was measured below the top of casing, and recorded to the nearest hundredth of a foot. Depth to water was converted to groundwater elevation using survey information for the top of casing in the wells. Depth to water data and groundwater elevations are summarized in Table 1 and 2.

References for Appendix A

Bouwer, H., and R.C. Rice 1976. A slug test method for determining hydraulic conductivity of unconfined aquifers with completely or partially penetrating wells, Water Resources Research, Vol. 12, No. 3, pp. 423-428.

Butler, J.J. 1998. The design performance and analysis of slug tests. Publisher: Lewis, Boca Raton, Florida.

Jeffries, Michael G., and Michael P. Davies 1991. Soil classification by the cone penetrometer test: Discussion, Can. Geotech. J. 28, 173-176.

Jeffries, Michael G., and Michael P. Davies 1993. Use of CPTu to Estimate Equivalent SPT N_{60} . Geotechnical Testing Journal. GTJODJ, Vol. 16, No. 4, 458-468.

Lunne, T. P.K. Robertson, and J.J.M. Powell 1997. Cone Penetration Testing in Geotechnical Practice, Blackie Academic and Professional, London.

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Key to Exploration Logs

Sample Description

Classification of soils in this report is based on visual field and laboratory abservations which include density/consistency, moisture condition, grain size, and plasticity estimates and should not be construed to imply field nor laboratory testing unless presented herein. Visual-manual classification methods of ASTM D 2488 were used as an identification guide.

Soil descriptions consist of the following:

Density/consistency, moisture, color, minor constituents, MAJOR CONSTITUENT, additional remarks.

Som density/consistenc	y in test pits is estimat	ed based on visual observe	rd Penetration Resistance. ation and is presented pare	enthetically on the test pit log
SAND or GRAVEL Density	Standard Penetration Resistance (N) in Blows/Foot	SILT or CLAY Consistency	Standard Penetration Resistance (N)	Approximate Shear Strength
2	•	consistency	in Blows/Foot	in TSF
Very loose	0 - 4	Very soft	0 - 2	<0.125
Loose	4 - 10	Soft	2 - 4	0.125 - 0.25
Medium dense	10 - 30	Medium stiff	4 - 8	0.25 - 0.5
Dense	30 - 50	Stiff	8 - 15	0.5 - 1.0
Very dense	>50	Very stiff	15 - 30	1.0 - 2.0
		Hard	>30	>2.0

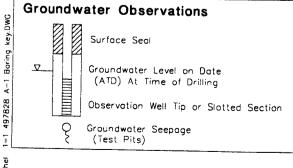
Moisture

Dry Little perceptible moisture

- Damp Some perceptible moisture, probably below optimum
- Moist Probably near optimum moisture content
- Wet Much perceptible moisture, probably above optimum

Legends

Sam	pling Test Symbols
BORIN	G SAMPLES
\square	Split Spoon
	Shelby Tube
Ē	Cuttings
	Core Run
*	No Sample Recovery
P TEST	Tube Pushed, Not Driven PIT SAMPLES
\boxtimes	Grab (Jar)
	Bag
	Shelby Tube



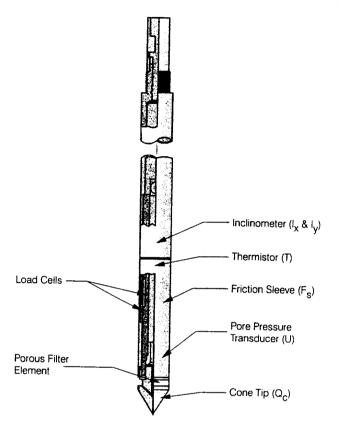
Minor ConstituentsEstimated PercentageNot identified in description0 - 5Slightly (clayey, silty, etc.)5 - 12Clayey, silty, sandy, gravelly12 - 30Very (clayey, silty, etc.)30 - 50

Test Symbols

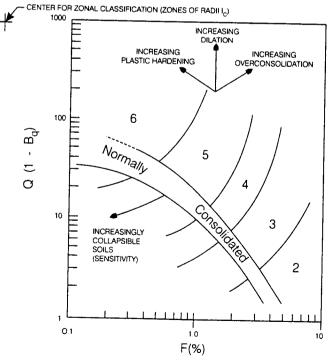
	GS CN UU	Grain Size Classification Consolidation
	UU	
Į		Unconsolidated Undrained Triaxial
2	CU	Consolidated Undrained Triaxial
4	CD	Consolidated Drained Triaxial
	QU	Unconfined Compression
{	DS	Direct Shear
ł	<	Permeability
F	эр	Pocket Penetrometer Approximate Compressive Strength in TSF
-	ΓV	Torvane Approximate Shear Strength in TSF
(CBR	California Bearing Ratio
N	٨D	Moisture Density Relationship
ļ	AL.	Atterberg Limits
		Water Content in Percent
		Liquid Limit
		Plastic Limit (NP=Non Plastic)
F	DIN	Photoionization Detector Reading
С	A:	Chemical Analysis
D	T	In Situ Density Test

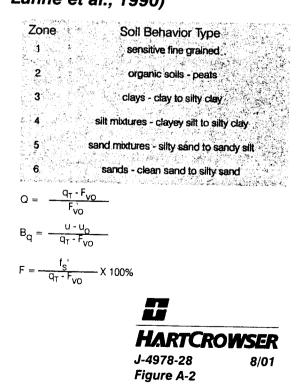


Electric (Piezocone) Cone Penetrometer Schematic of Electric Piezocone (Typical)



Simplified Classification Chart (Jefferies and Davies, 1993 after Lunne et al., 1990)





Boring Log HC99-B37 N 18,014, E 11,020

Descriptions ind Surface Elevation in Feet: 234.55	Depth in Feet	Sample	STANDARD PENETRATION RESISTANCE A Blows per Foot 1 2 5 10 20 50 100	LAB TEST
(Loose) to medium dense, wet, dork brown, slightly gravelly, very silty SAND.		G-1 III S-1 X		
Dense, wet, gray, silty, fine to medium SAND.	∠ 4/5/99	s-2		- GS
Very stiff, moist, dark brown to gray, sandy, clayey PEAT.		S-3		- AL
Very stiff, moist, gray, slightly gravelly, very sandy SILT.		S-4 X		- GS
Very dense, moist, gray, slightly gravelly, very silty SAND.		S-5* ∞	50,	/3
Bottom of Boring at 25.4 Feet. Completed 2/22/99.	25 30	S-6 🗙	50, 	/5
	- 35			
	+40 			
	- 50			
	-55			
	1 - 60	1 1	1 2 5 10 20 50 100 • Water Content in Percent	L

Refer to Figure A-1 for explanation of descriptions and symbols.

- and sympols.
 2. Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
 3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

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J-4978-06 2/99 Figure A-3

HARTCROWSER

Boring Log HC99-B38 N 18,012, E 10,819

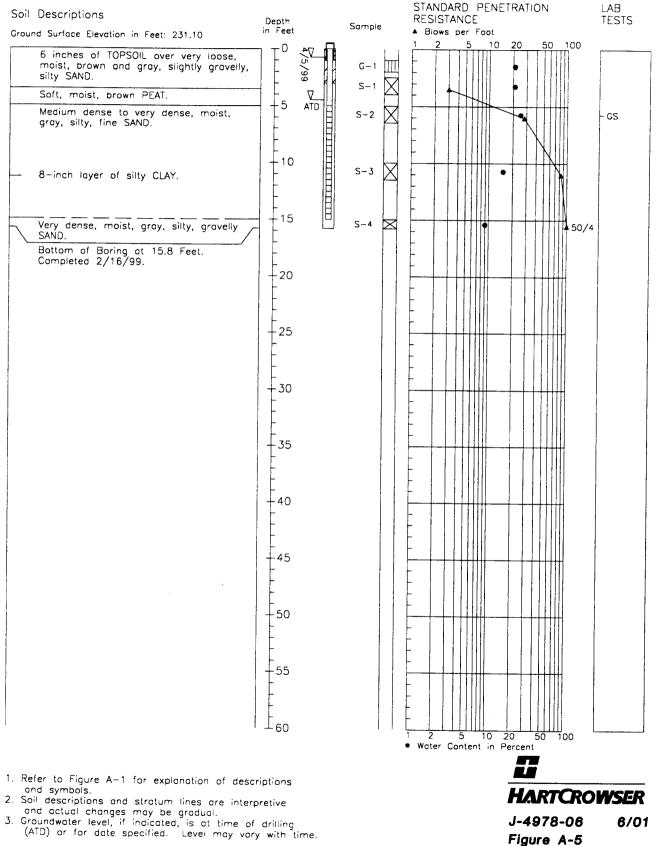
l Descriptions und Surface Elevation in Feet: 227.58	Depth in Feet	Sample	RESISTANCE ▲ Blows per Foot	TESTS
(Loose), moist, brown, silty SAND.		G-1 III		
Medium dense, moist, gray, very silty SAND.		S-1 🛛		
Stiff, moist, dark brown, sandy PEAT with occasional wood debris.	4/9/99	S-2		
Soft, moist, gray, slightly sandy SILT with occasional wood debris.	- 10 V ATD	S-3		
Very dense, moist to wet, gray, slightly gravelly, silty SAND.	15	S-4 X	• • • • • • • • •	0/6 - GS
Bottom of Boring at 20.3 Feet.	20 L	S_5 🗪)/4
Completed 2/22/99.				
	-25			
	- 30			
	- 35			
	+40			
	- 45			
	+50			
	-55			
	1 - 60		1 2 5 10 20 50 100 • Water Centent in Person	, L
			Water Content in Percent	
Refer to Figure A-1 for explanation of de			<u>ili</u>	

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and actual changes may be gradual.
Groundwater level, if indicated, is at time of drilling (ATD) or for date specifiea. Level may vary with time.

6/01 J-4978-06 Figure A-4

Boring Log HC99-B39 N 18,174, E 10,722

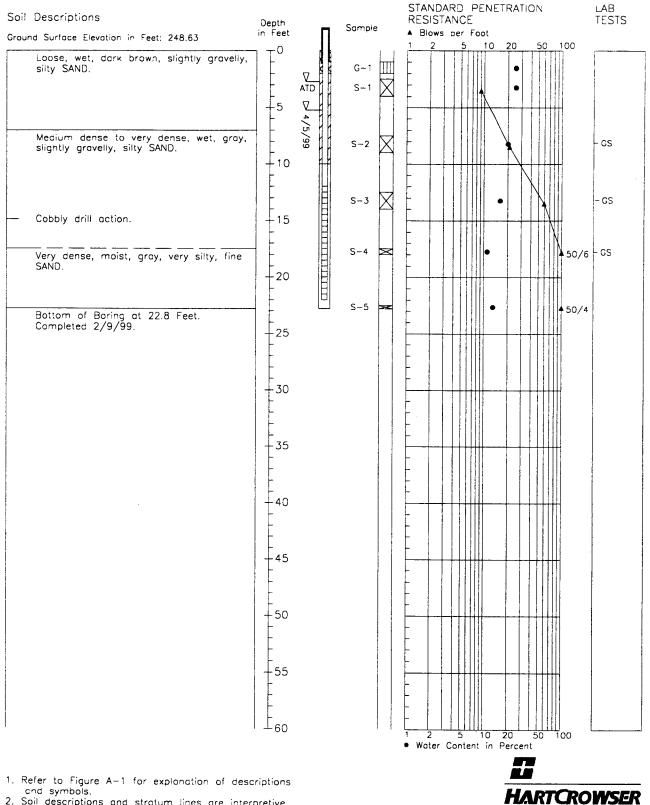


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J-4978-06 6/01 Figure A-5

N 18,285 E 11,026



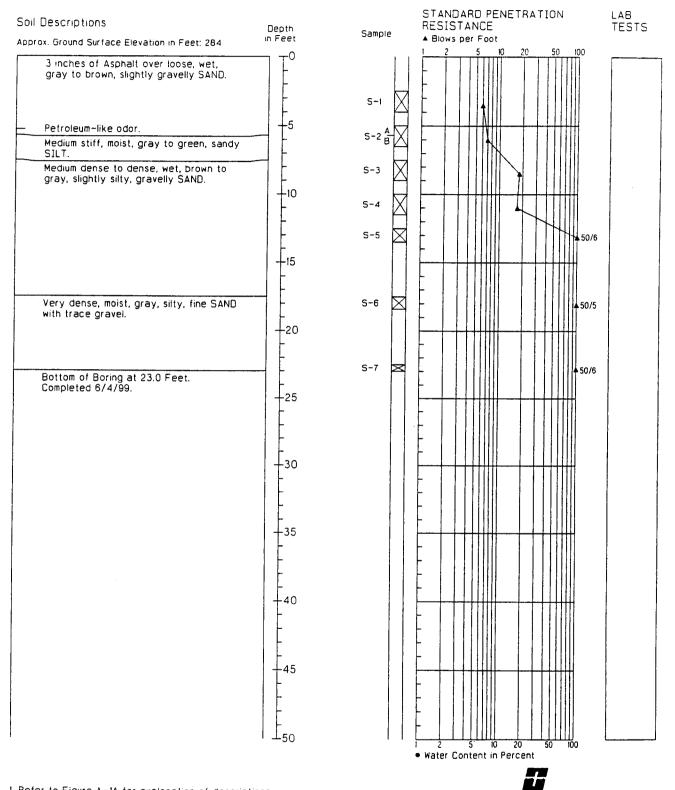
2. Soil descriptions and stratum lines are interpretive and actual changes may be gradual. 3. Groundwater level, if indicated, is at time of drilling

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- (ATD) or for date specified. Level may vary with time.

J-4978-06 2/99 Figure A-6

N 19310 E 11020



1. Refer to Figure A-1A for explanation of descriptions and symbols.

- 2. Soil descriptions and stratum lines are interpretive and actual changes may be gradual. 3. Groundwater level, if indicated, is at time of drilling
- (ATD) or for date specified. Level may vary with time.

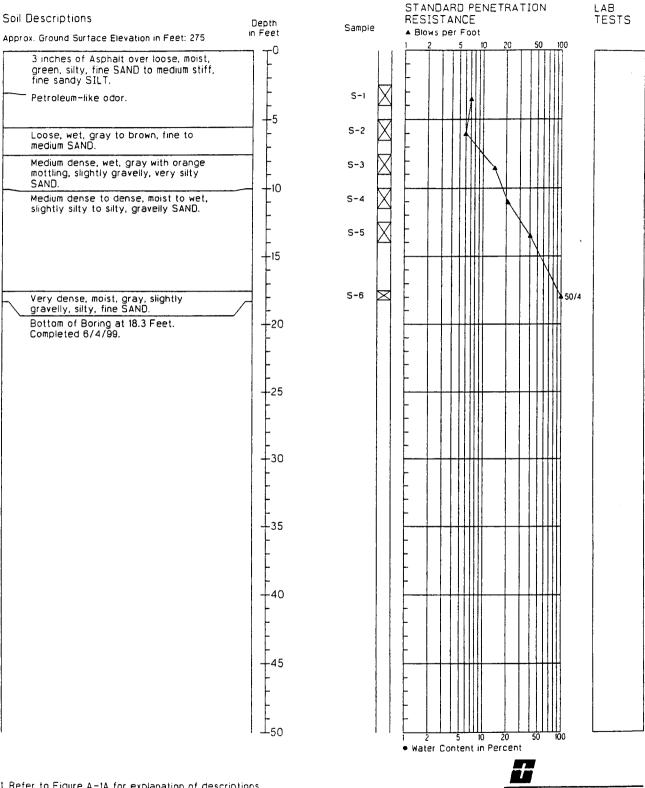
AR 051307

HARTCROWSER

8/99

J-4978-07

N 19311 E 10918



1. Refer to Figure A-1A for explanation of descriptions and symbols.

- 2. Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
- Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

AR 051308

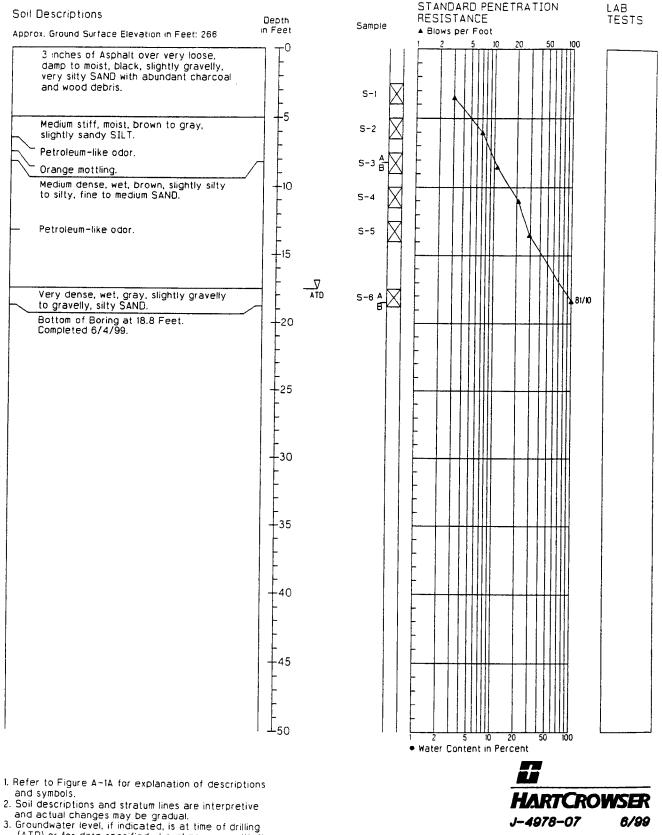
J-4978-07

Figure A-8

HARTCROWSER

6/99

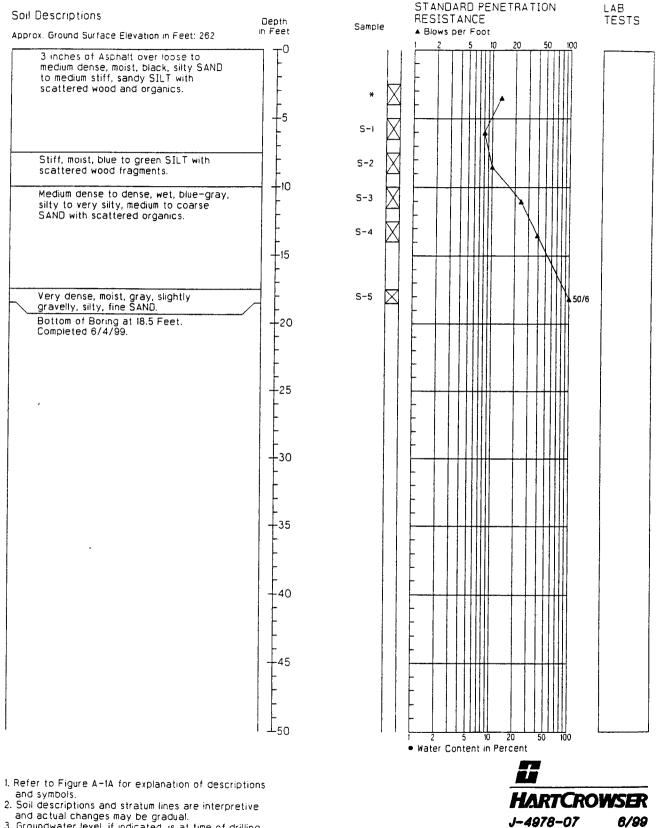
N 19312 E 10816



(ATD) or for date specified. Level may vary with time.

AR 051309

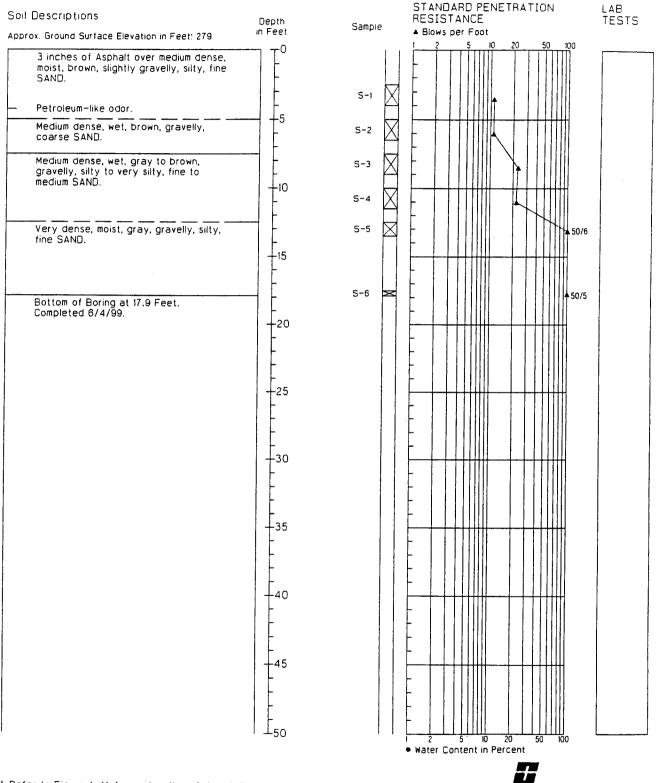
N 19311 E 10743



 Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

AR 051310

N 19309 E 10961

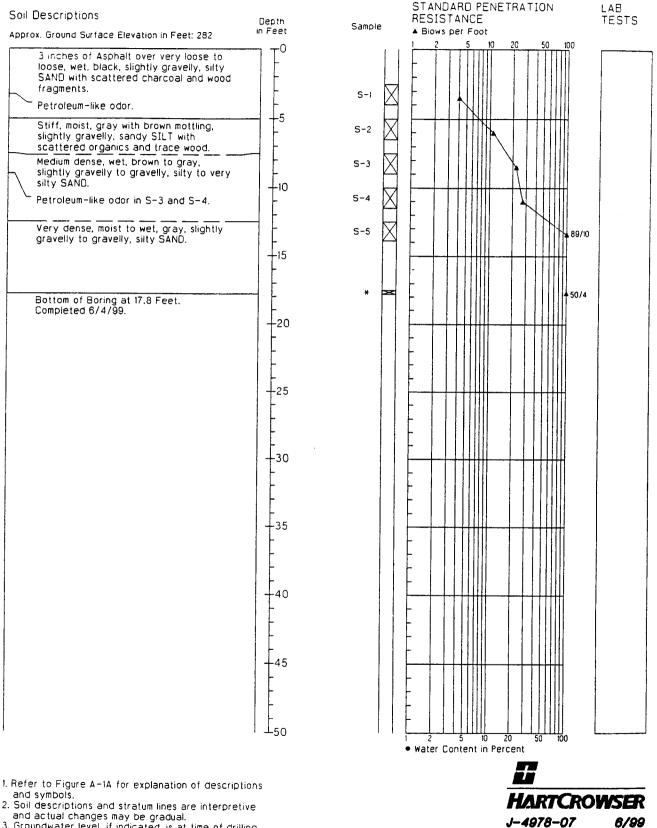


- 1. Refer to Figure A-1A for explanation of descriptions and symbols.
- 2. Soil descriptions and stratum lines are interpretive and actual changes may be gradual.

<u>ن</u>

 Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time. **HARTCROWSER** J-4978-07 6/99 Figure A-11

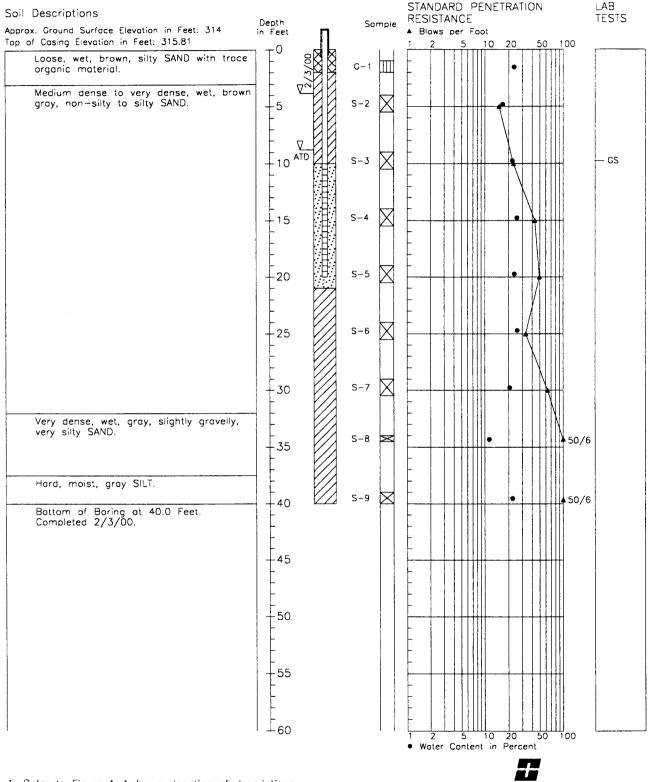
N 19311 E 10996



Soli descriptions and stratum mes are interpreted and actual changes may be gradual.
 Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

AR 051312

Boring Log HC00-B106 N 17284 E 10878



1. Refer to Figure A-1 for explanation of descriptions and symbols.

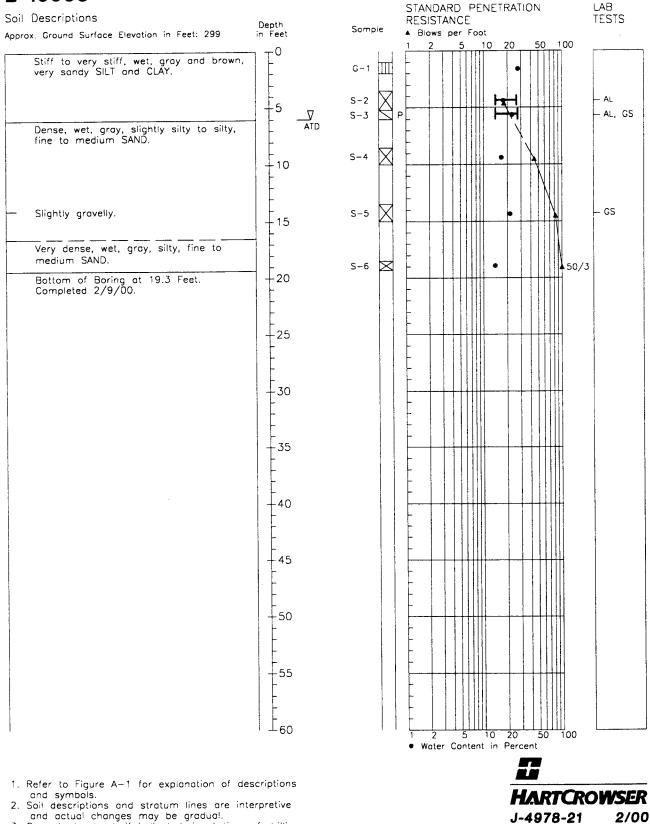
Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
 Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

HARTCROWSER J-4978-21 2/00 Figure A-13

hei 7/30/01 21 LOGS 497821

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Boring Log HC00-B107 N 17398 E 10953



and actual changes may be gradual.
Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

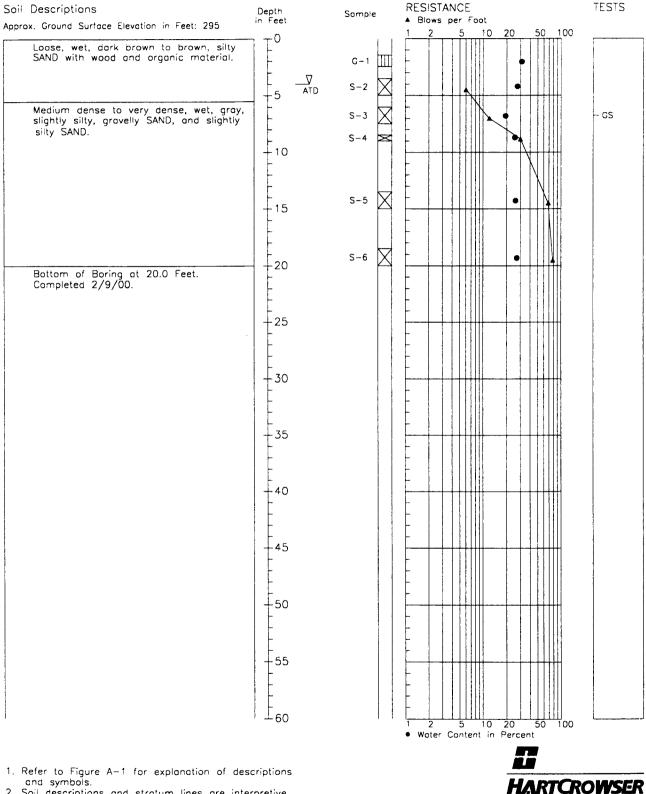
AR 051314

Figure A-14

hel 7/30/01 21 LOGS 497821

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Boring Log HC00-B108 N 17407 E 11067



STANDARD PENETRATION

LAB

2. Soil descriptions and stratum lines are interpretive

- and actual changes may be gradual. 3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

AR 051315

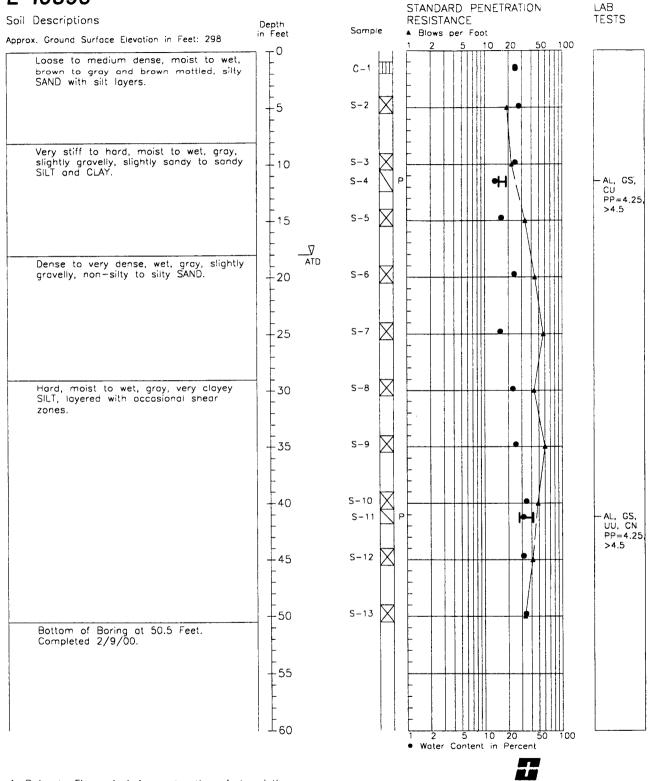
2/00

J-4978-21

Figure A-15

1 het 7/30/01 497821 1.0GS

Boring Log HC00-B110 N 17422 E 10895



1. Refer to Figure A-1 for explanation of descriptions and symbols.

- 2. Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
- and actual changes may be gradual. 3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

HARTCROWSER J-4978-21 2/00 Figure A-16

hei 7/30/01 497821 LOCS

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Boring Log HC00-B111 N 17617 E 11163

E 11163			STANDARD PENETRATION	LAB
Soil Descriptions	Depth	Sample	RESISTANCE	TESTS
Approx. Ground Surface Elevation in Feet: 285 Top of Casing Elevation in Feet: 286.06	in Feet		▲ Blows per Foot 1 2 5 10 20 <u>50 1</u> 00	
Loose, wet, brown, silty SAND.		G-1		
Stiff, wet, brown and gray mottled, slightly sandy, clayey SILT.	5	S-2 S-3	P	- AL, GS, CN PP=3.25
Medium dense, wet, gray, slightly gravelly, silty SAND.		S-4		- GS
Medium dense, wet, gray, slightly clayey, gravelly, very silty SAND.	- - 15 ∇ <u></u>	S-5 S-6	P	- AL, GS, CN
Hard, moist to wet, gray, very clayey SILT with layered sond and silt zones.		S-7 🗙		
	-25	S-8		
— Sheor zone noted.	-30	s-9 🗙		
	-35	S-10 🗙		
	40	S-11 S-12	P	- AL, GS, CU
	45	S-13 🗙		PP=>4.5
— Shear zone noted.	-50	5-14 🗙		
— Shear sane noted.	-55	S-15 🗙		
	-60	S-16 🗙		
	65	S-17 🗙		
	70	S-18 🗙	81	/10
Very dense, wet, gray, silty SAND.		S-19 🔀	- - ● ● 50	/6
Bottom of Boring at 74.5 Feet. Completed 2/1/00.				
I	1 180	ł	1 2 5 10 20 50 100	L
			Water Content in Percent	
			iii	

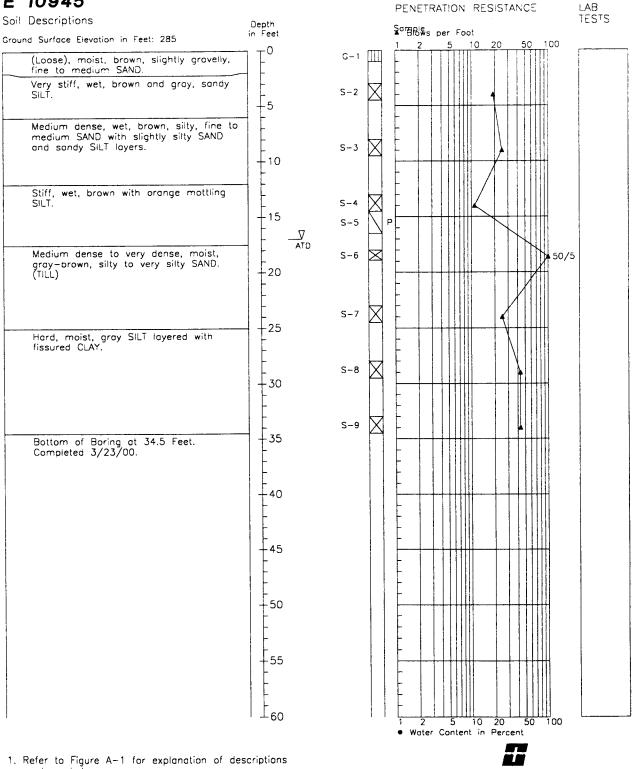
<u>e</u>

hel 7/30/01 1 497821 LOGS

Refer to Figure A-1 for explanation of descriptions and symbols.
 Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
 Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

HARTCROWSER 2/00 J-4978-21 Figure A-17

Boring Log HC00-B113 N 17607 E 10945



PENETRATION RESISTANCE

and symbols. 2. Soil descriptions and stratum lines are interpretive

- and actual changes may be gradual.
 Ground water level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

AR 051318

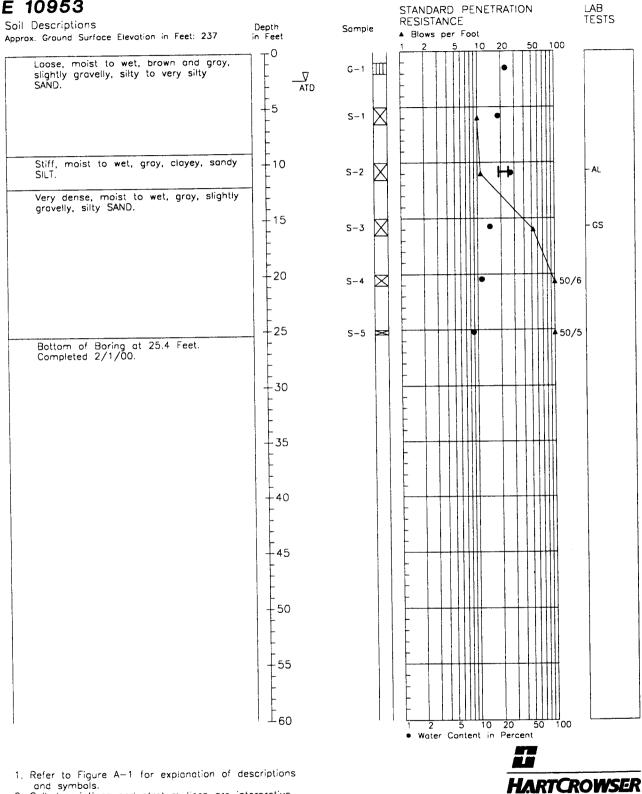
3/00

HARTCROWSER

J-4978-21

= hei 7/30/01 21 LOGS 497821

Boring Log HC00-B114 N 18122 E 10953



2. Soil descriptions and stratum lines are interpretive

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hel 7/30/01 497821 LOCS

and actual changes may be gradual. 3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

AR 051319

J-4978-21

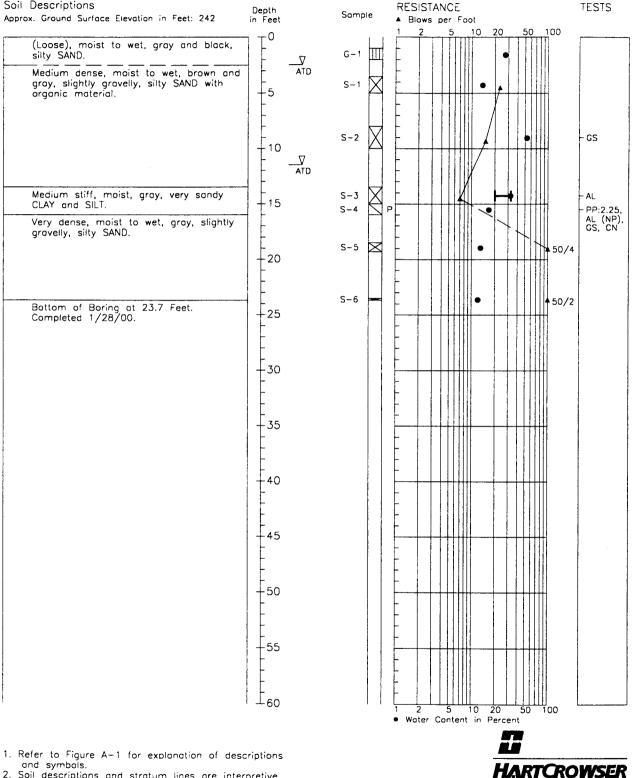
Figure A-19

2/00

Boring Log HC00-B115

N 18225 E 10990

Soil Descriptions



STANDARD PENETRATION

LAB

2. Soil descriptions and stratum lines are interpretive

- and actual changes may be gradual. 3. Groundwater level, if indicated, is at time of arilling (ATD) or for date specified. Level may vary with time.

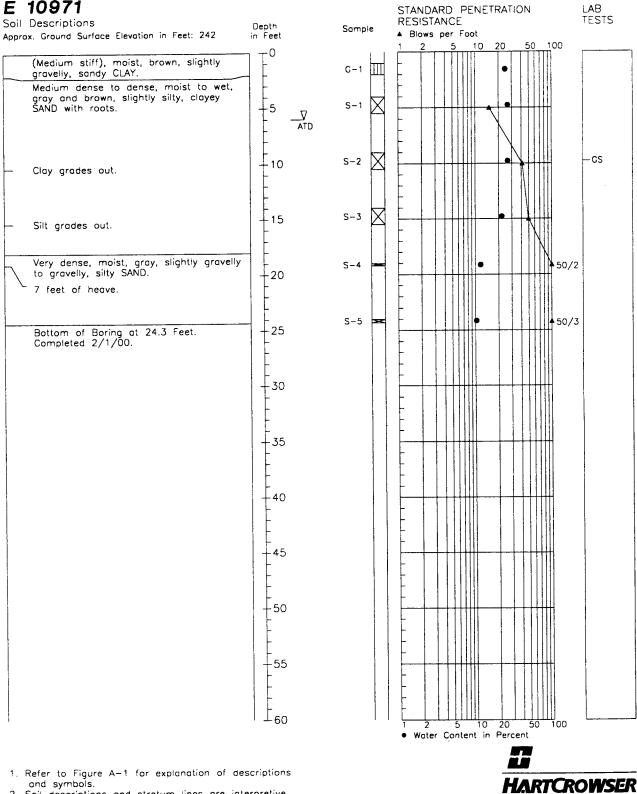
AR 051320

1/00

J-4978-21

1 hei 7/30/01 497821 LOGS

Boring Log HC00-B116 N 18291 E 10971



2. Soil descriptions and stratum lines are interpretive

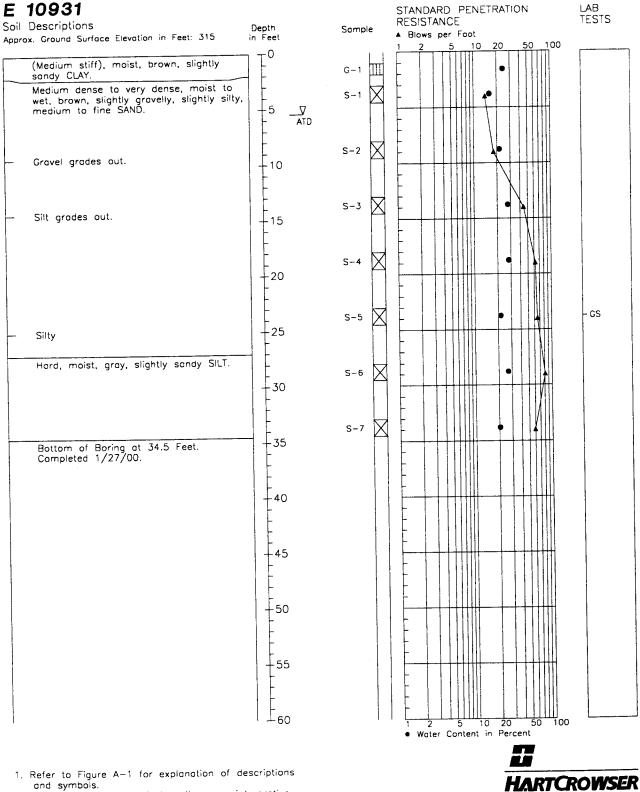
and actual changes may be gradual. 3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

J-4978-21 2/00 Figure A-21

Ĩ hel 7/30/01 497821 LOGS

Boring Log HC00-B117

N 17271



and symbols.

- 2. Soil descriptions and stratum lines are interpretive
- and actual changes may be gradual. 3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

1/00 J-4978-21 Figure A-22

AR 051322

[hei 7/30/01 497821 LOCS

Boring Log HC00-B118 N 17456 E 10947

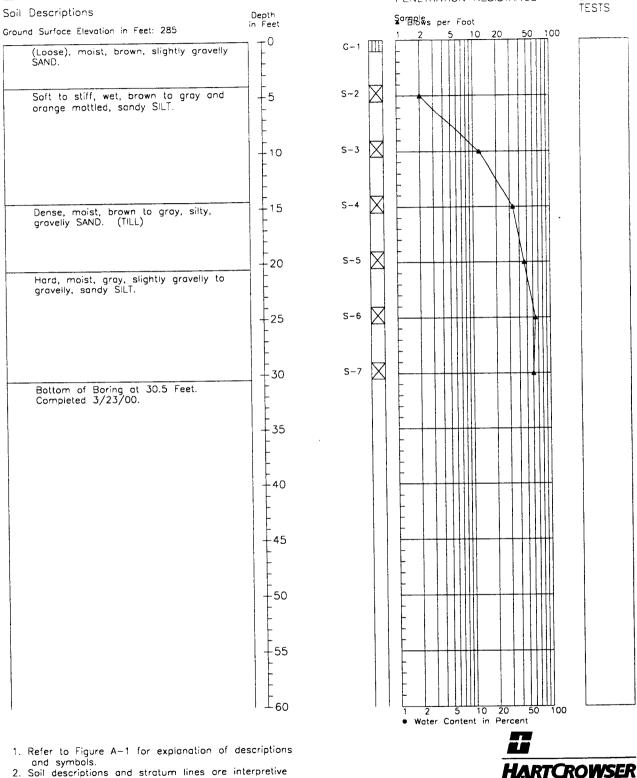
10947 Descriptions rox. Ground Surface Elevation in Feet: 298 of Casing Elevation in Feet: 298.61	Depth in Feet	Sample	STANDARD PENETRATION RESISTANCE ▲ Blows per Foot 1 2 5 10 20 50 100	LAB TESTS
(Loose), moist, dark brown, slightly silty SAND.		G-1		
Medium stiff, tan and orange mottled, slightly sandy, clayey SILT with organic material.		S-1 S-2	P	-AL, GS.
(Medium dense), wet, brown, slightly gravelly SAND.		S-3 A		UU, CN
(Very stiff), moist, gray, slightly sandy, slightly gravelly CLAY.		і вКл		-PP: 1.0 1.0, 2.0 -AL, GS
Hard, moist, gray, sandy CLAY to very clayey SILT.	-15	S-5 S-6		- AL
				4.5+, 4 AL, GS, CN
(Loose to) very dense, wet, gray SAND. Note: 5–7 N—value apparently not representative of in situ density, due to disturbance by groundwater.	-20 ATD	S−7 X		GS
	-25	S-8		
Hard, moist, gray, clayey SILT.	-30	S-9 X		
	-35	s-10		
Bottom of Boring at 35.5 Feet. Completed 1/27/00.				
	 40 -			
	-50			
	-55			
	I ⊥ ₆₀		1 2 5 10 20 50 100 • Water Content in Percent	L
Refer to Figure A-1 for explanation of descr and symbols. Soil descriptions and stratum lines are inter			HARTCI	ROWSE
and actual changes may be gradual. Groundwater level, if indicated, is at time of			J-4978-2	

Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

hei 7/30/01 1=1 497821 LOGS

AR 051323

Boring Log HC00-B119 N 17560 E 10894



PENETRATION RESISTANCE

LAB

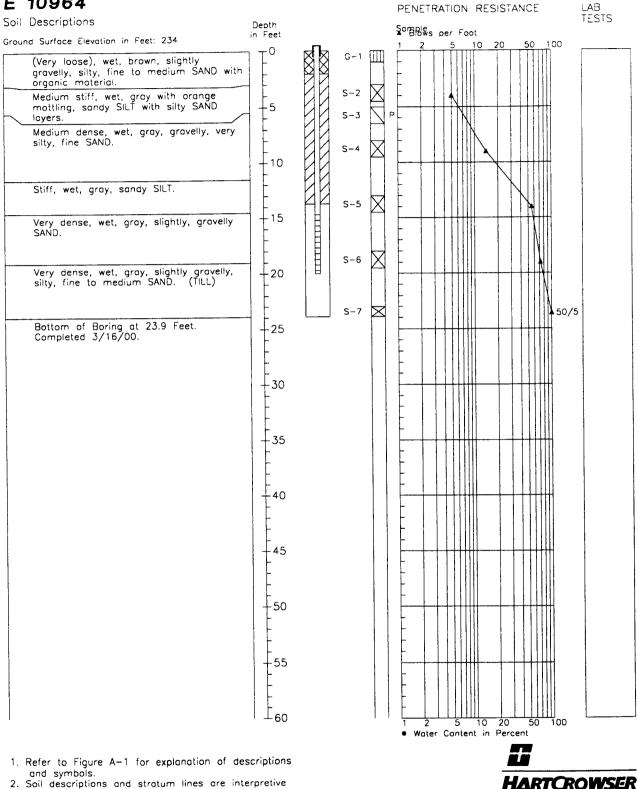
2. Soil descriptions and stratum lines are interpretive

and actual changes may be gradual. 3. Ground water level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

3/00 J-4978-21 Figure A-24

я, hel 7/30/01 497821 LOGS

Boring Log HC00-B120 N 17834 E 10964



2. Soil descriptions and stratum lines are interpretive

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hei 7/30/01 497821 LOGS

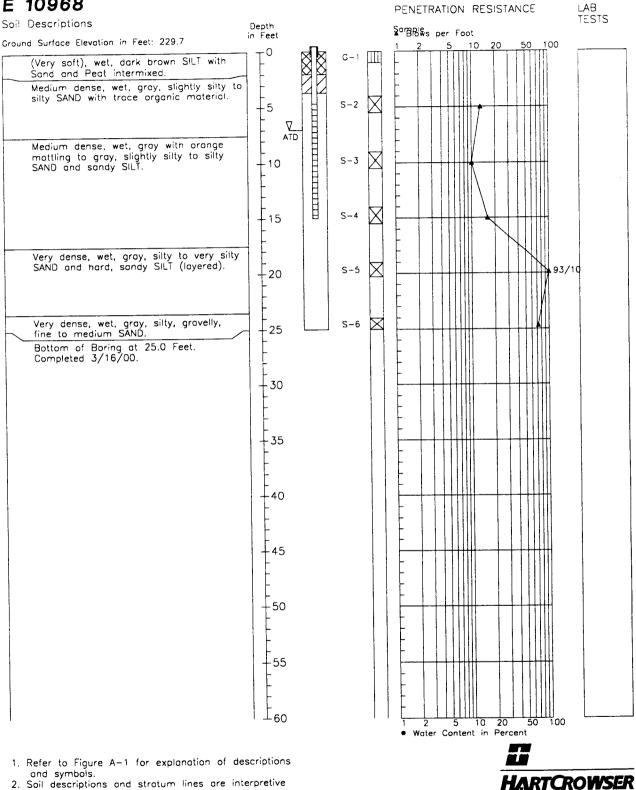
and actual changes may be gradual. 3. Ground water level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

Figure A-25

3/00

J-4978-21

Boring Log HC00-B121 N 17881 E 10968



PENETRATION RESISTANCE

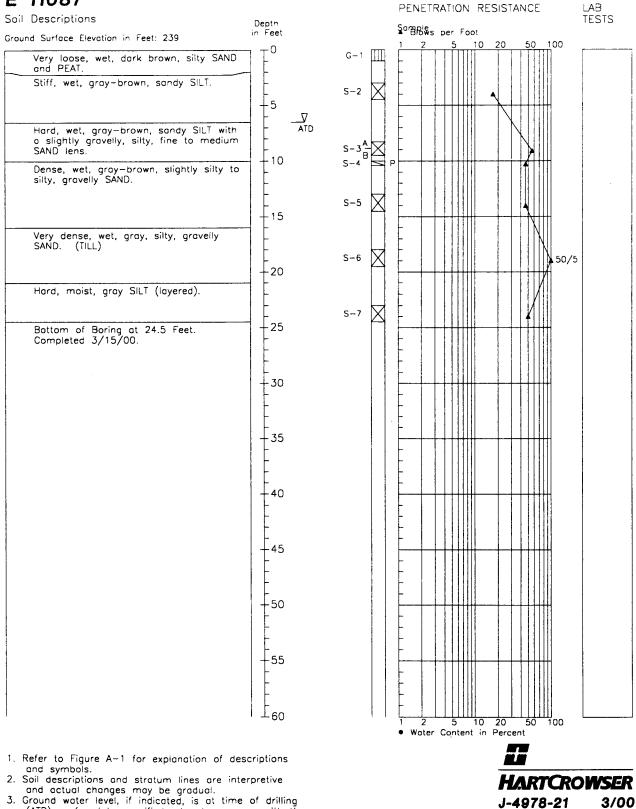
and actual changes may be gradual. 3. Ground water level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

AR 051326

J-4978-21 Figure A-26 3/00

2 hei 7/30/01 21 LOGS 497821

Boring Log HC00-B122 N 17840 E 11087



and actual changes may be gradual.
 Ground water level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

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hel 7/30/01 497821 LOGS

AR 051327

Boring Log HC00-B123 N 17958 E 11114 LAB PENETRATION RESISTANCE TESTS Soil Descriptions Depth Sangpies per Foot in Feet Ground Surface Elevation in Feet: 234.7 10 20 50 100 т0 G-1 III \boxtimes (Soft). (PEAT) wet, dark brown, sandy SILT Ø V. \sim S-2 Loose, wet, brown, very silty, fine to medium SAND with sandy SILT layers. ATD **4**5 5-3 Soft to medium stiff, wet, brown to gray, sandy SILT with interbedded layers of peat, sand and silty sand. 5-4 MEdium stiff, wet, gray and brown, slightly sandy PEAT with very silty SAND S-5 -10 layers. Medium dense, wet, gray, silty, gravelly SAND with sandy SILT layers. S-6 S-7 \square -15 Dense, wet, gray, slightly gravelly, very silty SAND. X S-8 +20 Stiff, wet, gray, sandy CLAY. \square S-9 -25 S-10 Very dense, moist, gray, slightly silty to silty, gravelly SAND. \$ 50/6 S-11 2 +30 \$50/6 S-12 🖂 35 Bottom of Boring at 34.5 Feet. Completed 3/22/00. 40 +45 -50 -55 160 ໂດດ 10 20 50 • Water Content in Percent 1. Refer to Figure A-1 for explanation of descriptions and symbols. HARTCROWSER

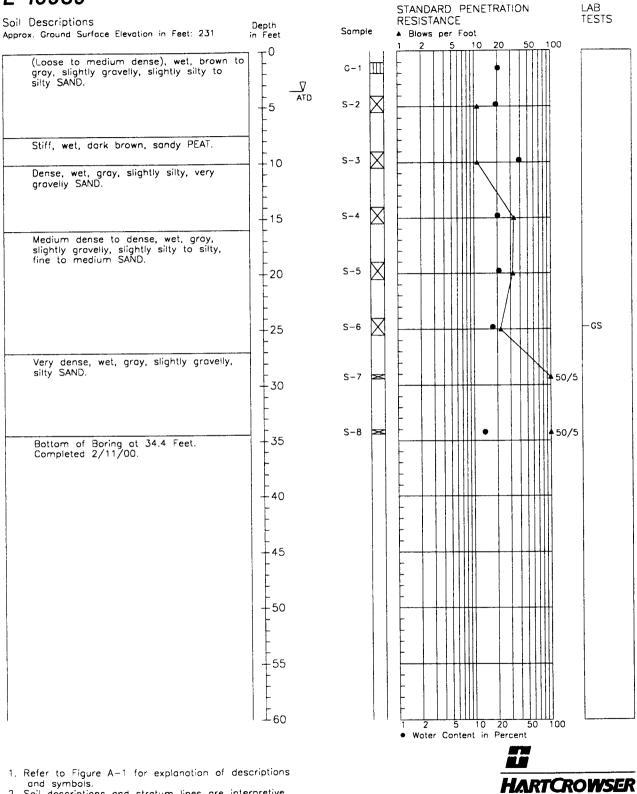
- 2. Soil descriptions and stratum lines are interpretive
- and actual changes may be gradual. 3. Ground water level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

AR 051328

J-4978-21 Figure A-28 3/00

H hei 7/30/01 21 LOCS 497821

Boring Log HC00-B124 N 18016 E 10939



and symbols. 2. Soil descriptions and stratum lines are interpretive

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hei 7/30/01 497821 LOCS

and actual changes may be gradual. 3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

AR 051329

2/00

J-4978-21

Boring Log HC00-B125 N 17686 E 10916

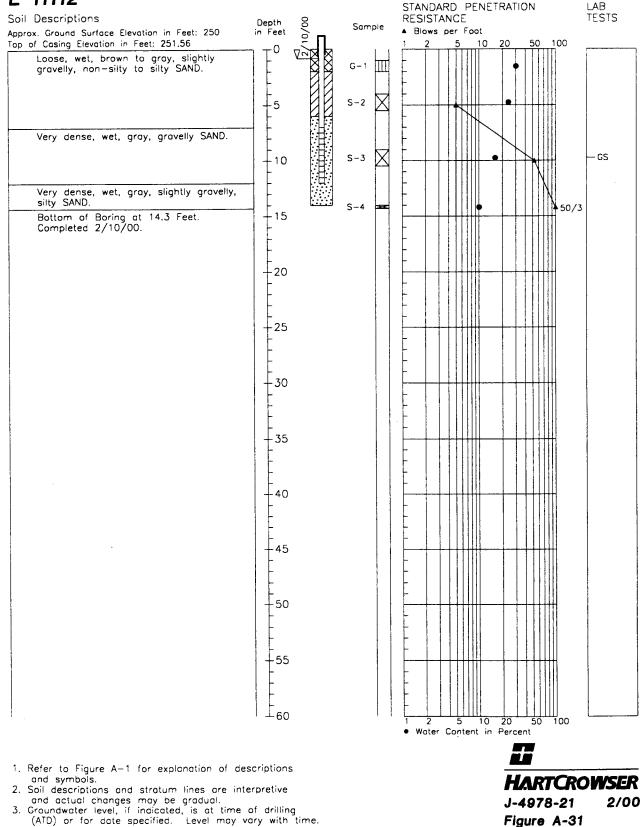
S ^{or} BR%s per Foot G-1 S-2 S-1A S-3 S-4 S-5 S-5 S-5 S-5 S-5 S-5 S-5 S-5
G-1 S-2 S-1A S-3 S-4 S-4 S-4 S-50/5
S-3 X 50/4
S-5 X

(ATD) or for date specified. Level may vary with time.

hei 7/30/01 1=1 497821 LOCS

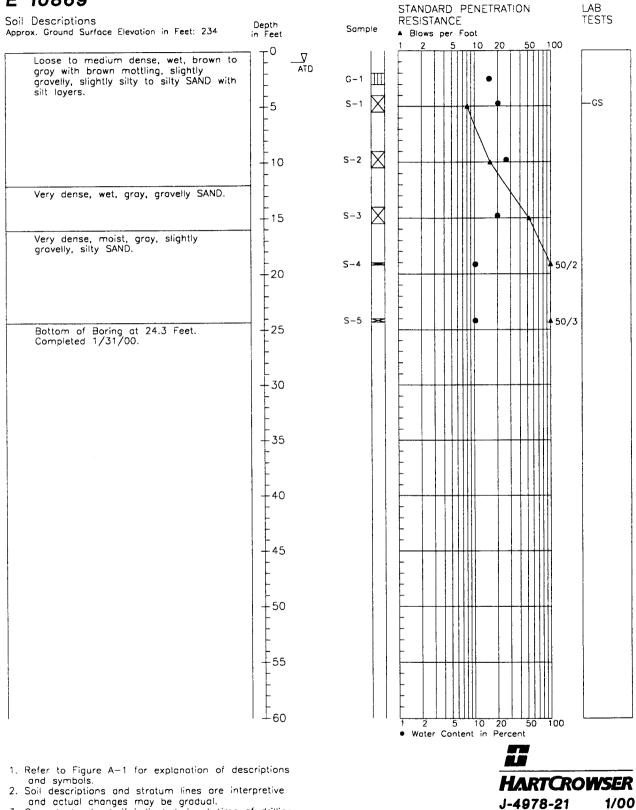
AR 051330

Boring Log HC00-B126 N 18232 E 11112



hei 7/30/01 1=1 497821 LOGS

Boring Log HC00-B127 N 18215 E 10869

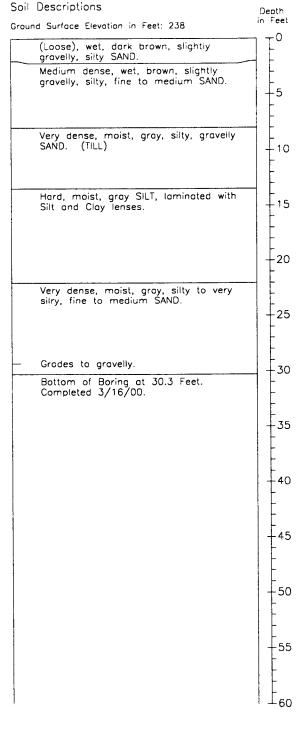


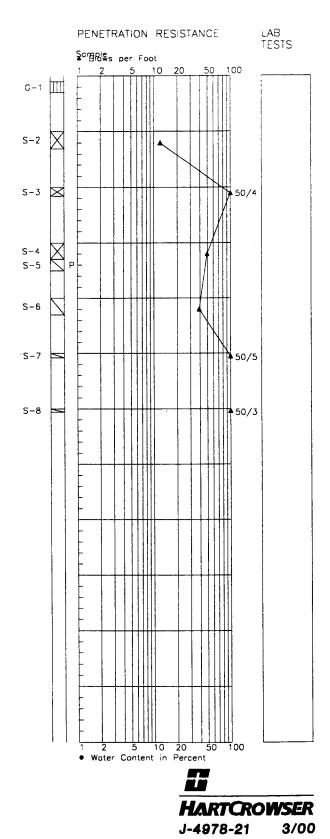
and actual changes may be gradual. 3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

Figure A-32

hel 7/30/01 1=1 497821 ∟0GS

Boring Log HC00-B128 N 17786 E 10918





and symbols.
Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
Ground water level, if indicated, is at time of drilling

(ATD) or for date specified. Level may vary with time.

1. Refer to Figure A-1 for explanation of descriptions

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hel 7/30/01 497821 LOGS

AR 051333

Boring Log HC00-B129 N 18256 E 11004

E 11004 Soil Descriptions Approx. Ground Surface Elevation in Feet: 243 Top of Casing Elevation in Feet: 245.83	Depth 00/01	Somple	STANDARD PENETRATIC RESISTANCE ▲ Blows per Foot 1 2 5 10 20	DN LAB TESTS
Medium dense, wet, brown to gray medium to fine SAND.		G-1		
(Stiff), wet, gray, slightly sandy CLAY.	TF MM	K3	⊳⊢ ⊢⊣	- AL, GS,
Medium dense, wet, brown, slightly silty, medium to fine SAND.		S-4		UU PP=1.75
Very dense, wet, gray, silty, slightly gravelly to gravelly SAND.		S-5		97/10
Bottom of Boring at 19.3 Feet. Completed 2/10/00.		S-6 ≫		4 50/3
	-25 -30 -35 -40 -40 -55 -55 	d hole		50 100
1. Refer to Figure A-1 for explanation of de	scriptions		Water Content in Percer	nt 11
 and symbols. Soil descriptions and stratum lines are interand actual changes may be gradual. Groundwater level, if indicated, is at time 	erpretive			ARTCROWSER 978-21 2/00

3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

hei 7/30/01 1=1 497821 LOCS

AR 051334

Boring Log HC00-B130 N 17901 E 10839

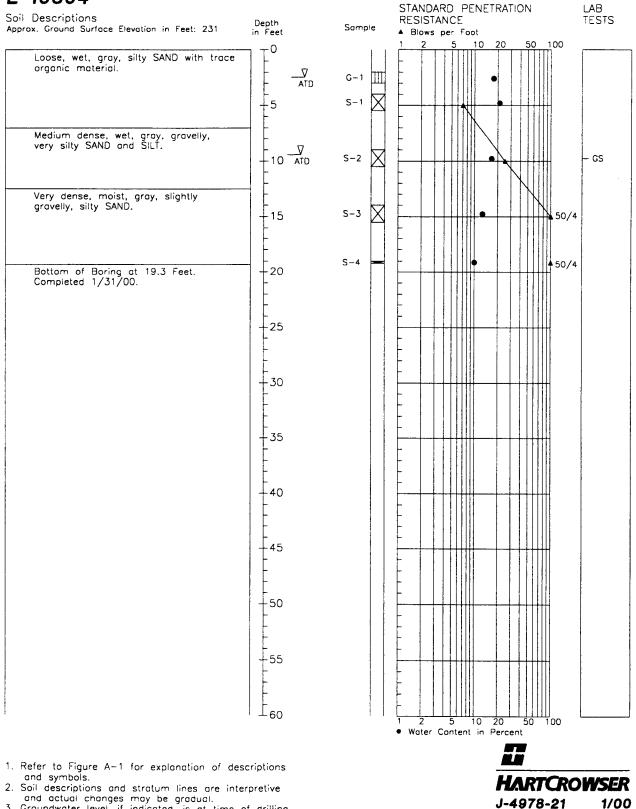
round Surface Elevation in Feet: 223.1	in Feet	SanBhows per Foot 1 2 <u>5 10</u>	20 50 100
(Medium dense), wet, gray and brown to gray, silty to very silty, fine to medium SAND with sandy SILT and SAND layers.	S-2		
Very dense, moist to wet, gray, silty, gravelly SAND. (TILL)	- ∇ ATD S-4 X - 10 S-5 ×		7 4/10 ▲ 50/3
Bottom of Boring at 14.8 Feet. Completed 3/21/00.	15 S-6 🔀		▲ 50/3
	- 20		
	-25		
	- 30 30		
	- 35		
	L - 40 -		
	L + 45 L		
	+50 L		
	- 55		
	60	1 2 5 10 • Water Content in	20 50 100
		- Water Content M	
 Refer to Figure A-1 for explanation of de and symbols. Soit descriptions and stratum lines are inti 			HARTCROWS

. Ground water level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

hei 7/30/01 1=1 497821 LOCS

AR 051335

Boring Log HC00-B131 N 18329 E 10804



and actual changes may be gradual.
Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

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hel 7/30/01 21 LOCS

497821

AR 051336

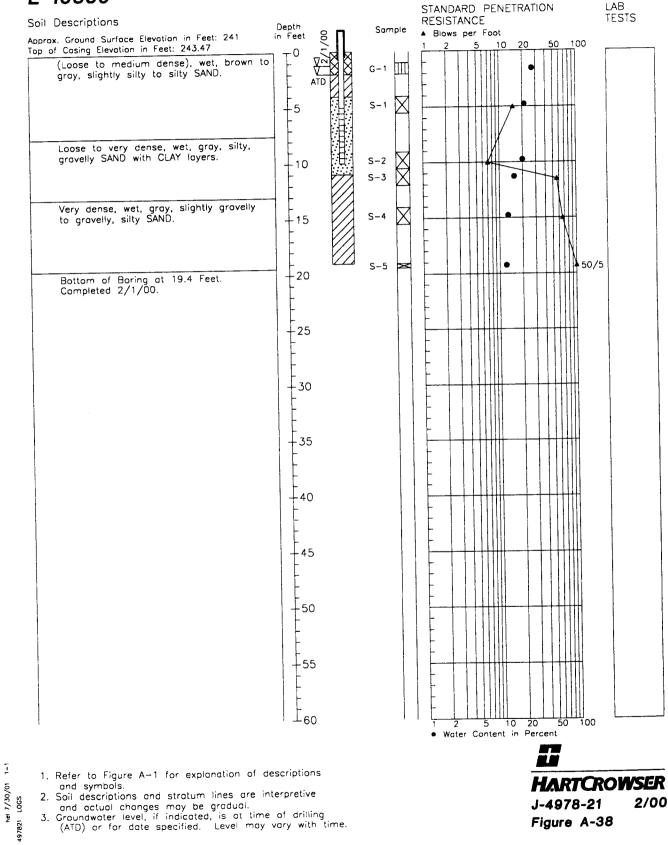
Boring Log HC00-B132 N 17904 F 10924

E 10924		PENETRATION RESISTANCE	LAB TESTS
Soil Descriptions	Depth in Feet	SanBles per Foot	
Ground Surface Elevation in Feet: 227.4 (Loose), wet, dark brown, silty SAND and very soft, wet, gray, slightly sandy SILT with Peat layers.	G-1)
Very dense, wet, gray SAND with Silt	$\begin{array}{c c} +5 & 5-2 \\ 5-3 \\ 5-4 \\ -10 \\ \overline{} & 5-5 \end{array}$		
Medium stiff, wet, gray, slightly sandy	ATD 5-6 - 5-6 - 15 5-7 S-8		
CLAY. Very dense, moist, gray, slightly gravelly, silty, fine to medium SAND.			60/3
Bottom of Boring at 25.5 Feet. Completed 3/20/00.			
	+ 30		
	+ 35		
	+40 [-		
	- 45 		
	- 50		
	-55		
	F 60	Water Content in Percent	00
 Refer to Figure A-1 for explanation of d and symbols. Soil descriptions and stratum lines are in 		HART	ROWS
and actual changes may be gradual. 3. Ground water level, if indicated, is at tim (ATD) or for date specified. Level may		J-4978- Figure A	21 3.

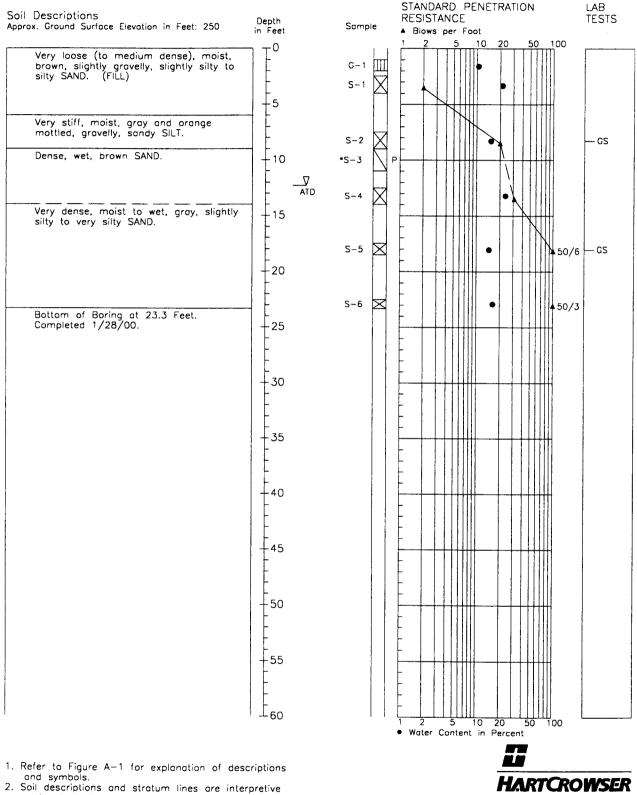
AR 051337

Boring Log HC00-B133 N 18471 E 10859

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Boring Log HC00-B134 N 18438 E 10953

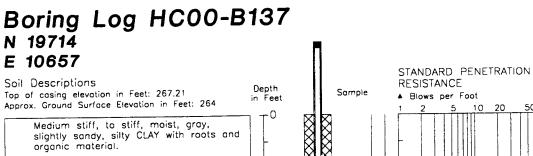


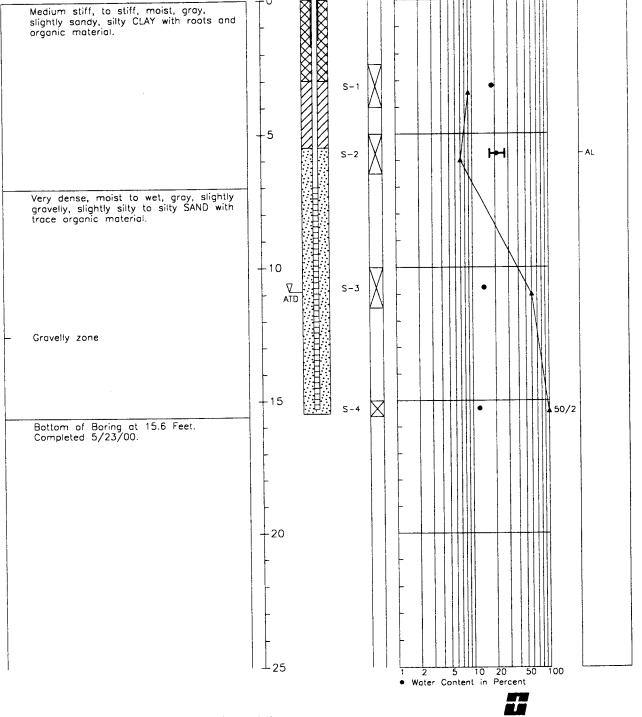
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hei 7/30/01 497821 LOCS

and actual changes may be gradual.
 Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

J-4978-21 1/00 Figure A-39





1. Refer to Figure A-1 for explanation of descriptions and symbols.

2. Soil descriptions and stratum lines are interpretive

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hei 7/30/01 BORINCS and actual changes may be gradual.
Ground water level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

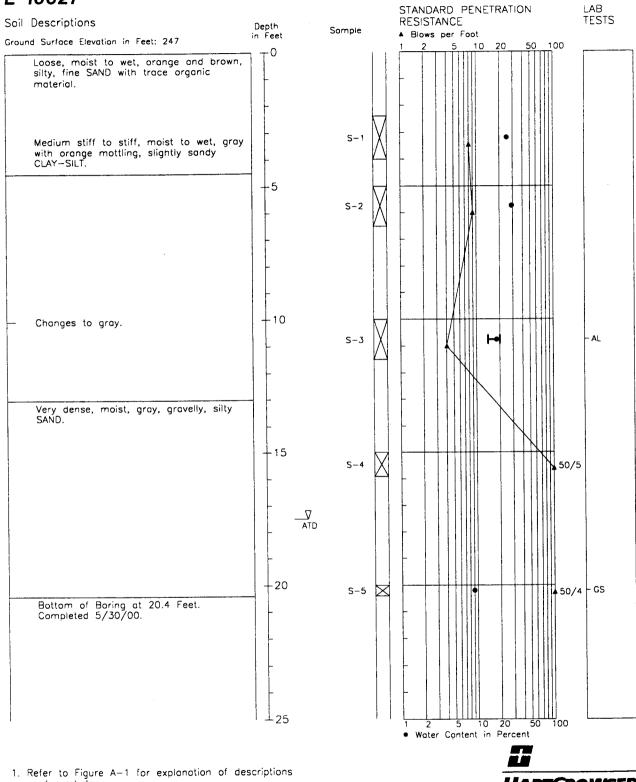
5/00 J-4978-26 Figure A-40

HARTCROWSER

LAB TESTS

50 100

Boring Log HC00-B138 N 19096 E 10627



1=1 hei 7/30/01 BORINGS

and symbols. 2. Soil descriptions and stratum lines are interpretive

and actual changes may be gradual. 3. Ground water level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

HARTCROWSER J-4978-26 5/00 Figure A-41

Boring Log HC00-B139 N 18759 E 10889

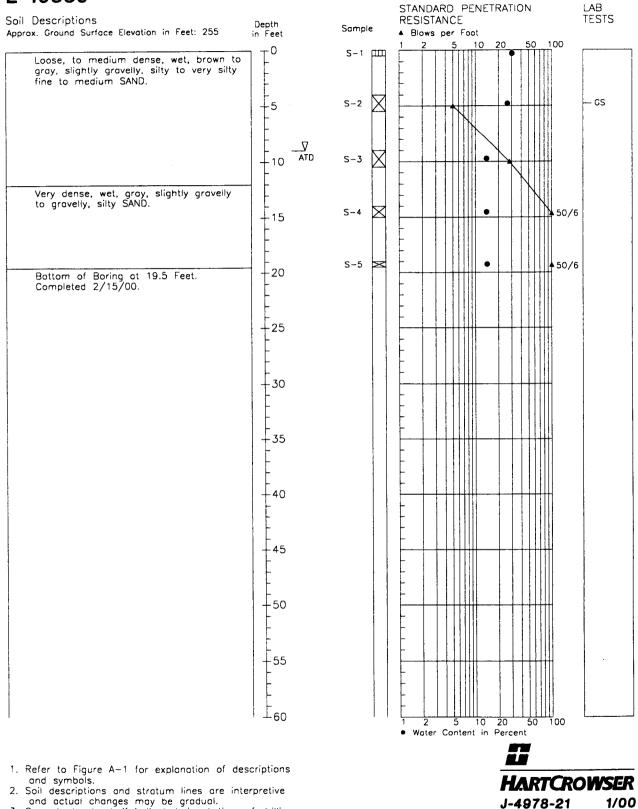


Figure A-42

- and actual changes may be gradual.
 Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

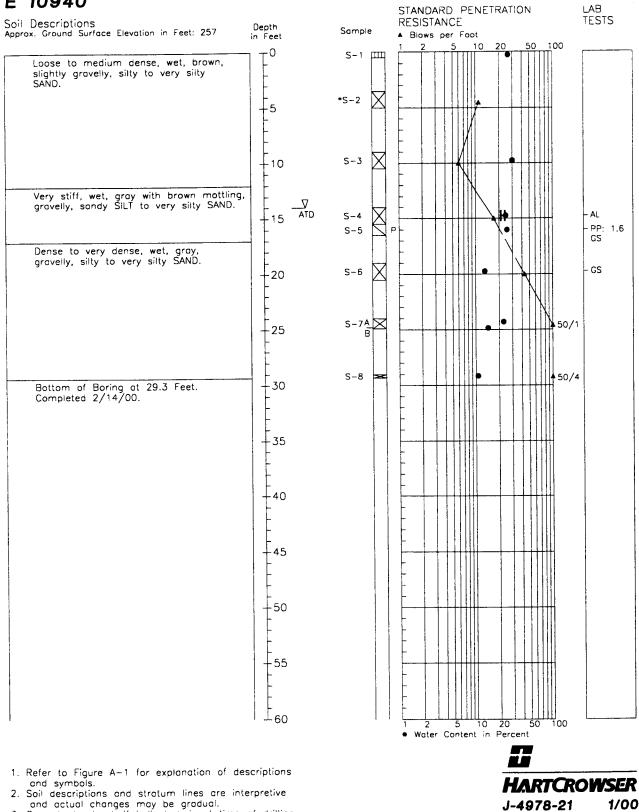
2 hel 7/30/01 21 LOGS 497821

Boring Log HC00-B140 N 18498 E 10940

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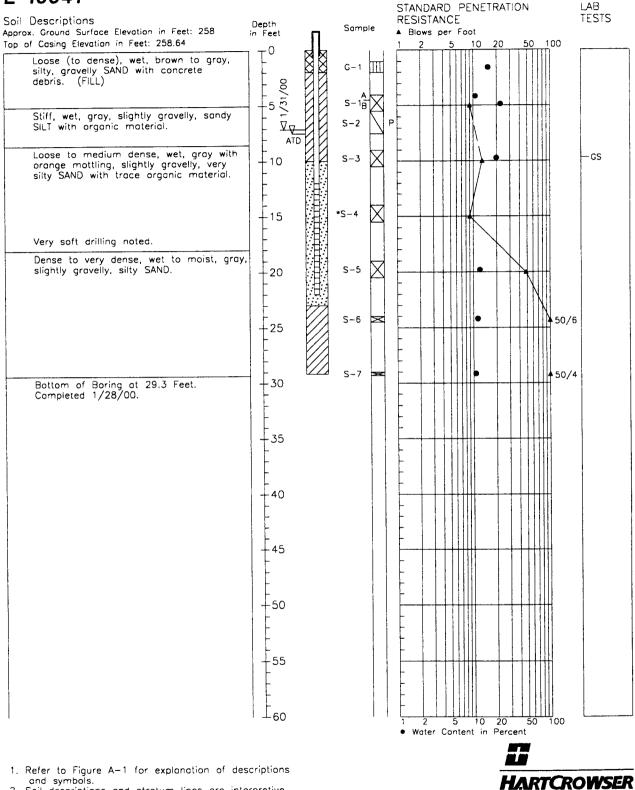
hel 7/30/01 497821 LOGS



and actual changes may be gradual. 3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

AR 051343

Boring Log HC00-B141 N 18597 E 10941



and symbols. 2. Soil descriptions and stratum lines are interpretive

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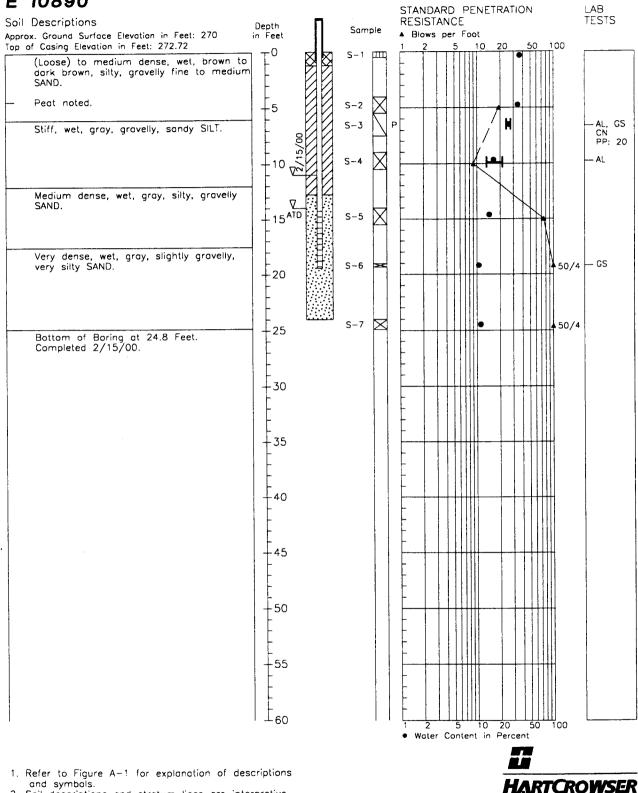
hei 7/30/01 497821 LOGS

and actual changes may be gradual. 3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

AR 051344

J-4978-21 Figure A-44 1/00

Boring Log HC00-B142 N 19263 E 10890



and symbols.2. Soil descriptions and stratum lines are interpretive and actual changes may be gradual.

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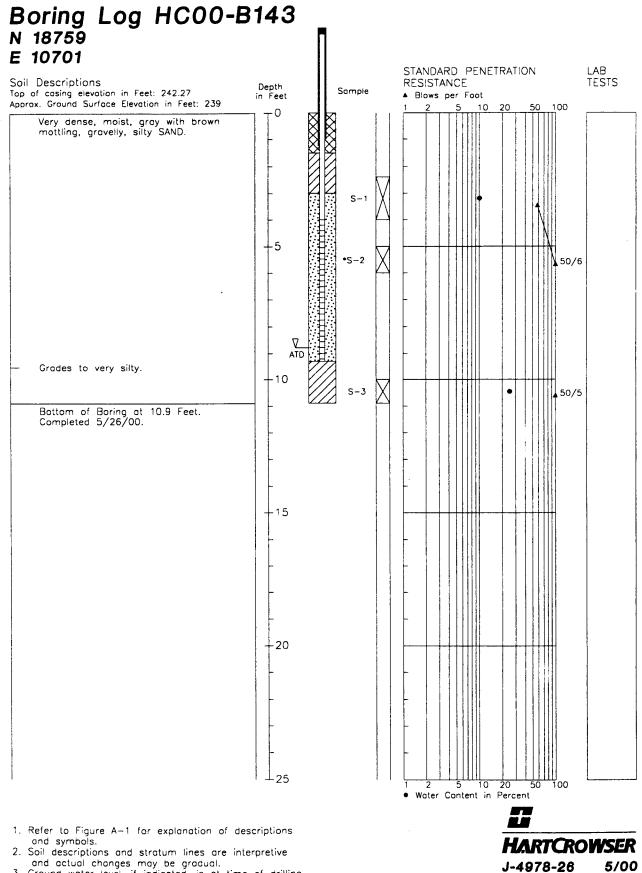
hel 7/30/01 497821 1.0GS

and actual changes may be gradual. 3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

AR 051345

1/00

J-4978-21



- and actual changes may be gradual.
 3. Ground water level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

AR 051346

Figure A-46

hei 7/30/01 1=1 497826 BORINGS

Boring Log HC00-B144 N 18792 E 10787

rox. Ground Surface Elevation in Feet: 247 ir	Depth n Feet	Sample	STANDARD PENETF RESISTANCE A Blows per Foot		LAB TESTS
of Casing Elevation in Feet: 248.99 Loose to medium dense, dark brown to gray, slightly gravelly, non—silty to silty SAND with trace organic material.		S-1 III		20 50 100	
	-5	S-2			– CS
Very dense, wet, gray, silty, slightly gravelly to very gravelly SAND.		s-3			
Bottom of Boring at 14.4 Feet. Completed 2/15/00.	- <u>()</u> - 15	S-4 🕿		50/5	5
	- 				
	- 				
	- 30				
	- - - -				
	-40 				
	- - - -				
	-50				
	1 ₆₀		- 1 2 5 10 • Water Content in F		
Refer to Figure A-1 for explanation of descionary symbols.				HARTCR	OW
Soil descriptions and stratum lines are interp and actual changes may be gradual. Groundwater level, if indicated, is at time of (ATD) or for date specified. Level may var	drilling			J-4978-21 Figure A-47	1

hei 7/30/01 1±1 497821 LOGS

Boring Log HC00-B145 N 18964 E 10866

E 10866 Soil Descriptions Approx. Ground Surface Elevation in Feet: 263	Depth in Feet	Sample	STANDARD PENETRATION RESISTANCE ▲ Blows per Foot 1 2 5 10 20 50 10	LAB TESTS
Top of Casing Elevationin Feet: 265.11 Loose, wet, dark brown to gray and brown, slightly gravelly, silty SAND with trace organic material.		G-1		
Medium dense, wet, tan, slightly gravelly, silty SAND.	10	s-3 🗙		– GS
Very dense, moist, gray, slightly gravelly, silty SAND.	-15	S-4 X		88/10
Bottom of Boring at 19.8 Feet. Completed 2/7/00.	- 20 - 20 - 25	s−5 ⊠		50/3
	- 30			
	40			
	+45			
	-55			
	60		Water Content in Percent	00
 Refer to Figure A-1 for explanation of des and symbols. Soil descriptions and stratum lines are inte and actual changes may be gradual. Groundwater level, if indicated, is at time (ATD) or for date specified. Level may v 	erpretive of drilling		HARTO J-4978-2 Figure A	

hei 7/30/01 1=1 497821 LOCS

Boring Log HC00-B146 N 19242 E 10784

c. Ground Surface Elevation in Feet: 261 f Casing Elevation in Feet: 263.55	in Feet	Sample	▲ Blows per Foot 1 2 5 10	20 50 100	
Soft, wet, dark brown, sandy PEAT.		S-1 III			
Stiff, wet, gray, gravelly, very sandy SILT and CLAY with trace organic material.	2/14/00 5	S-2 S-3	P-		- AL
Very dense, wet, gray, slightly gravelly, very silty SAND.		S-4			- G
		S-5 🗙		50/	5
Bottom of Boring at 14.4 Feet. Completed 2/14/00.	+15				
	-20				
	-25				
	+ 30				
	-40				
	+45 				
	+50				
	上 ₆₀			20 50 100	
			 Water Content in 	Percent	

Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
 Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

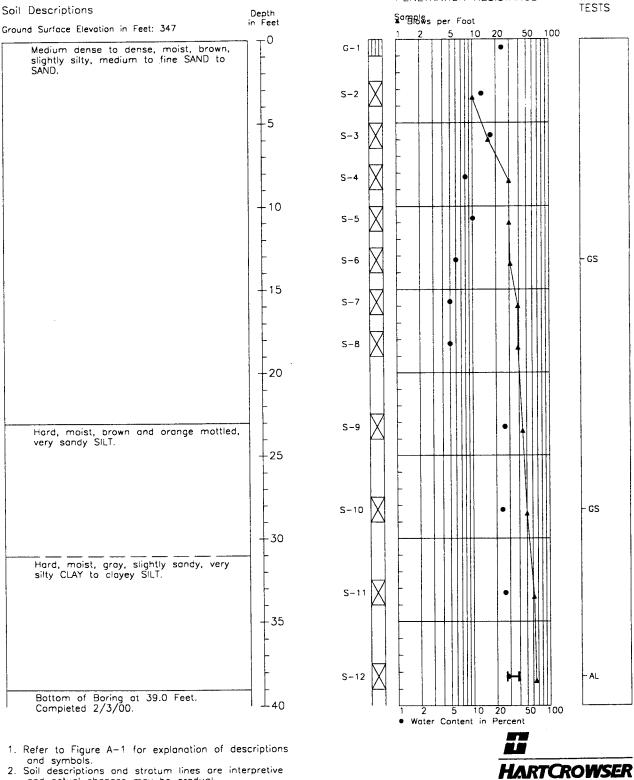
J-4978-21 2/00 Figure A-49

AR 051349

hel 7/30/01 497821 LOGS

1

Boring Log HC00-B147 N 16694 E 10507 Soil Descriptions



PENETRATION RESISTANCE

LAB

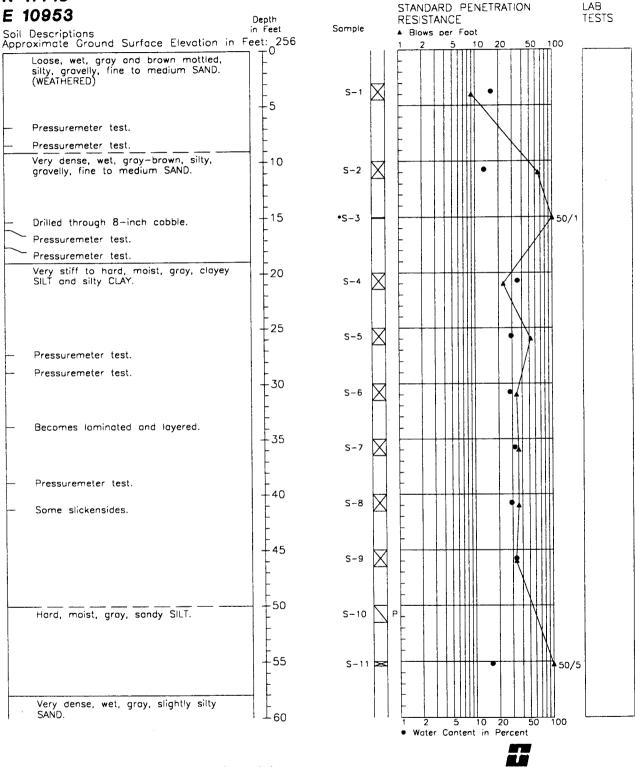
2. Soil descriptions and stratum lines are interpretive

and actual changes may be gradual. 3. Ground water level, if indicated, is at time of drilling

(ATD) or for date specified. Level may vary with time.

2/00 J-4978-22 Figure A-50

Boring Log HC00-B221 N 17715 E 10953



1. Refer to Figure A-1 for explanation of descriptions and symbols.

- 2. Soil descriptions and stratum lines are interpretive
- and actual changes may be gradual. 3. Ground water level, if indicated, is at time of drilling

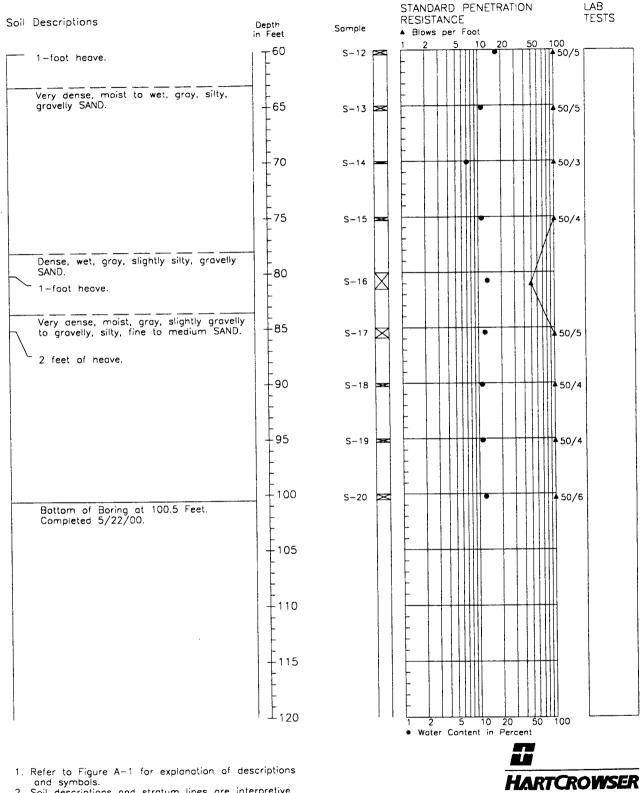
hei 7/30/01 1=1 497827\LOGS.dwg

(ATD) or for date specified. Level may vary with time.

HARTCROWSER 5/00 J-4978-27 1/2 Figure A-51

AR 051351

Boring Log HC00-B221



2. Soil descriptions and stratum lines are interpretive

l = 1 5 M p

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and actual changes may be gradual. 3. Ground water level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

AR 051352

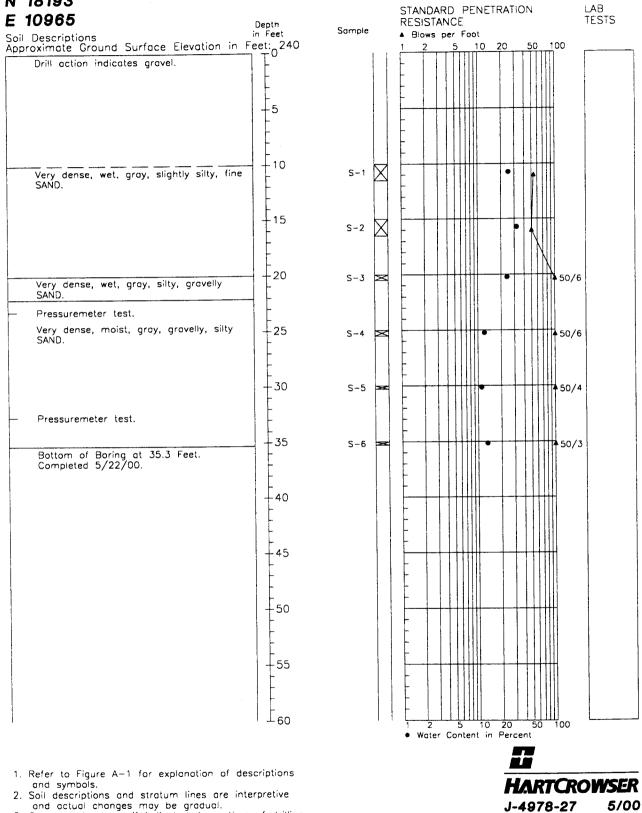
J-4978-27

Figure A-51

5/00

2/2

Boring Log HC00-B224 N 18193



and actual changes may be gradual. 3. Ground water level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

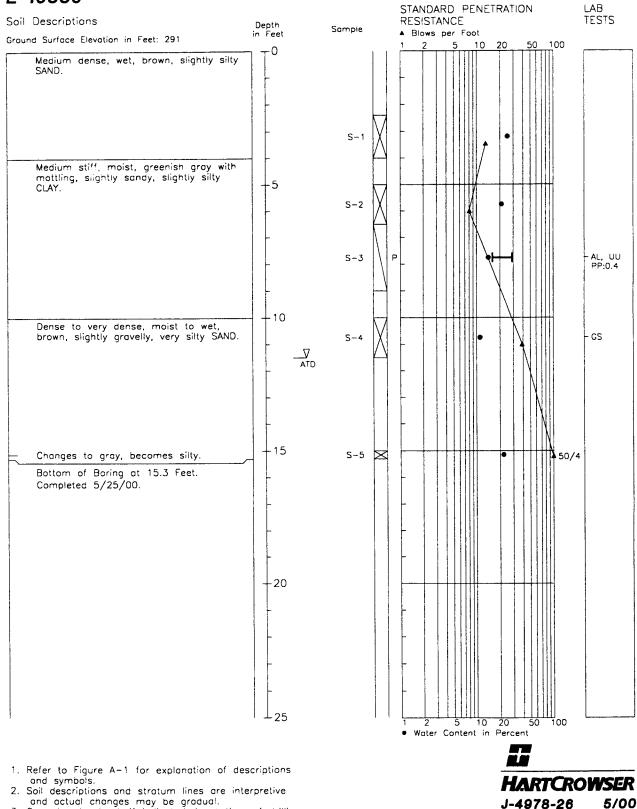
1-

hel 7/30/01

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AR 051353

Boring Log HC00-B300 N 19818 E 10889



and actual changes may be gradual.
Ground water level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

Ē

hei 7/30/01 1= 497826 BORINGS

AR 051354

Figure A-53

LAB

Monitoring Well Log HC00-B301

Northing (ft): 21476

Water Content in Percent	Northing (ft): 21476 Easting (ft): 10560	D 11		STANDARD PENETRATION RESISTANCE	LAB TESTS
(Stiff, damp, brown, sandy SILT with trace organics. 0 0 0 Medium dense, most, brown and gray, silty SAND with some peet over (soft) most, brown PEAT. 0 0 0 Stiff, most, gray to brown SILT with some PEAT and trace organics. 10 5 0 5 Loose to medium dense, wei, brown to gray, fine to coarse SAND. 5 5 7 4 Hard, most, gray, slightly gravelly, slightly sity, fine to coarse SAND. 5 9 4 5 Bottom of Boring at 24.0 Feet. Completed 08/11/00. 20 5 5 7 4 -30 -30 -35 -35 -40	Soil Descriptions Ground Surface Elevation in Feet: 264.65	in Feet	Sample		
Stiff: moist, gray to brown SILT with some PEAT and trace organics. 10 4. S-4. 5.6 5.6 Loose to medium dense, wet, brown to gray, fine to coarse SAND. 5.7 5.7 Hard, moist, gray, slightly gravelly, sandy SILT. 5.8 5.9 Dense, moist to wet, gray, slightly gravelly. slightly silty, fine to coarse SAND. 5.9 Bottom of Boning at 24.0 Feet. Completed 08/11/00. 25	(Stiff), damp, brown, sandy SILT with trace		G-1		
Siff, moist, gray to brown SLI with some PEAT and trace organics. Image: some source organics of the some source organics of the source org of the source organics of the source organi	SAND with some peat over (soft) moist, brown PEAT.		4. S-1A S-3 4. S-2A S-4 4. S-3A		
Hard, moist, gray, slightly gravelly, sandy SILT. Dense, moist to wet, gray, slightly gravelly. slightly silty, fine to coarse SAND. Bottom of Boring at 24.0 Feet. Completed 08/11/00. 	PEAT and trace organics.				
Hard, moist, gray, slightly gravelly, sandy SILT. Dense, moist to wet, gray, slightly gravelly. slightly silty, fine to coarse SAND. Bottom of Boring at 24.0 Feet. Completed 08/11/00. 	Loose to medium dense, wet, brown to gray, fine to coarse SAND.	- 15			
Bottom of Boring at 24.0 Feet. Completed 08/11/00.			S-9		
Completed 08/11/00.	Dense, moist to wet, gray, slightly gravelly, slightly silty, fine to coarse SAND.		S-10		
		- - - - - - - - - - - - - - - - - - -			



Figure A-54

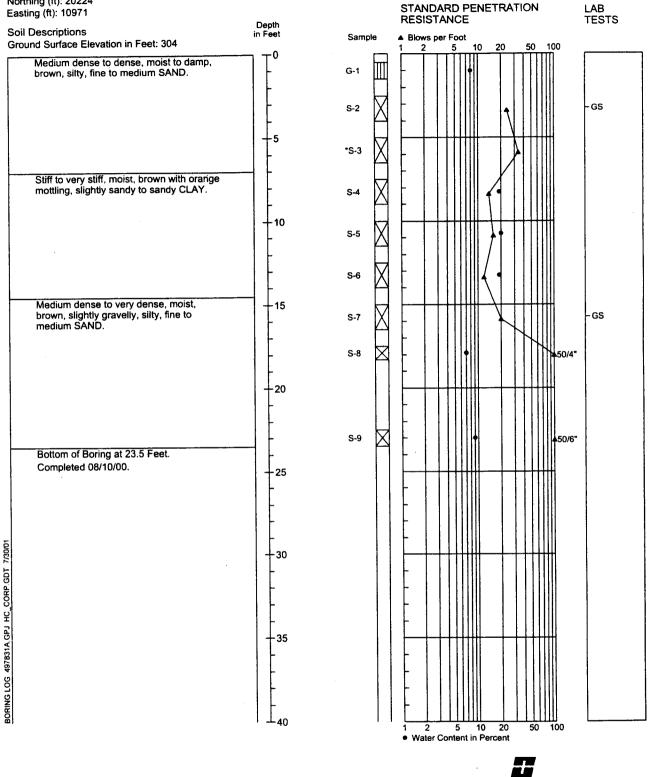
 Refer to Figure A-1 for explanation of descriptions and symbols.
 Soil descriptions and stratum lines are interpretive and actual changes may be gradual.

Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

Boring Log HC00-B303

Northing (ft): 20224

Easting (ft): 10971



 Refer to Figure A-1 for explanation of descriptions and symbols.
 Soil descriptions and stratum lines are interpretive and actual changes may be gradual.

3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

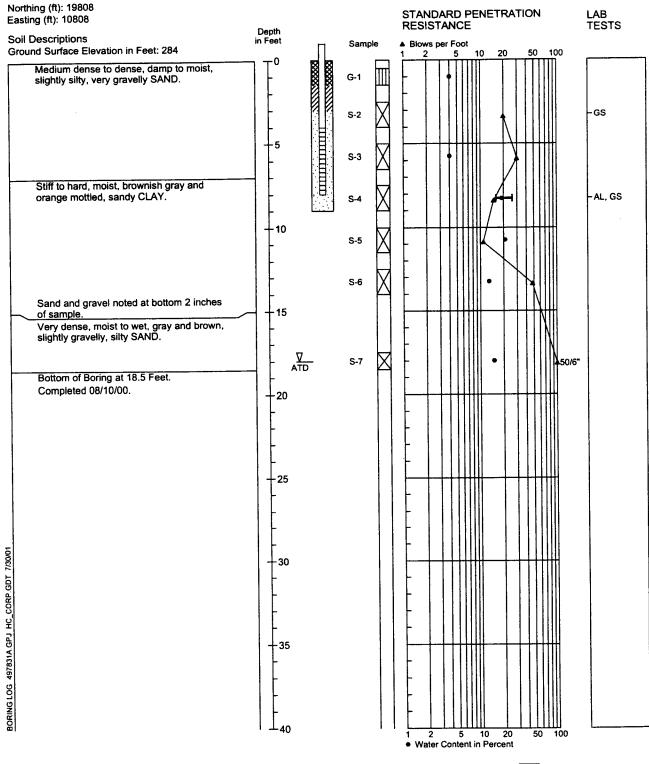
AR 051356

08/00

HARTCROWSER

4978-31

Monitoring Well Log HC00-B305



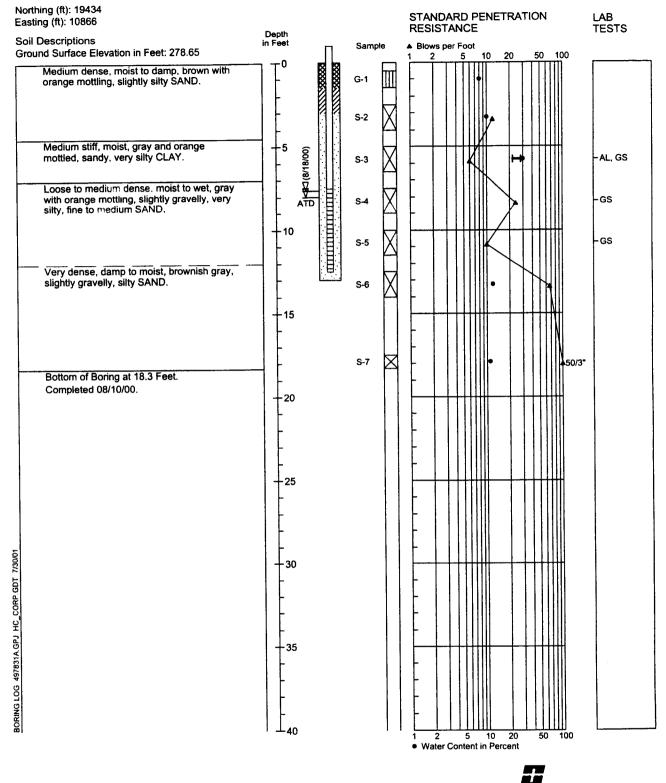


Refer to Figure A-1 for explanation of descriptions and symbols.
 Soil descriptions and stratum lines are interpretive and actual changes

 Soil descriptions and stratum lines are interpretive and actual change may be gradual.

3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

Monitoring Well Log HC00-B306



1. Refer to Figure A-1 for explanation of descriptions and symbols.

Soil descriptions and stratum lines are interpretive and actual changes may be gradual.

3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

AR 051358

08/00

HARTCROWSER

4978-31

Boring Log HC00-B307 Northing (ft): 19082

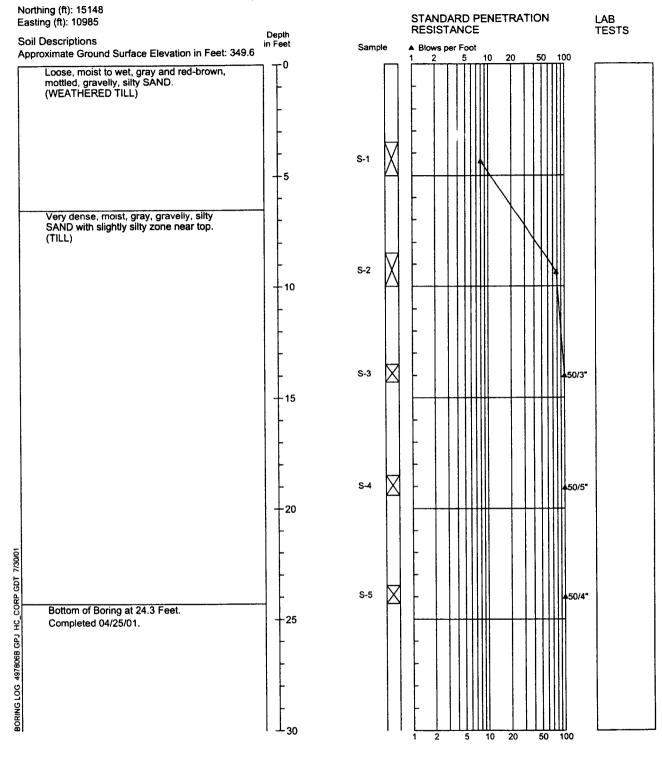
Soil Descriptions Ground Surface Elevation in Feet: 267 Medium dense to dense, moist to damp, brown and gray, gravelly, silty SAND.	Depth in Feet	Sample G-1	▲ Blows per Foot 1 2 5 10 20 50 100 	
Medium dense to dense, moist to damp, brown and gray, gravelly, silty SAND.		s-2		
Noted gravels in run				
Noted gravels in nun	+5	- H		
Noted gravels in nun		S-3		
Noted gravels in run.	-	•s-4		
	+10	•s-5		
Very dense, moist to wet, brownish gray to gray, slightly gravelly, slightly silty to silty SAND.		S-6		
	- 15			
Bottom of Boring at 18.8 Feet.		s-7	- • • • • • • • • • • • • • \bullet \bullet	10"
Completed 08/09/00.	+20			
	-25			
20201 7/2001	-30			
HC. CORP.C				
7831A GPJ	-35			
BORING LOG 497831A GPJ HC_CORP GDT 7/30/01				
B	⊥ ₄₀		1 2 5 10 20 50 100 • Water Content in Percent	L



Figure A-58

Refer to Figure A-1 for explanation of descriptions and symbols.
 Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
 Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

Boring Log HC01-B401



Refer to Figure A-1 for explanation of descriptions and symbols.
 Soil descriptions and stratum lines are interpretive and actual changes may be gradual.

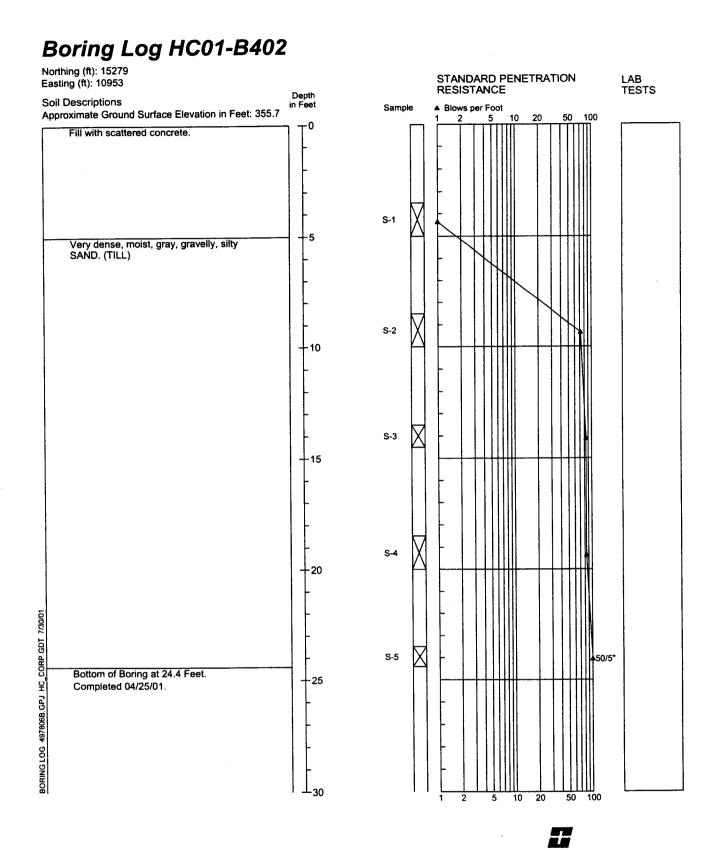
3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

AR 051360

04/01

HARTCROWSER

J-4978-06



Refer to Figure A-1 for explanation of descriptions and symbols.
 Soil descriptions and stratum lines are interpretive and actual changes

may be gradual.3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

HARTCROWSER J-4978-06 04/01 Figure A-60

Boring Log HC01-B403

asting (ft): 10759	Depth		STANDARD PENETRATION RESISTANCE	LAB TESTS
oil Descriptions pproximate Ground Surface Elevation in Feet: 361.7	in Feet	Sample	Blows per Foot	
Fill and gravelly roadbase.	ĨŢ			
Medium dense, moist, gray and brown mottled, slightly gravelly, silty SAND. (WEATHERED TILL)	- - - 	s-1		
Very dense, moist, gray, gravelly, silty SAND. (TILL)				
		S-2		
	15	s-3	- - - - 	
		s4 🛛	- - - - -	
		s-5 🔀		
Bottom of Boring at 28.5 Feet. Completed 04/25/01.		S-6 🗶		

Refer to Figure A-1 for explanation of descriptions and symbols.
 Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
 Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

Figure A-61

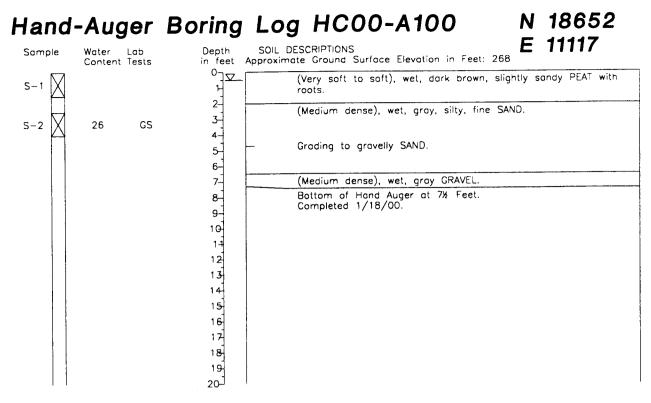
HARTCROWSER

Si

J-4978-06

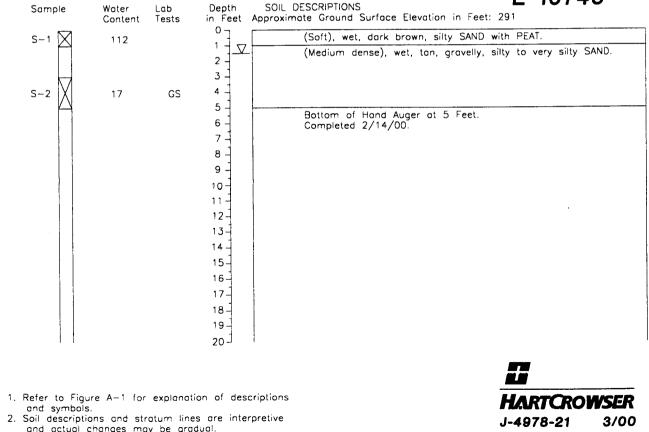
AR 051362

04/01



Hand-Auger Boring Log HC00-A105

N 17373 E 10745



and actual changes may be gradual. 3. Groundwater conditions, if indicated, ore at the time of excavation. Conditions may vary with time.

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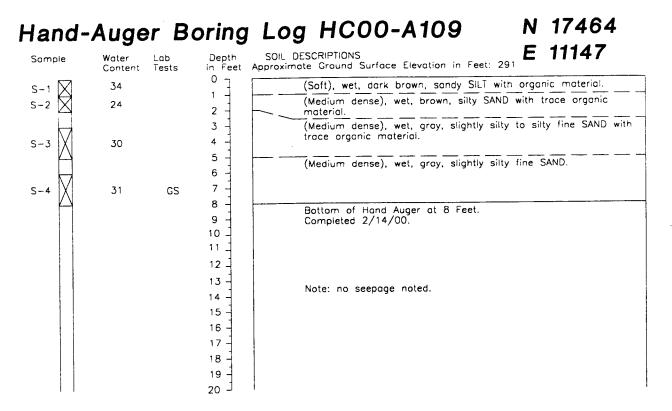
497821

R

7/30/01

1e

AR 051363



Hand-Auger Boring Log HC00-A137

10636 F SOIL DESCRIPTIONS Depth Sample Water Lab in Feet Approximate Ground Surface Elevation in Feet: 235 Tests Content 0 (Loose), wet, brown, silty SAND with PEAT. 又. 1 94 2 S-1 3 (Loose), wet, brown, very silty SAND. 33 S-2 4 5 GS S-3 29 6 (Medium dense), wet, gray SAND. S-4 30 7 (Stiff), wet, gray, gravelly, sandy SILT and CLAY. S-5 27 AL 8 Bottom of Hand Auger at 8 Feet. 9 Completed 2/14/00. 10 11 12 13 14 15 16 17 18 19 20 1. Refer to Figure A-1 for explanation of descriptions HARTCROWSER

and symbols. 2. Soil descriptions and stratum lines are interpretive

and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time

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of excavation. Conditions may vary with time.

AR 051364

J-4978-21

Figure A-63

2/00

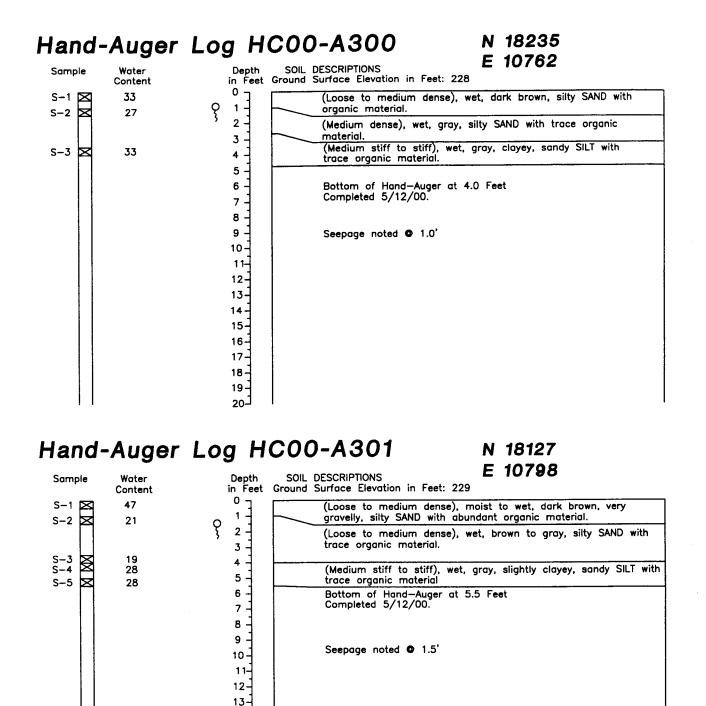
N 18666

Hand-	Auge	ər B	oring	Log HC00-A143 N 18733 E 10621
Sample	Water Content	Lab Tests		SOIL DESCRIPTIONS Approximate Ground Surface Elevation in Feet: 237
S-1 X	23			(Loose), wet, brown, slightly gravelly, very silty SAND with trace organic material.
S-2	47		2 - 3 -	(Very soft), wet, brown, sandy, organic SILT with organic material.
S-3	39	GS	4 - 5 -	(Loose), wet, brown, very sitty SAND with zones of sandy silt with organic material.
			6 - 7 - 8 -	
S-4	30		9 - 10 -	(Medium dense), wet, gray SAND.
			11	Bottom of Hand Auger at 11 Feet. Completed 2/14/00.

1. Refer to Figure A-1 for explanation of descriptions

Control and Symbols.
 Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
 Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.





1. Refer to Figure A-1 for explanation of descriptions and symbols.

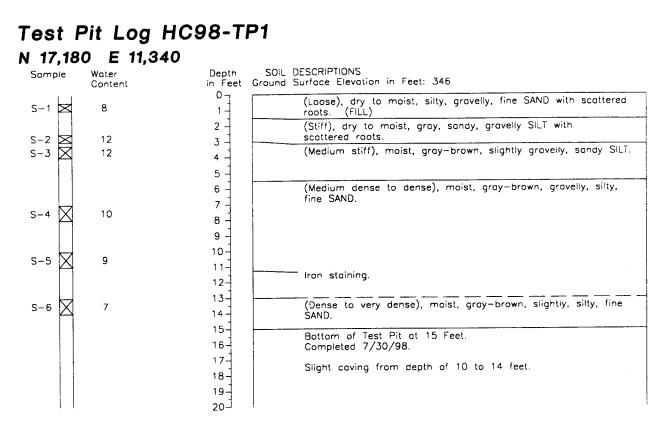
14-15-16-17-18-19-20-

- Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
 Groundwater conditions, if indicated, are at the time
- Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.

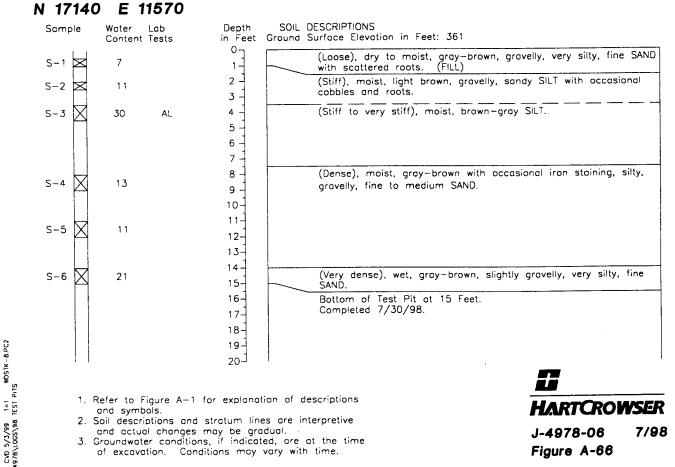
HARTCROWSER J-4978-26 5/00 Figure A-65

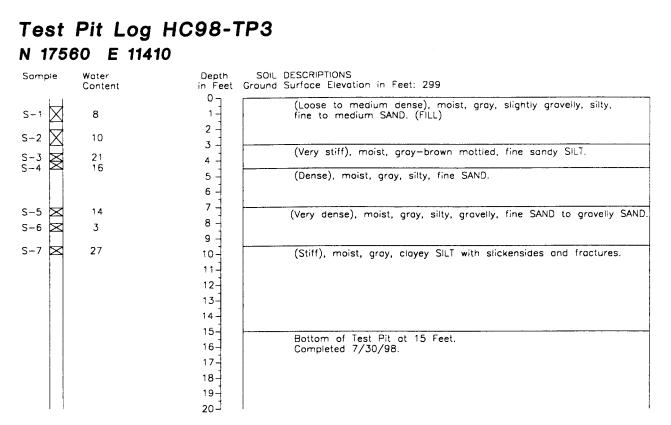
AR 051366

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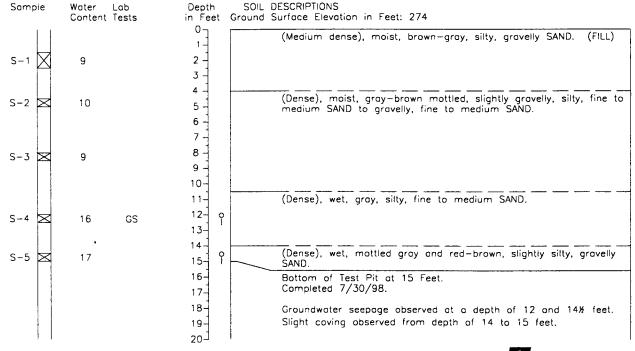


Test Pit Log HC98-TP2





Test Pit Log HC98-TP4 N 17780 E 11380

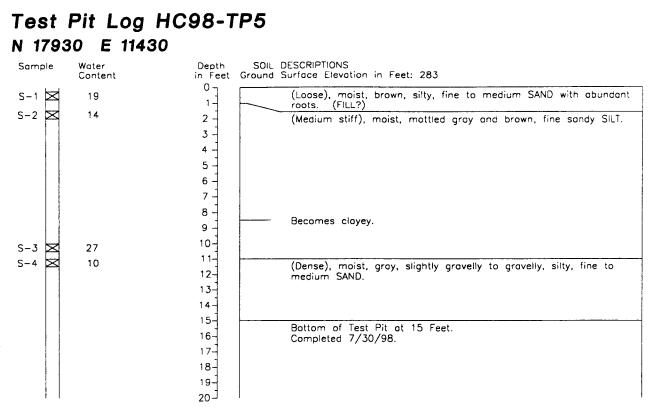


hel 7/30/01 1=1 1978\LOGS\98 TEST PITS

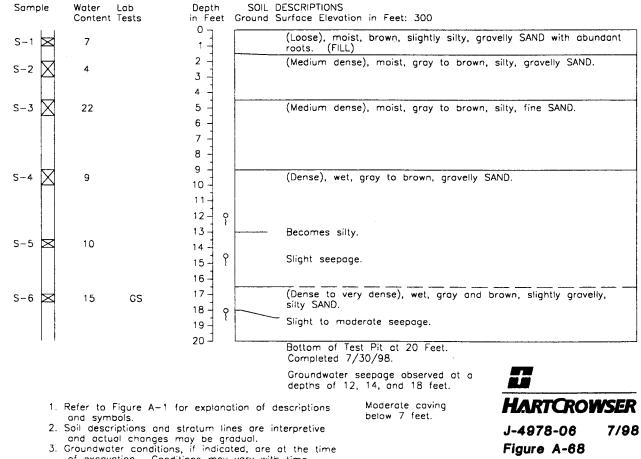
- 1. Refer to Figure A-1 for explanation of descriptions and symbols. 2. Soil descriptions and stratum lines are interpretive
- and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.

HARTCROWSER

J-4978-06 7/98 Figure A-67



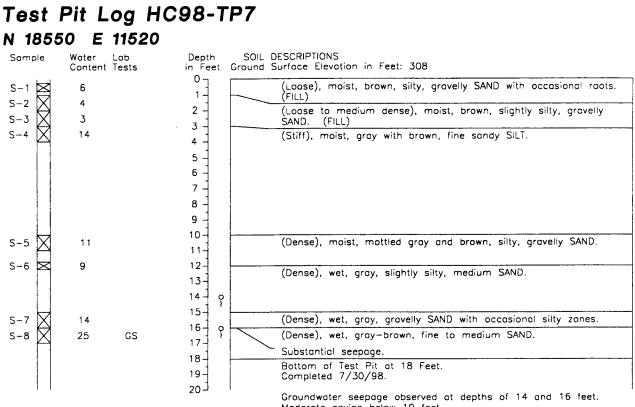
Test Pit Log HC98-TP6 N 18330 E 11500



of excavation. Conditions may vary with time.

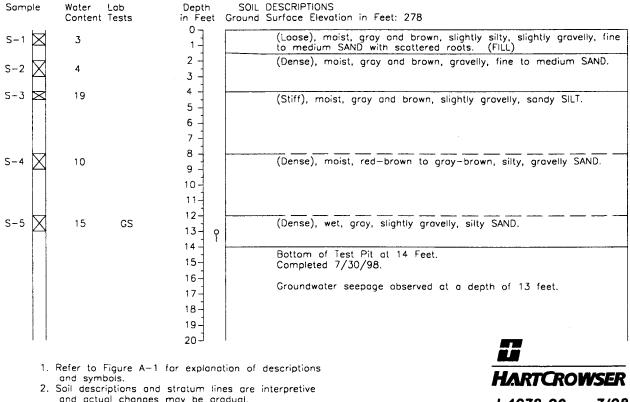
PITS

hei 7/30/01 1=1 1978\LOGS\98 TEST



Test Pit Log HC98-TP8 N 18306 E 11340

Moderate caving below 10 feet.

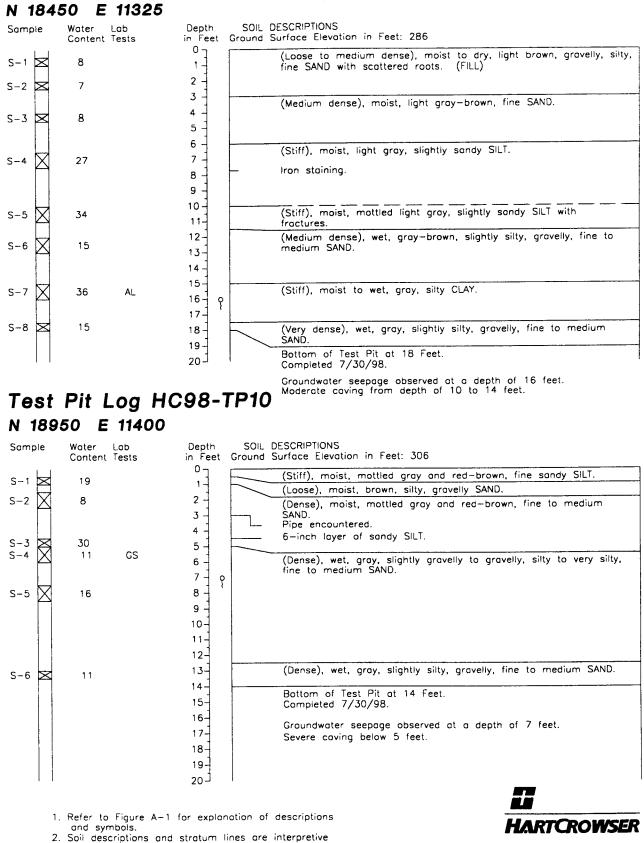


and actual changes may be grodual. 3. Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.

J-4978-08 7/98 Figure A-69

CVD 5/3/99 1=1 WDSTK-B.PC2 1978\LOGS\98 TEST PITS

Test Pit Log HC98-TP9



2. Soil descriptions and stratum lines are interpretive

PITS

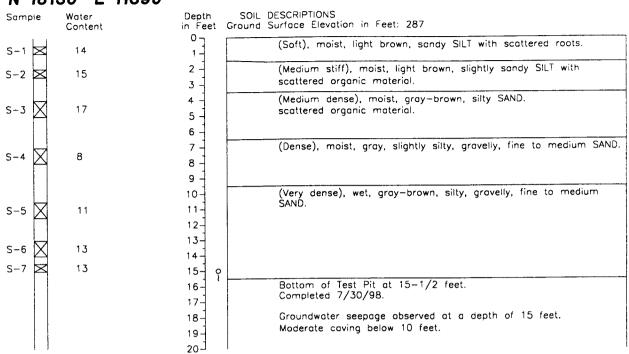
1=1 TEST

hei 7/30/01 4978\LOGS\98 TI

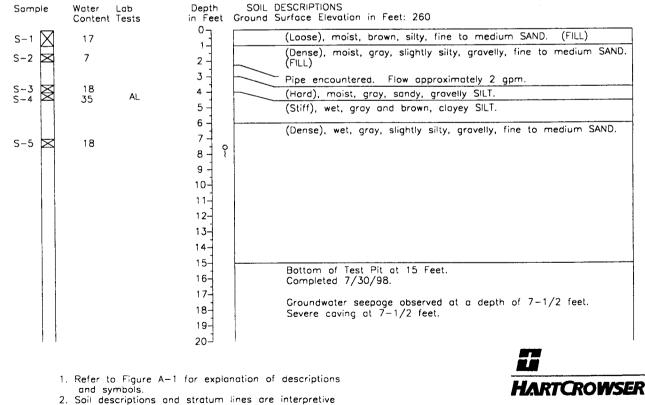
and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.

7/98 J-4978-06 Figure A-70

Test Pit Log HC98-TP11 N 18130 E 11390

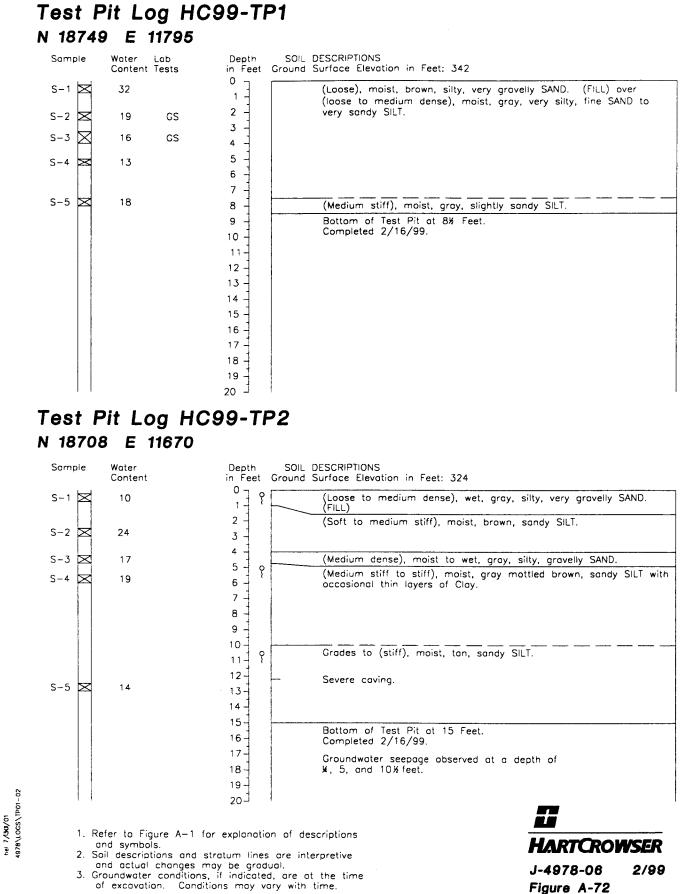


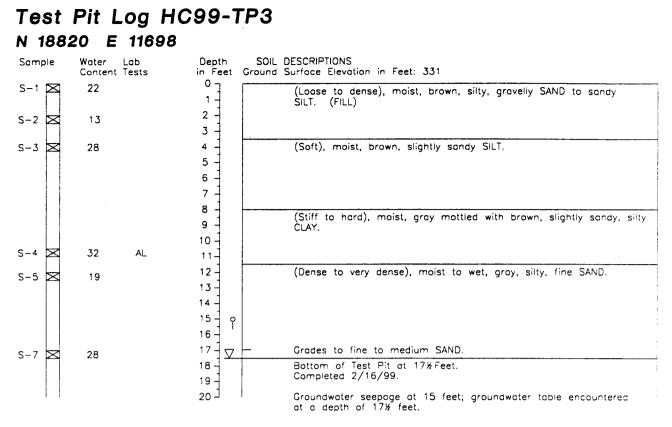
Test Pit Log HC98-TP12 N 17930 E 11330



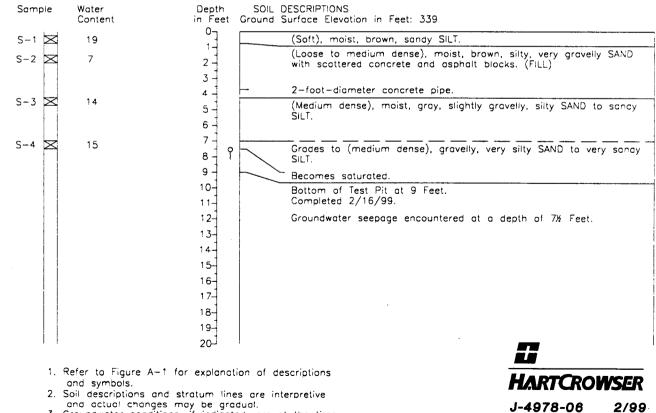
and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.

7/98 J-4978-06 Figure A-71





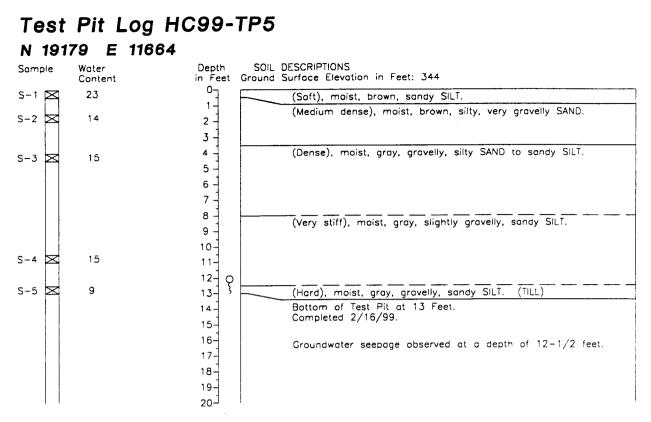
Test Pit Log HC99-TP4 N 19100 E 11643



and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.

DTN 11/13/00 1=1 WDS1K-B.PC2 4978\LOGS\99 TESTPIT\TP03-04

AR 051374



Test Pit Log HC99-TP6 N 19284 E 11656

SOIL DESCRIPTIONS Sample Water Depth in Feet Ground Surface Elevation in Feet: 348 Content 0 S-1 🖂 8 (Loose), moist, dark gray, slightly silty SAND. (FILL) 1 (Soft), moist, brown, slightly gravely, sandy SILT. S-2 🖂 12 2 -3 -4 S-3 🖂 22 5 6 -7 -(Dense), moist, gray, gravelly, very silty SAND to very sandy 8 -ŜΙLΤ. 9 S-4 🗙 12 10 11-Bottom of Test Pit at 11 Feet. Completed 2/16/99. 12-13-14 15-16-17-18-19-20 HARTCROWSER 1. Refer to Figure A-1 for explanation of descriptions and symbols. 2/99 J-4978-06

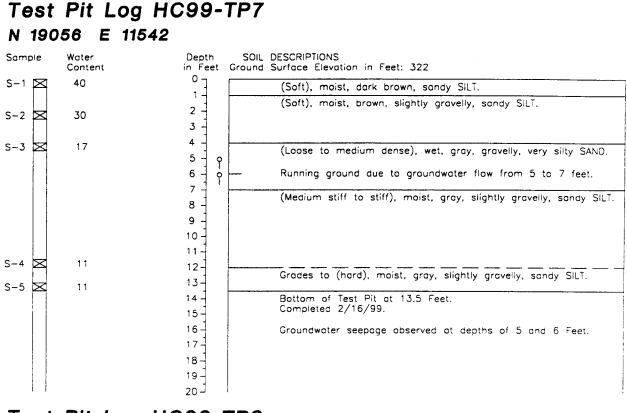
 Soil descriptions and stratum lines are interpretive and actual changes may be gradual.

1=1 WUSTK-8.PC2 TEST PHS\IP05-06

DTN 11/13/00 1 49/8\LOCS\99 1

> and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.

> > AR 051375



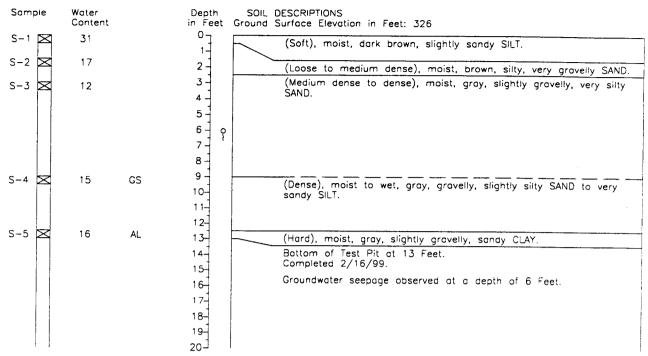
Test Pit Log HC99-TP8 N 19180 E 11860

Sample Water Depth SOIL DESCRIPTIONS Content in Feet Ground Surface Elevation in Feet: 364 0 -S-1 🖂 34 (Soft), moist, dark brown, sandy SILT. (TOPSOIL) 1 S-2 🖂 14 (Soft to medium stiff), moist, gray, sandy SILT. 2 4 (Medium stiff), moist, brown, slightly gravelly, sandy SILT. S-3 🖂 21 3 4 _ S-4 🖂 18 (Medium dense), moist, gray, very silty SAND to very sandy SILT. 1 5 Ŷ Red stains observed. 6 7 8 9 10 S-5 🖂 15 (Dense), moist, gray, very silty SAND. 11 Moderate caving from 10 to 14 feet depths. 12 -13 14] ? S-6 🖂 14 15. (Hard), moist, gray, slightly sandy, gravelly SILT. 16 Bottom of Test Pit at 15 Feet. Completed 2/16/99. 17 -18 Groundwater seepage observed at a depth of 5 and 14 Feet. WDSTK-8.PC2 19 \TPD7--08 20 ocs/ = HARTCROWSER $\frac{6}{2}$ 1. Refer to Figure A-1 for explanation of descriptions 8 and symbols. 2. Soil descriptions and stratum lines are interpretive J-4978-06 2/99

and actual changes may be gradual.
Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.

AR 051376

Test Pit Log HC99-TP9 N 19190 E 11517



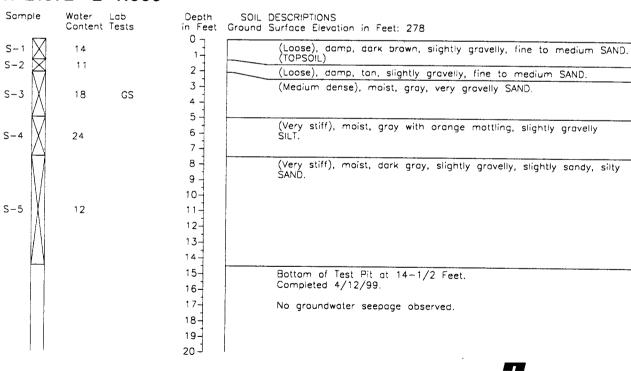
Test Pit Log HC99-TP10 N 21672 E 11059

WDSTK -- 8. PC2 PLTS \ TP09-- 10

test

'n

011 11/13/00 1 4978\LOGS\99 1



1. Refer to Figure A-1 for explanation of descriptions and symbols.

2. Soil descriptions and stratum lines are interpretive and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time

of excavation. Conditions may vary with time.

HARTCROWSER

J-4978-06 2/99 Figure A-76

AR 051377

Test Pit Log HC99-TP34 N 20,164 E 10,952 SOIL DESCRIPTIONS Sample Water Lab Depth Field in Feet

Content Tests Test ٦ 3 inches of Sod over (medium dense), damp, reddish-brown, slightly gravelly, silty, fine SAND with occasional arganic materic: 13 5-1 1-S-2 6 2 (Medium dense to dense), damp, gray-brown, silty to very silty, S-3 🖂 16 3 fine SAND. 4-(Dense), damp, gray, fine to medium SAND with occasional 5gravei lenses. 6-7-Grades to sandy GRAVEL. 8-Ŷ (Very stiff to hard), moist, gray with orange mottling, sandy, 9 PP=4.2 silty CLAY with occasional gravel. 10-PP=2.0 12 AL PP=3.0 13 S-4 🖂 24 14 -S-5 🖾 14 (Dense), wet, gray, slightly silty to silty SAND with occasiona 15gravel. 16-Bottom of Test Pit at 15% Feet. 17-Completed 9/28/99. 18-19-20

Ground Surface Elevation in Feet: 303

Test Pit Log HC99-TP35 N 20,374 E 10.976

Ground Surface Elevation in Feet: 305 SOIL DESCRIPTIONS Water Lab Depth Sample Content Tests in Feet 0--2 inches of Sod over (medium dense to dense), dry to damp, X 1brown, silty, very gravely SAND. (FILL) S-1 8 S-2 6 2-(Dense), damp, reddish-brown, slightly gravelly, silty SAND with concrete debris in upper 3 feet. (FILL) 3-4-(Medium stiff to stiff), moist, gray with orange mottling, slightly sandy, silty CLAY with trace gravel. 5-6-7--8-9-S-3 🖾 24 AL 10-11-(Medium dense to dense), moist to wet, brown, slightly gravelly, 12silty SAND. 13-S-4A 🔀 15 14-Ŷ S-48 🖂 Becomes wet. 14 15-Bottom of Test Pit at 15½ Feet. 16-Completed 9/28/99. 17-18-19 20 1. Refer to Figure A-1 for explanation of descriptions and symbols. HARTCROWSER 2. Soil descriptions and stratum lines are interpretive and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time J-4978-16 9/99 Figure A-77

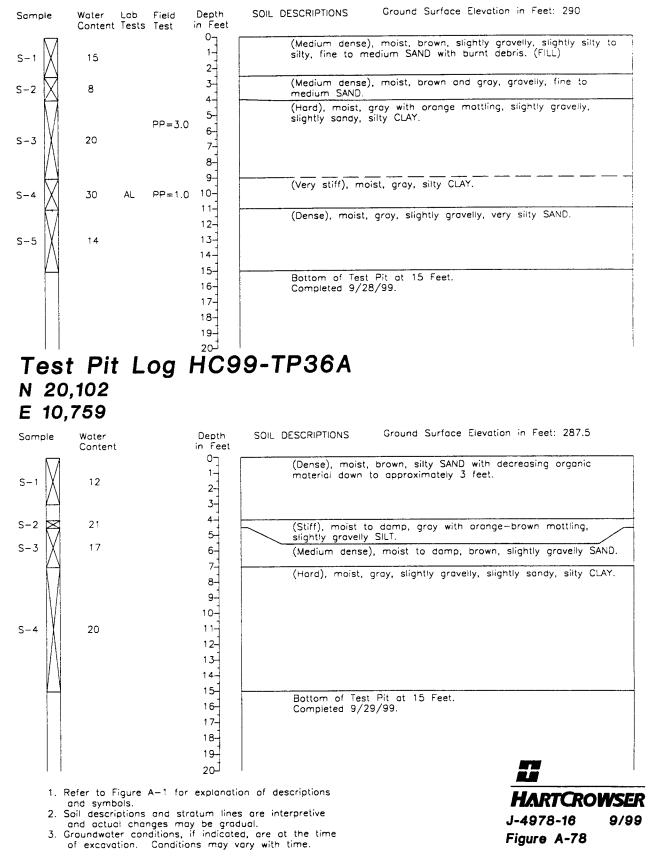
of excavation. Conditions may vary with time.

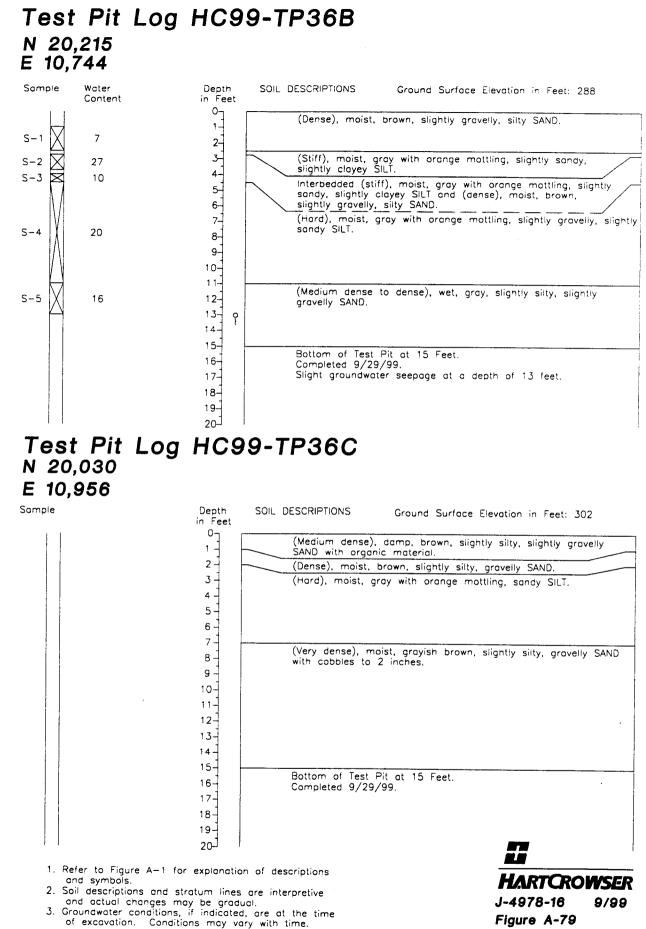
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Test Pit Log HC99-TP36 N 20,287 E 10,730

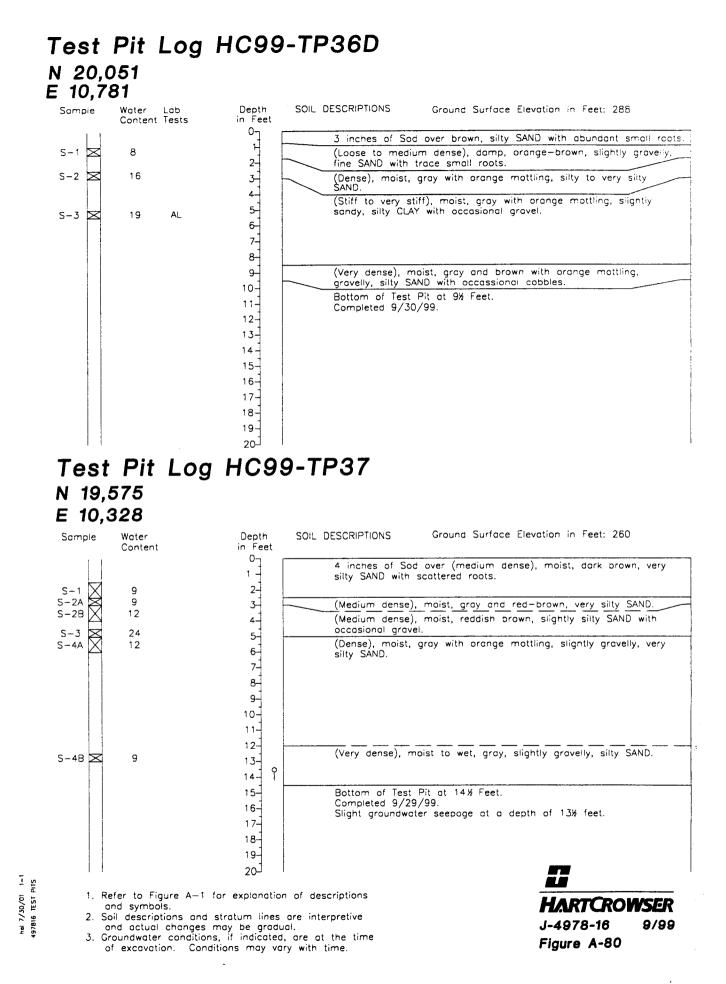
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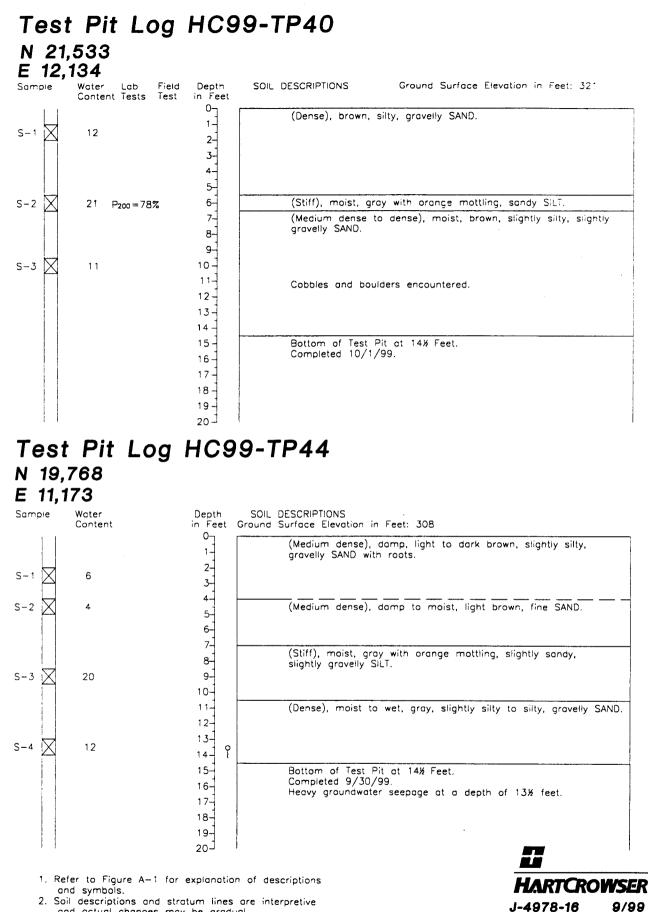




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and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time

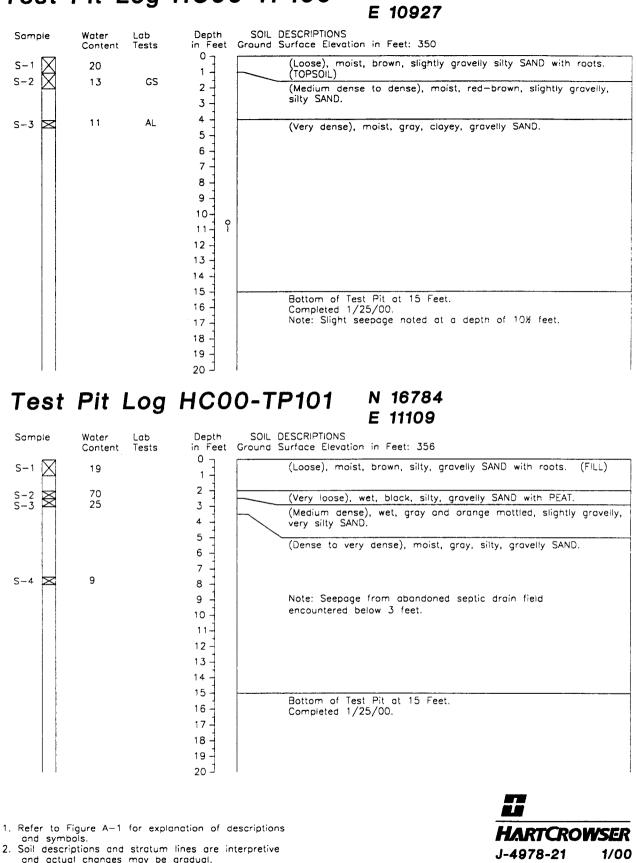
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DIN 11/13/00 1=1 497816 TEST PITS

of excavation. Conditions may vary with time.

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N 16778



and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.

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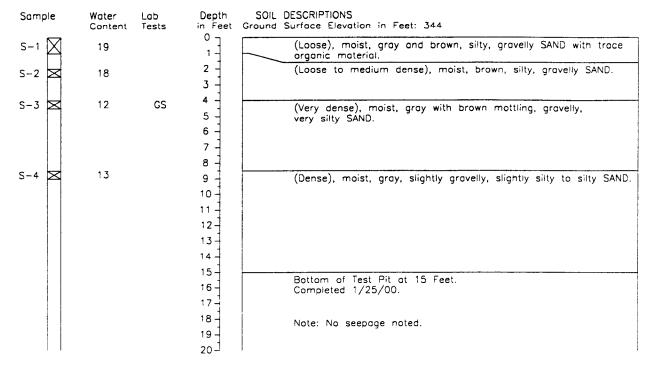
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AR 051383

Figure A-82

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N 17003 E 11158



Test Pit Log HC00-TP103

N 17037 E 11018

SOIL DESCRIPTIONS Sample Water Lab Depth in Feet Ground Surface Elevation in Feet: 339 Content Tests 0 (Loose), moist, brown, gravelly, silty SAND. (FILL) 1 X GS S-1 15 2 3 25 15 S-2 S-3 Ř (Loose), moist, dark brown, silty, gravelly SAND. (TOPSOIL) 4 (Medium dense), moist, brown, silty, gravelly SAND. 5 6 S-4 X 13 (Very dense), moist, gray with brown mottling to gray, silty, 7 gravelly SAND. 8 1 9 10 -Ŷ 11-12 -13 14 15 Bottom of Test Pit at 15 Feet. 16 Completed 1/25/00. Note: Slight groundwater seepage noted at a depth of 10½ feet. 17 -18 Septic drainfield encountered 2 to 3 feet deep. 19 20

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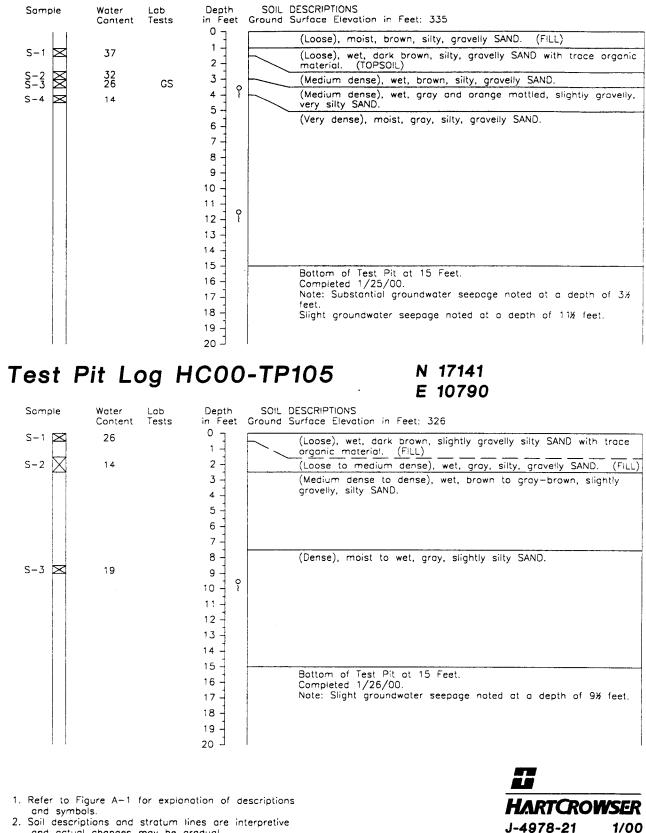
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- 1. Refer to Figure A-1 for explanation of descriptions and symbols.
- 2. Soil descriptions and stratum lines are interpretive and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time
- of excavation. Conditions may vary with time.

-HARTCROWSER J-4978-21 1/00 Figure A-83

AR 051384

N 17030 E 10874



and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time

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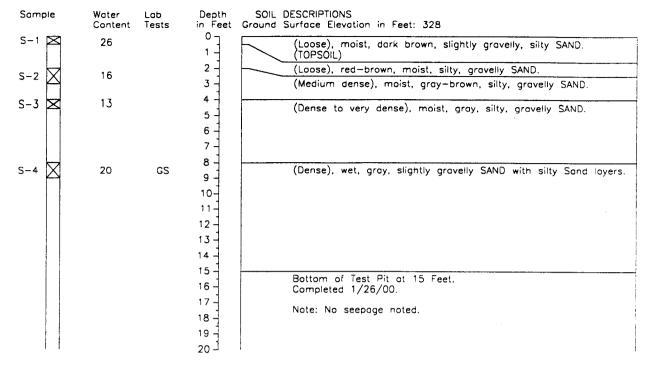
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of excavation. Conditions may vary with time.

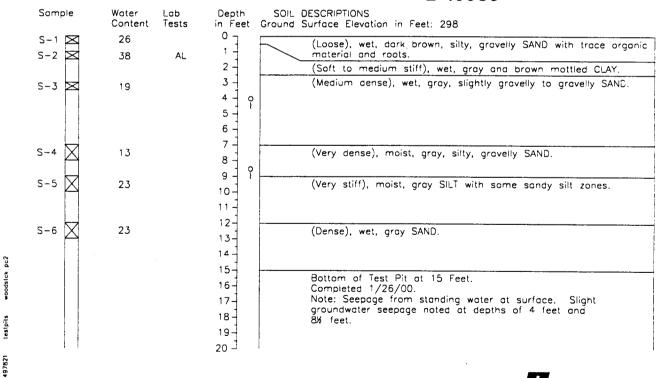
AR 051385

N 17151 E 10925



Test Pit Log HC00-TP107

N 17415 E 10989



1. Refer to Figure A-1 for explanation of descriptions and symbols.

2. Soil descriptions and stratum lines are interpretive

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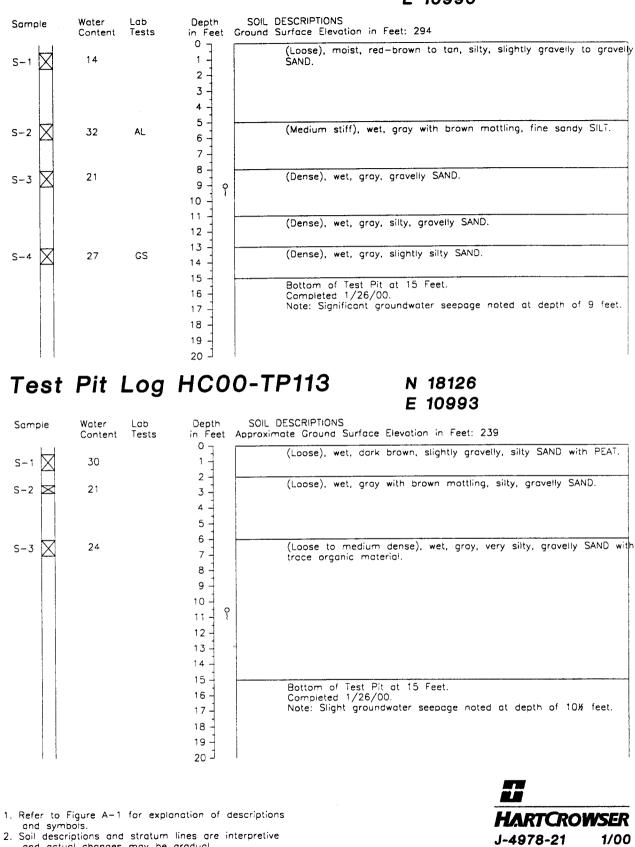
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and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time

 Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time. **HARTCROWSER** J-4978-21 1/00 Figure A-85

AR 051386

N 17477 E 10990



 and actual changes may be gradual.
 Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.

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AR 051387

N 17647 E 10813

Sample	Water Content	Lab Tests	-		DESCRIPTIONS Surface Elevation in Feet: 256
S-1 🛛	43		0- 1-	\square	(Loose), wet, dark brown, slightly gravelly, silty SAND with organic material. (TOPSOIL)
S-2 🛛	40		2-		(Loose), wet, brown, slightly silty, gravelly, silty SAND.
S-3 🛛	32	GS	2- 3- 0 4-		(Loose), wet, light brown, fine to medium SAND.
S-4 🔀	12		5- 6- 7-		(Medium dense), wet, gray with brown mottling, silty, gravelly SAND.
			8- 9- 10- 11-		Grades to very dense.
S-5 🔀	32	AL	12 13		(Hard), moist, gray CLAY with silt partings.
			14 15 16 17 18 19 20		Bottom of Test Pit at 14 Feet. Completed 3/15/00. Note: Groundwater seepage noted at depth of 3 feet. Sand from 3 to 5 feet highly transmissive and loose, free-face flows as a sand slurry.

Test Pit Log HC00-TP111



Sample Water Lab Depth SOIL DESCRIPTIONS in Feet Ground Surface Elevation in Feet: 285 Content Tests 07 (Loose), moist to wet, dark brown, slightly silty, slightly gravelly, fine to medium SAND with roots and trace organic material. S-1 21 Roots grade out. S-2 🖾 Ŷ 26 (Medium stiff), wet, gray-prown, sandy SILT. S-3 🖂 26 AL (Medium stiff), wet, gray with brown mottling, non-sandy to, slightly sendy CLAY. (Loose), wet, gray, gravelly, fine to medium SAND. S-4 🖂 .18 Grades to medium dense. Bottom of Test Pit at 15 Feet. 16 Completed 3/15/00. 17 Note: Slight groundwater seepage noted at depth of 6½ feet. 18 19 20]

1. Refer to Figure C-1 for explanation of descriptions and symbols.

2. Soil descriptions and stratum lines are interpretive

and actual changes may be gradual.3. Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.



J-4978-21 3/00 Figure A-87

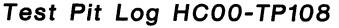
AR 051388

N 17736 E 10990

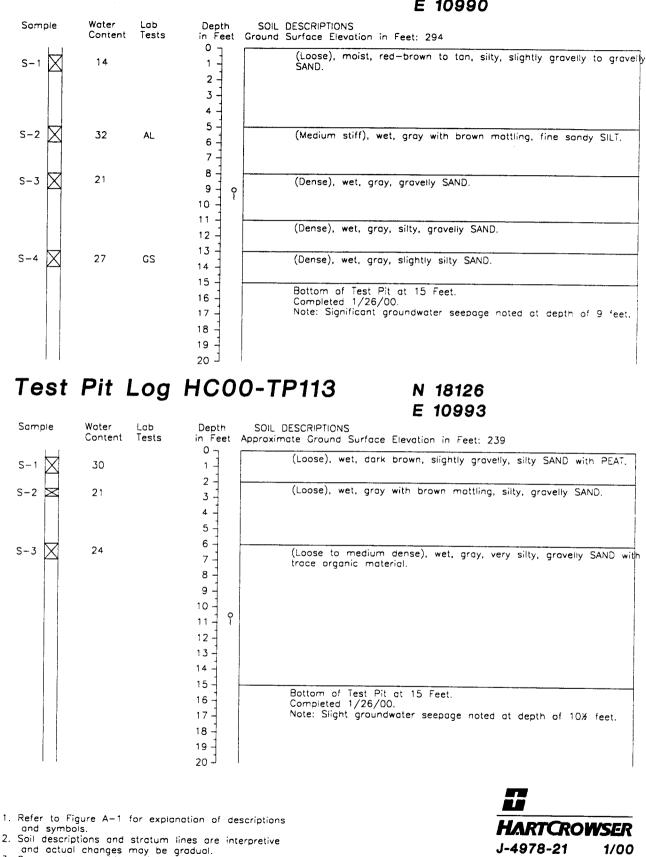
Sample	Water Lab Content Tests	in Feet	OIL DESCRIPTIONS Approximate Ground Surface Elevation in Feet: 266.5
S-1 🗙	14	0 1- 2- 3- 4+ 5-	(Loose), wet, brown, silty, gravelly SAND with trace organic material. (FILL and TOPSOIL)
S-2 🔀	14 CS	6- 7- 8- 9- 10-	(Dense), wet, gray with brown mottling, non-grovelly to slightly gravelly, silty to very silty, fine to meaium SAND. Grades to gravelly.
S-3 🛛	11	11- 12- 13- 14-	(Very dense), moist, gray with orange mottling, silty, gravelly SAND.
		15- 16- 17- 18- 19- 20-	Bottom of Test Pit at 15 Feet. Completed 3/15/00. Note: Septic drain field seepage from a depth of 4 feet. Groundwater seepage noted at a depth of 7-1/2 feet.

- 1. Refer to Figure C-1 for explanation of descriptions and symbols.
- 2. Soil descriptions and stratum lines are interpretive
- and actual changes may be gradual.3. Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.





N 17477 E 10990



and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.

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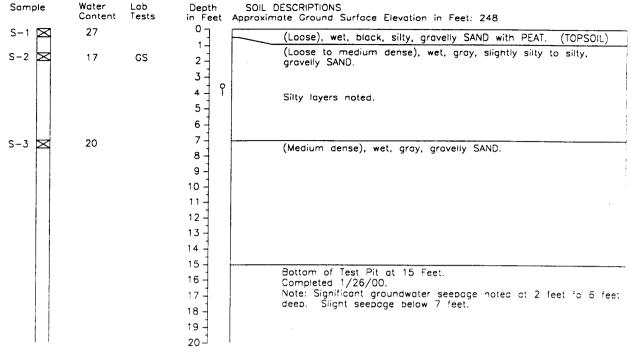
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N 18220 E 11068



SOIL DESCRIPTIONS

Test Pit Log HC00-TP115

Depth

in Feet

0 -

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5 6

7

9

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Lab

AL

Tests

N 18325 E 11041

(Loose), moist, dark brown, slightly gravelly, silty SAND with trace

(Loose), moist, gray and brown, silty, gravelly SAND. (FILL)

S-2 S-3 S-4 S-5 🛛

Sample

S-1 🖂

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Water

26

36

14

25

17

Content

3 -(Loose), wet, dark brown, slightly gravely, silty SAND with peat and trace organic material. (TOPSOIL) 4 1 (Medium dense), wet, gray, slightly silty SAND (Medium dense), wet, slightly gravelly SAND. 8 -10-11-(Stiff), wet, gray, slightly gravelly, sendy CLAY with sand lenses. 12 13. (Dense), wet, gray, gravelly SAND. 14 -15. Bottom of Test Pit at 15 Feet. 16-Completed 1/26/00. 17-18 -Note: Observed abandoned septic drain line with gravel at

3½ feet deep; no seepage noted.

Approximate Ground Surface Elevation in Feet: 250

organic material and roots. (FILL)

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1. Refer to Figure A-1 for explanation of descriptions and symbols.

2. Soil descriptions and stratum lines are interpretive and actual changes may be gradual. 3. Groundwater conditions, if indicater are at the time

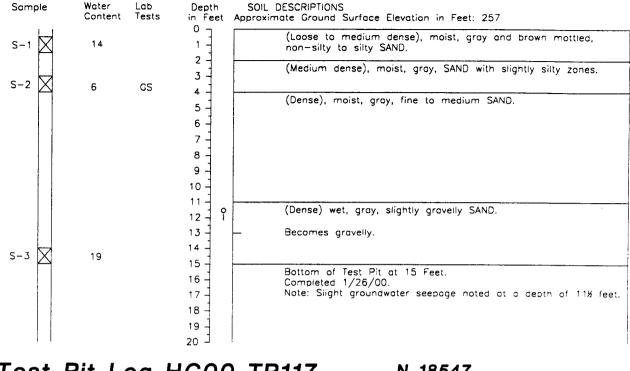
of excavation. Conditions may vc y with time.

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J-4978-21 1/00 Figure A-90

AR 051391

N 18396 E 11005



Test Pit Log HC00-TP117

N 18547 E 10999

Sample Water Lab Depth SOIL DESCRIPTIONS Content Tests Ground Surface Elevation in Feet: 261 in Feet 0 (Loose), moist, silty SAND. (FILL) 1 Concrete slab. S-1 🖾 8 2 (Medium dense), moist, gray-brown, slightly siity SAND. 3 4 S-2 🖾 18 (Dense), moist, gray and orange mottled, gravelly, very silty 5 SAND. 6 7 8 X S-3 12 (Dense), wet, gray, slightly silty, very gravelly SAND. 1 9 10 11 12 Grades to sandy GRAVEL. 13 14 Х S-4 18 15 Bottom of Test Pit at 15 Feet. 16 Completed 1/26/00. 17 18 Note: No seepage noted. 19 20 Ŀ

1. Refer to Figure A-1 for explanation of descriptions and symbols.

- 2. Soil descriptions and stratum lines are interpretive
- and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time

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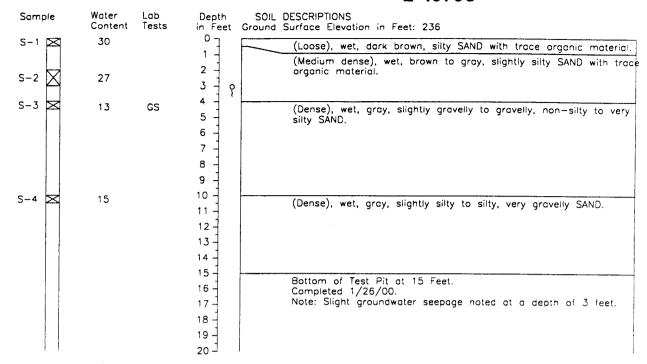
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of excavation. Conditions may vary with time.

HARTCROWSER J-4978-21 1/00 Figure A-91

N 18412 E 10798



Test Pit Log HC00-TP119

N 18529 E 10757

Sample Depth SOIL DESCRIPTIONS Water Lab in Feet Ground Surface Elevation in Feet: 247 Content Tests 0 0 -S-1 🖂 17 (Loose), moist, dark brown, silty SAND with trace organic material and some roots. S-2 🖂 19 2 -(Loose to medium dense), moist, gray-brown, silty fine SAND. 3 -4 -5 s-3 🛛 (Dense), moist, gray with orange mottling, slightly gravely, silty SAND with non-silty lenses. 15 6 1 7 8 -9 -10 -11 12 -(Dense), moist, gray, silty, gravelly SAND. 13 -14 S-4 Х 12 15 -Bottom of Test Pit at 15 Feet. 16 Completed 1/26/00. 17 18 Note: no seepage reported. 19 20 J

1. Refer to Figure A-1 for explanation of descriptions and symbols.

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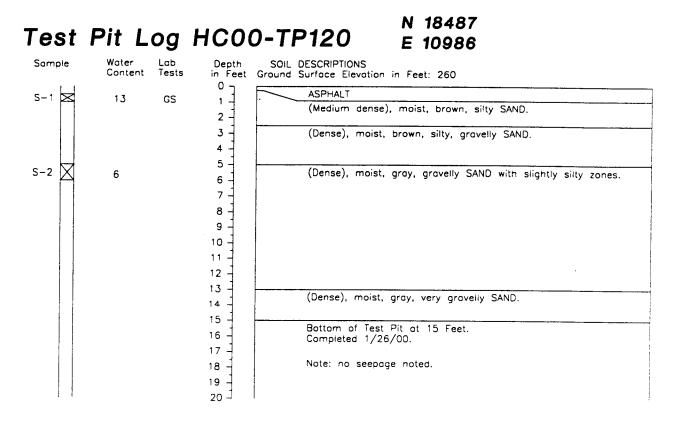
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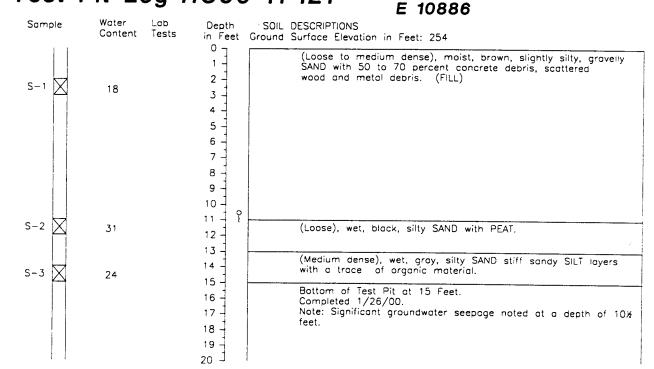
2. Soil descriptions and stratum lines are interpretive

and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.

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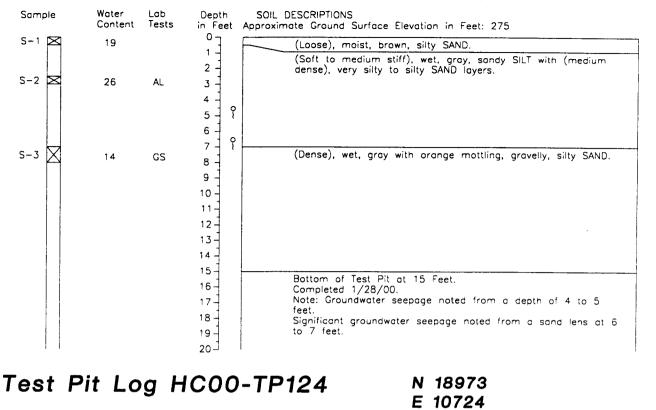
1. Refer to Figure A-1 for explanation of descriptions and symbols.

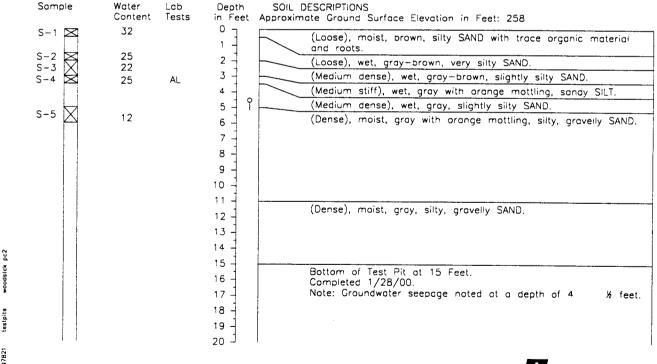
2. Soil descriptions and stratum lines are interpretive

and actual changes may be gradual.3. Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.

HARTCROWSER J-4978-21 1/00 Figure A-93

N 19126 E 10873





1. Refer to Figure A-1 for explanation of descriptions and symbols.

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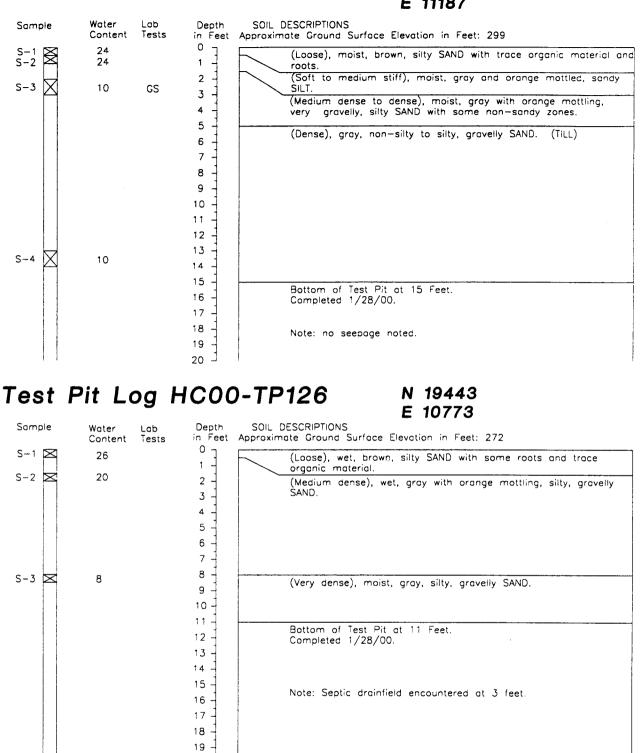
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2. Soil descriptions and stratum lines are interpretive and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time

of excavation. Conditions may vary with time.

HARTCROWSER J-4978-21 1/00 Figure A-94

N 19168 E 11187



1. Refer to Figure A-1 for explanation of descriptions and symbols.

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2. Soil descriptions and stratum lines are interpretive

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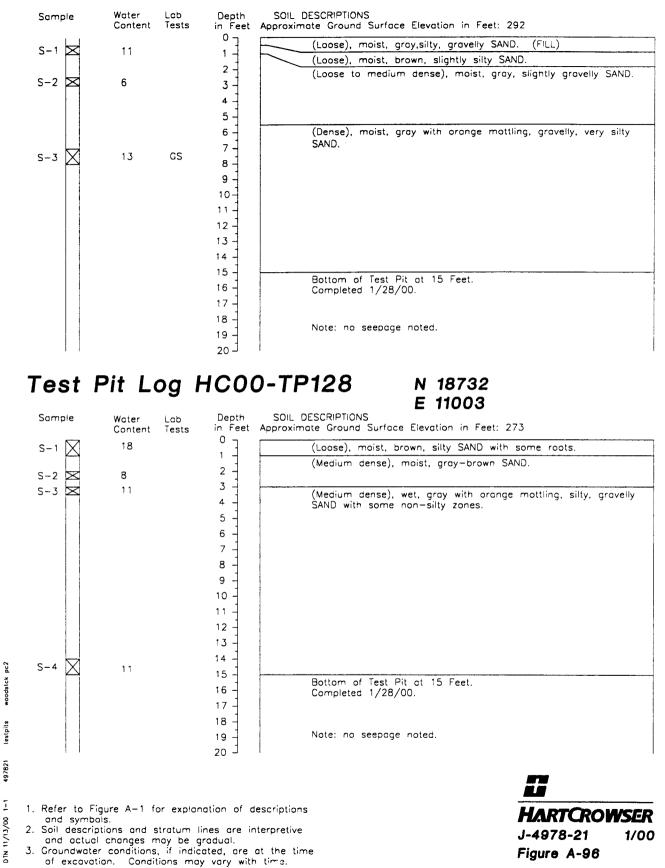
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and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.

HARTCROWSER J-4978-21 1/00 Figure A-95

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N 19166 E 11086



N 18899 E 10623

Sample	Water Content	Lob Tests	Depth in Feet	SOIL DESCRIPTIONS Approximate Ground Surface Elevation in Feet: 250
S-1	28		0	(Loose), moist, brown, very silty SAND with trace organic material and roots.
S−2 🔀	22	GS	2- 3-	(Medium dense), wet, gray, very sandy SILT.
S-3 🔀	× 16	4- 5- 6-	(Stiff), wet, gray and orange mottled, sandy SILT.	
			7- 8- 9- 10- 11- 12- 13- 14-	(Dense), moist, gray and orange mottled, silty, gravelly SAND.
			15- 16- 17- 18- 19- 20-	Bottom of Test Pit at 15 Feet. Completed 1/28/00.

 Refer to Figure A-1 for explanation of descriptions and symbols.
 Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
 Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.

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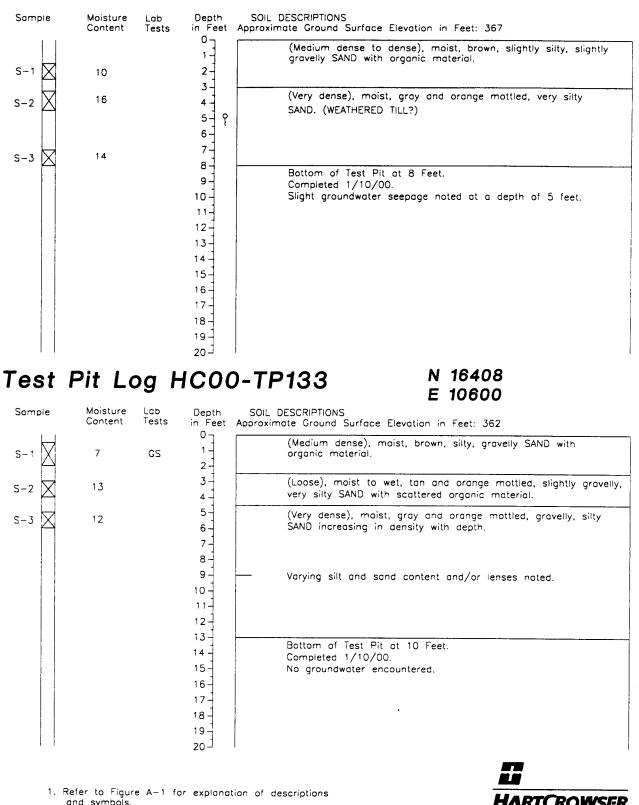
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J-4978-21 1/00 Figure A-97

N 16290 E 10592



and symbols. 2. Soil descriptions and stratum lines are interpretive

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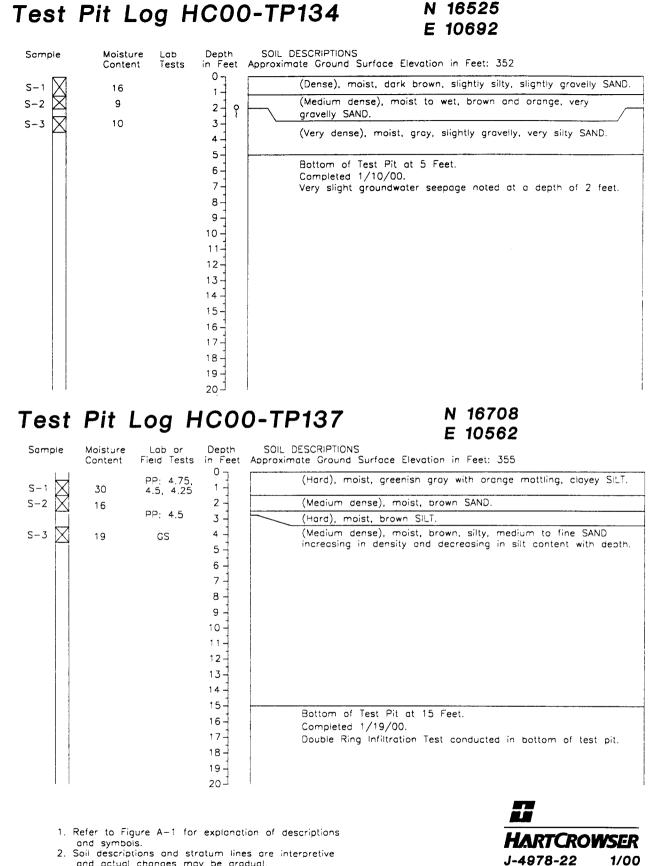
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and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time

of excavation. Conditions may vary with time.

HARTCROWSER

J-4978-22 1/00 Figure A-98



and actual changes may be gradual.3. Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.

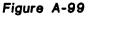
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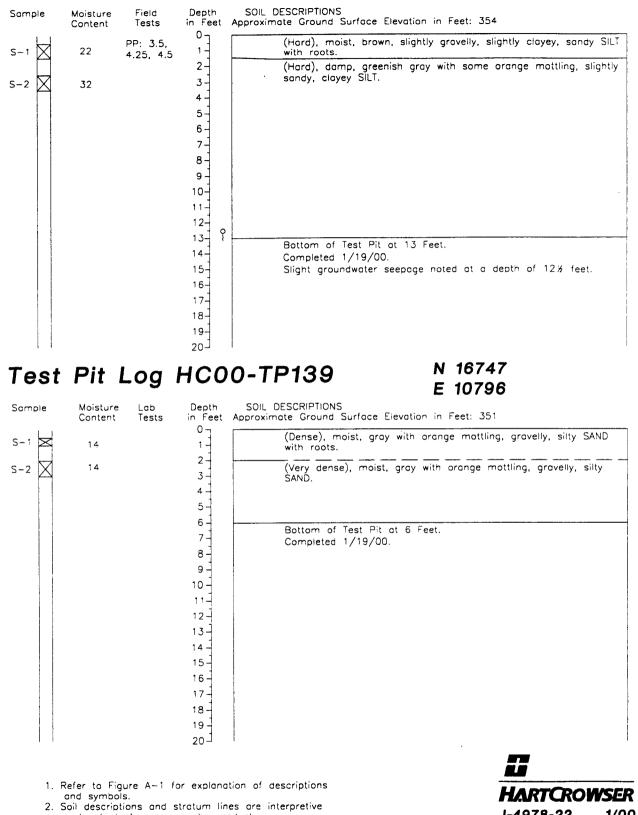
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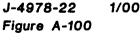


N 16702 E 10635



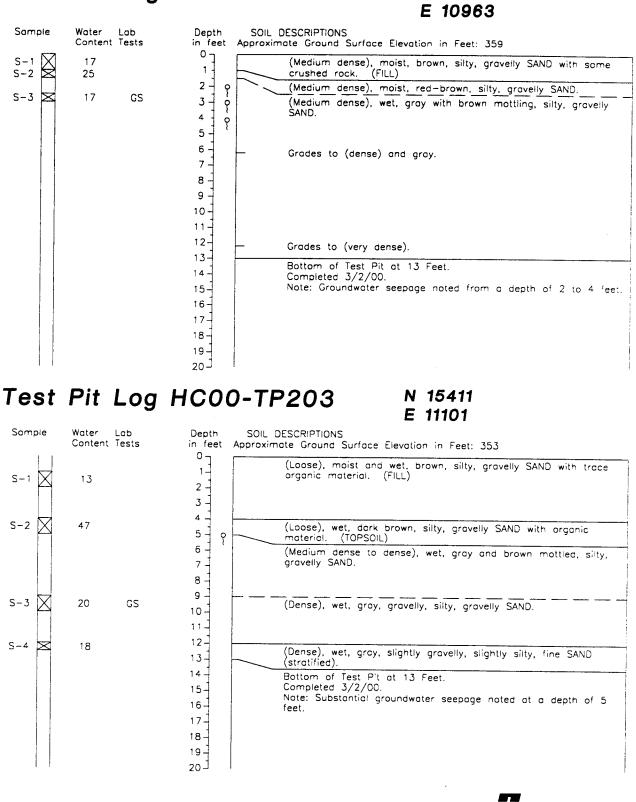
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of excavation. Conditions if indicated, are at the time of excavation.



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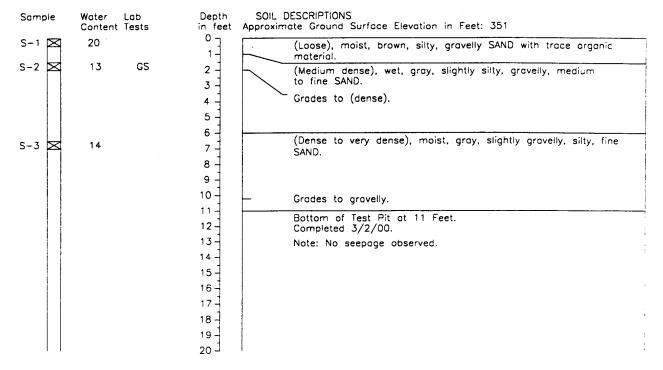
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- 1. Refer to Figure A-1 for explanation of descriptions and symbols.
- 2. Soil descriptions and stratum lines are interpretive and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time
 - of excavation. Conditions may vary with time.

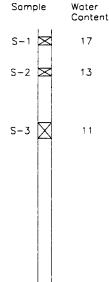
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Figure A-101

N 15161 E 10951



Test Pit Log HC00-TP205



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Depth in feet	SOIL DESCRIPTIONS Approximate Ground Surface Elevation in Feet: 356
0 - 1 -	(Loose), moist, red-brown, silty, gravelly SAND.
2 - 3 - 4 - 2 5 -	(Medium dense to dense), moist to wet, gray with brown mottling, silty, gravely SAND.
5 - 6 -	
7 - 8 - 9 -	(Dense), moist, gray, silty, gravelly, fine to medium SAND. (TILL)
10-	Bottom of Test Pit at 9 Feet. Completed 3/2/00.
11- 12-	Note: Groundwater seepage noted at a depth of 3½ feet. Very hard digging at 9 feet.
13-	
14-	
15- 16-	
17-	
18-	
19-	

N 15157 E 11181

1. Refer to Figure A-1 for explanation of descriptions and symbols.

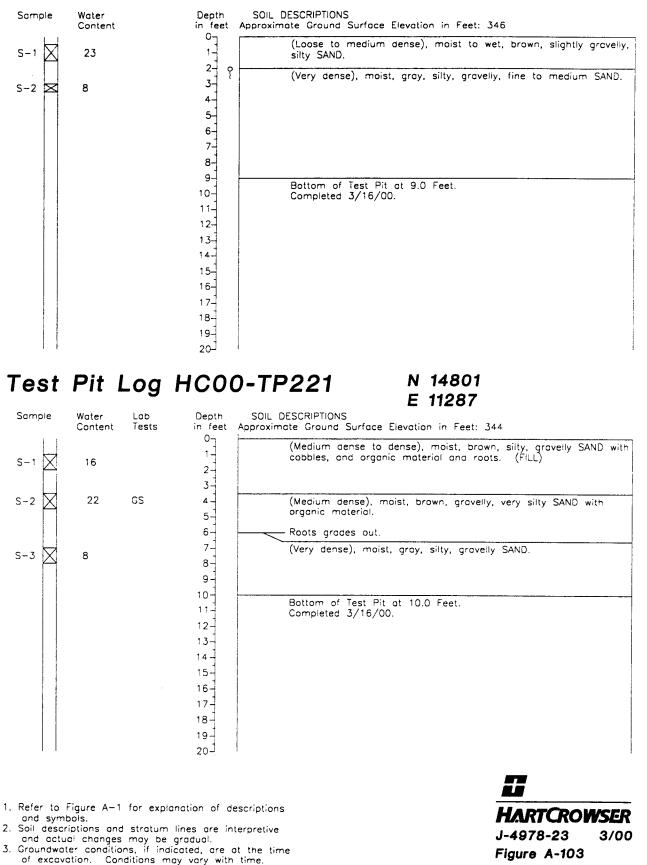
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- 2. Soil descriptions and stratum lines are interpretive
- and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.

HARTCROWSER J-4978-23 3/00 Figure A-102

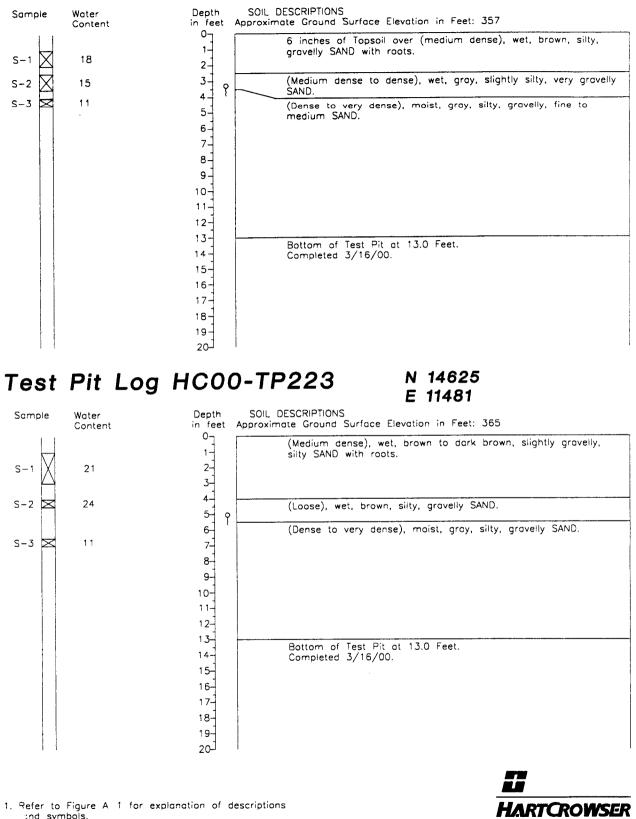
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N 14590 E 11265



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N 14579 E 11381



ind symbols. 2. Eail descriptions and stratum lines are interpretive

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and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.

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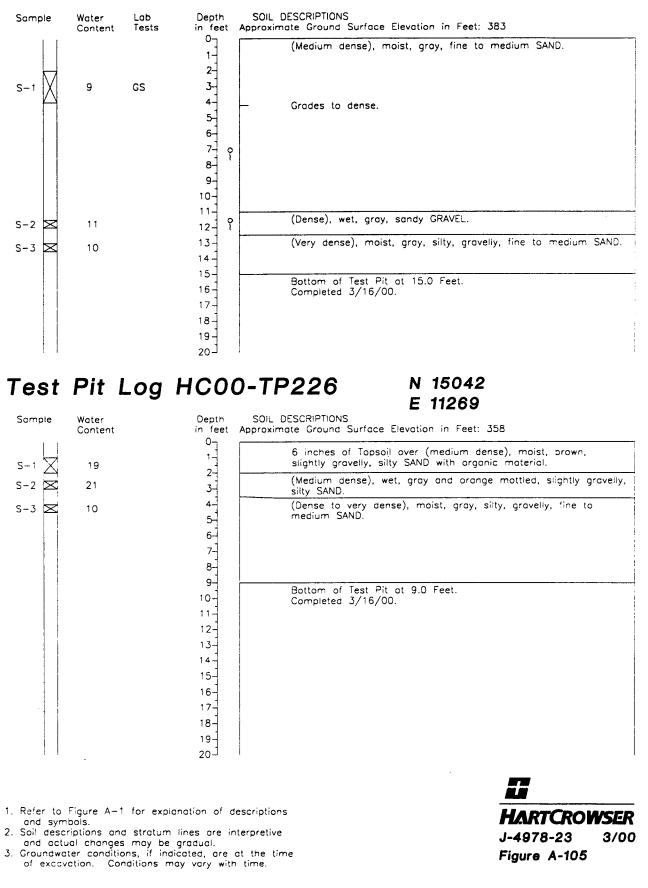
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Figure A-104

497823 Pits 6/7/00

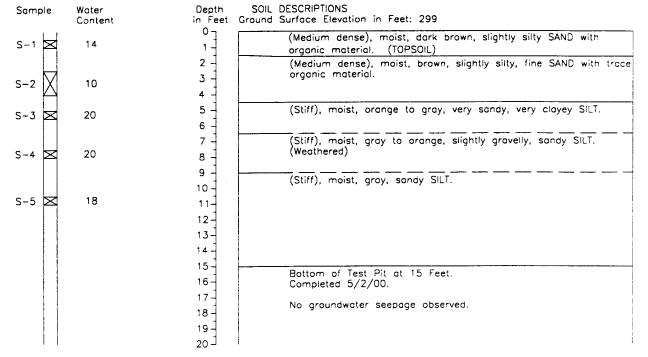
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N 15532 E 12085





N 20404 E 10845



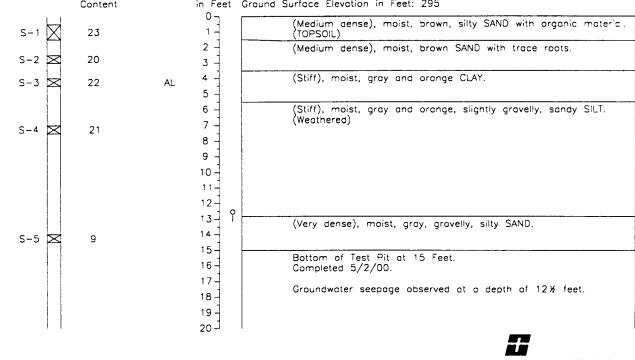
Test Pit Log HC00-TP301

Sample

Water



SOIL DESCRIPTIONS Depth in Feet Ground Surface Elevation in Feet: 295

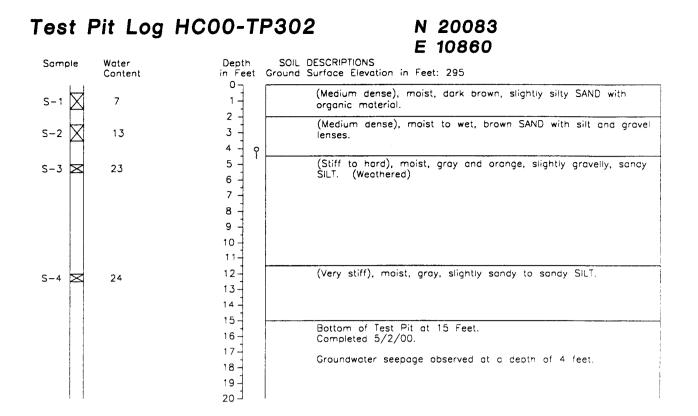


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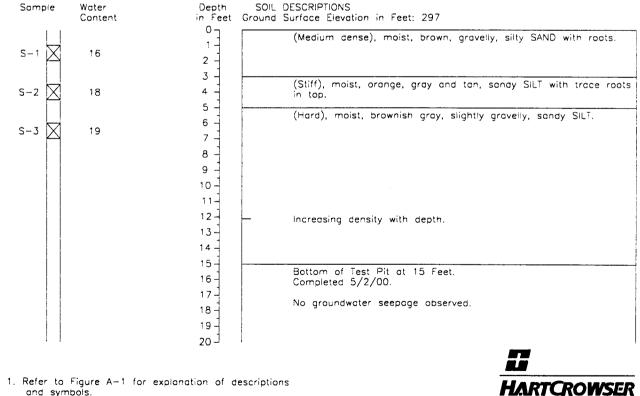
- 1. Refer to Figure A-1 for explanation of descriptions and symbols.
- 2. Soil descriptions and stratum lines are interpretive

and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.

HARTCROWSER J-4978-26 5/00 Figure A-106







and symbols. 2. Soil descriptions and stratum lines are interpretive

and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time

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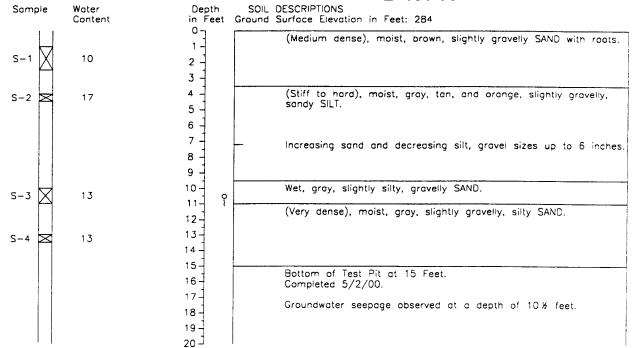
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of excavation. Conditions may vary with time.

J-4978-26 5/00 Figure A-107

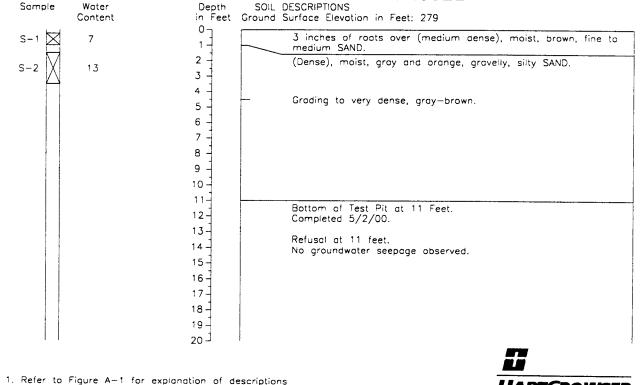
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Test Pit Log HC00-TP305





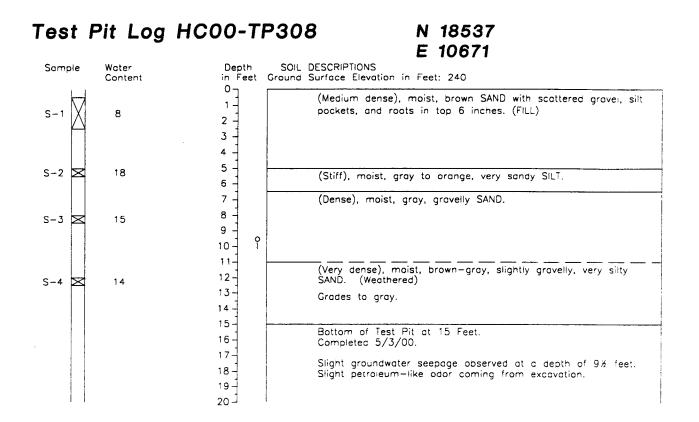
and symbols. 2. Soil descriptions and stratum lines are interpretive

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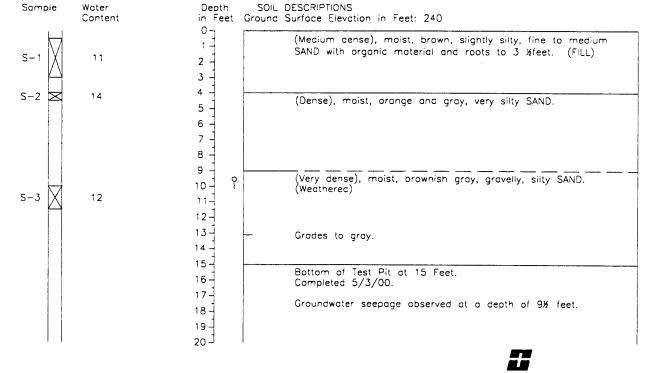
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- DTN 11/13/00 TESTPITS
 - and actual changes may be gradual.
 Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.

HARTCROWSER J-4978-26 5/00 Figure A-108







1. Refer to Figure A-1 for explanation of descriptions and symbols.

2. Soil descriptions and stratum lines are interpretive

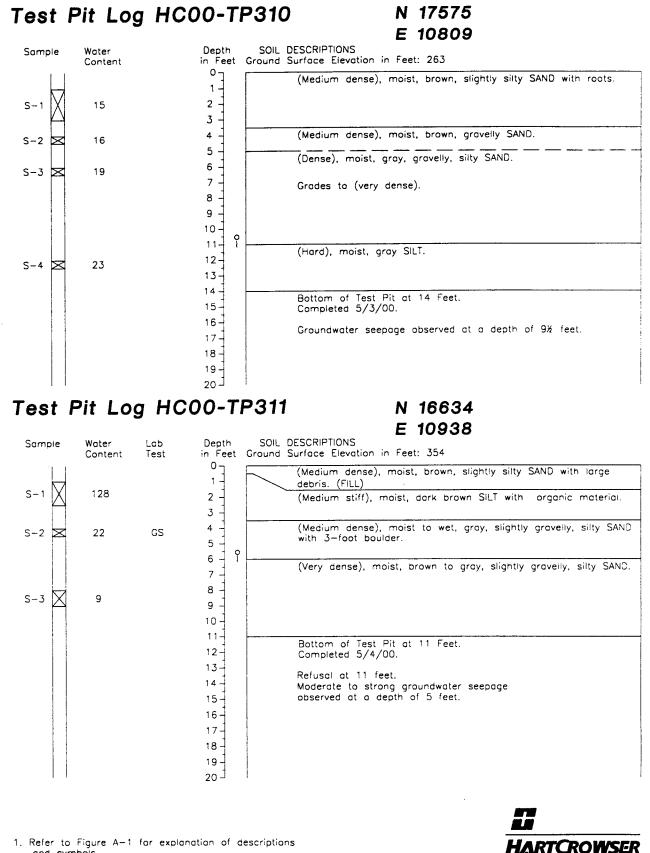
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and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.

HARTCROWSER J-4978-26 5/00 Figure A-109



1. Refer to Figure A-1 for explanation of descriptions and symbols.

2. Soil descriptions and stratum lines are interpretive and actual changes may be gradual. 3. Groundwater conditions, if indicated, are at the time

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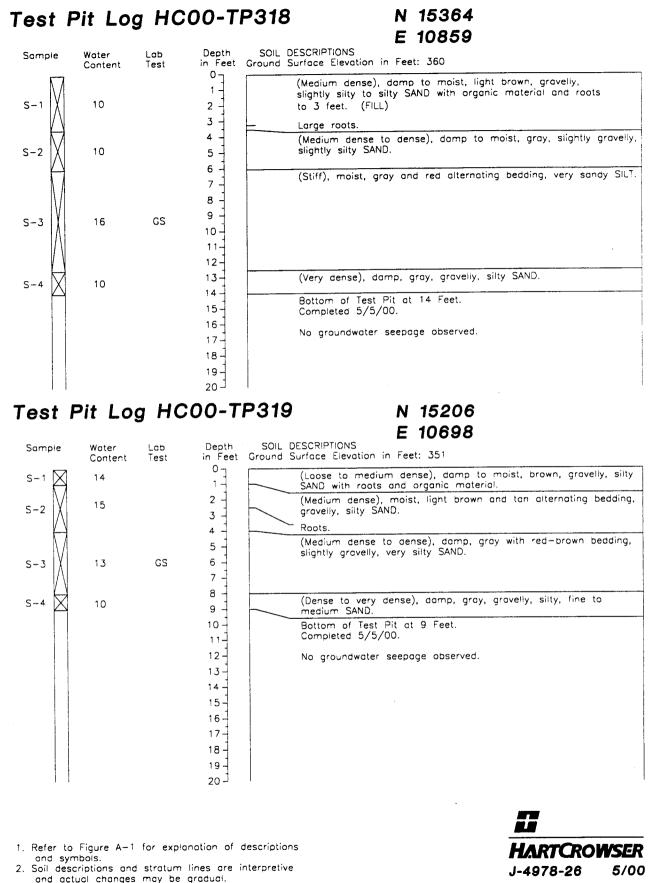
01N 8/30/00 IESIPIIS

of excavation. Conditions may vary with time.

Figure A-110

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J-4978-26



and actual changes may be gradual.
Groundwater conditions, if indicated, are at the time of excavation. Conditions may vary with time.

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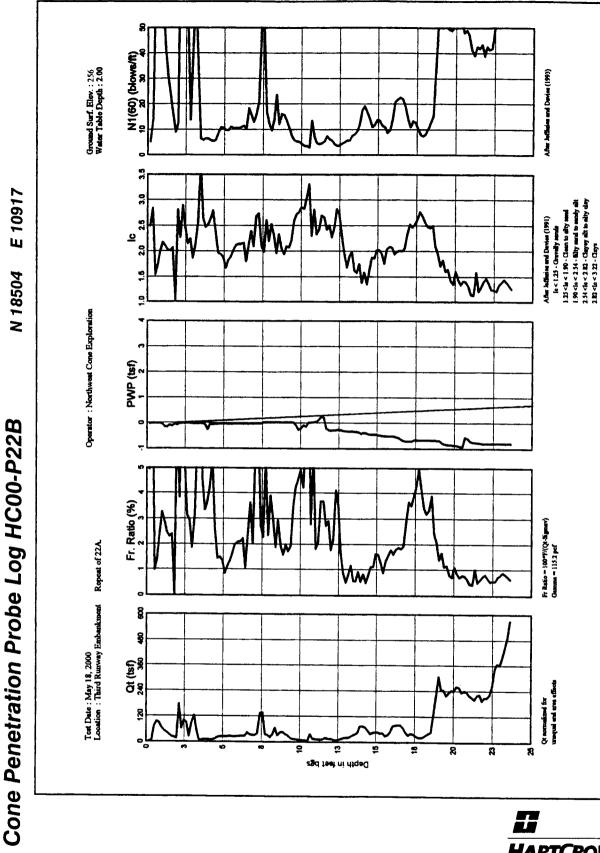
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Figure A-111

Hart Crowser CVD 6/16/00 4978270.cdr 8 N1(60) (blows/ft) 10 20 30 40 Ground Swrf. E 253 0.00 Water Table Depth : 2.00 After Juffinies and Davies (1993) 0 9.0 30 N 18507 E 10928 1.90 <le < 2.34 - Sity and to andy ait 2.34 <lo < 2.82 - Clayey ait to aity day 2.82 <lo < 3.22 - Claye 15 15 After Jaffmins and Device (1991) 1.25 < lo < 1.90 - Clean to aily 1 20 la < 1.25 - Canvel 5 5 Operator : Northwest Cone Exploration PWP (tsf) Cone Penetration Probe Log HC00-P22A DRAWN BY: Kelth Brown Fr. Ratio (%) Refuned at 5 feet depth. Pr Ratio = 100°F/(Qt-Signary) Octava = 115.2 pcf DATE: May 24, 2000 Test Dato : May 18, 2000 Location : Third Runway Esnbankment 8 ş 0t (tsf) 28 380 380 Qi noundized for unquel and are effect 8 PROJECT NO. J-4878-27 2 5 15 18 8 ន 8 sgd teet ni ritged Z HARTCROWSER 8/01

J-4978-27 Figure A-112



HARTCROWSER J-4978-26 8/01

Hart Crowser

DRAWN BY: Kelth Brown

DATE: May 24, 2000

PROJECT NO. J-4878-27

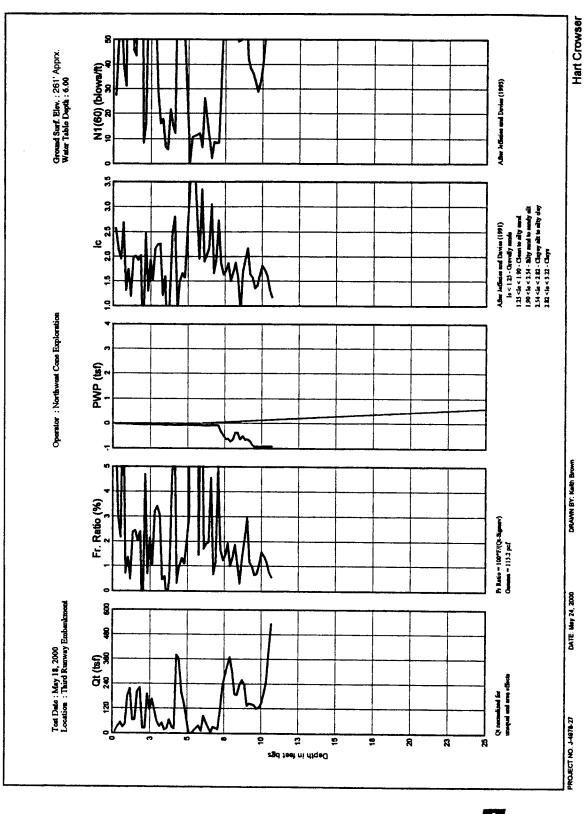
DTN 8/30/00 497827P.cdr

J-4978-26 8, Figure A-113

Cone Penetration Probe Log HC00-P23

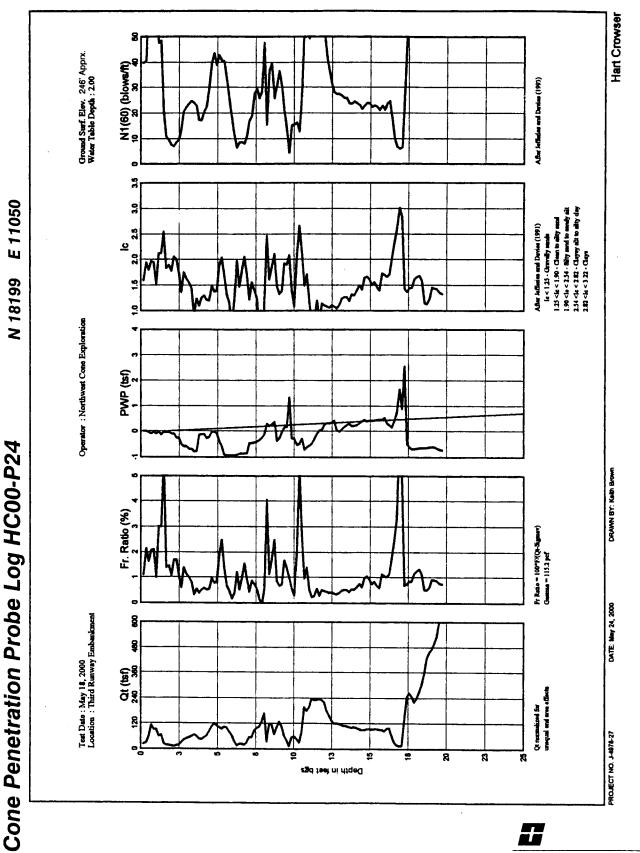
E 11000

N 18590



HARTCROWSER J-4978-27 8/01 Figure A-114

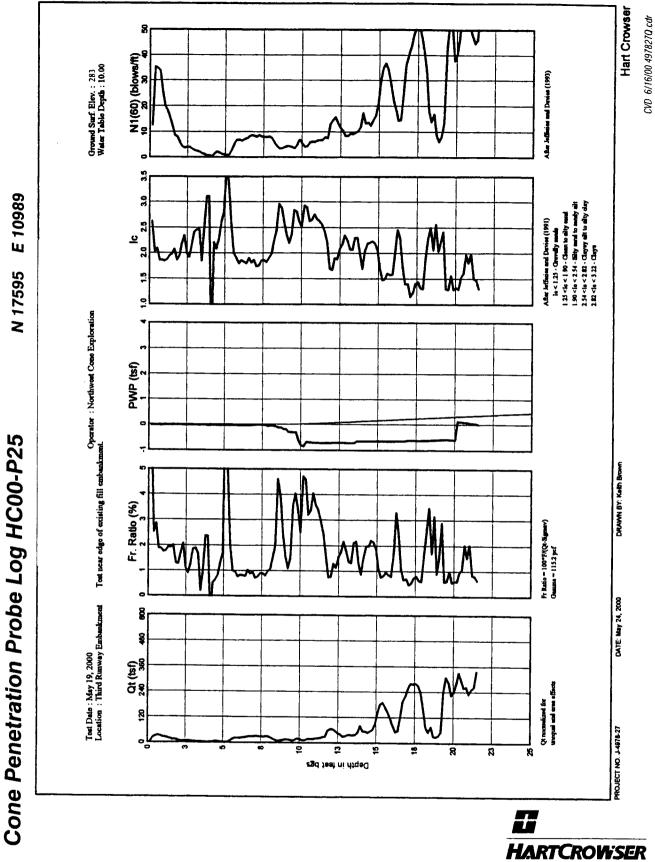
CVD 6/16/00 497827C.cdr



HARTCROWSER J-4978-27

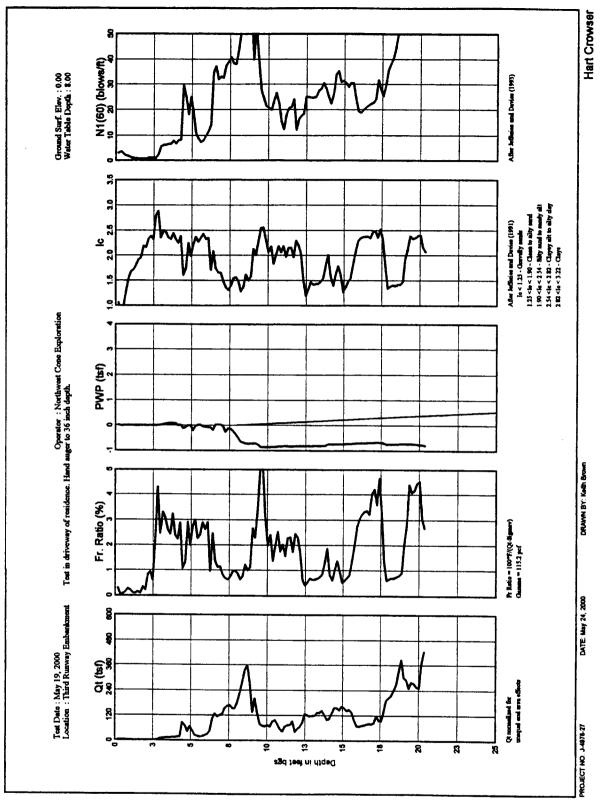
DTN 8/30/00 497826D.cdr

^{8/01} Figure A-115



J-4978-27 8/01 Figure A-116

Cone Penetration Probe Log HC00-P26



HARTCROWSER J-4978-27 8/01 Figure A-117

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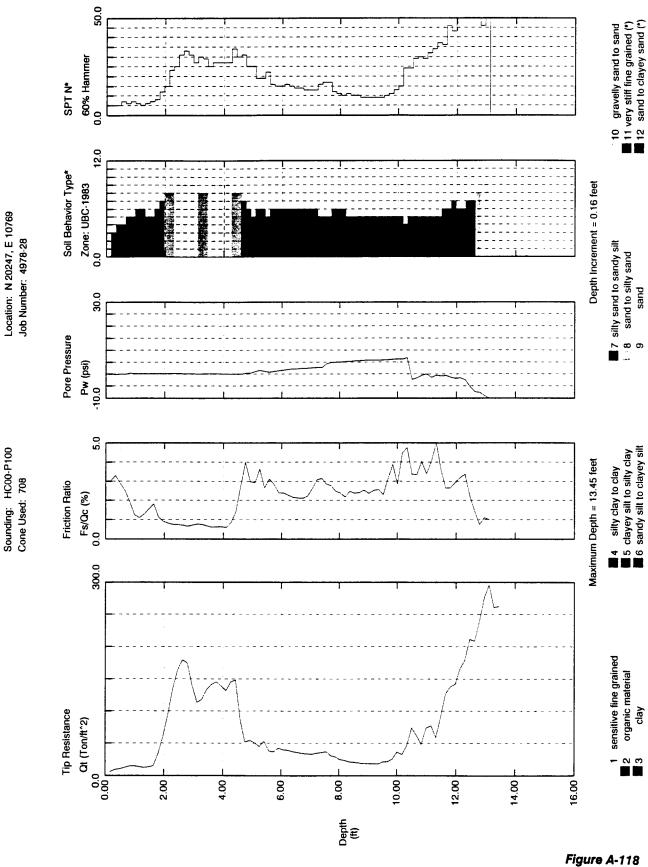
DTN 8/30/00 497827R.cdr

*Soil behavior type and SPT based on data from UBC-1983

Hart Crowser, Inc.

Operator: KRB

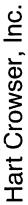
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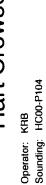


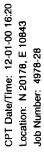
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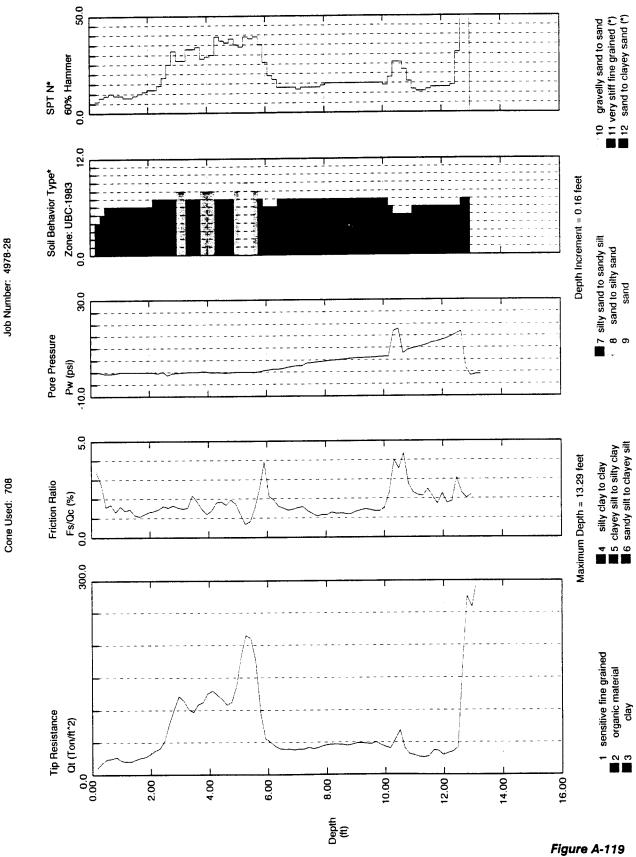
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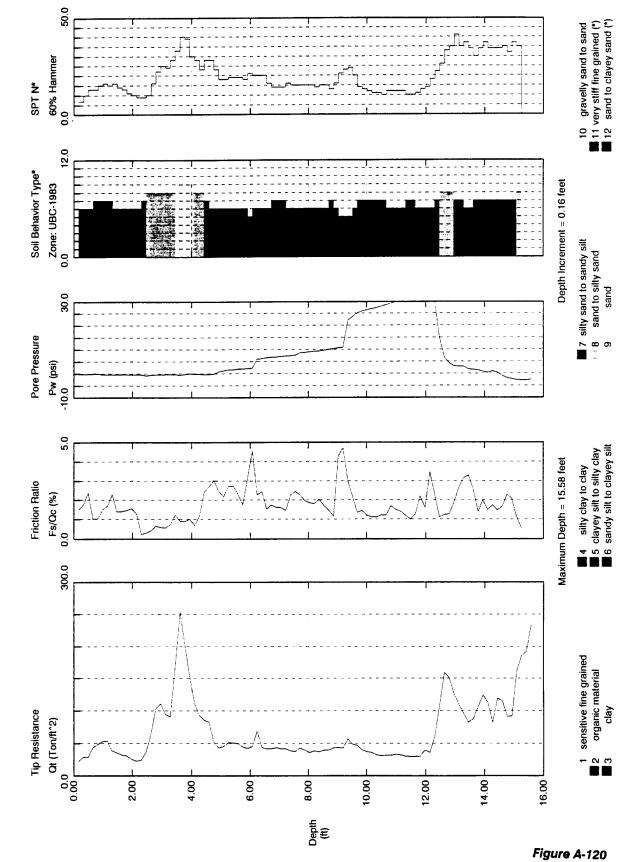


*Soil behavior type and SPT based on data from UBC-1983

Hart Crowser, Inc.

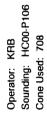


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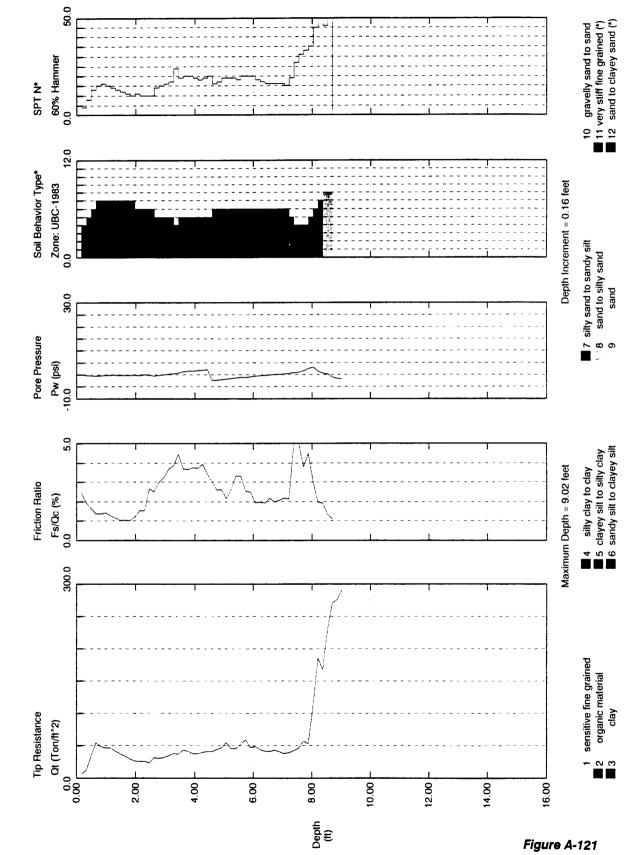


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Hart Crowser, Inc.



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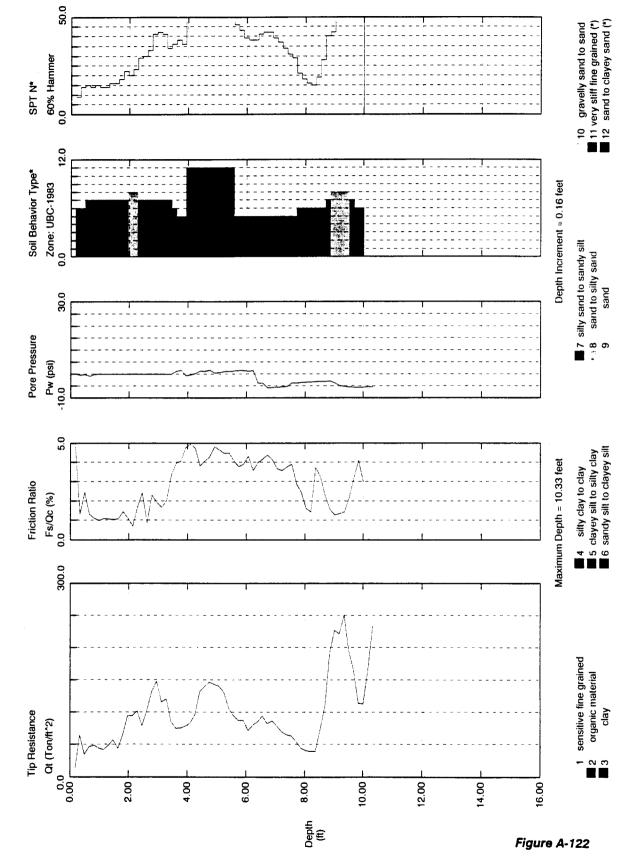


*Soil behavior type and SPT based on data from UBC-1983

Hart Crowser, Inc.

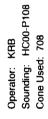


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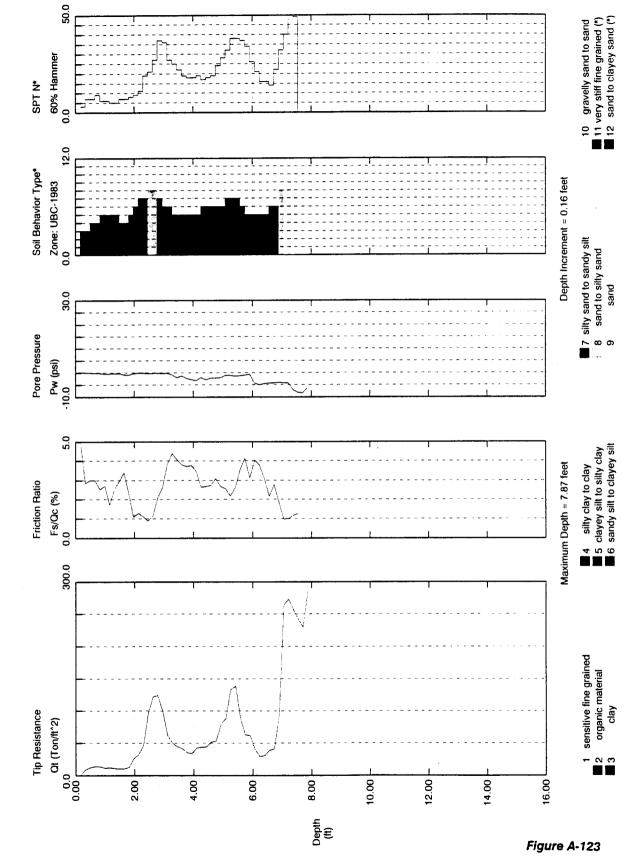


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Hart Crowser, Inc.



CPT Date/Time: 12-04-00 17:33 Location: N 19780, E 10674 Job Number: 4978-28

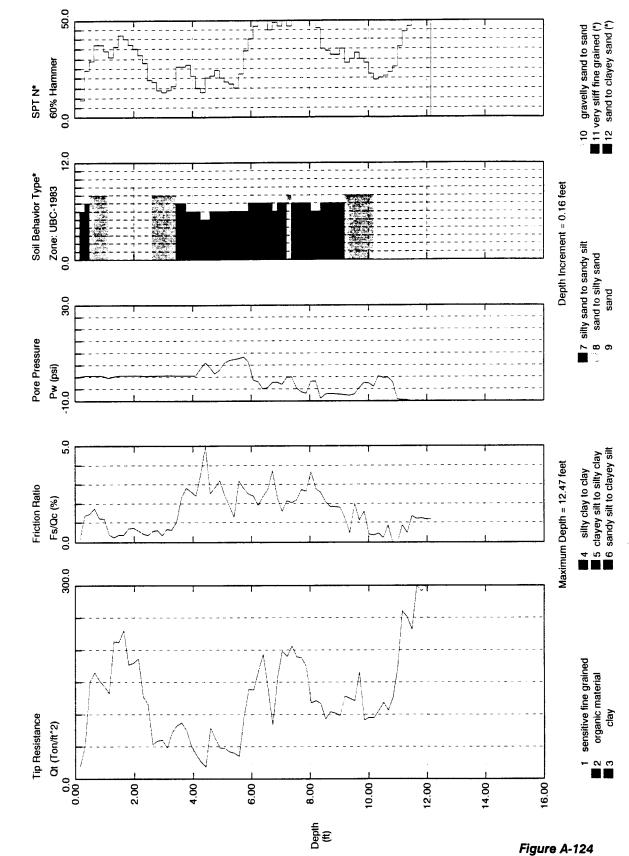


*Soil behavior type and SPT based on data from UBC-1983

Hart Crowser, Inc.



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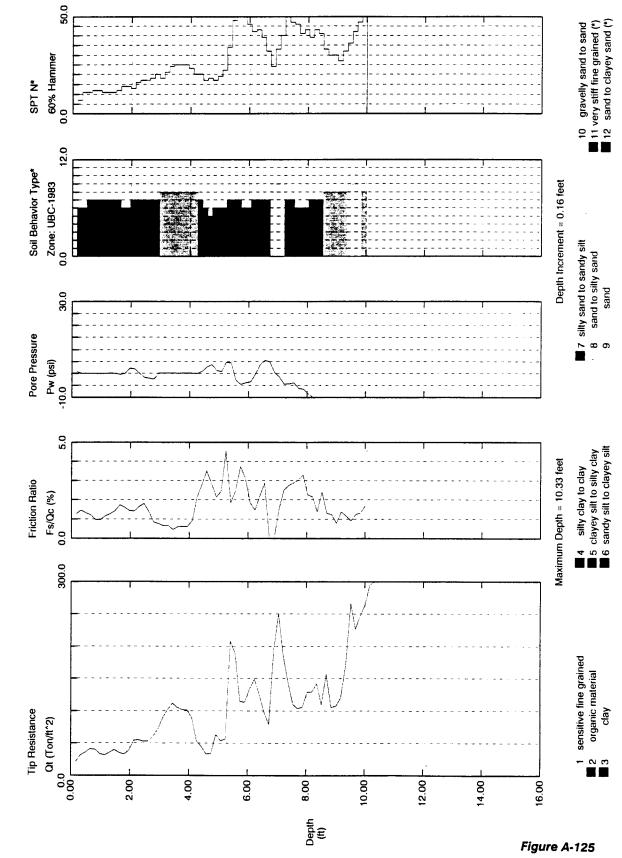
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Hart Crowser, Inc.



CPT Date/Time: 12-01-00 17:29 Location: N 19435, E 10877 Job Number: 4978-28

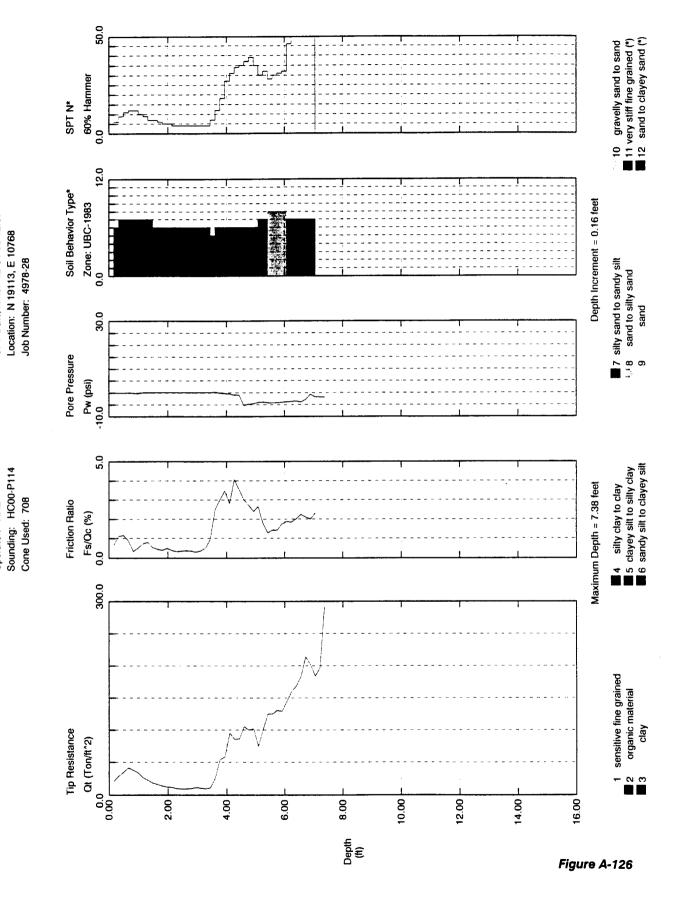


*Soil behavior type and SPT based on data from UBC-1983

Hart Crowser, Inc.

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Operator: KRB



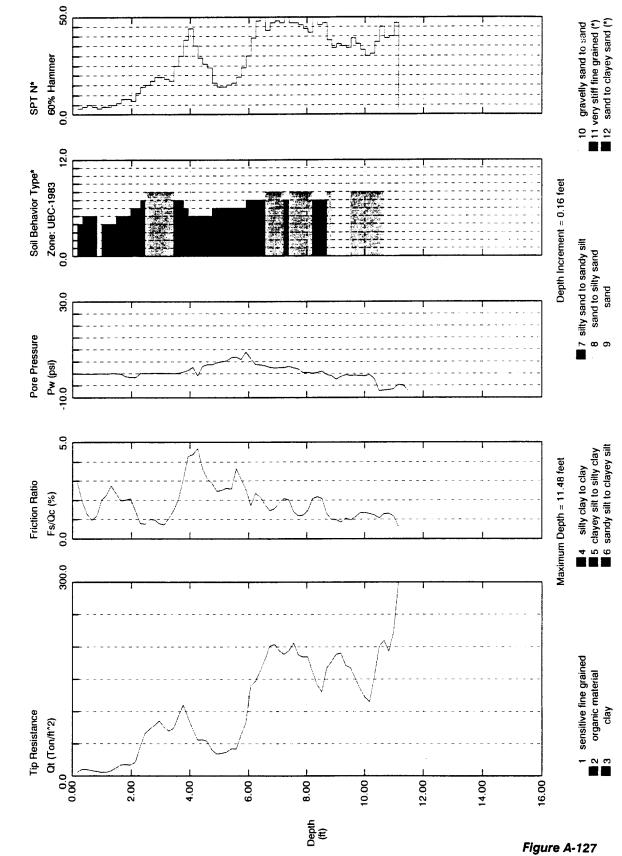
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Hart Crowser, Inc.

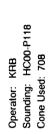


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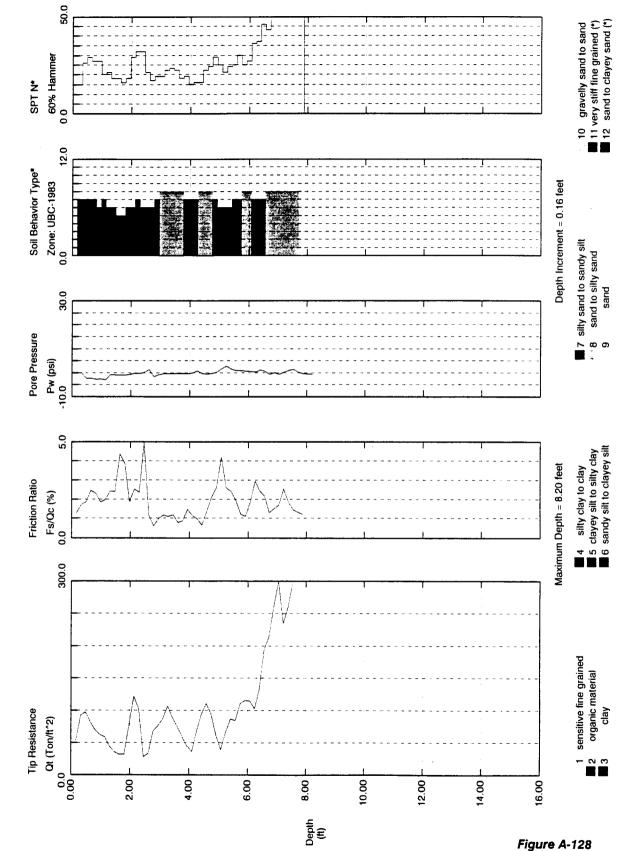


*Son uchavior type and SPT based on data from UBC-1983

Hart Crowser, Inc.







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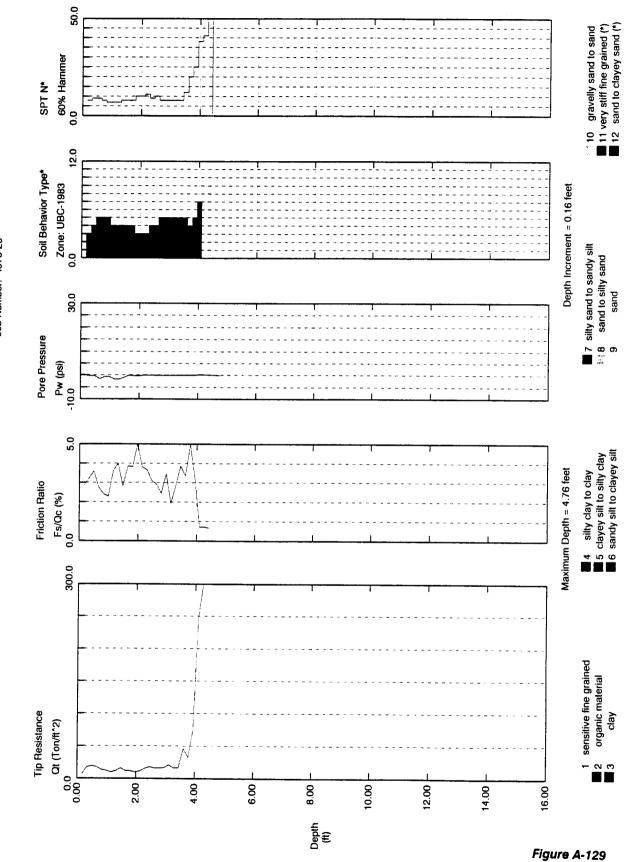
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Hart Crowser, Inc.

Operator: KRB Sounding: HC00-P119 Cone Used: 708

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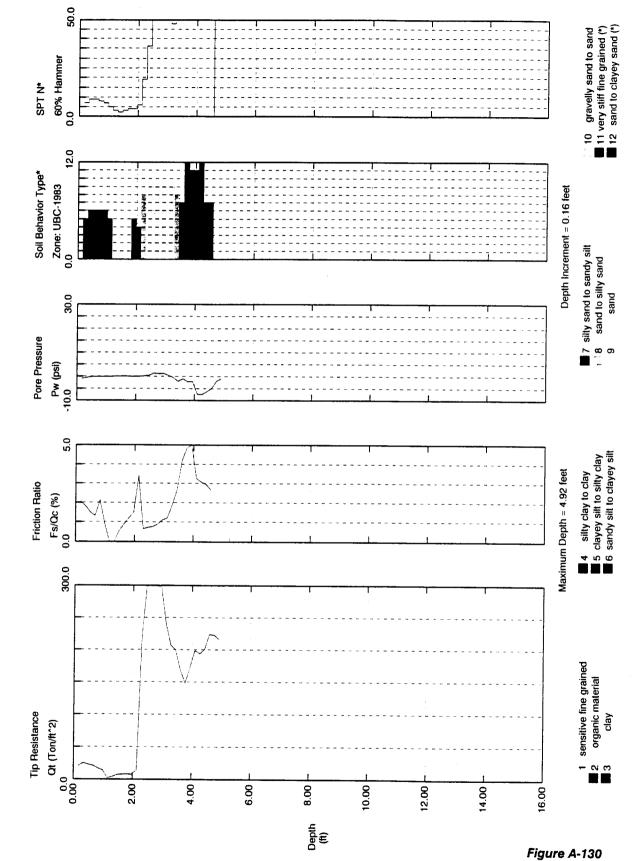


*Soil behavior type and SPT based on data from UBC-1983

Hart Crowser, Inc.

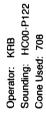
Operator: KRB Sounding: HC00-P120 Cone Used: 708

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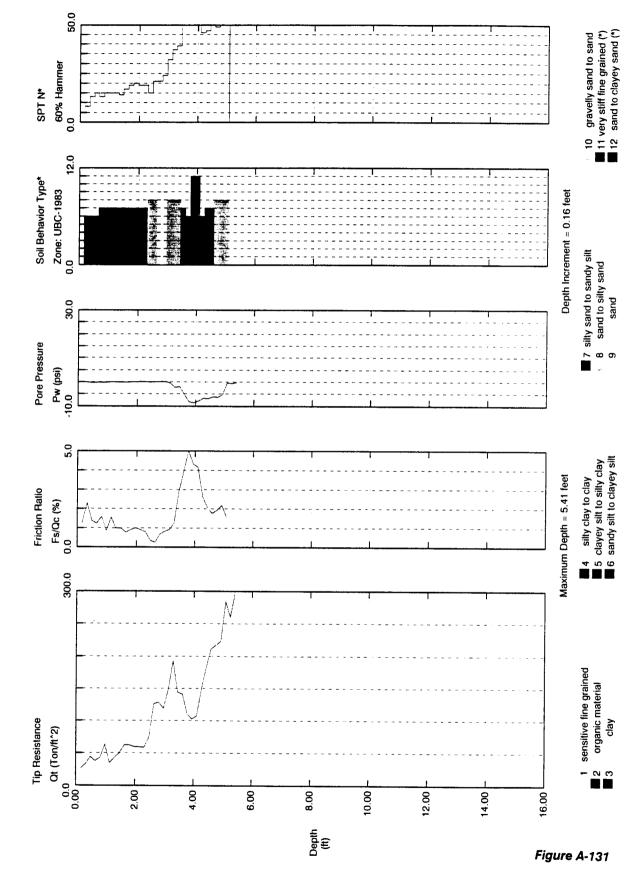


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Hart Crowser, Inc.



CPT Date/Time: 12-04-00 16:41 Location: N 15441, E 10929 Job Number: 4978-28

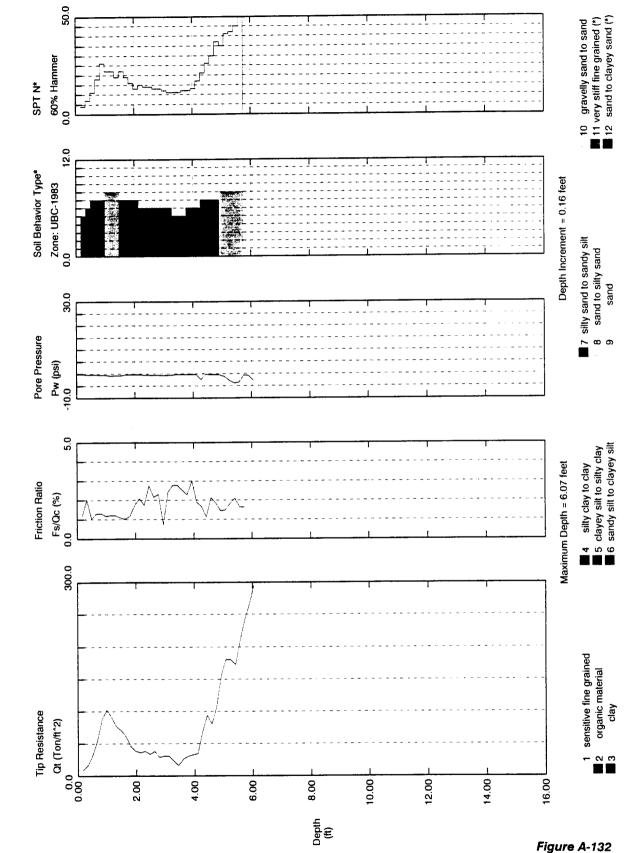


*Soil behavior type and SPT based on data from UBC-1983

Hart Crowser, Inc.



CPT Date/Time: 12:04-00 16:53 Location: N 15143, E 10983 Job Number: 4978-28



APPENDIX B LABORATORY TESTING PROGRAM

Hart Crowser 4978-28 September 21, 2001

APPENDIX B LABORATORY TESTING PROGRAM

Laboratory testing was accomplished by Hart Crowser to evaluate the basic index and geotechnical engineering properties of the site soils. Both disturbed and relatively undisturbed samples were tested. The tests performed and the procedures followed are outlined below.

Additional laboratory results obtained by Applied Geotechnology, Inc. (AGI) are presented at the end of Appendix C.

Appendix D presents results of additional classification and strength tests on peat and Appendix E presents results of triaxial strength tests on clay and silt soils.

<u>NOTE:</u> Laboratory test results presented in this report were compiled from earlier reports. These reports should be consulted in the interpretation of the laboratory test data.

Soil Classification

Field Observation and Laboratory Analysis. Soil samples from the explorations were visually classified in the field and then taken to our laboratory where the classifications were verified in a relatively controlled laboratory environment. Field and laboratory observations include density/consistency, moisture condition, and grain size and plasticity estimates.

The classifications of selected samples were checked by laboratory tests such as Atterberg limits determinations and grain size analyses. Classifications were made in general accordance with the Unified Soil Classification (USC) System, ASTM D 2487, as presented on Figure B-1.

Water Content Determinations

Water contents were determined for most samples recovered in the explorations in general accordance with ASTM D 2216, as soon as possible following their arrival in our laboratory. Water contents were not determined for very small samples nor samples where large gravel contents would result in values considered unrepresentative. The results of these tests are plotted at the respective sample depth on the exploration logs. In addition, water contents are routinely determined for samples subjected to other testing. These are also presented on the exploration logs.

Grain Size Analysis (GS)

Grain size distribution was analyzed on representative samples in general accordance with ASTM D 422. Wet sieve analysis was used to determine the size distribution greater than the U.S. No. 200 mesh sieve. The size distribution for particles smaller than the No. 200 mesh sieve was determined by the hydrometer method for a selected number of samples. The results of the tests are presented as curves on Figures B-2 through B-44 plotting percent finer by weight versus grain size.

Atterberg Limits (AL)

We determined Atterberg limits for selected fine-grained soil samples. The liquid limit and plastic limit were determined in general accordance with ASTM D 4318. The results of the Atterberg limits analyses and the plasticity characteristics are summarized in the Liquid and Plastic Limits Test Report, Figures B-45 through B-63. This relates the plasticity index (liquid limit minus the plastic limit) to the liquid limit. The results of the Atterberg limits tests are shown graphically on the boring logs as well as where applicable on figures presenting various other test results.

Pocket Penetrometer (PP) and Torvane (TV)

The pocket penetrometer and torvane procedures provide quick approximate tests of the consistency (undrained shear strength) of a cohesive soil sample. The pocket penetrometer device consists of a calibrated spring mechanism which measures penetration resistance of a 1/4-inch-diameter steel tip over a given distance. The penetration resistance is correlated to the unconfined compressive strength of the soil, which is typically twice the undrained shear strength of a saturated, cohesive soil.

The torvane device consists of a 1-inch-diameter plate with eight equally spaced and radially arranged 1/4-inch vanes. The vanes are pressed into the soil and the device is rotated. The vanes force a shear failure to take place over the area of plate face. The resistance at failure, as measured by a calibrated spring, correlates to the undrained shear strength of the sample tested. The exploration logs show the results of the pocket penetrometer and torvane tests in tsf (tons per square foot).

Consolidation Test (CN)

The one-dimensional consolidation test provides data for estimating settlement and preconsolidation pressure. The test was performed in general accordance with ASTM D 2435. A relatively undisturbed, fine-grained sample was carefully trimmed and fit into a rigid ring with porous stones placed on the top and bottom of the sample to allow drainage. Vertical loads were then applied incrementally to the sample in such a way that the sample was allowed to consolidate under each load increment. Measurements were made of the compression of the sample (with time) under each load increment. Rebound was measured during the unloading phase. In general, each load was left in place until the completion of 100 percent primary consolidation, as computed using Taylor's square root of time method. The next load increment was applied soon after attaining 100 percent primary consolidation. The test results plotted in terms of axial strain and coefficient of consolidation versus applied load (stress) are presented on Figures B-64 through B-71.

Triaxial Unconsolidated Undrained Compression Test (TUU)

The triaxial unconsolidated undrained compression test estimates the undrained shear strength of the soil. A relatively undisturbed fine-grained sample was trimmed to a length of about 6 inches, encased in a rubber membrane, and placed in the triaxial cell. An all-around confining pressure was applied hydraulically, but the sample was not allowed to consolidate, and no back pressure was applied. An axial load was then applied at a constant strain rate to the sample without allowing drainage from the specimen. The stress-strain behavior was recorded until failure occurred.

The failure stress was generally taken as the maximum load on the sample or the load recorded at 20 percent strain, whichever was greater. The test results plotted in terms of axial strain versus deviator stress are presented on Figure B-72 through B-76. The shear strength is considered to be one-half the maximum stress difference based on the $\phi = 0$ concept and a total stress analysis.

Consolidated Undrained Triaxial Compression Test (CU)

The consolidated undrained triaxial compression test with pore pressure measurement is used to determine effective strength of soil at various stress levels. We performed this test in general accordance with ASTM D 4767. A relatively undisturbed fine-grained soil sample was extruded from a shelby tube and trimmed to a length of about 6 inches, encased in a rubber membrane, and placed in the triaxial cell. The triaxial cell was then filled with de-aired water and a back pressure was applied to saturate the sample and force any remaining air bubbles into solution. The back pressure was applied to both, the sample and cell simultaneously to avoid consolidating the sample. After saturation was achieved, the cell pressure was raised to the desired confining pressure while keeping the back pressure constant and the sample was allowed to consolidate to the all-around equal confining pressure. During the consolidation phase, drainage occurred through porous stones and filter paper strips placed around the sample. Once consolidation was completed, the drainage lines were closed and the sample was loaded to failure under undrained conditions by application of increasing axial load at a constant strain rate.

During loading, we recorded the magnitude of excess pore water pressure developed. To obtain shear strength data, three soil samples were tested for each set of CU tests.

The CU test data are presented on Figures B-77 through B-84. These figures include Mohr envelopes and Mohr circles for effective stresses and for total stresses at failure, stress-strain plots (deviator stress vs. axial strain), and stress path plots.

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Unified Soil Classification (USC) System Soil Grain Size

Coarse-Grained Soils

Γ	Size of Opening In Inches								Number of Mesh per Inch (US Standard)					Grain Size in Millimetres							
12			N -2	× –	3/4 5/8	1/4	3/8	t	10	20	40	60	90	200 06	04	03	05	00 00 00 00	003	003	00
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0		-								Grain Size in	h Millir	metres									
Γ	COBBL	COBBLES GRAVEL			SAND					SILT and CLAY											

Coarse-Grained Soils

GW	GP	GM	GC	SW	SP	SM	SC			
Clean GRAV	EL <5% fines	GRAVEL witl	h >12% fines	Clean SANE) <5% fines	SAND with >12% fines				
GRA	VEL >50% coarse	fraction larger thar	n No. 4	SAND >50% coarse fraction smaller than No. 4						
·		Coarse-	Grained Soils >50	% larger than No. 2	00 sieve					

G W and S W
$$\left(\frac{D_{60}}{D_{10}}\right) > 4$$
 for G W $4 1 \le \left(\frac{(D_{30})^2}{D_{10} \times D_{60}}\right) \le 3$

G P and S P Clean GRAVEL or SAND not meeting requirements for G W and S W

G C and S C Atterberg limits above A Line with PI >7

Fine-Grained Soils

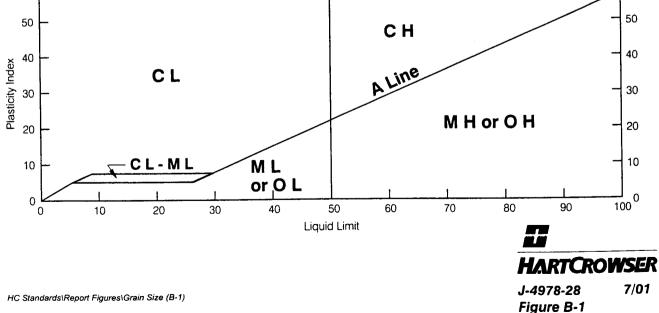
G M and S M Atterberg limits below A line with Pl <4

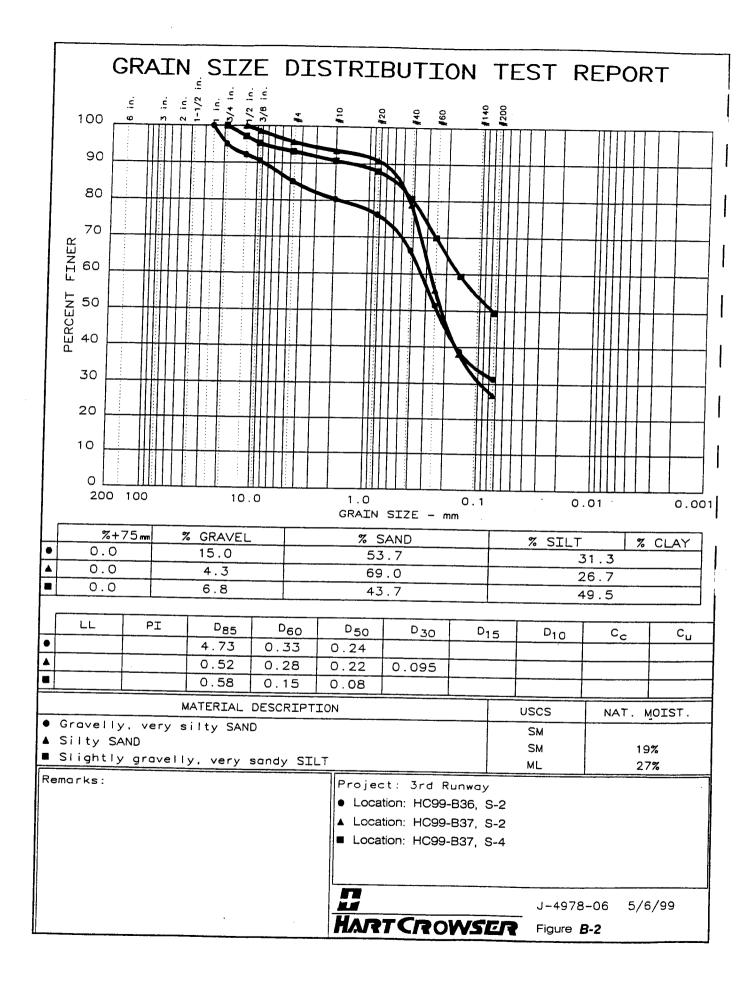
* Coarse-grained soils with percentage of fines between 5 and 12 are considered borderline cases required use of dual symbols.

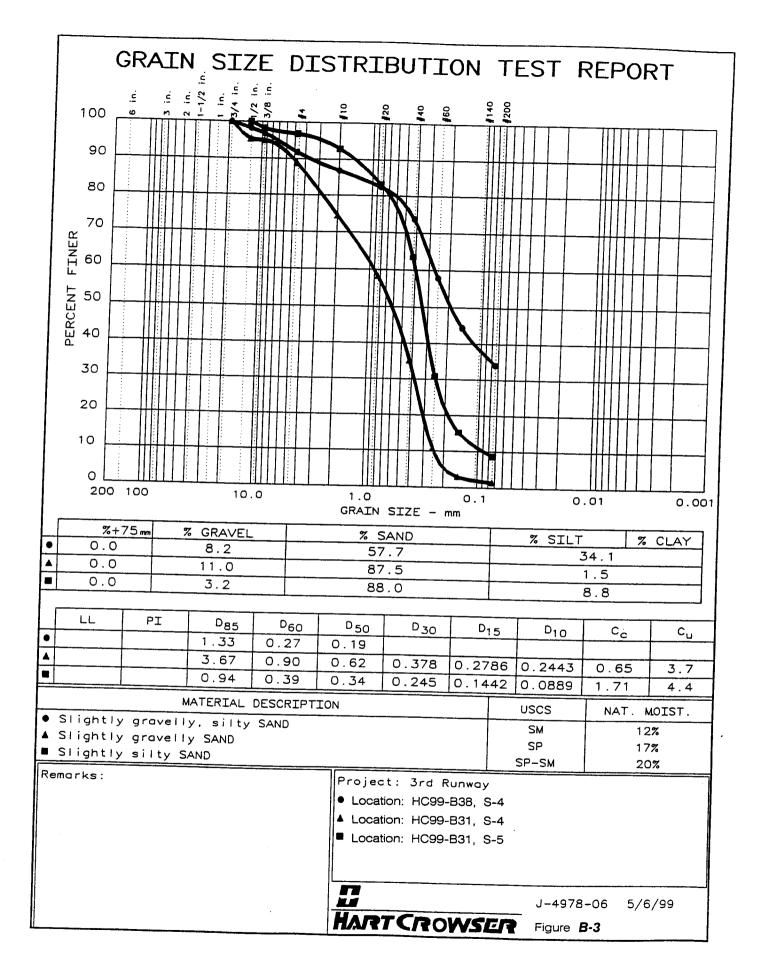
D₁₀, D₃₀, and D₆₀ are the particles diameter of which 10, 30, and 60 percent, respectively, of the soil weight are finer.

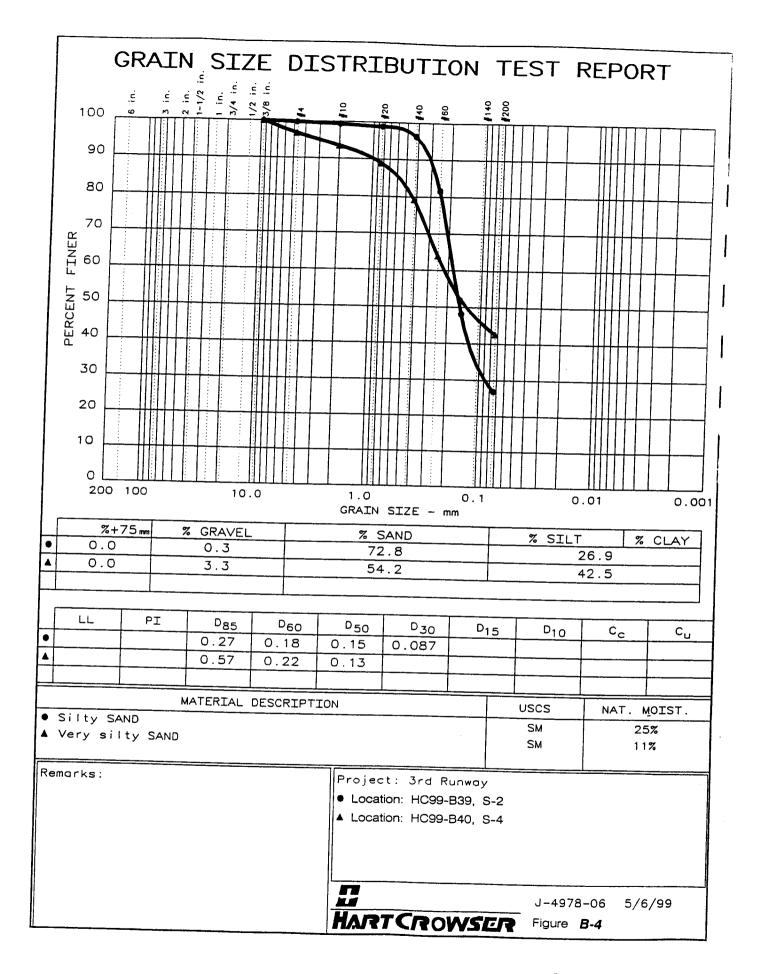
Fine-Grained Soils

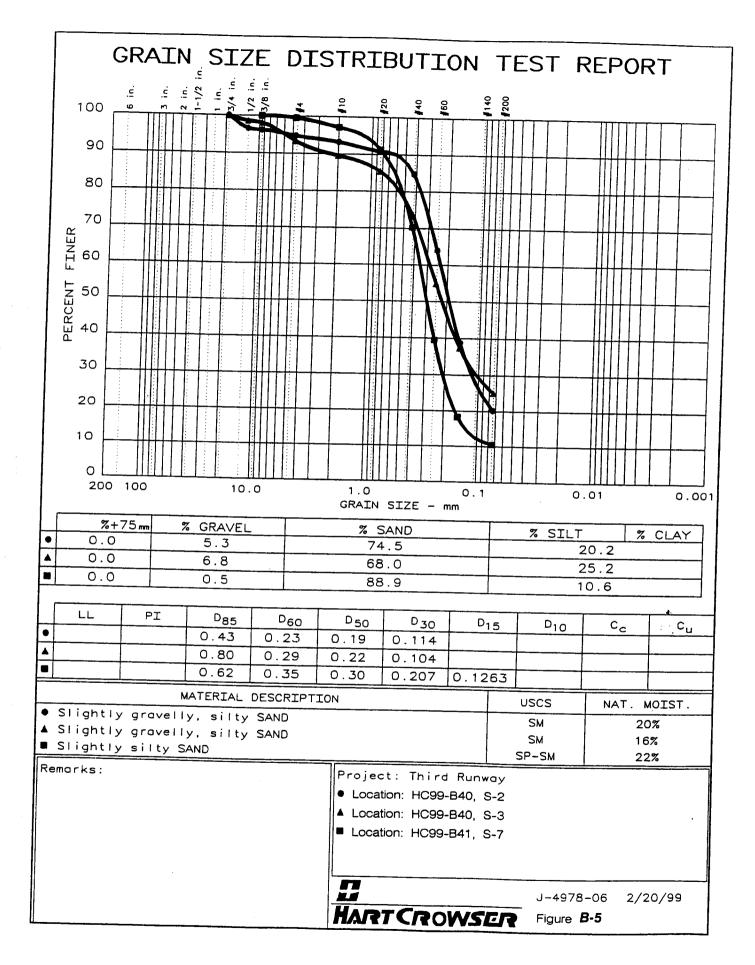
ML	CL	OL	ΜH	СН	ОН	Pt
SILT	CLAY	Organic	SILT	CLAY	Organic	Highly Organic
Soi	ls with Liquid Limit <	50%	Soi	ils with Liquid Limit >	50%	Soils
		Fine-Grained Sc	oils >50% smaller that	an No. 200 sieve		
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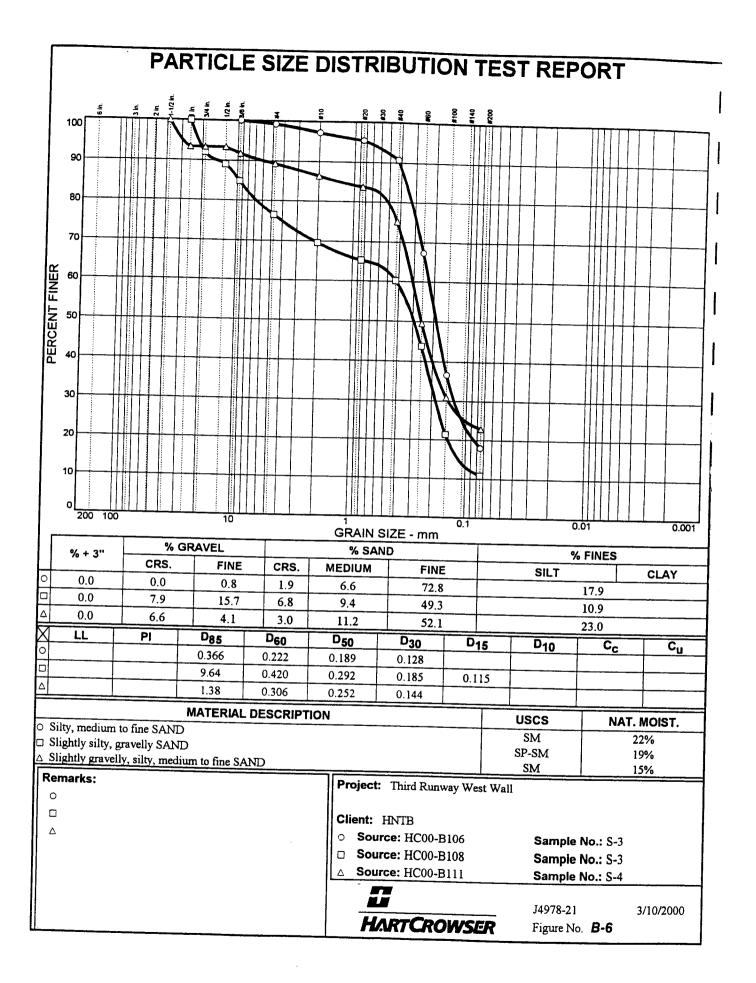


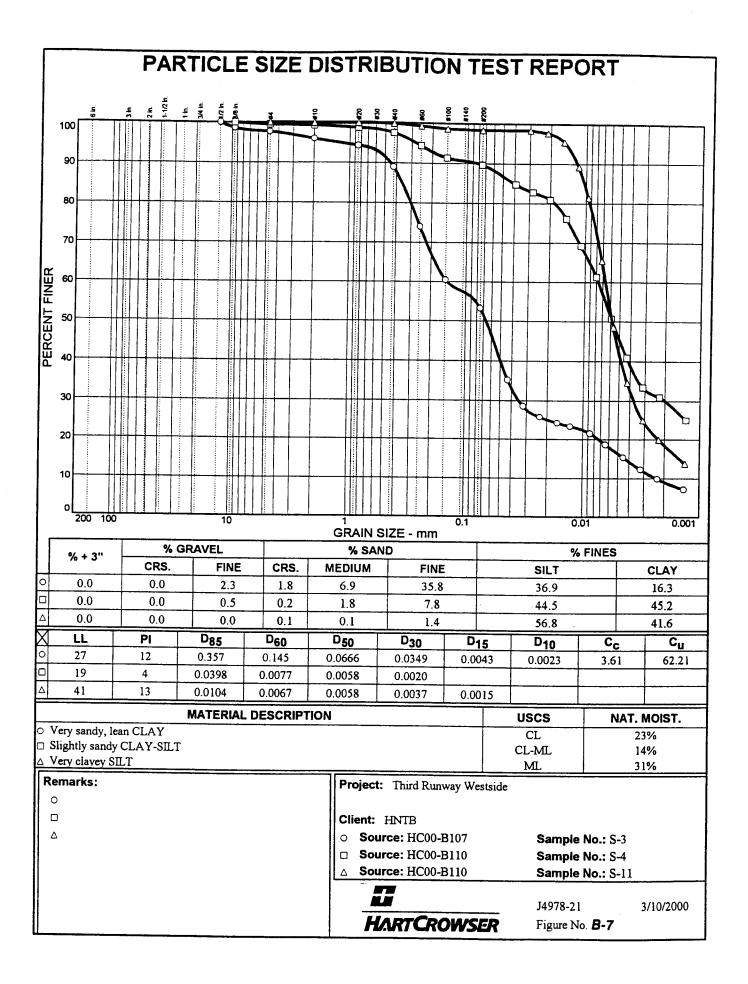


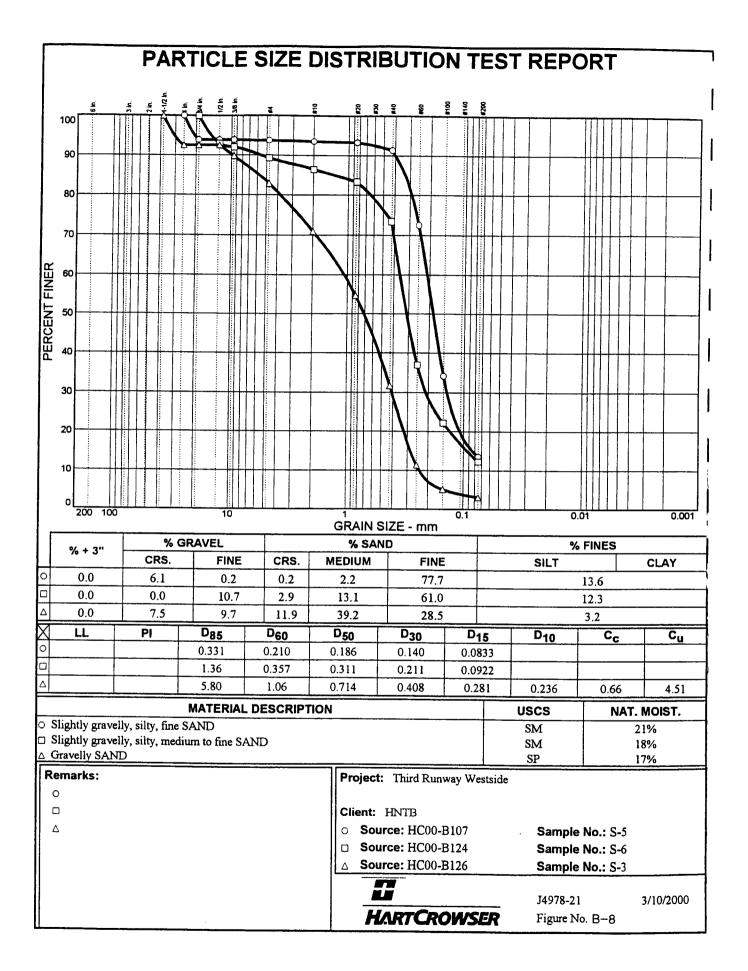


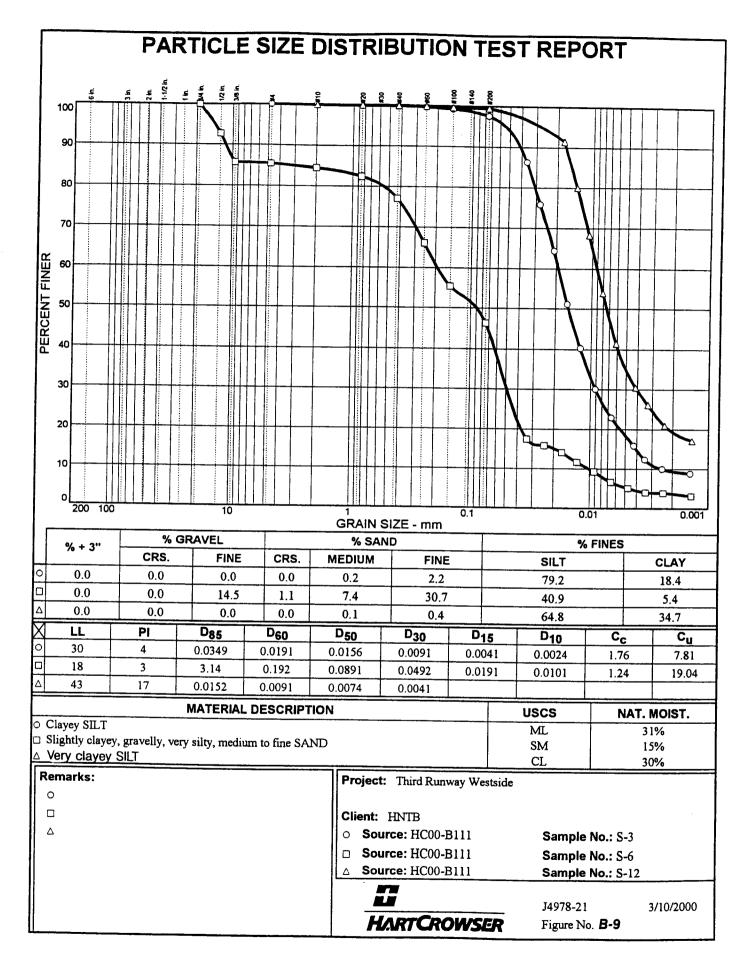


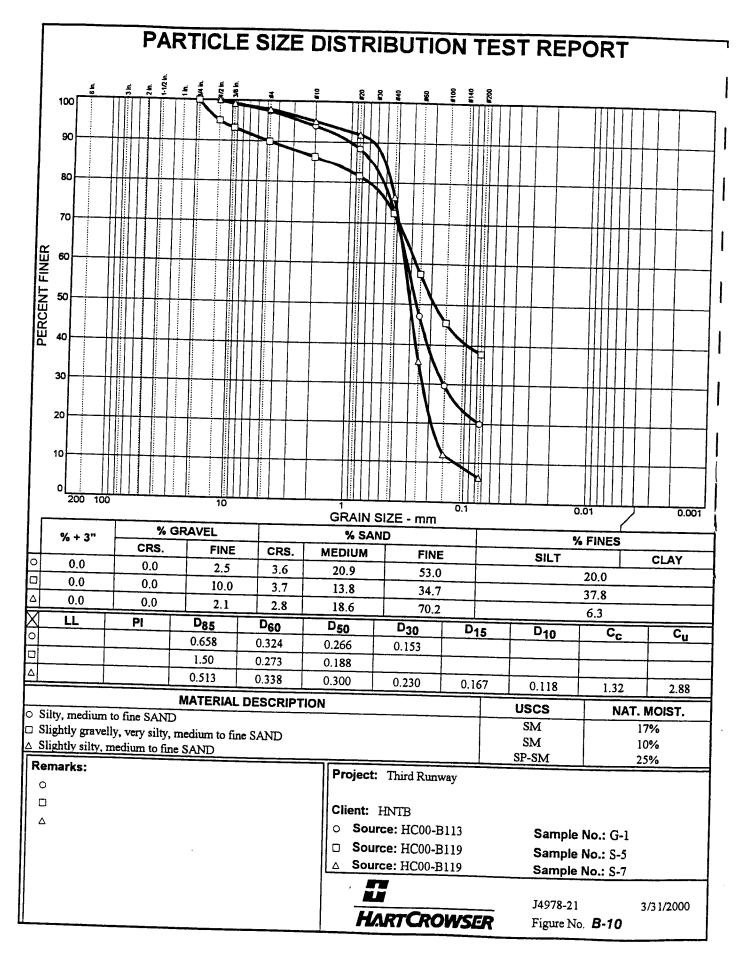




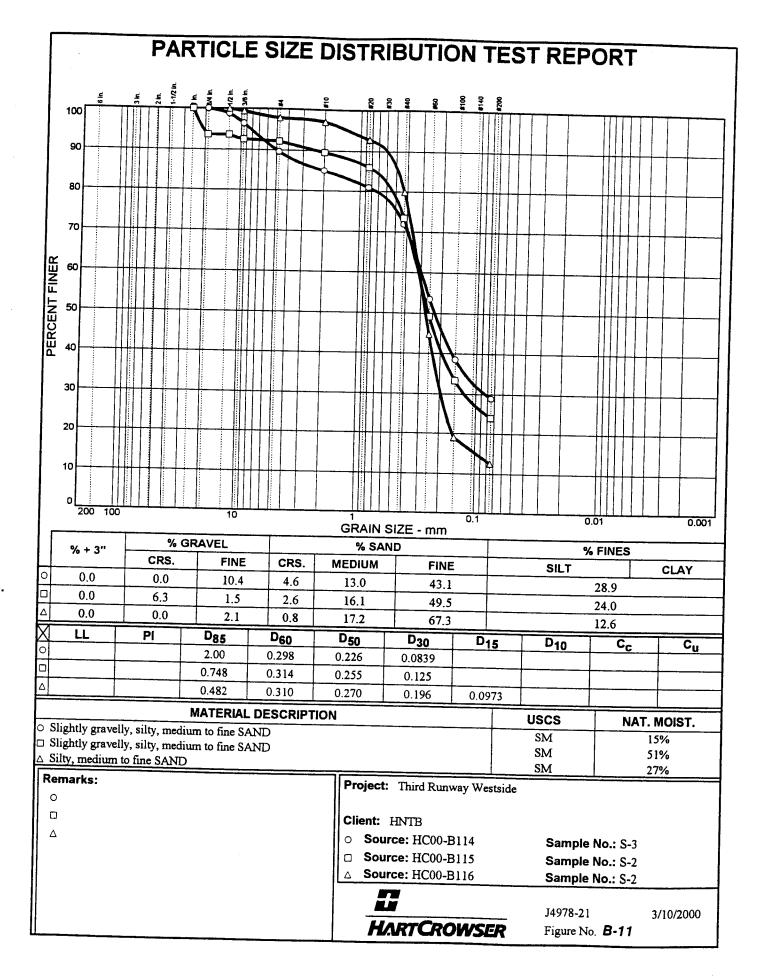


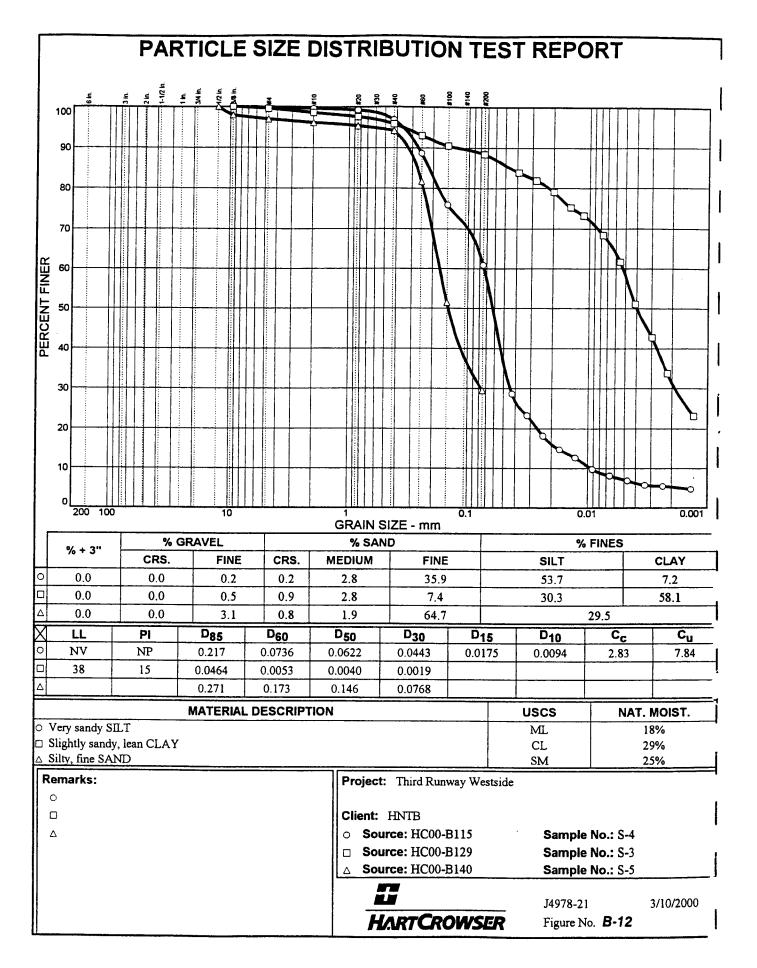


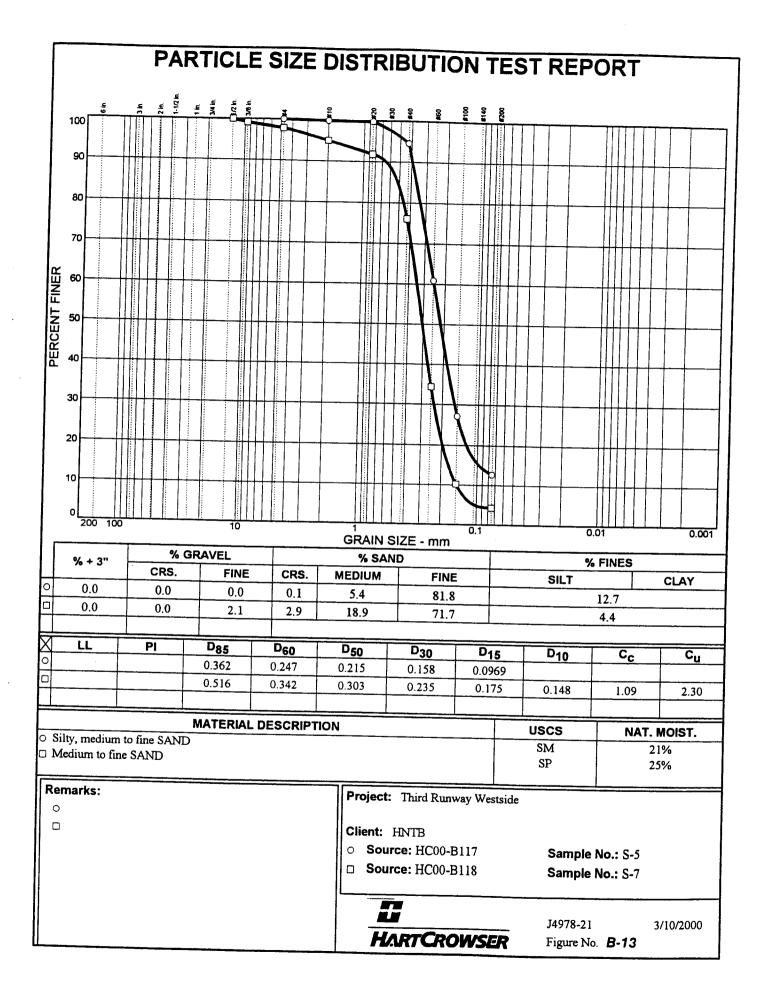


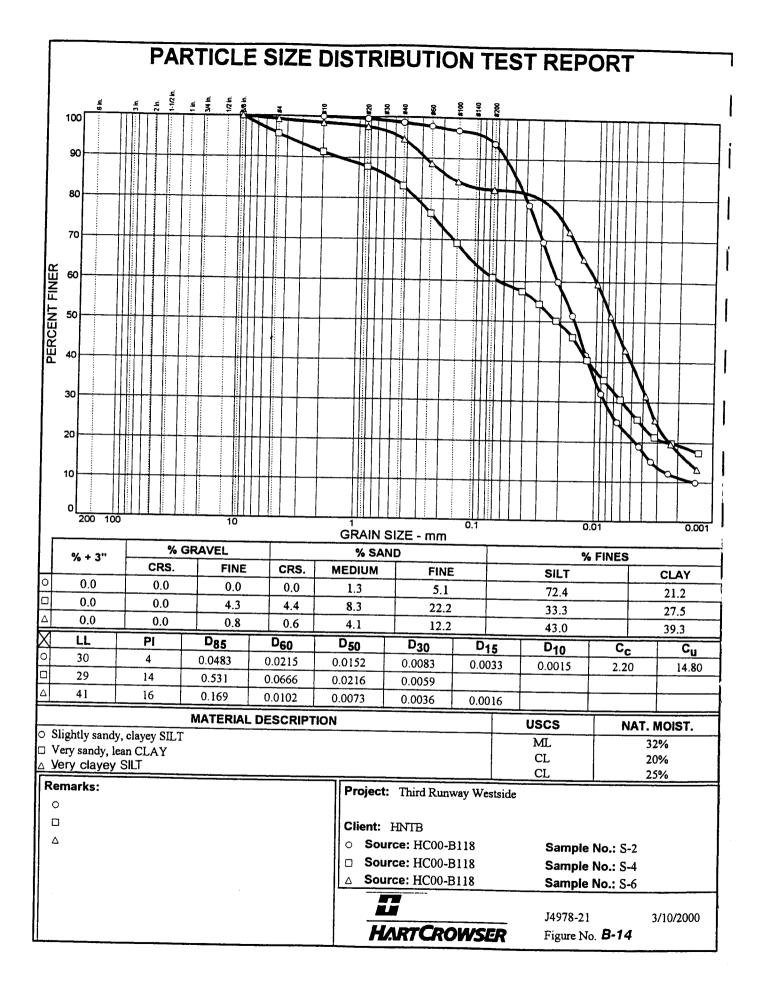


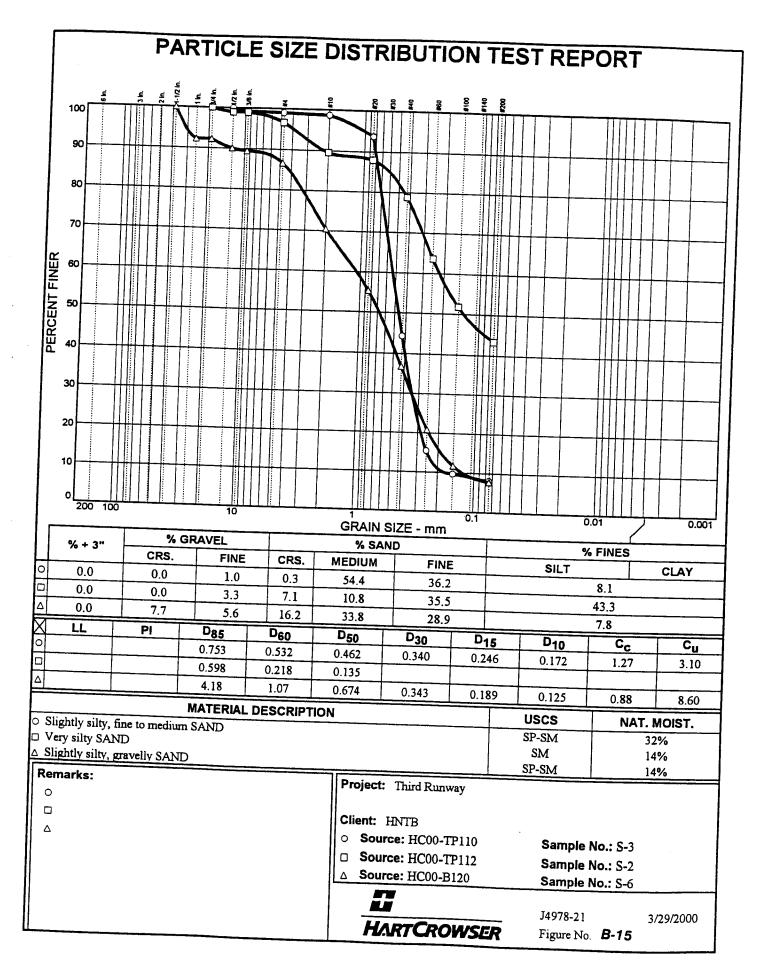
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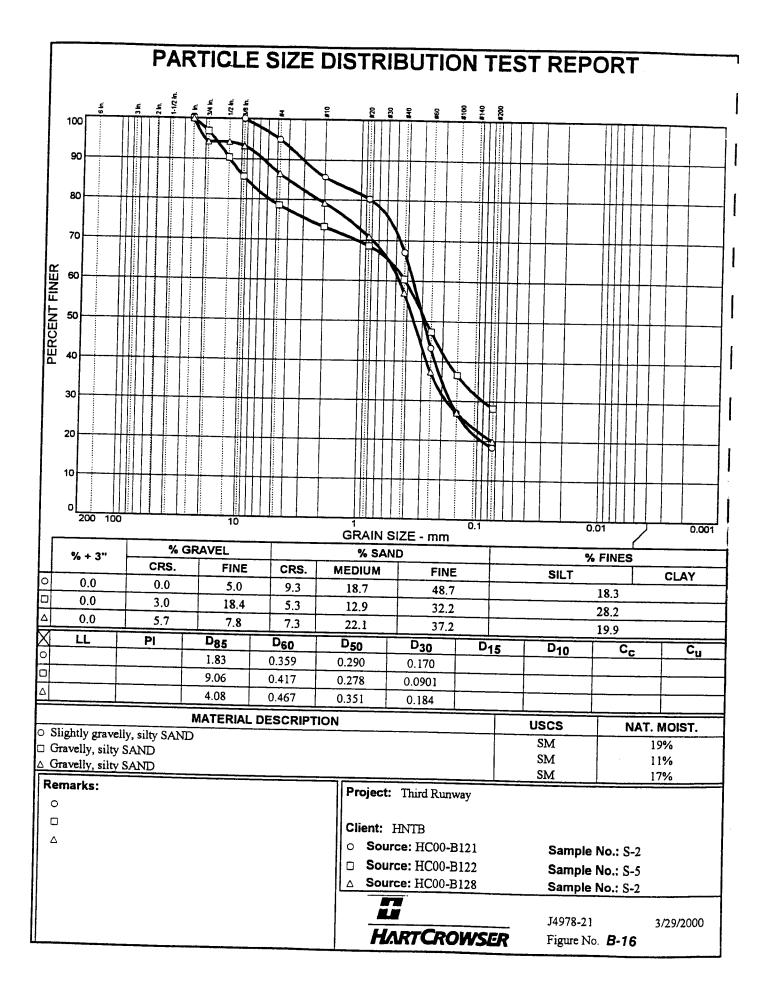


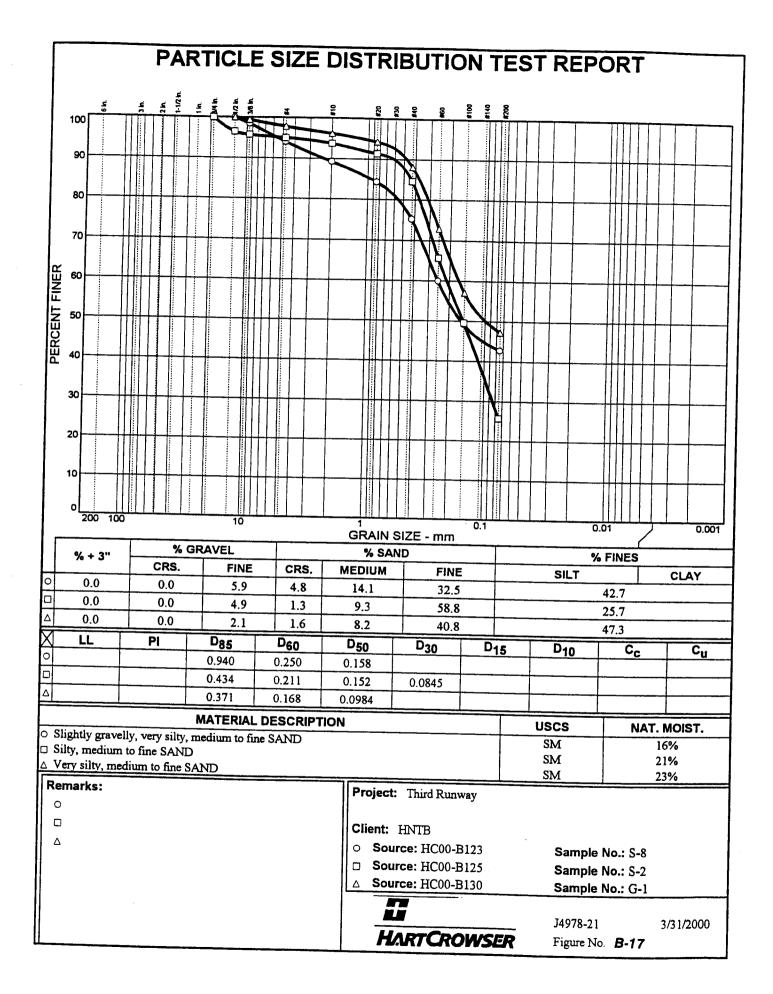


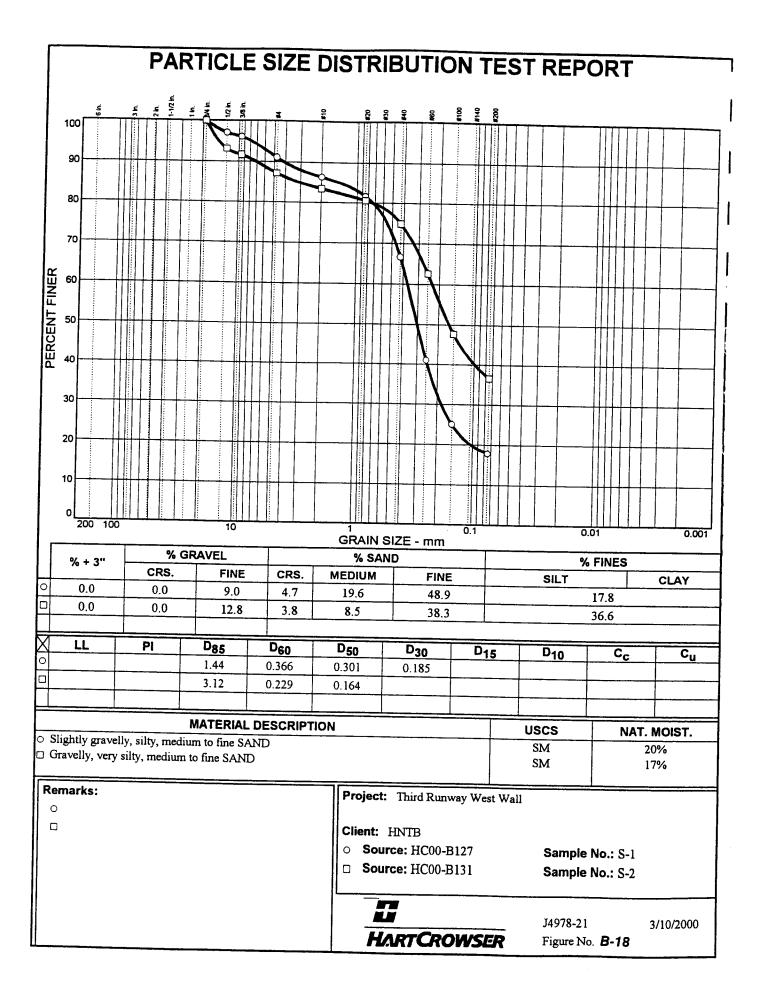


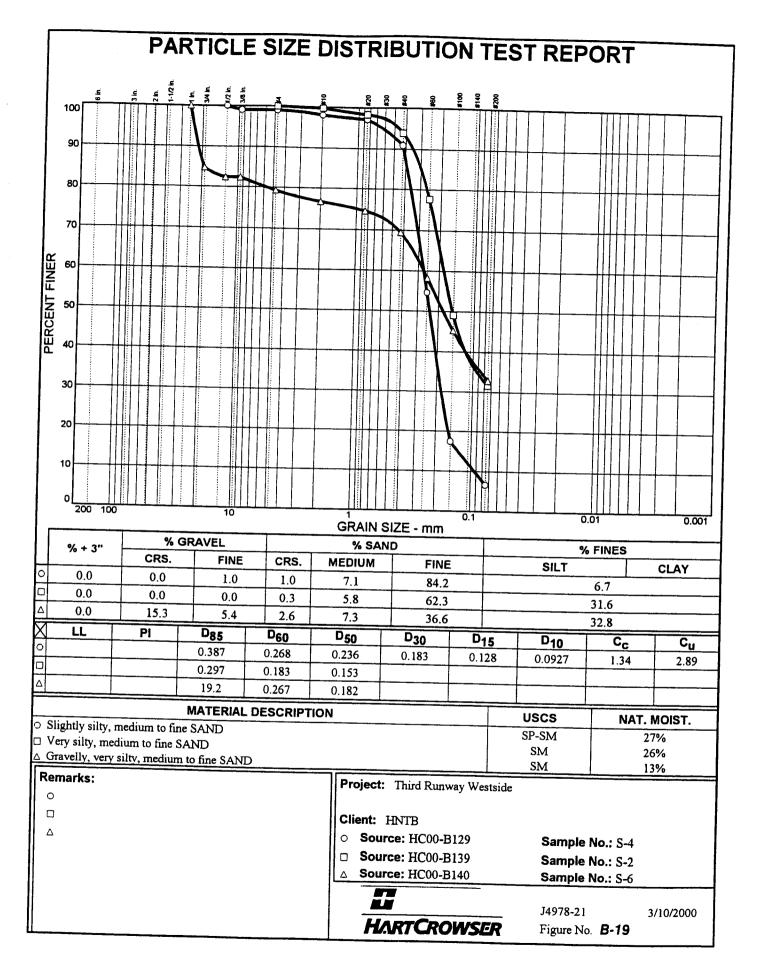


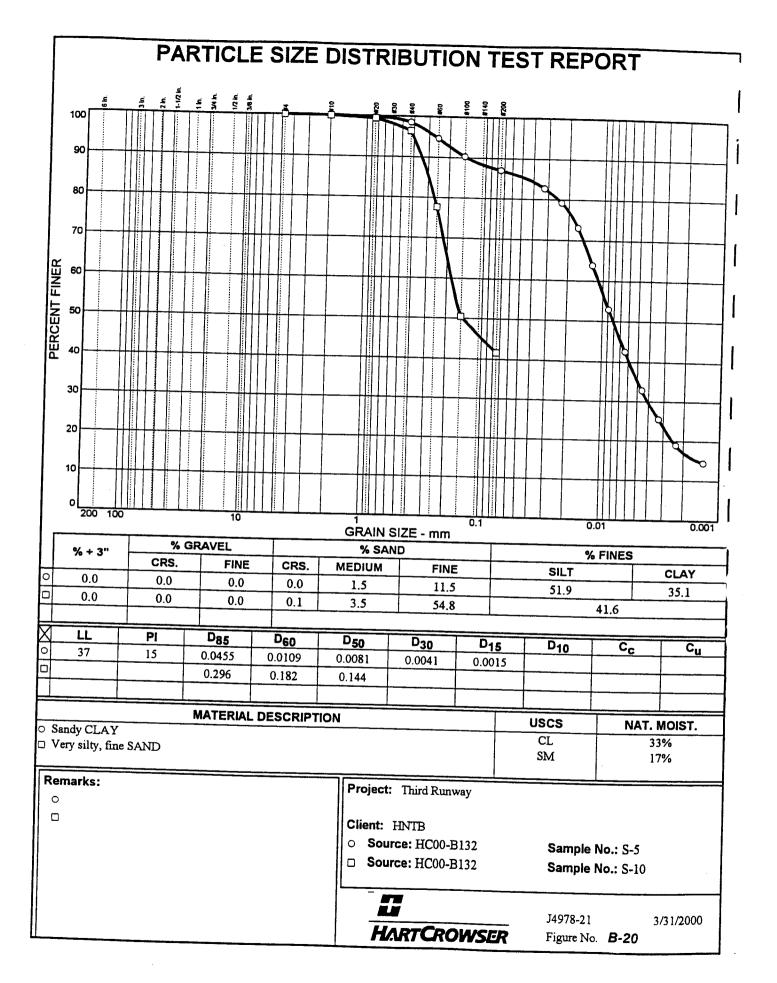
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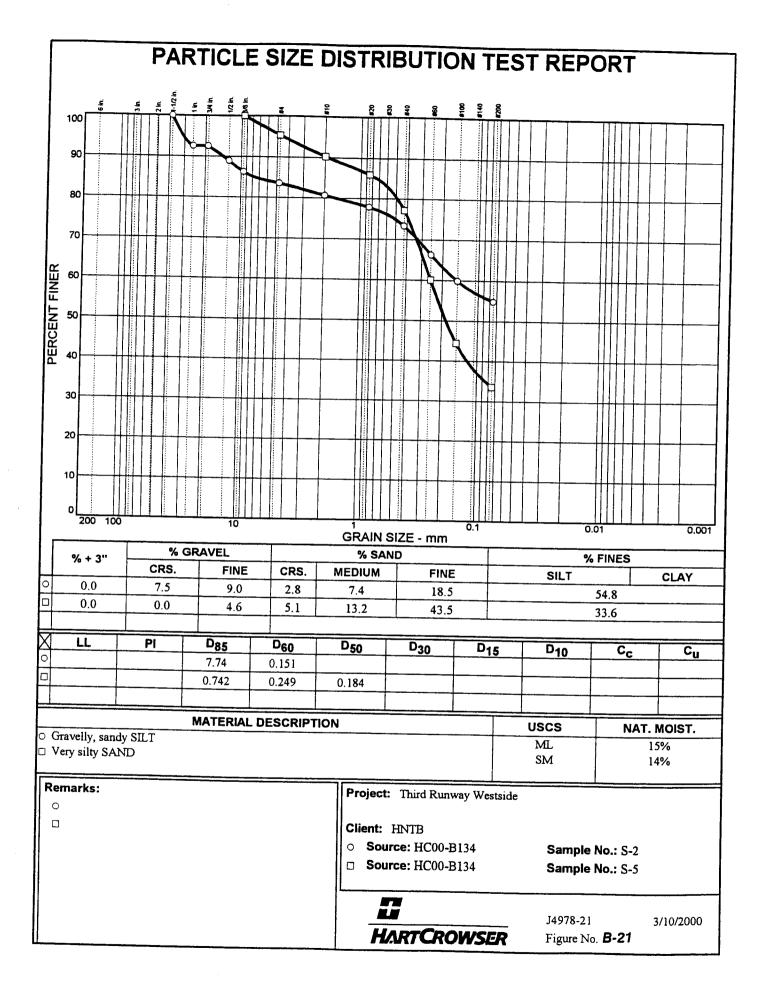


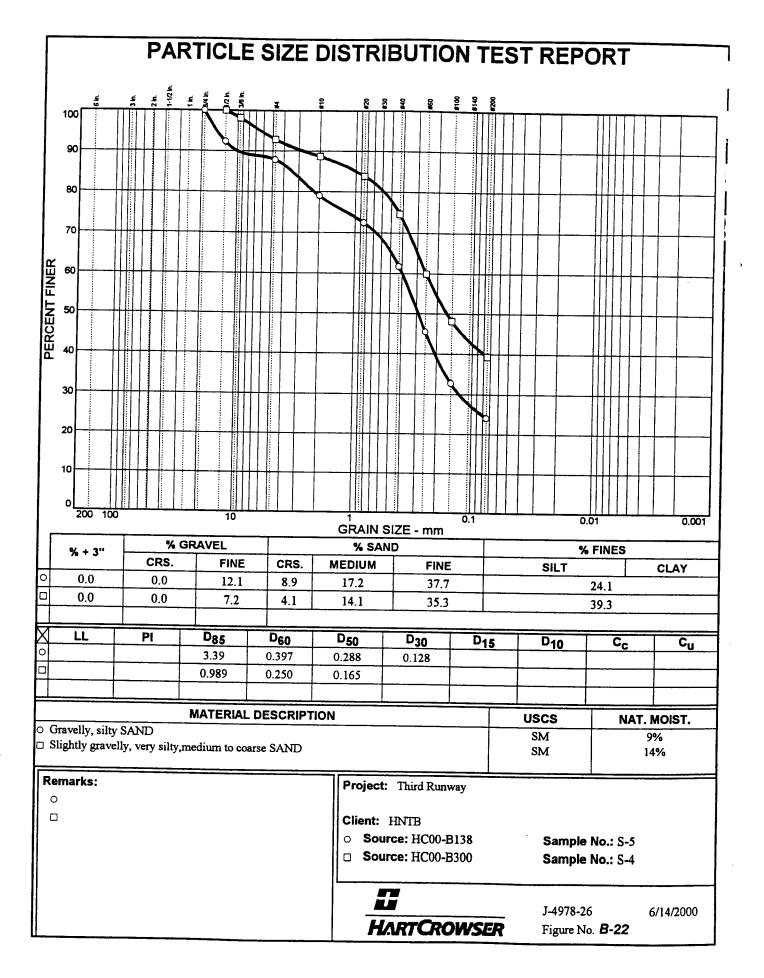


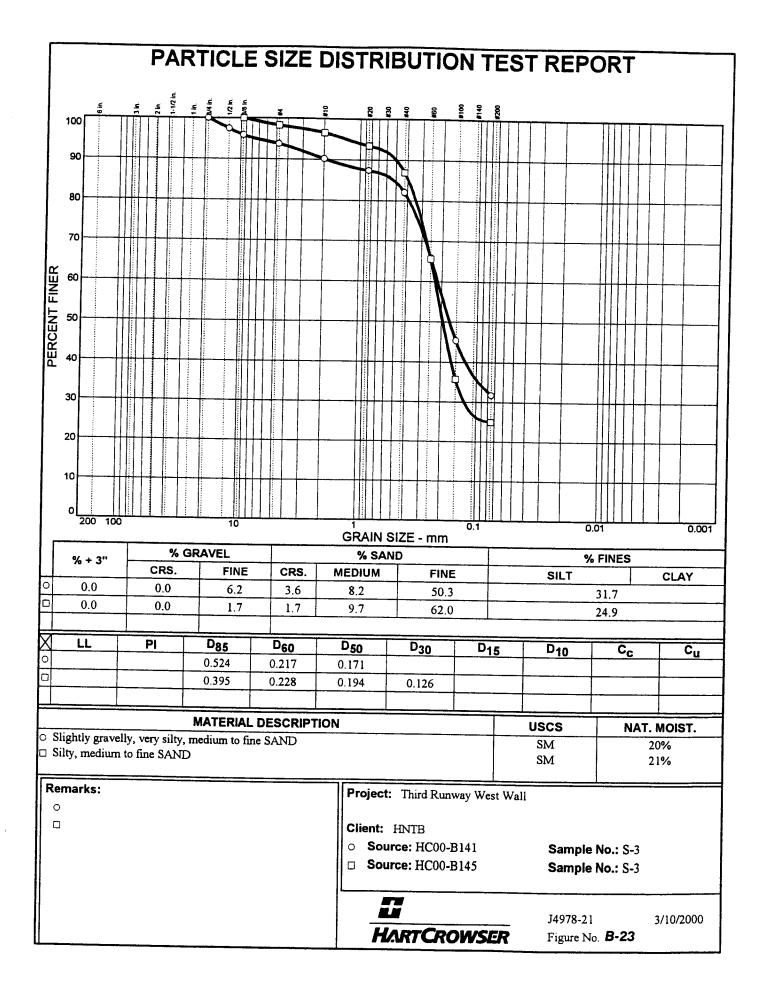


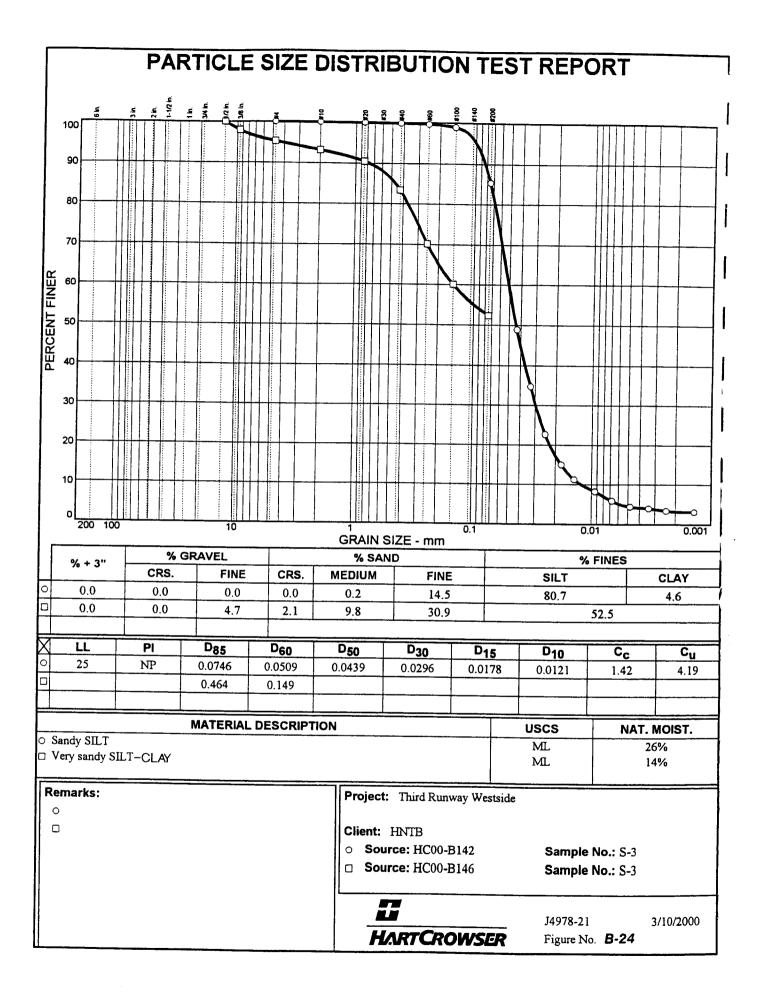


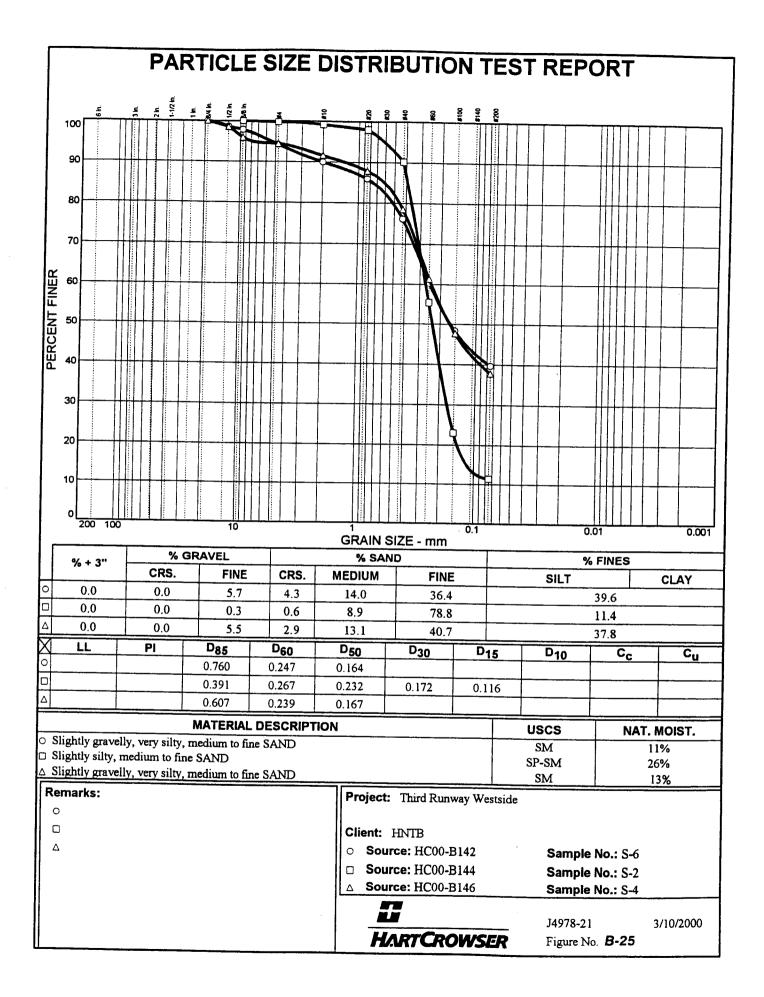


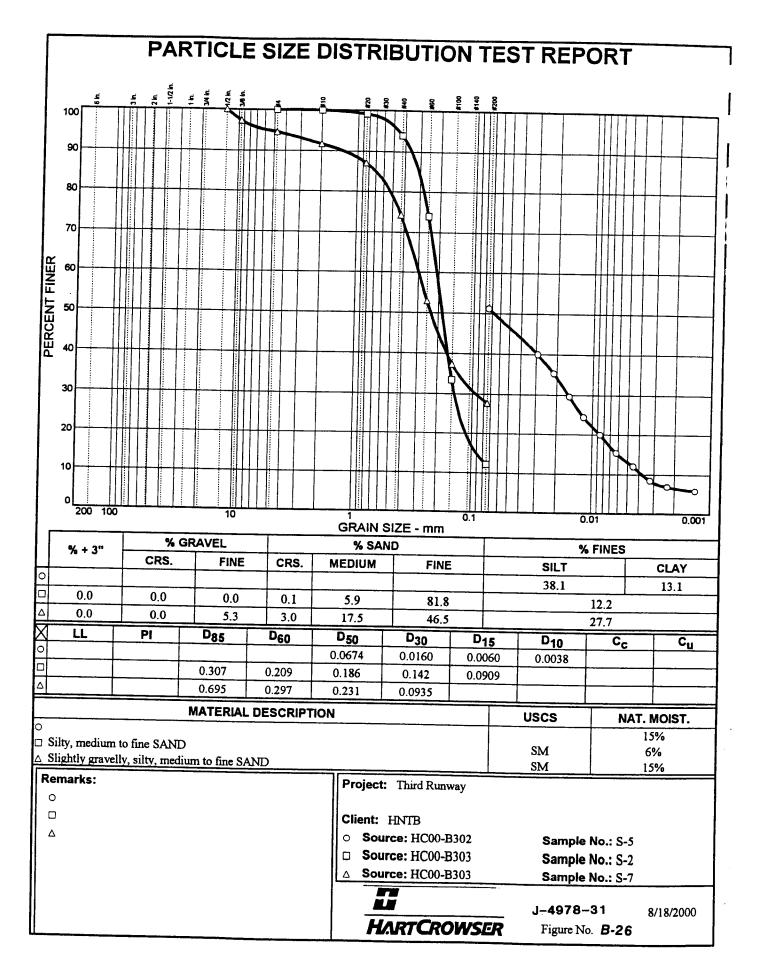


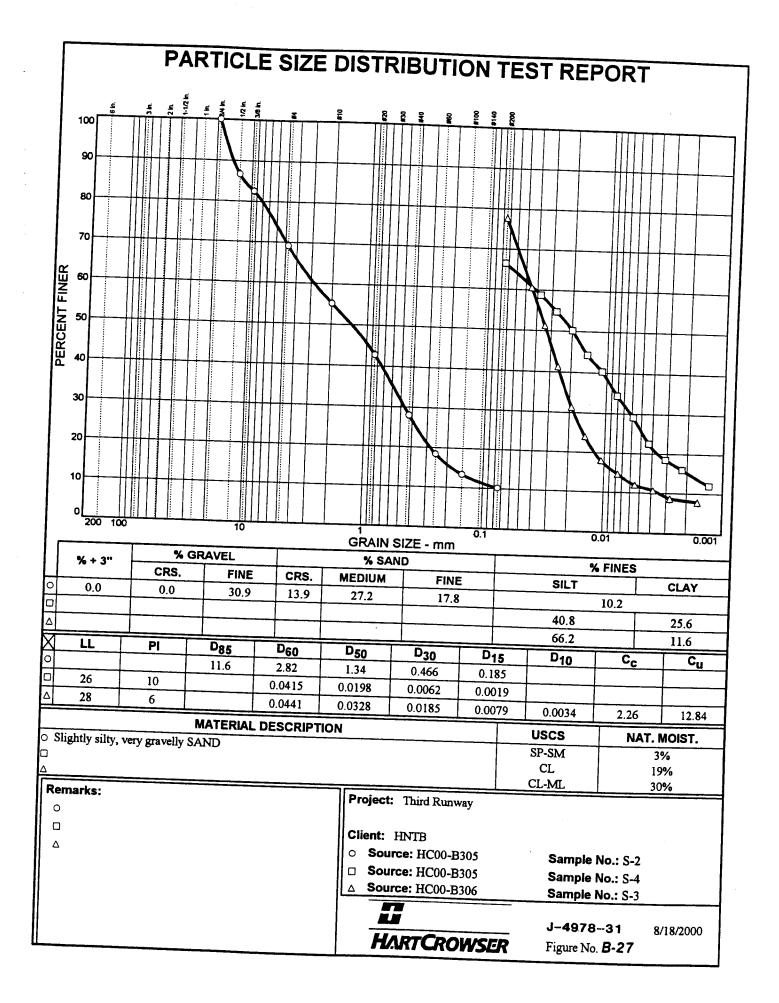


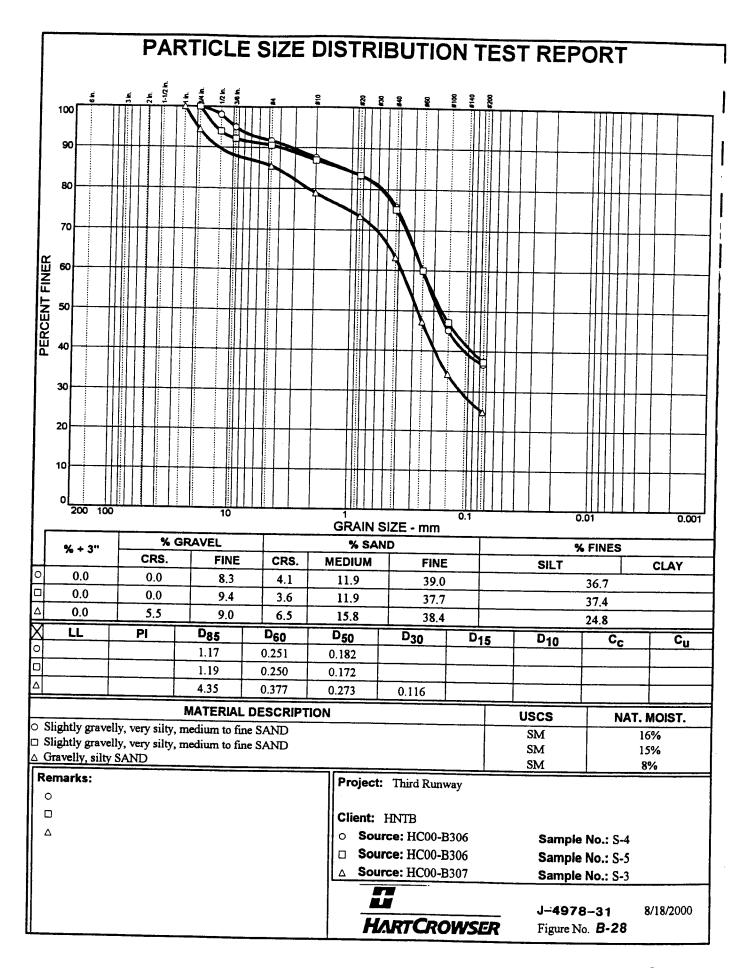


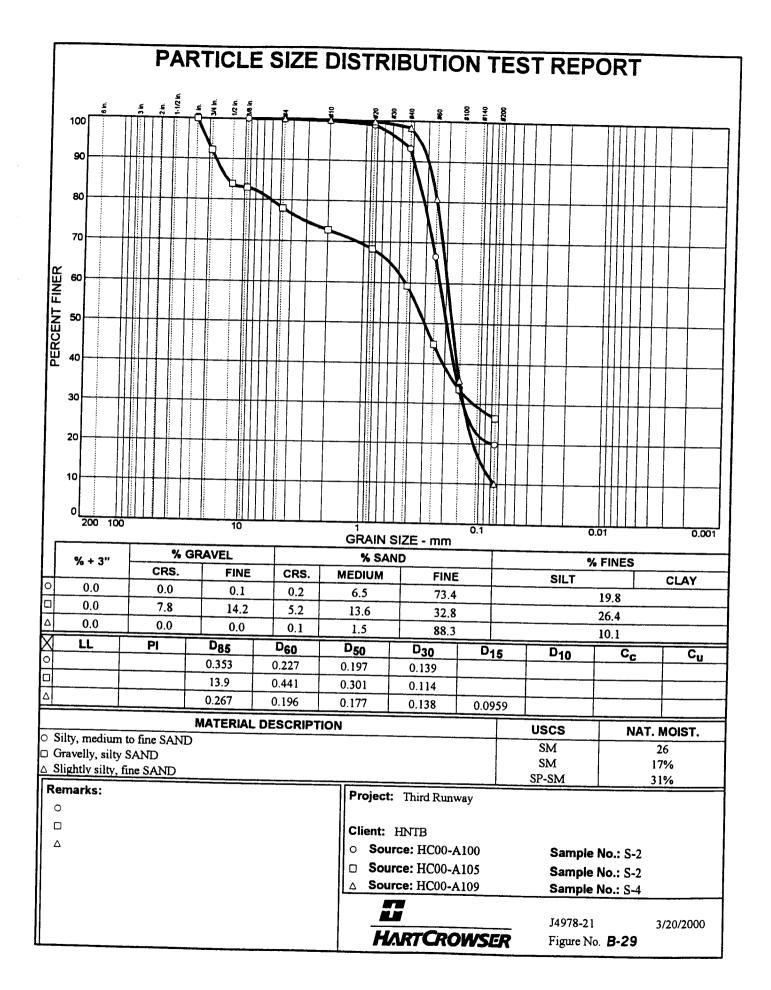


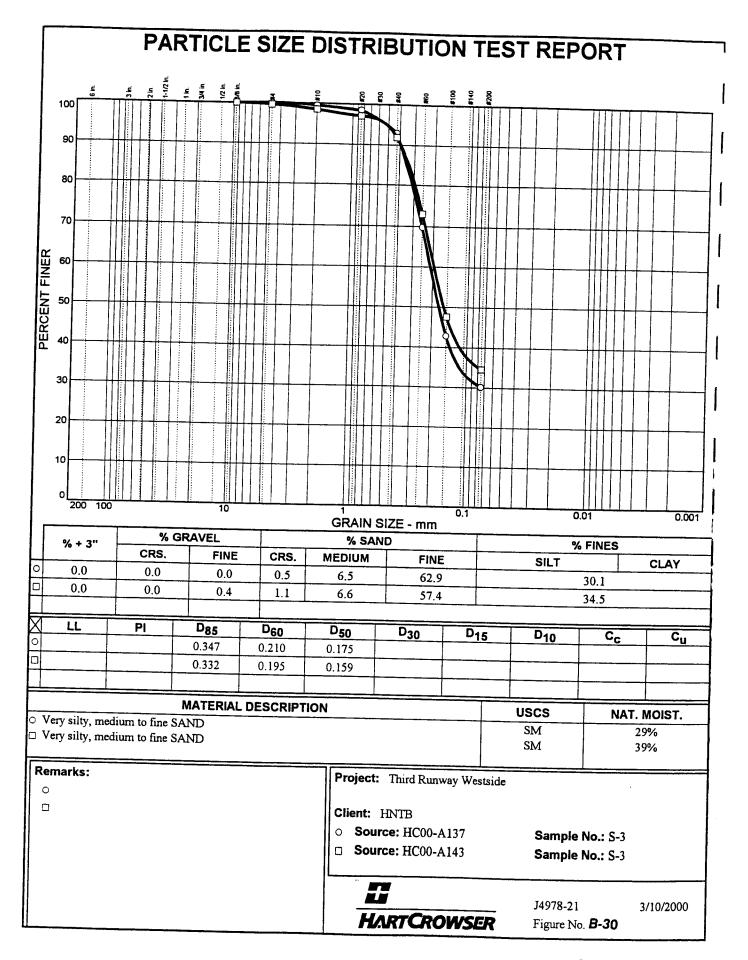


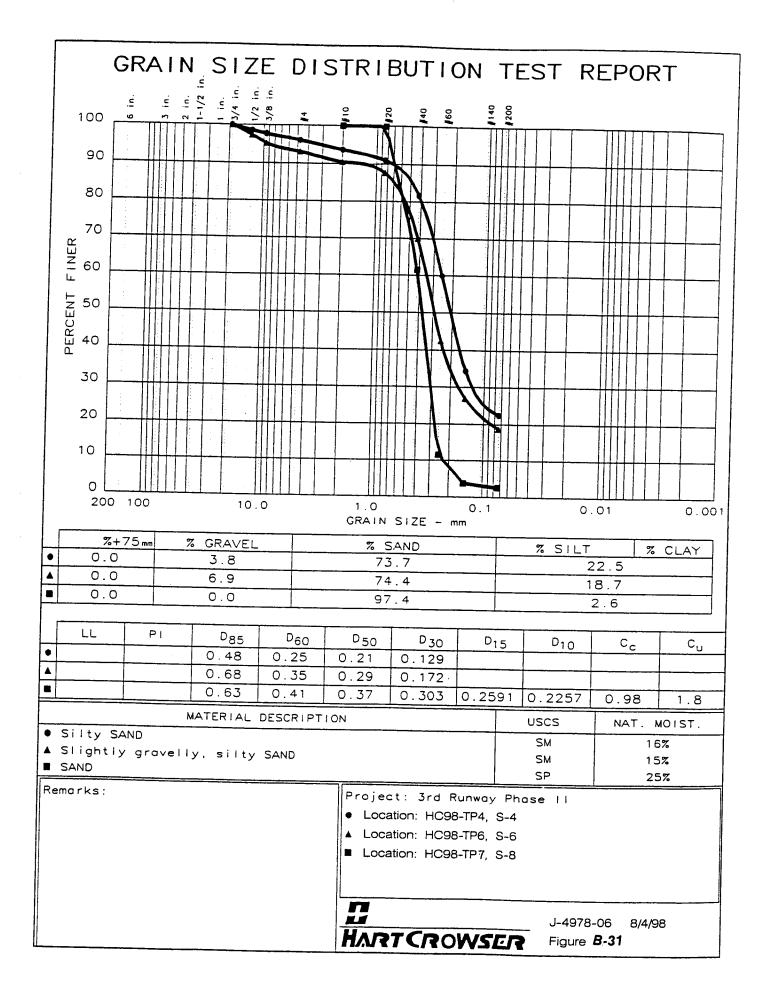


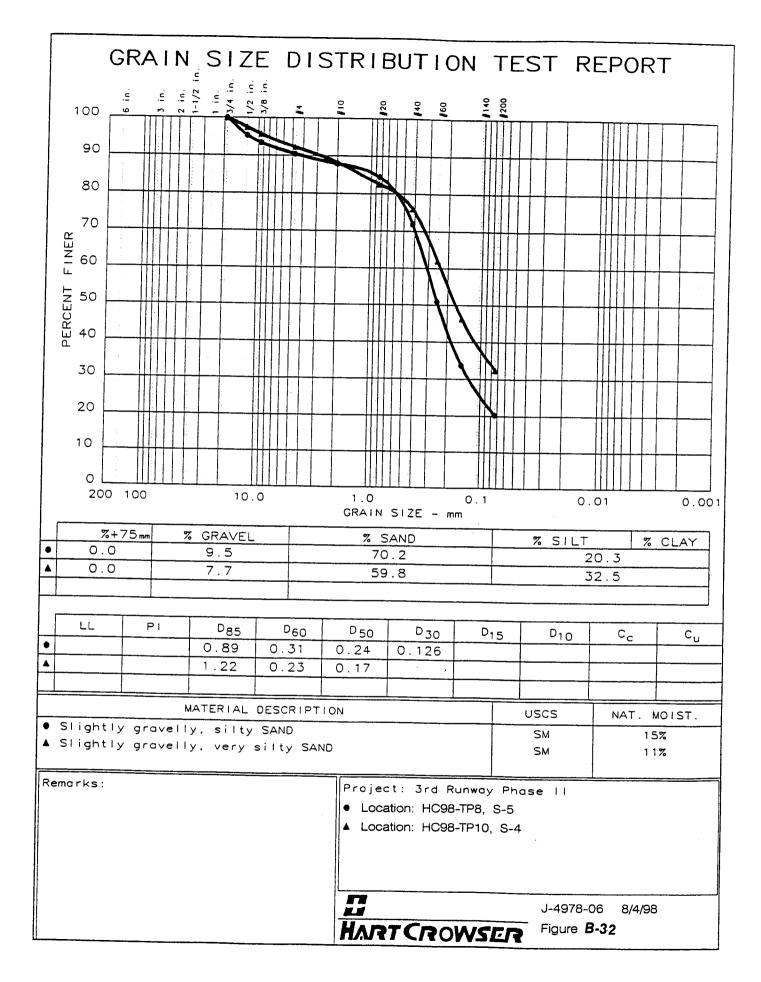


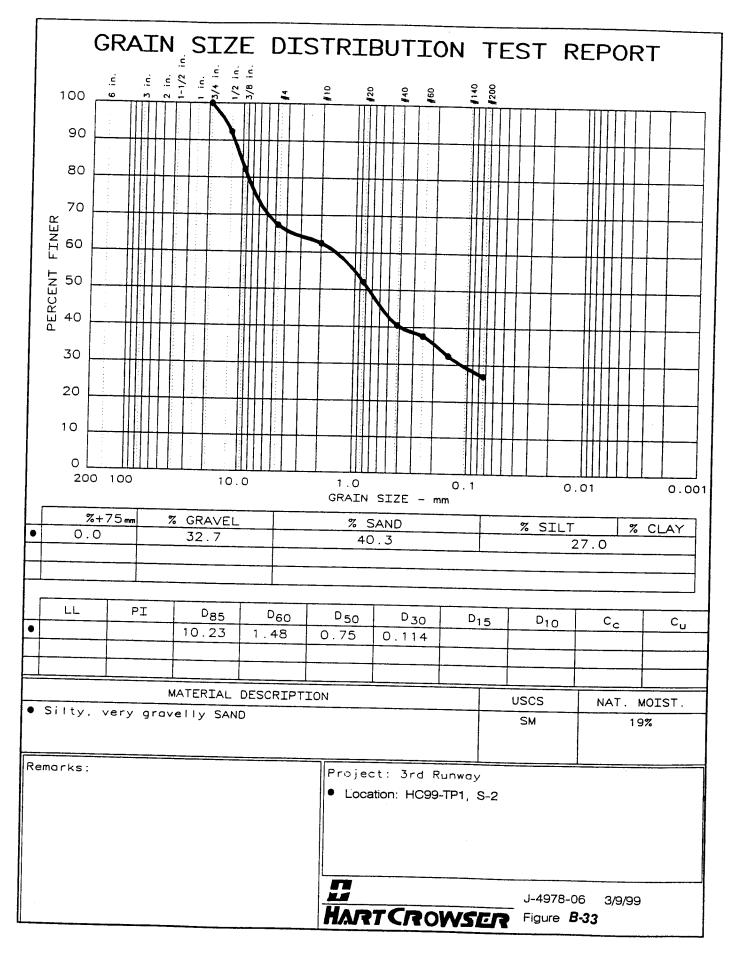




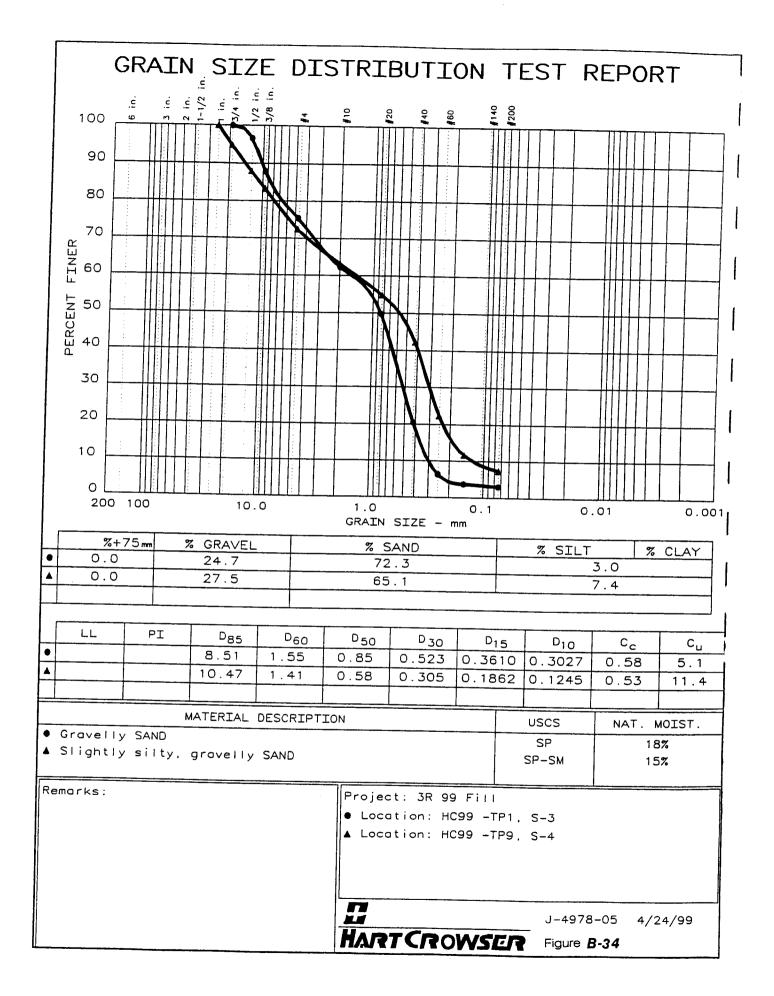


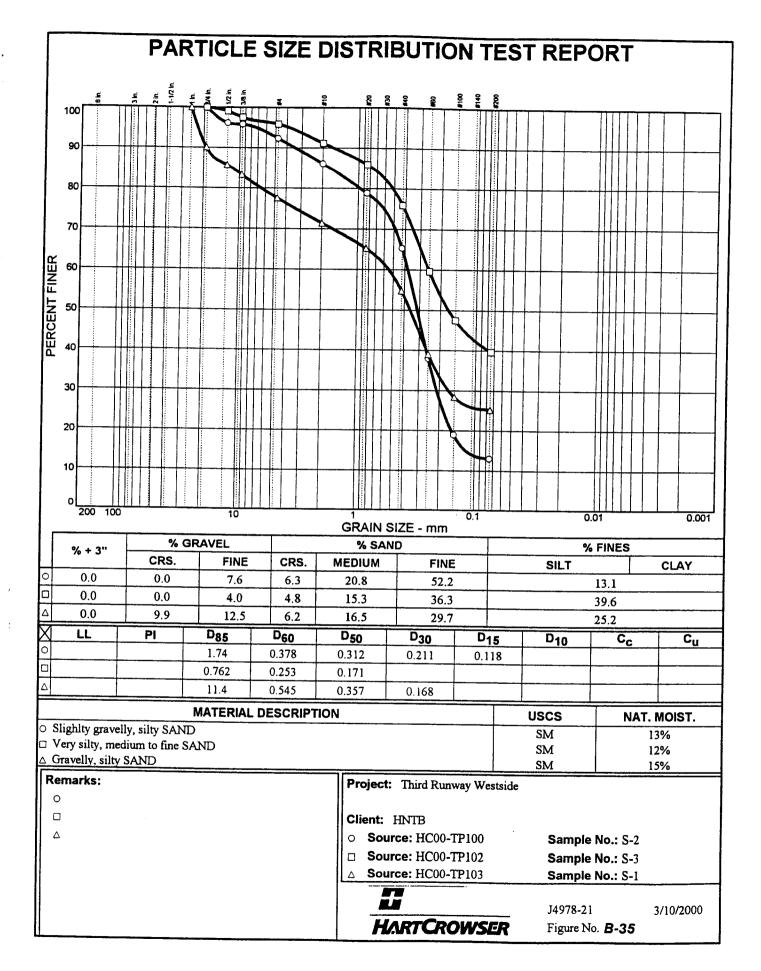




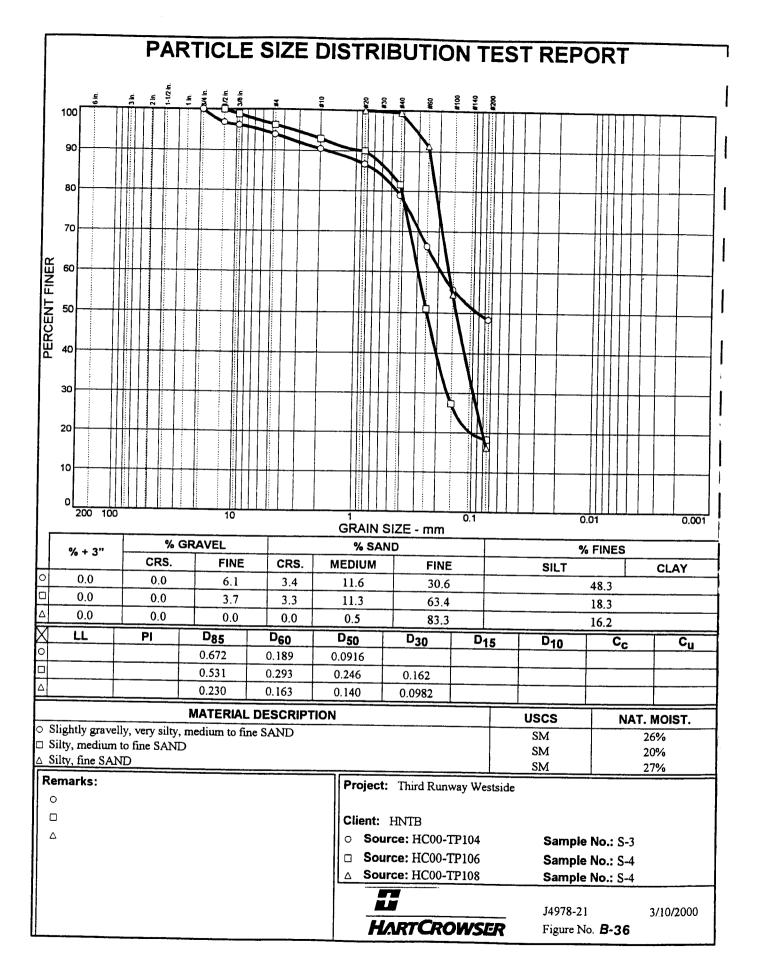


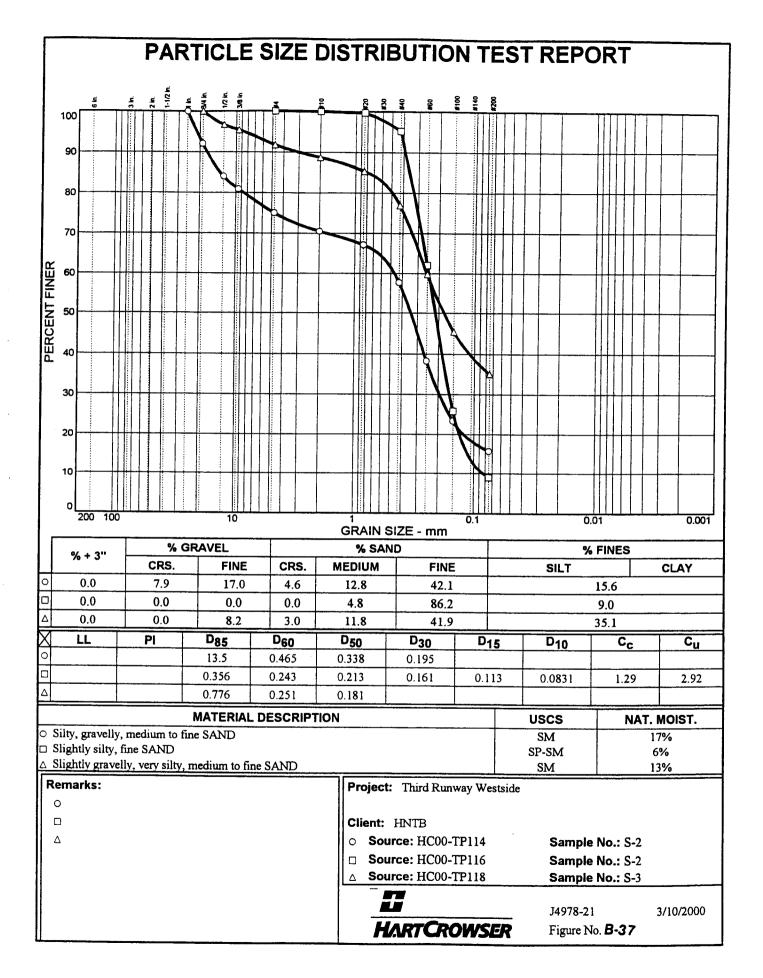
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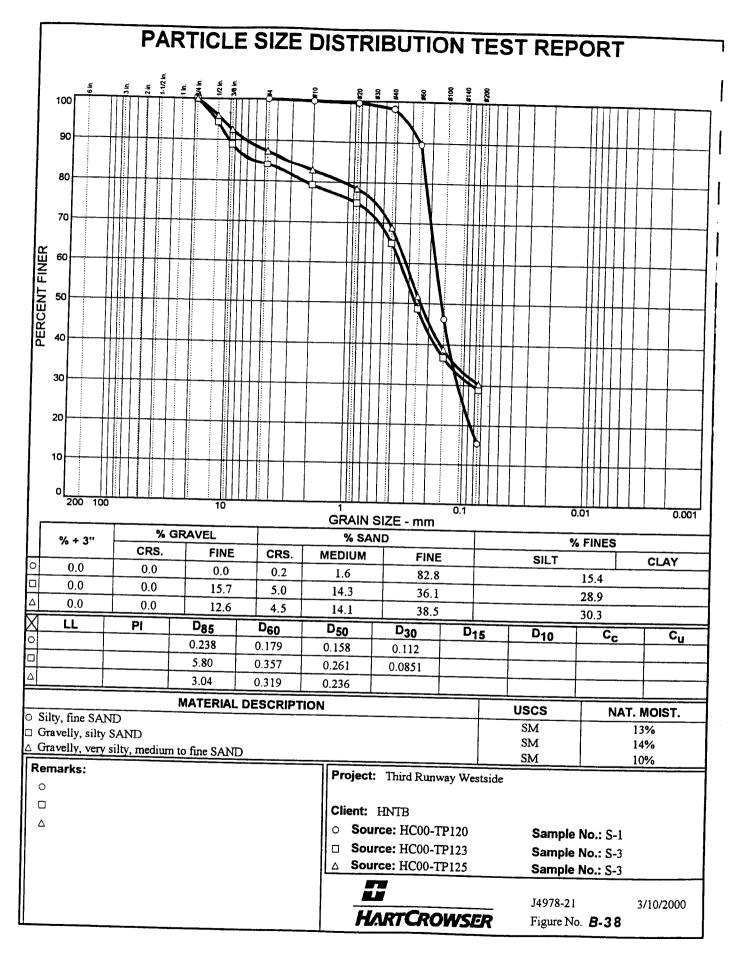


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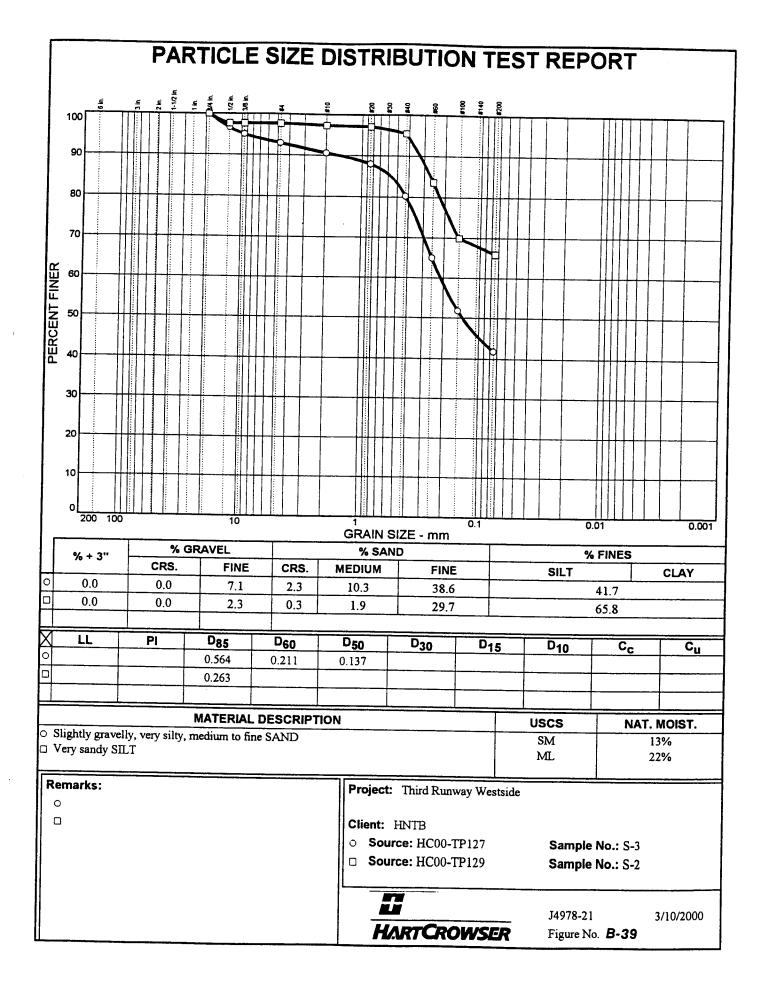


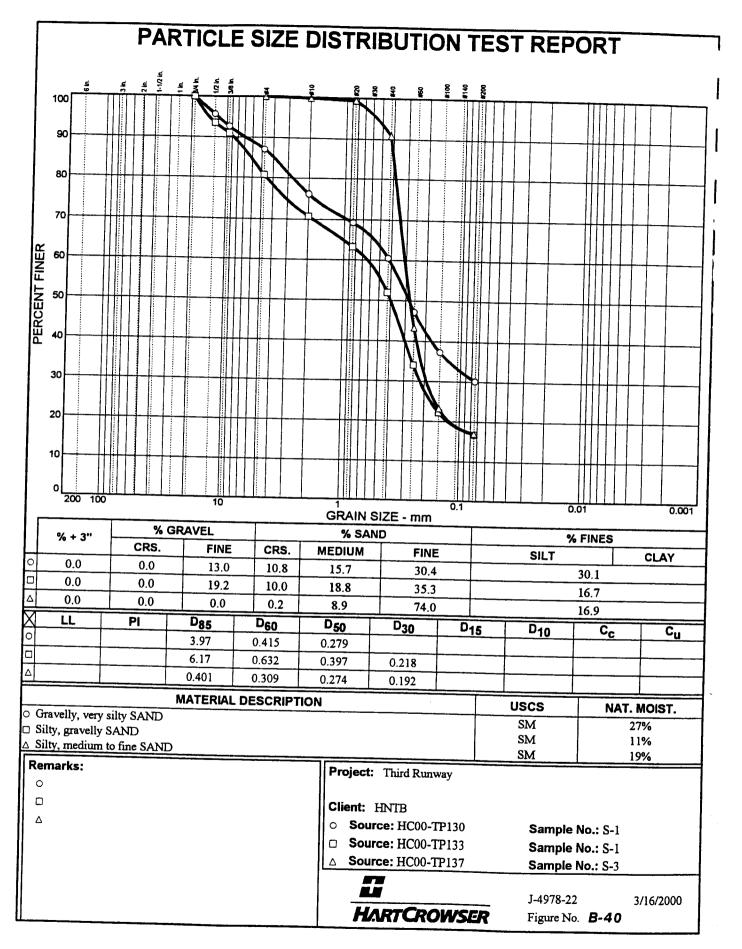


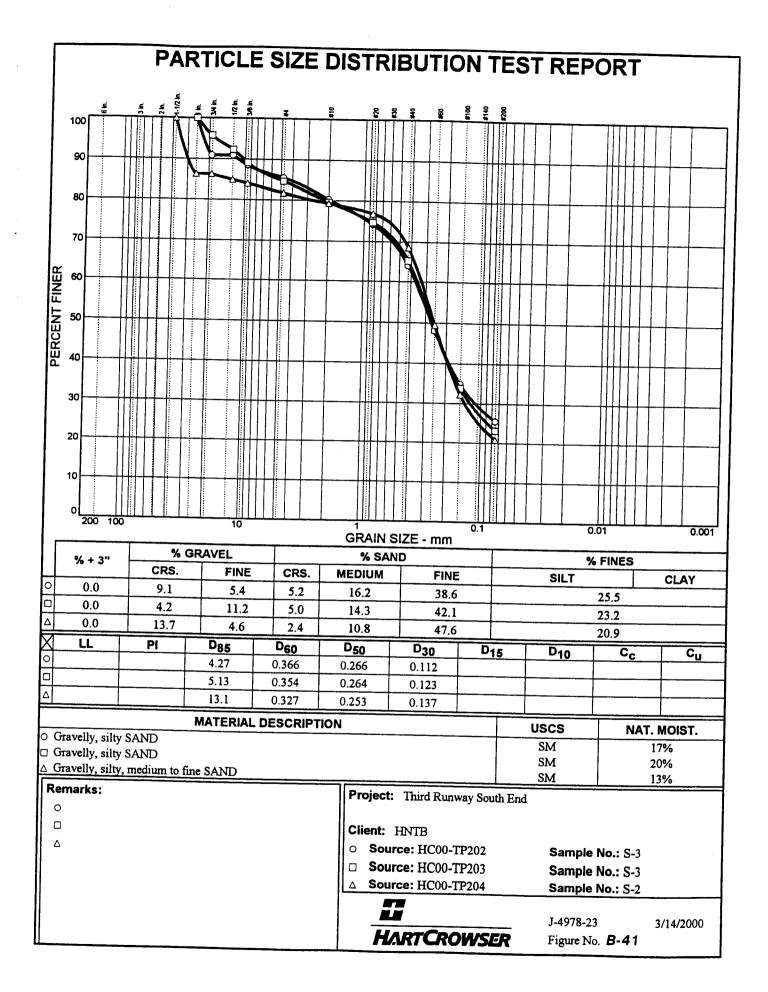
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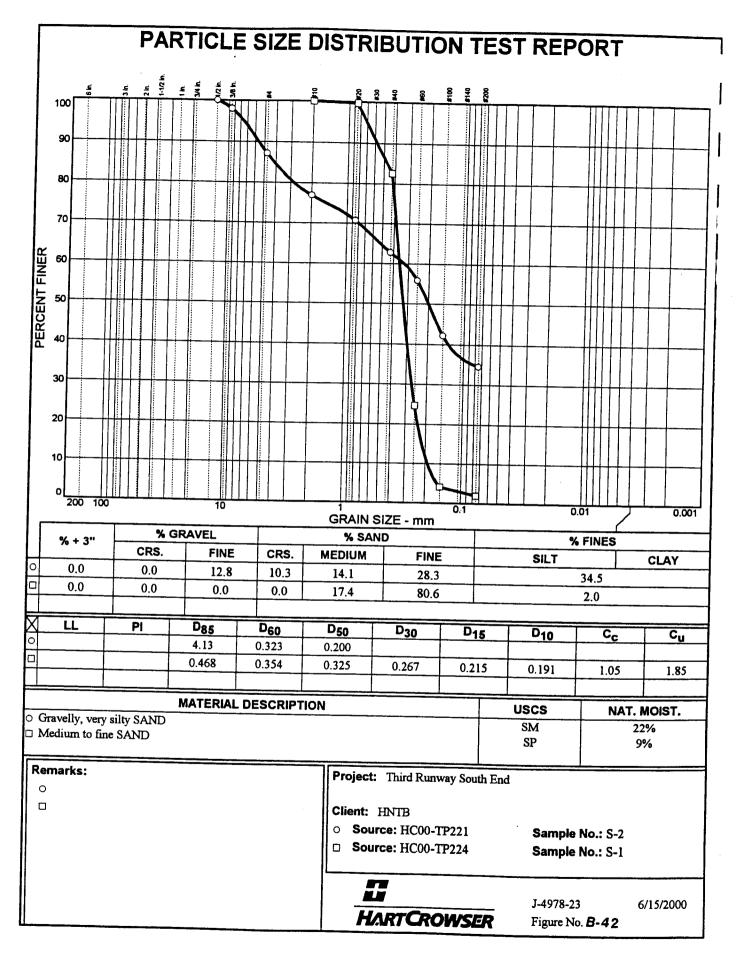


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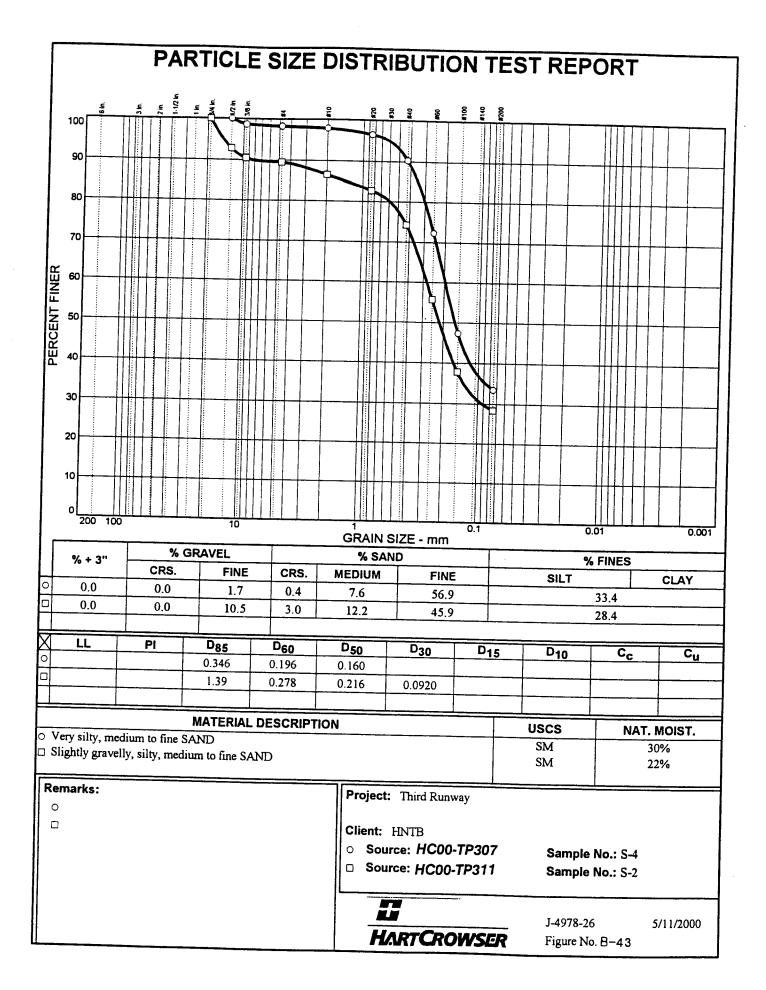


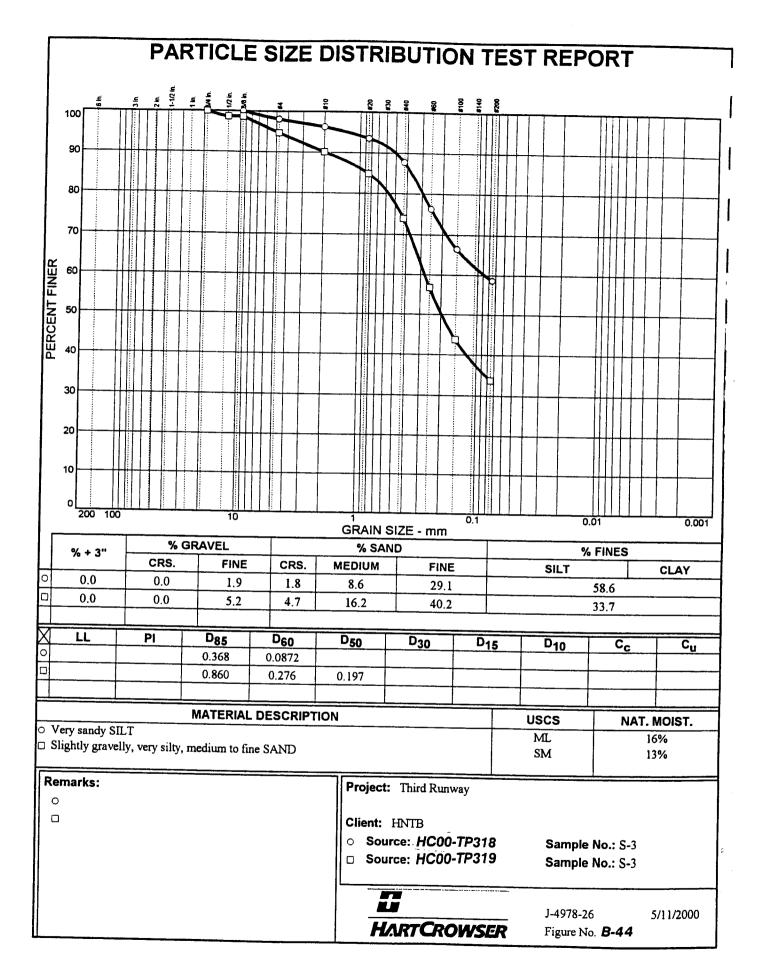


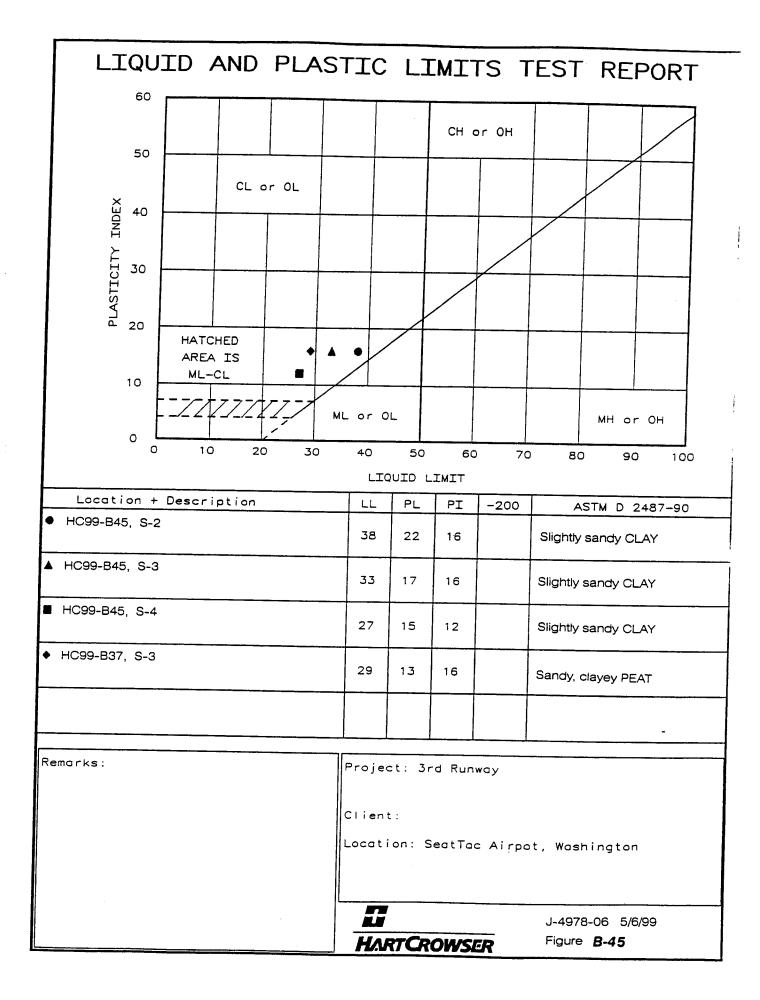


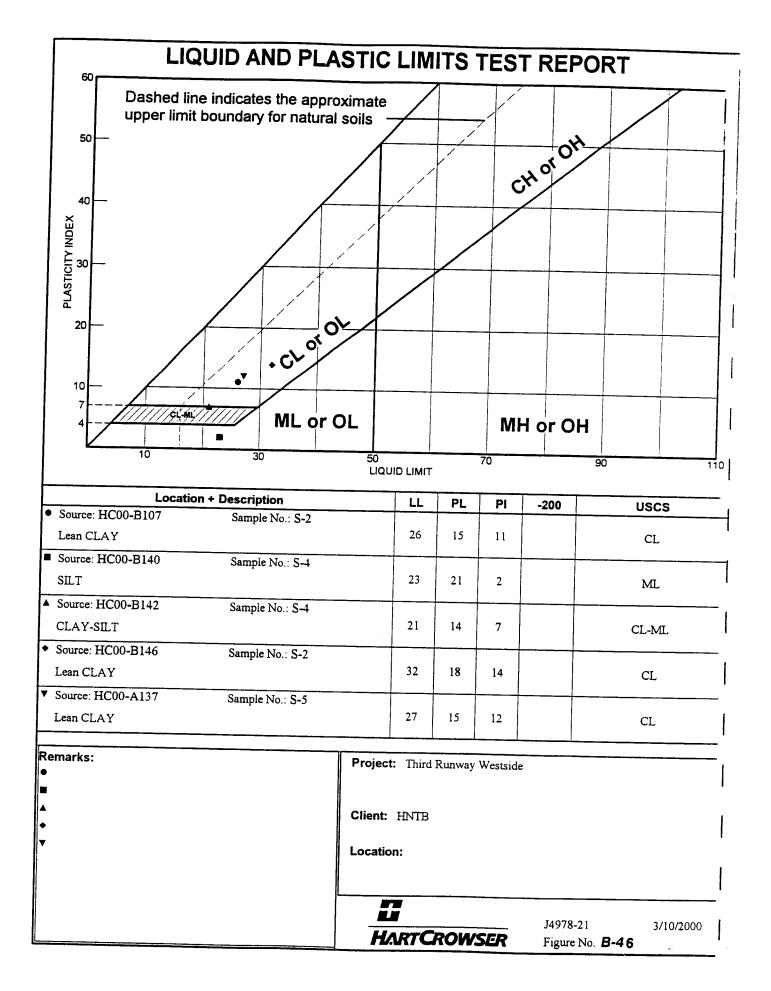


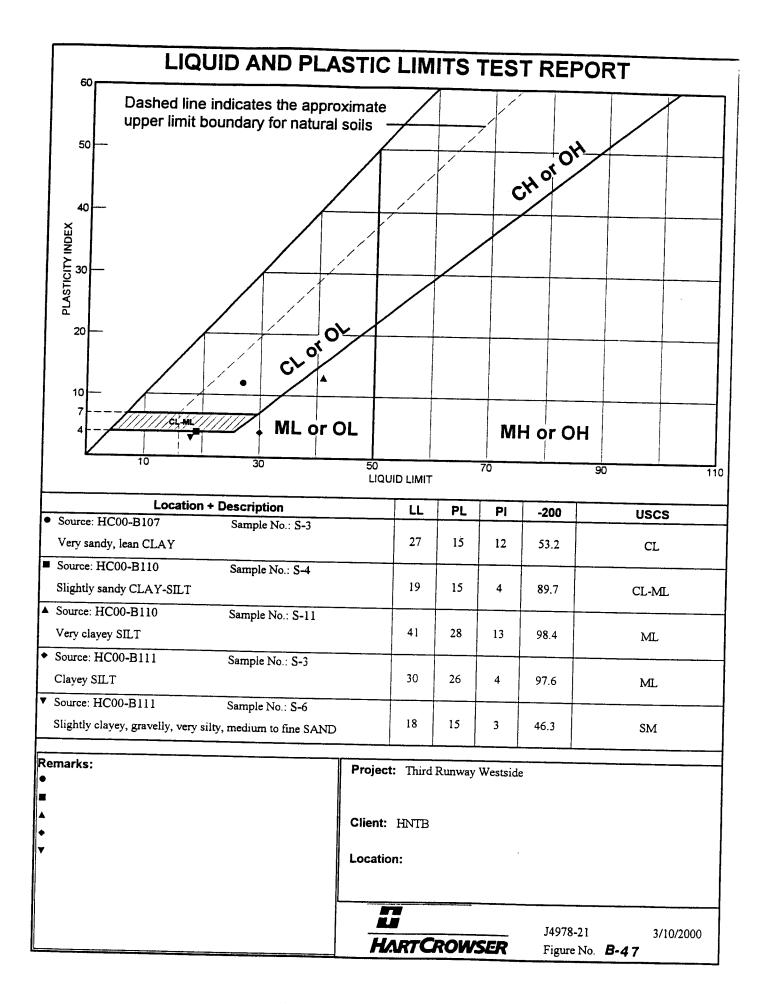
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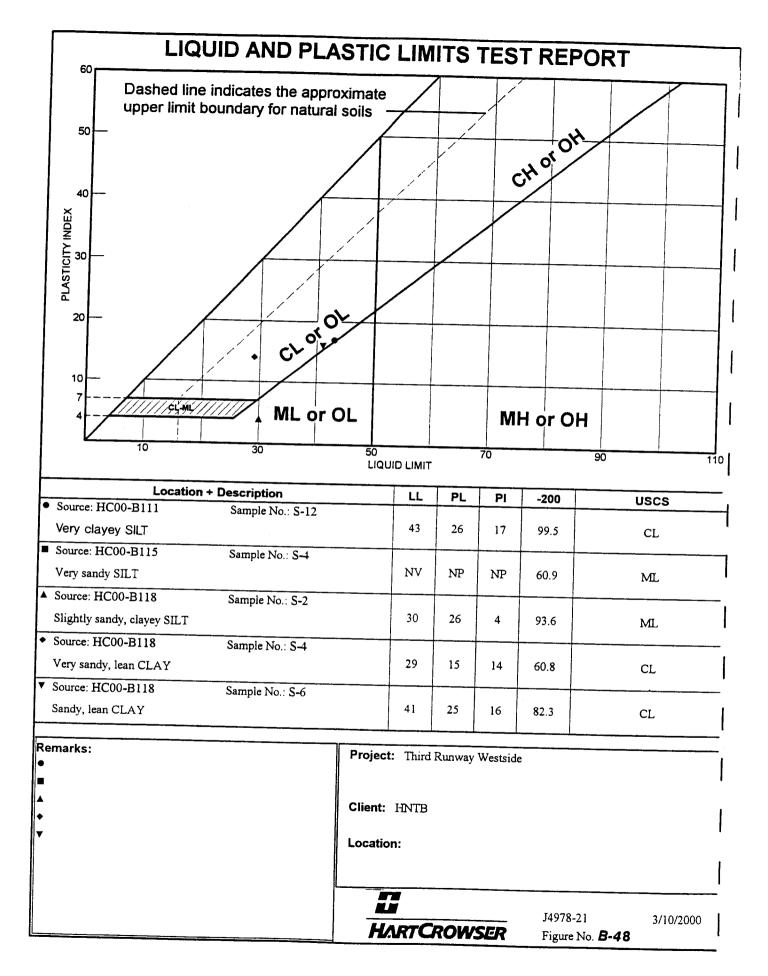


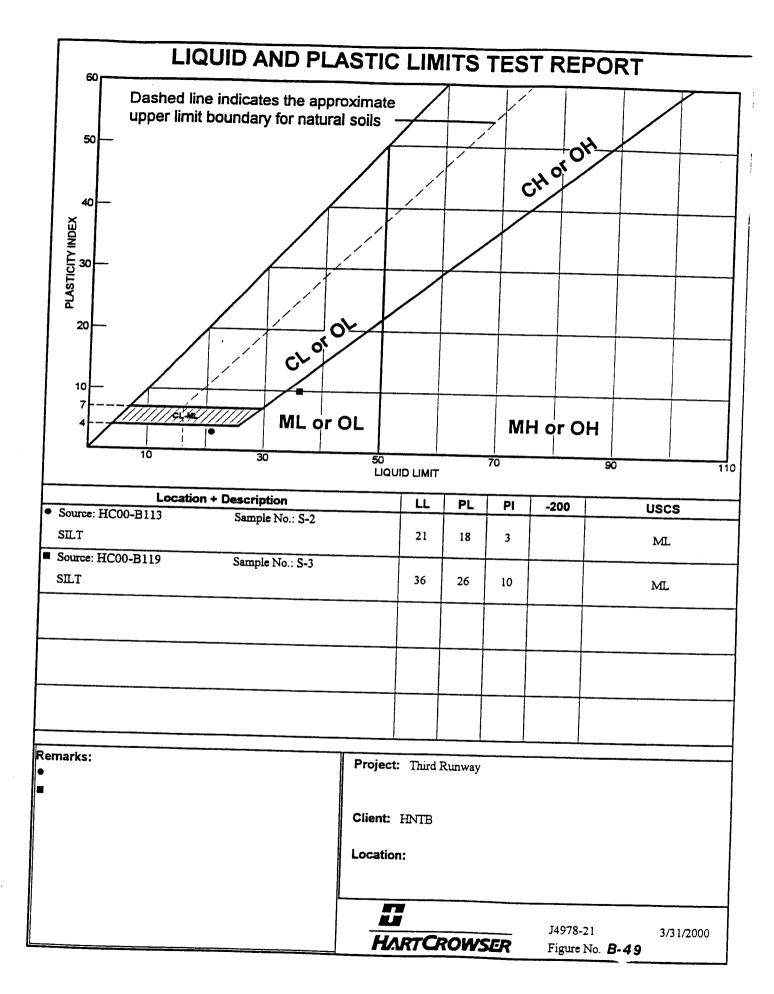


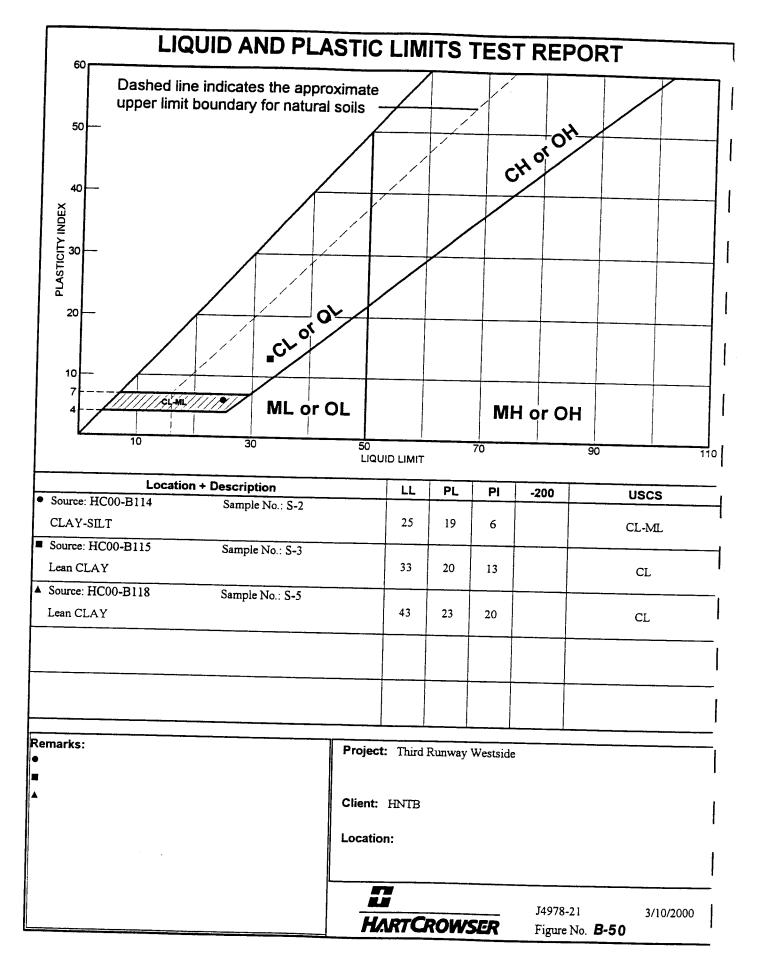


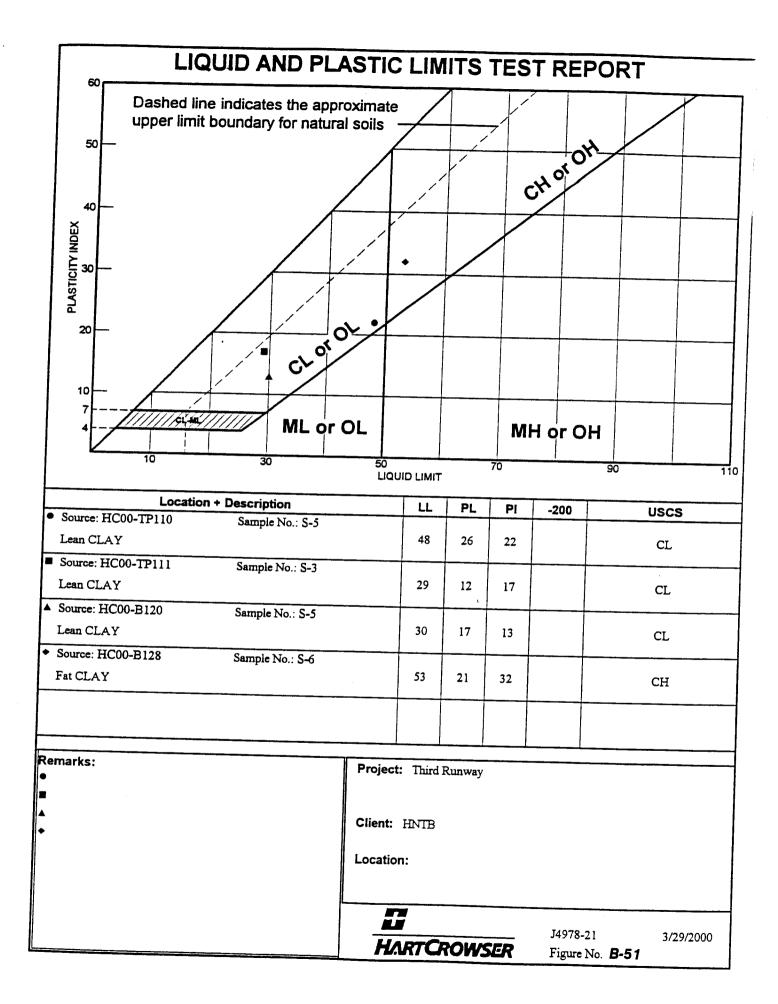


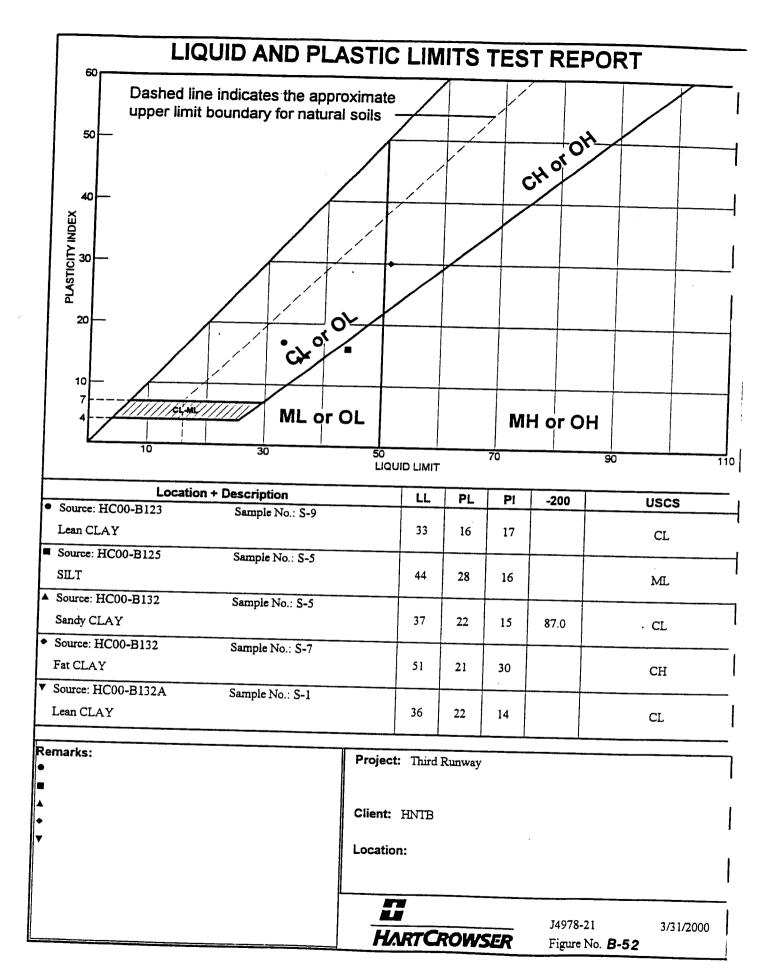


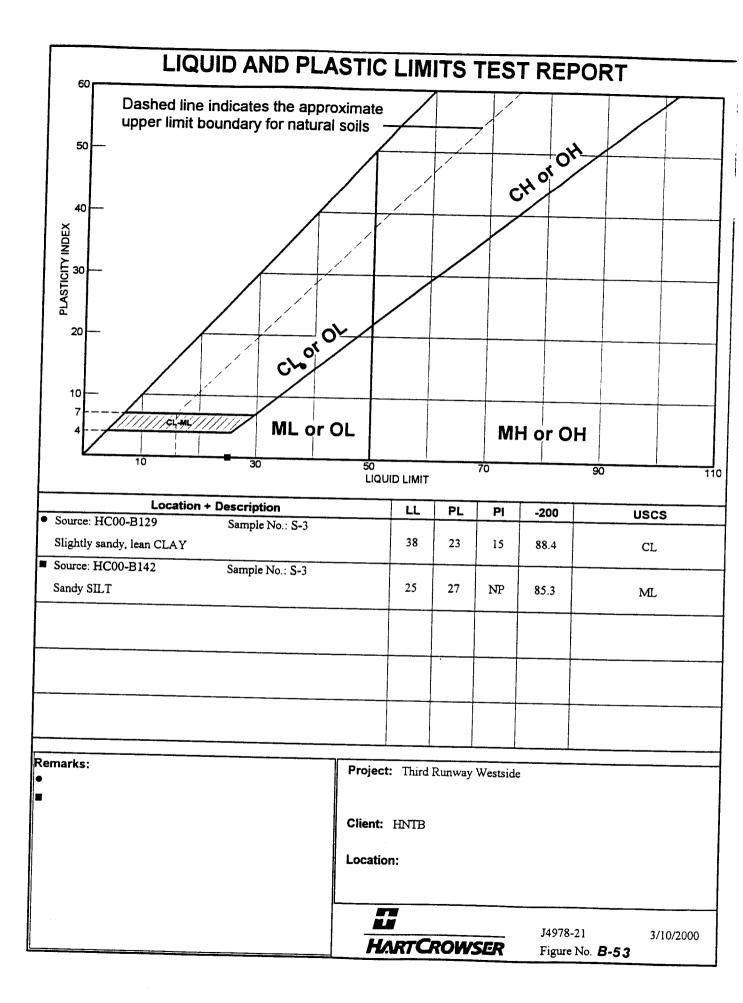


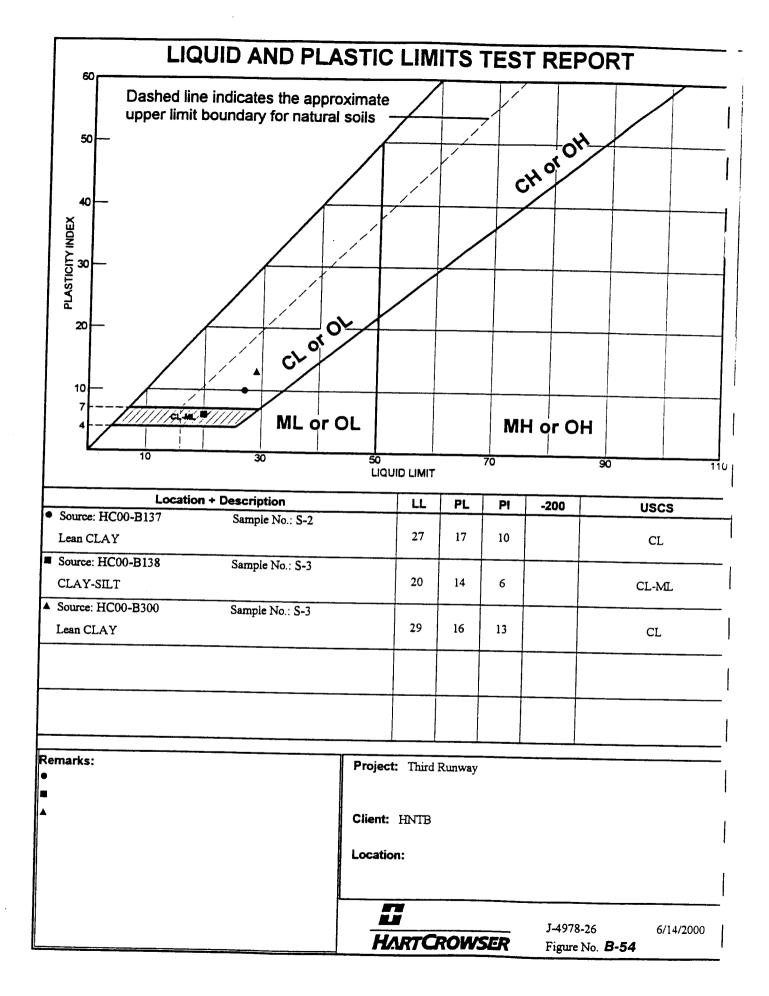


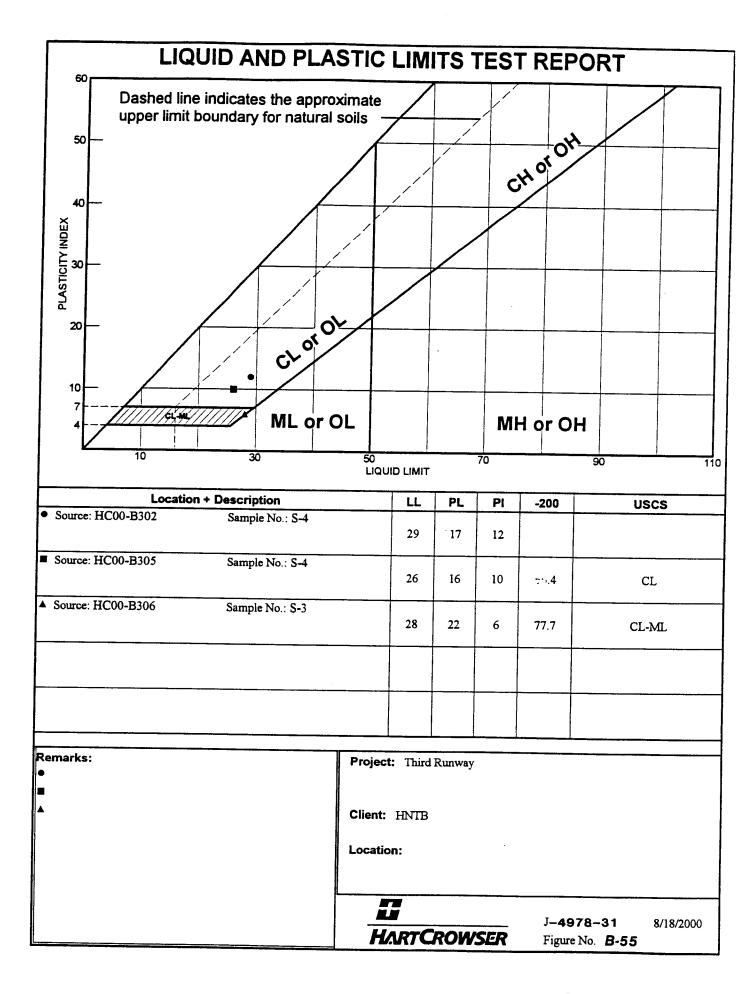


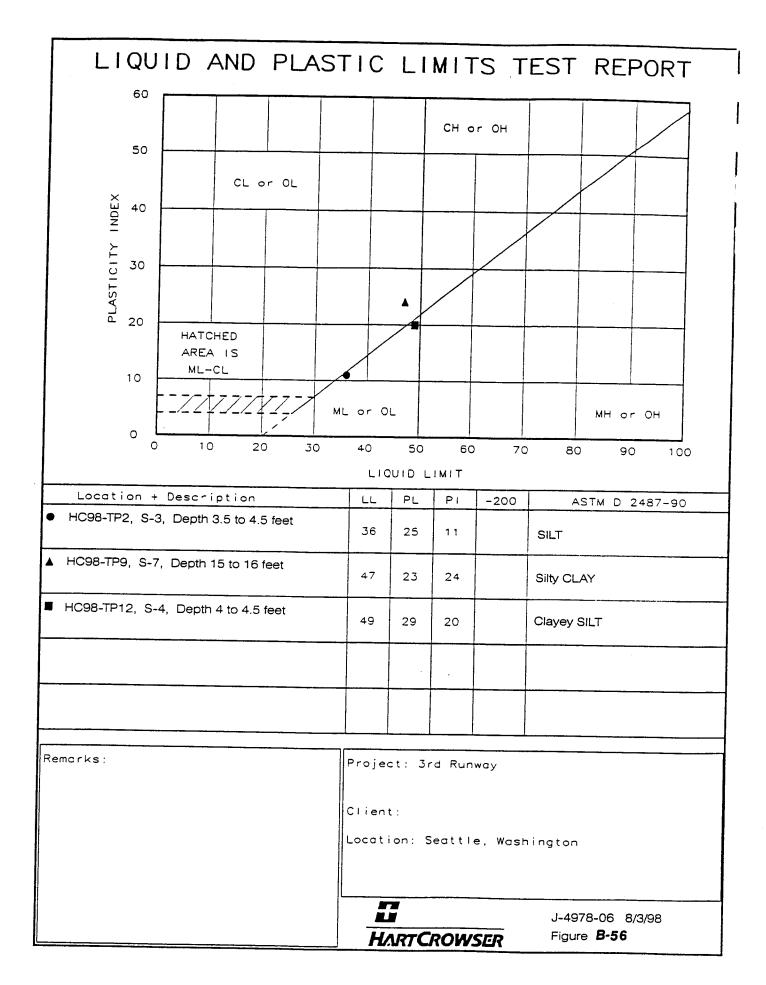


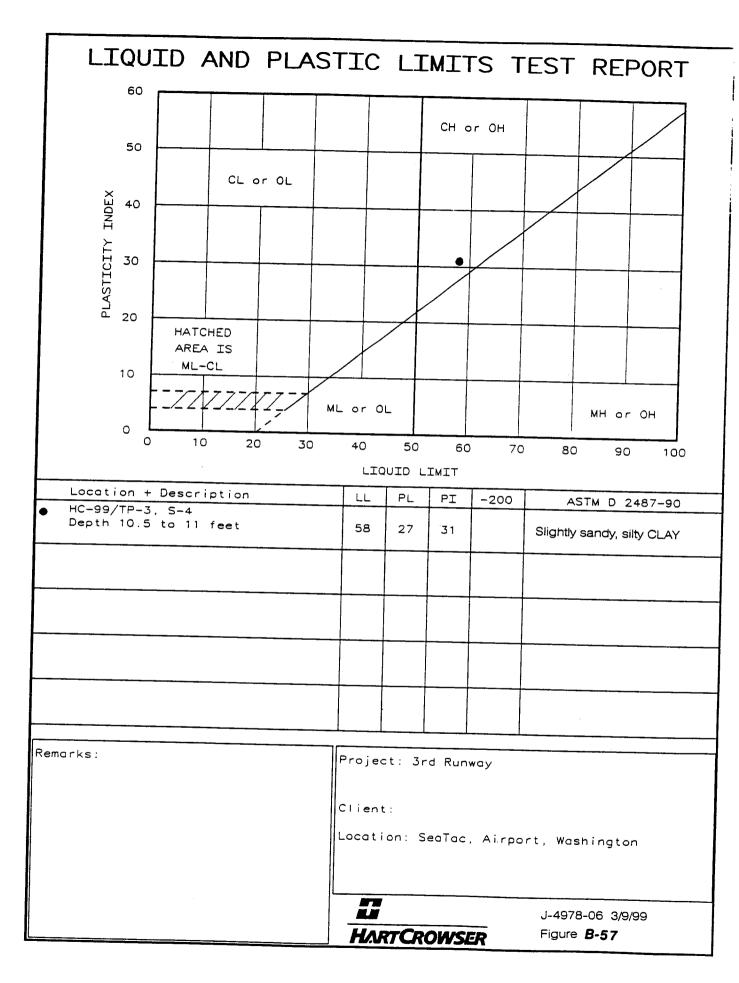


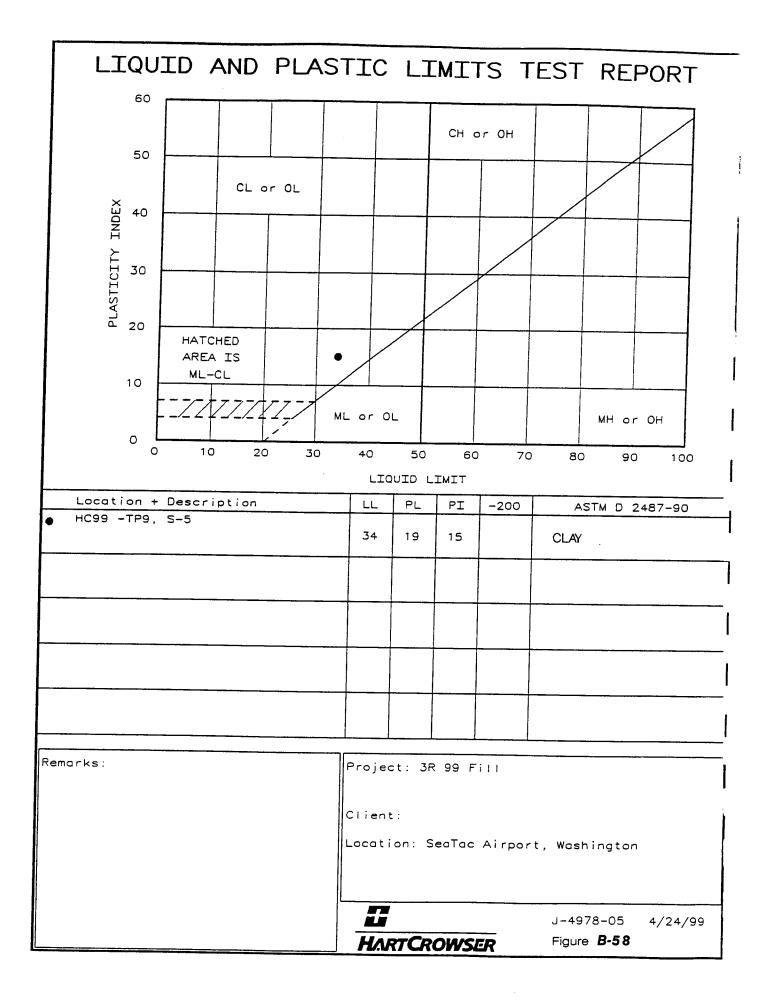


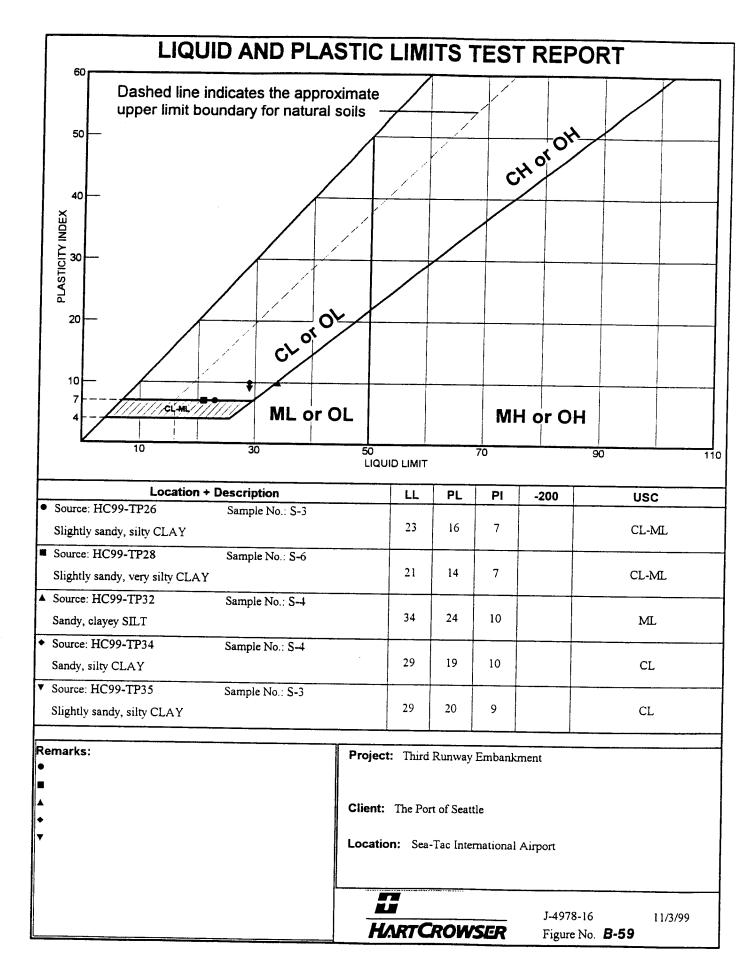


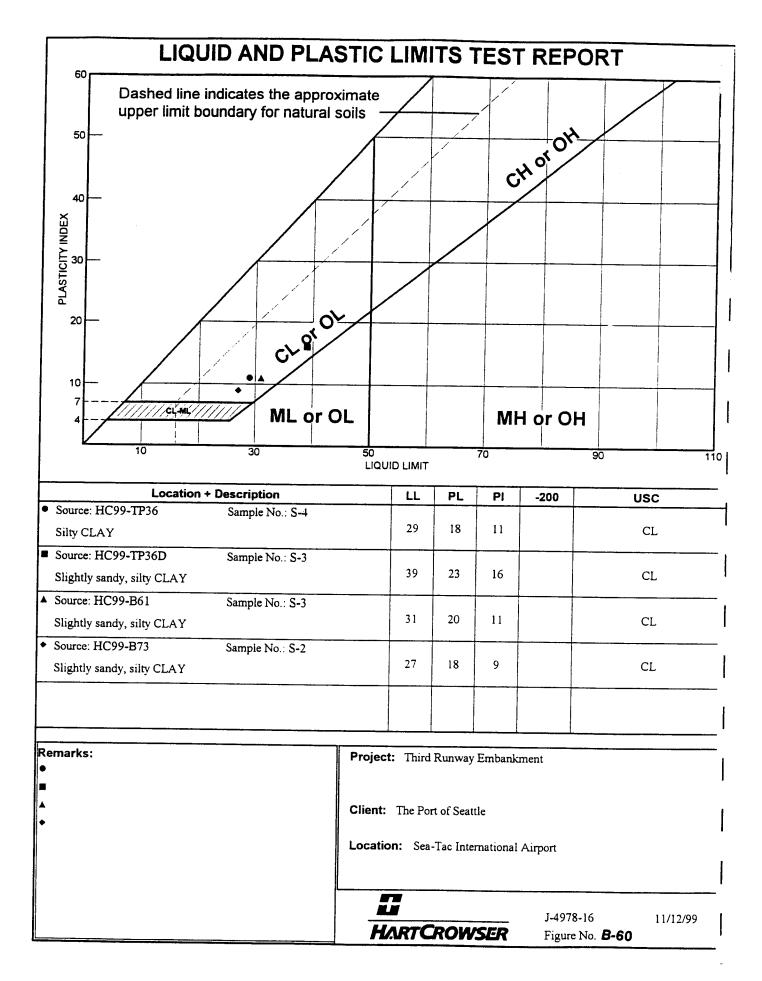


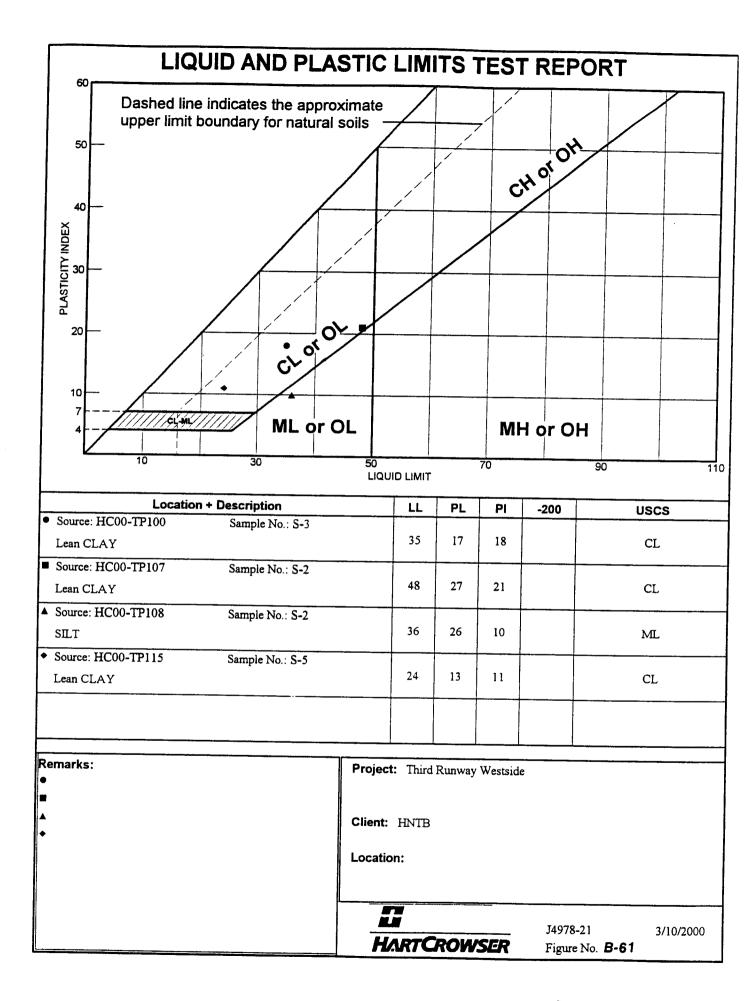


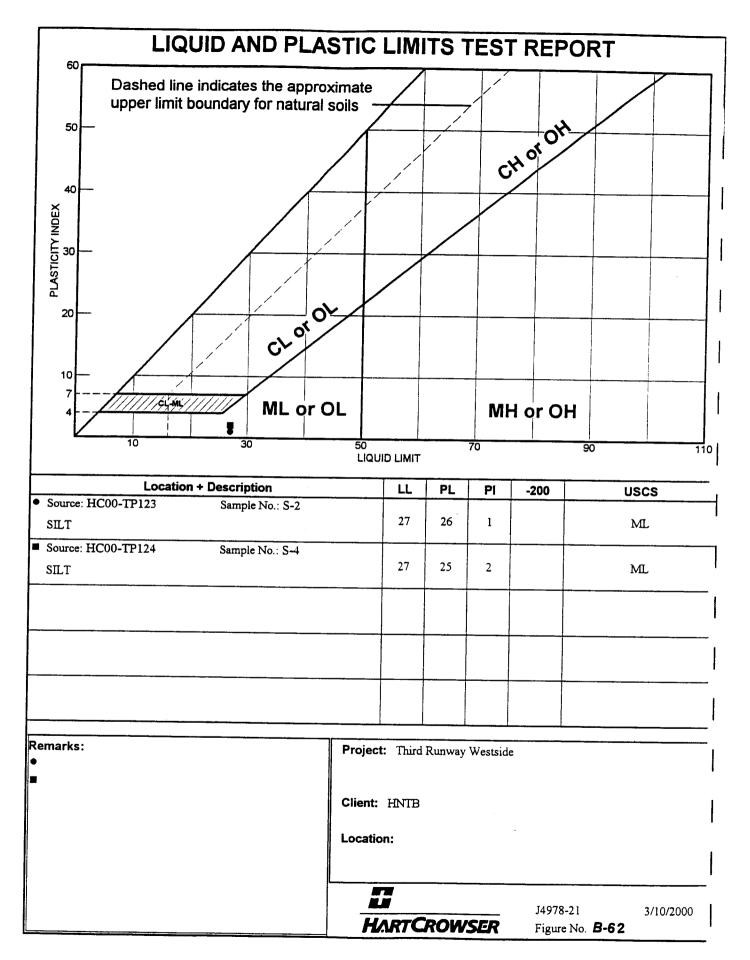


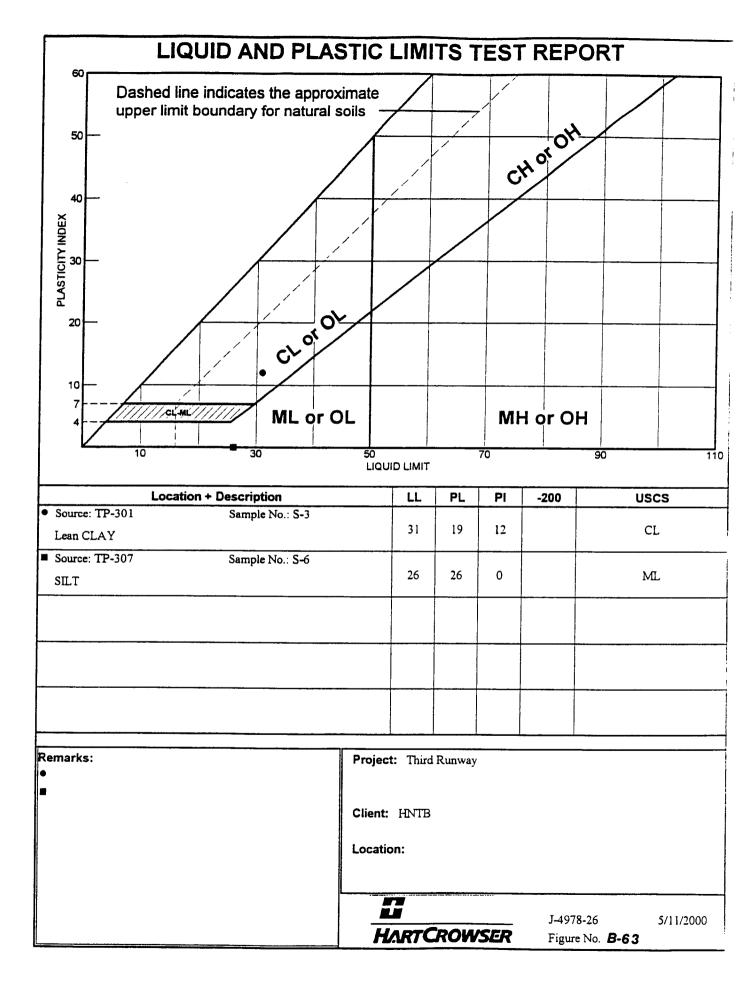


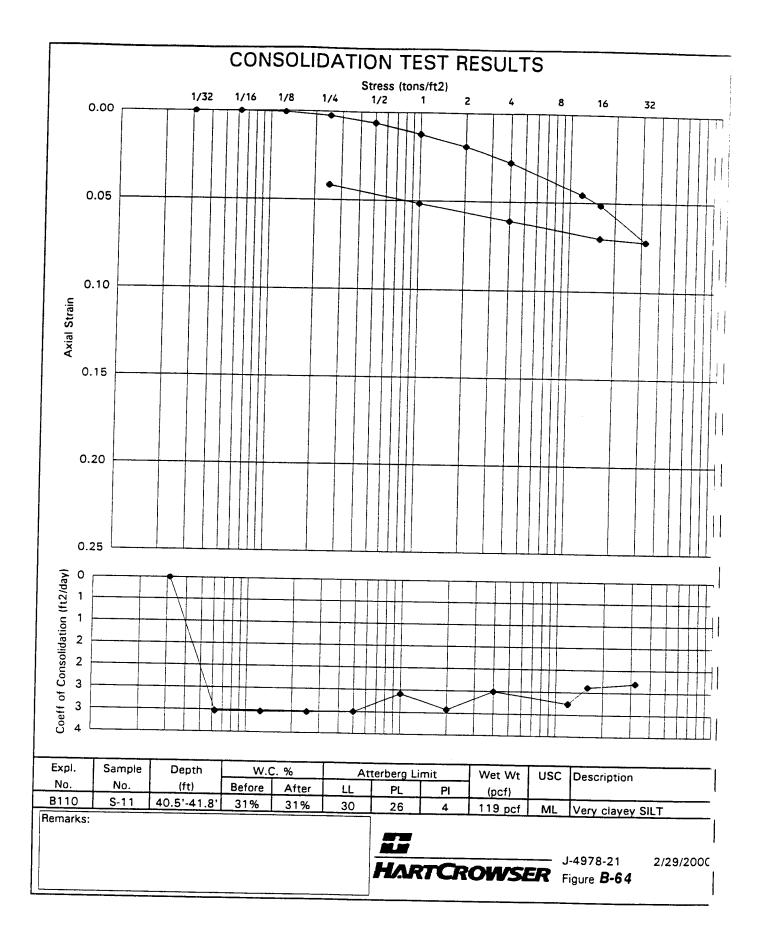


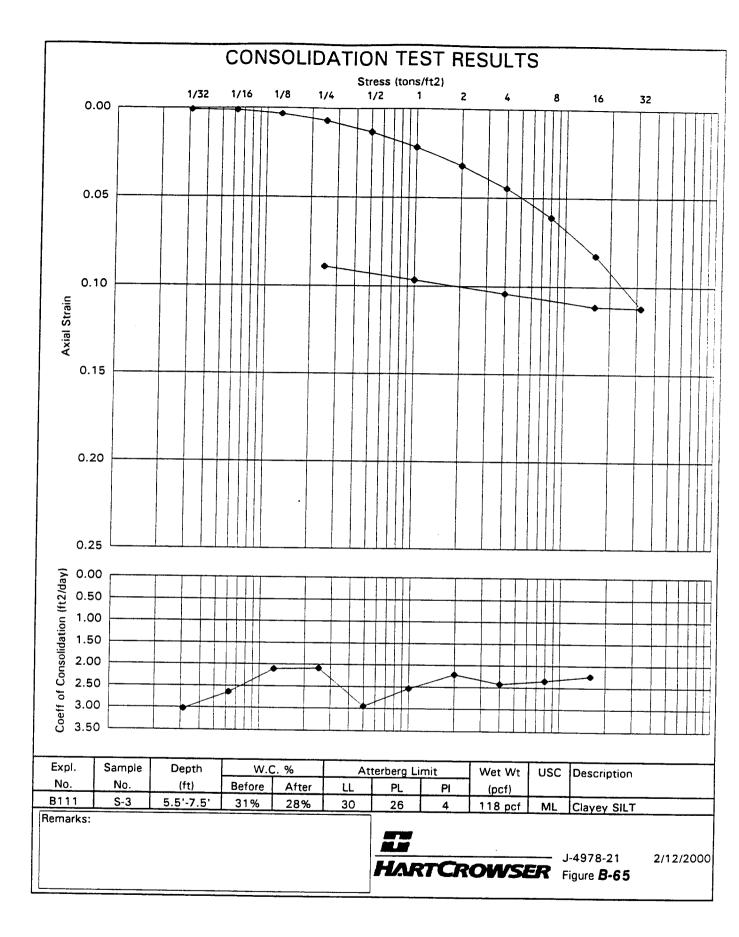


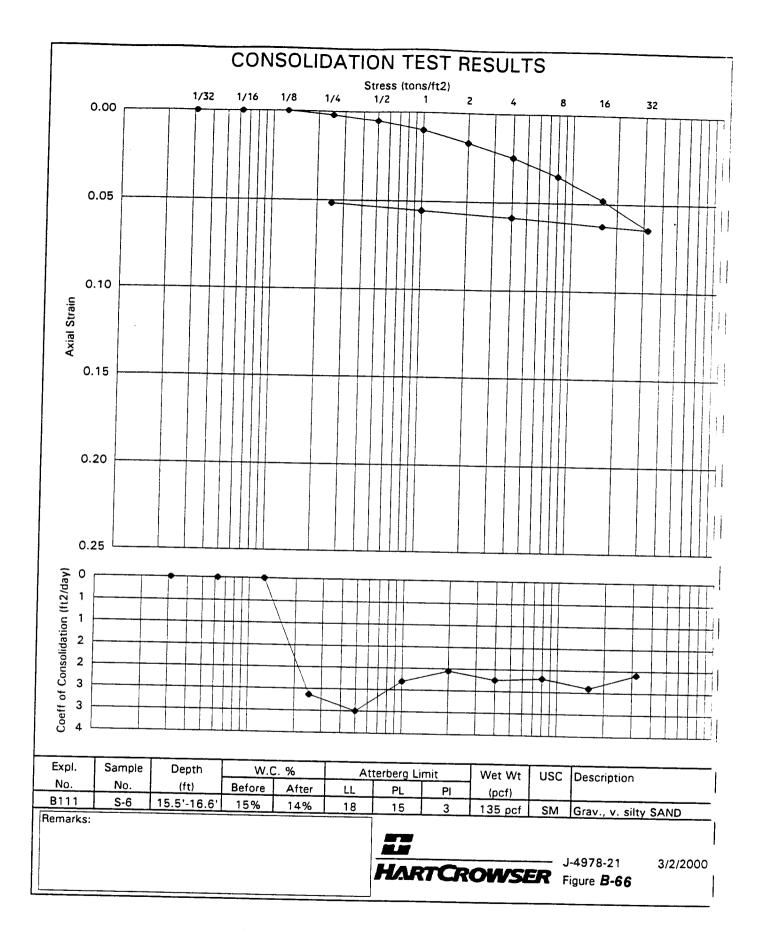


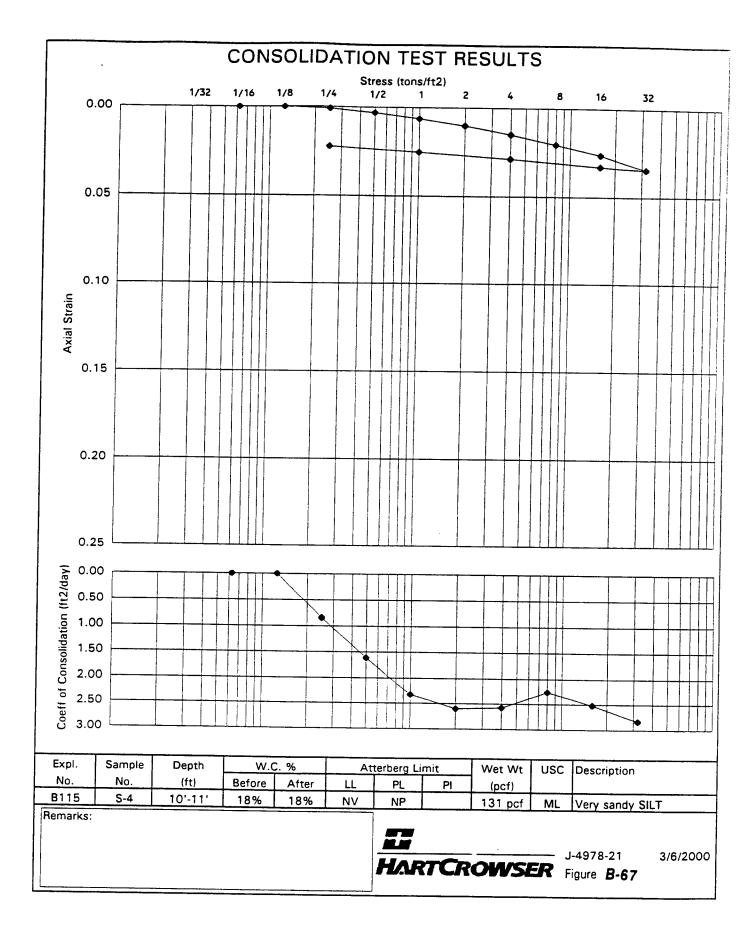




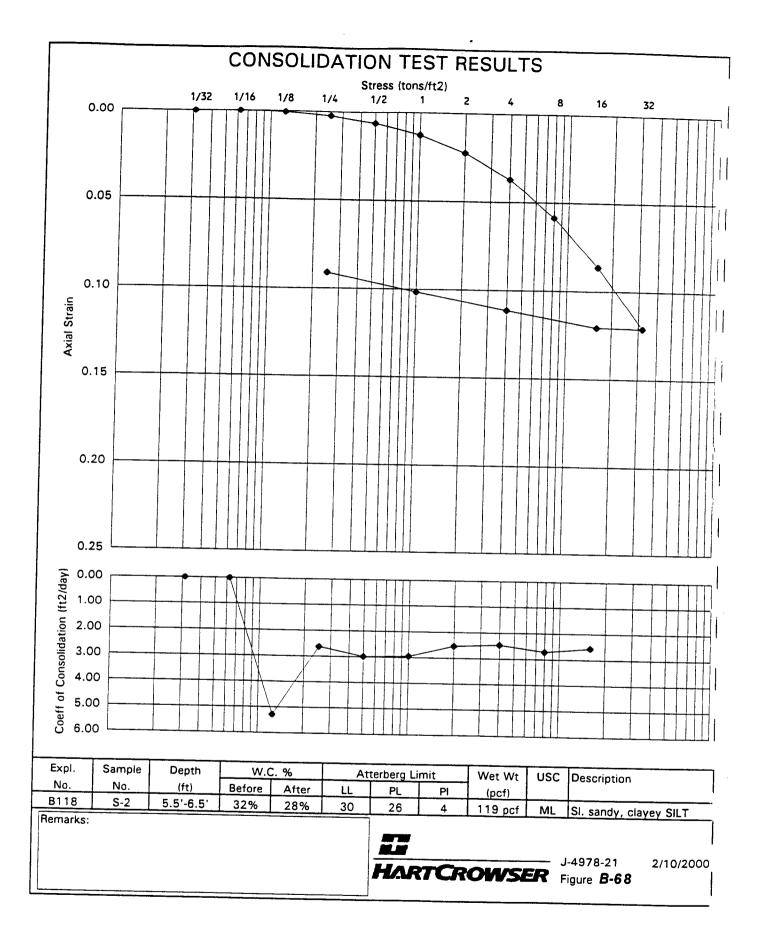


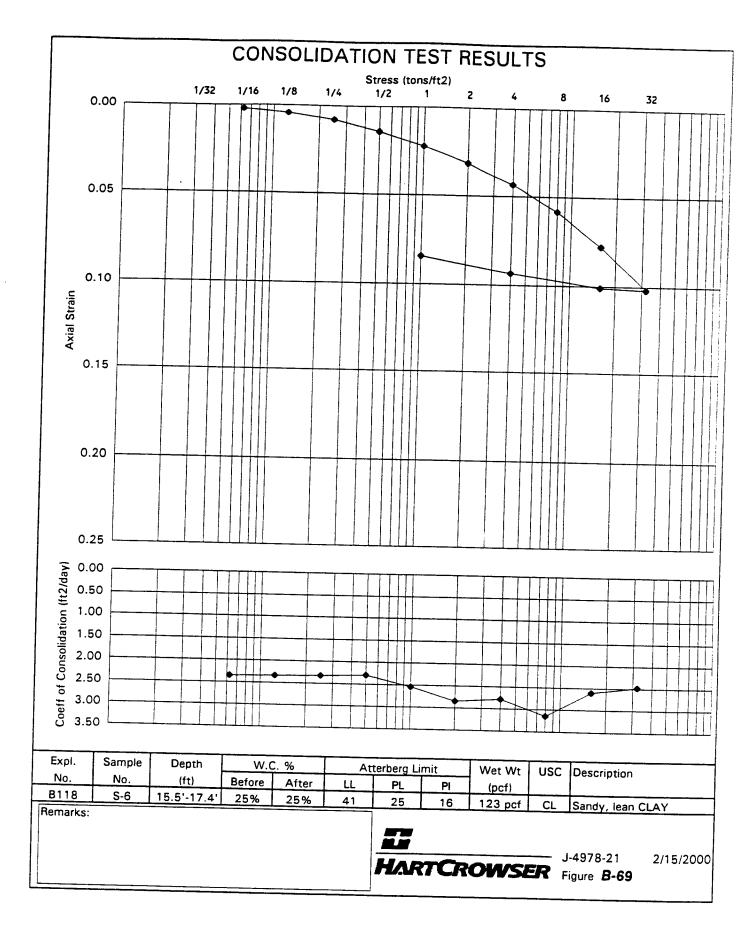




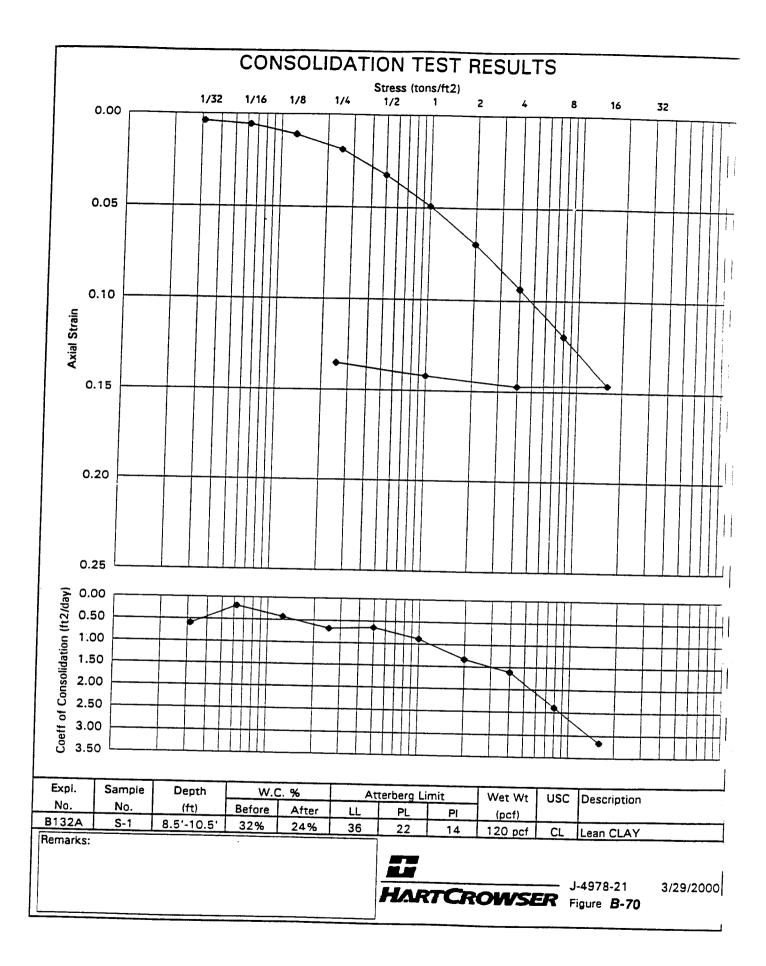


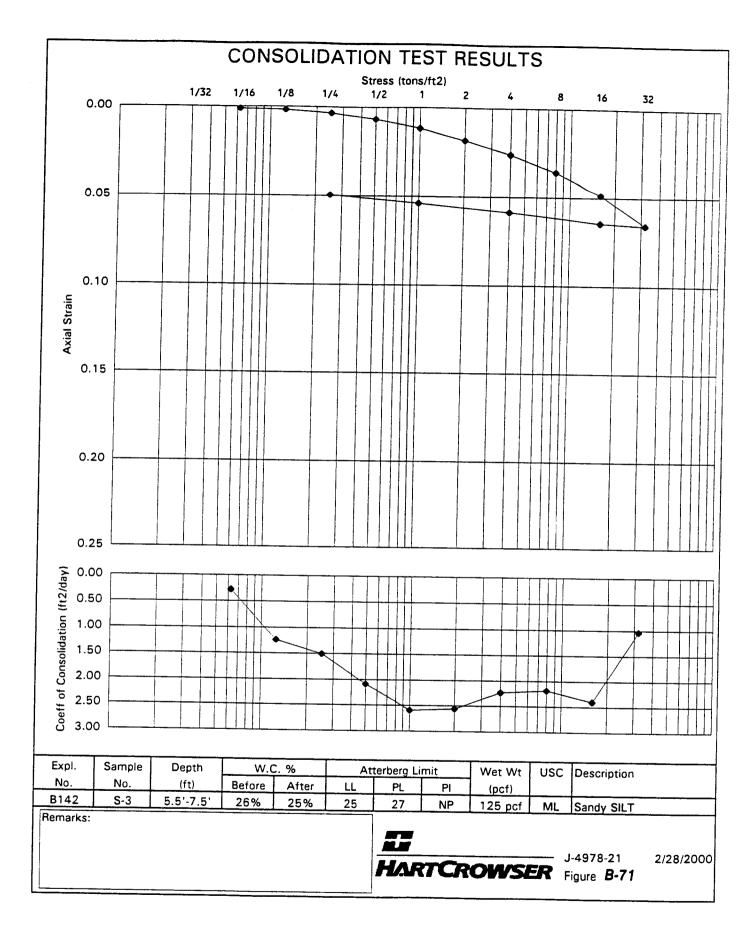
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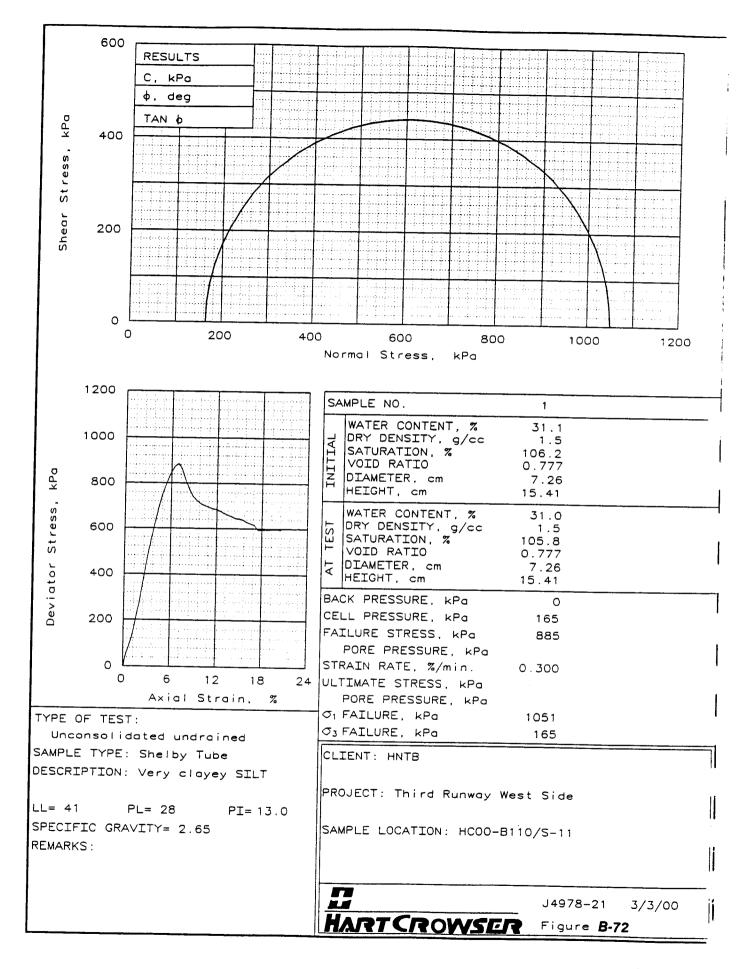


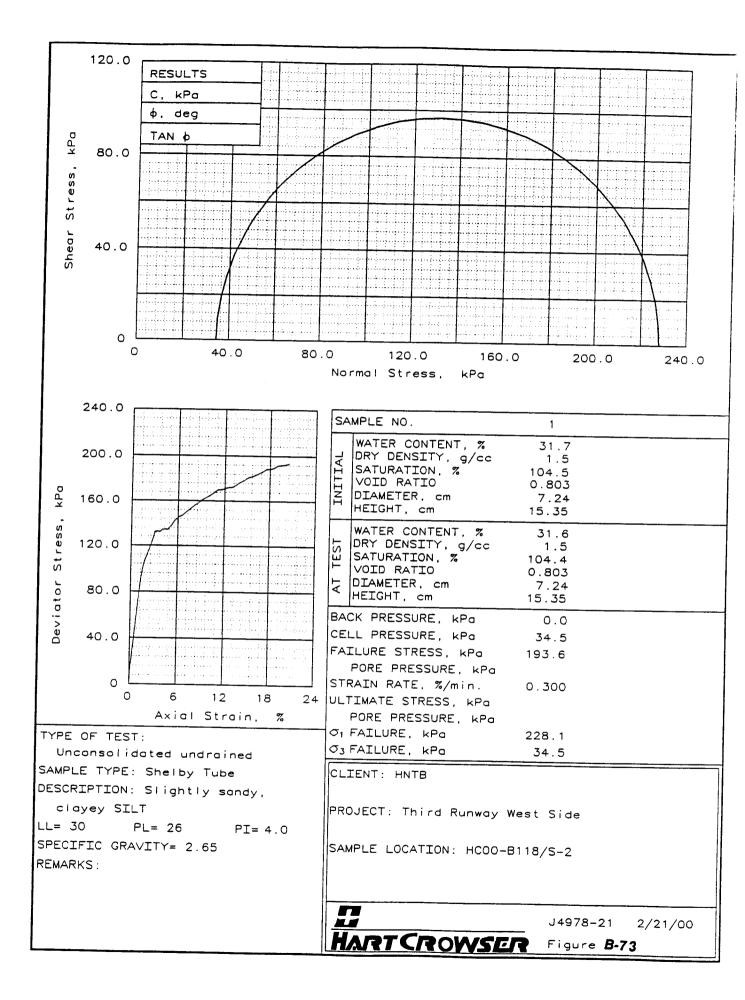


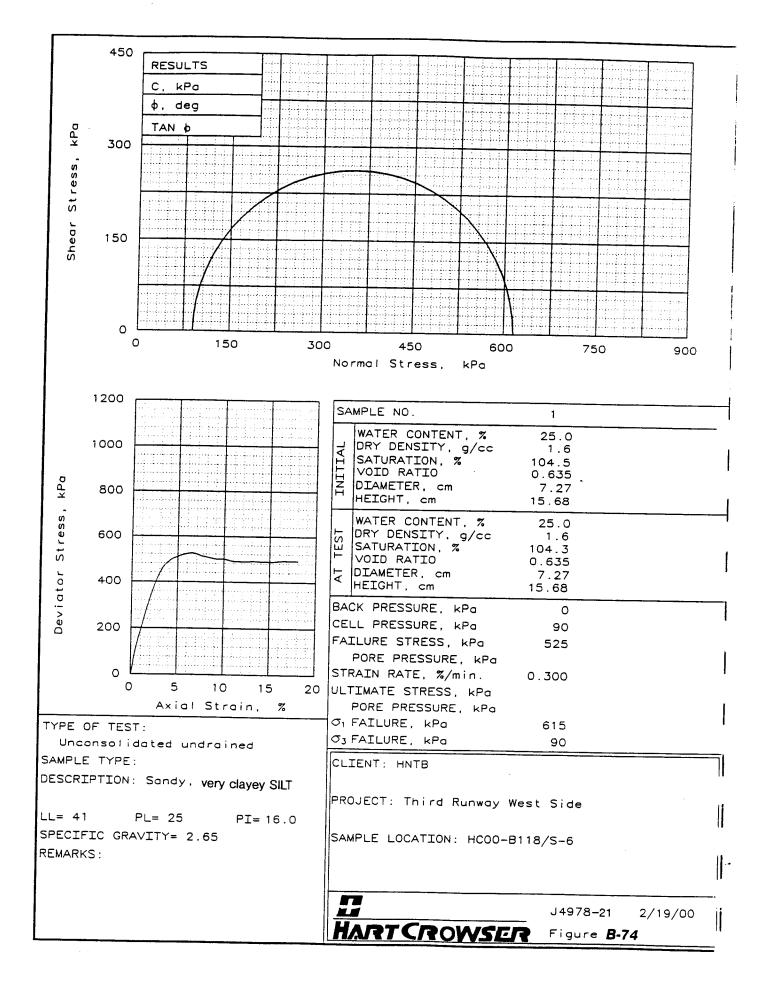
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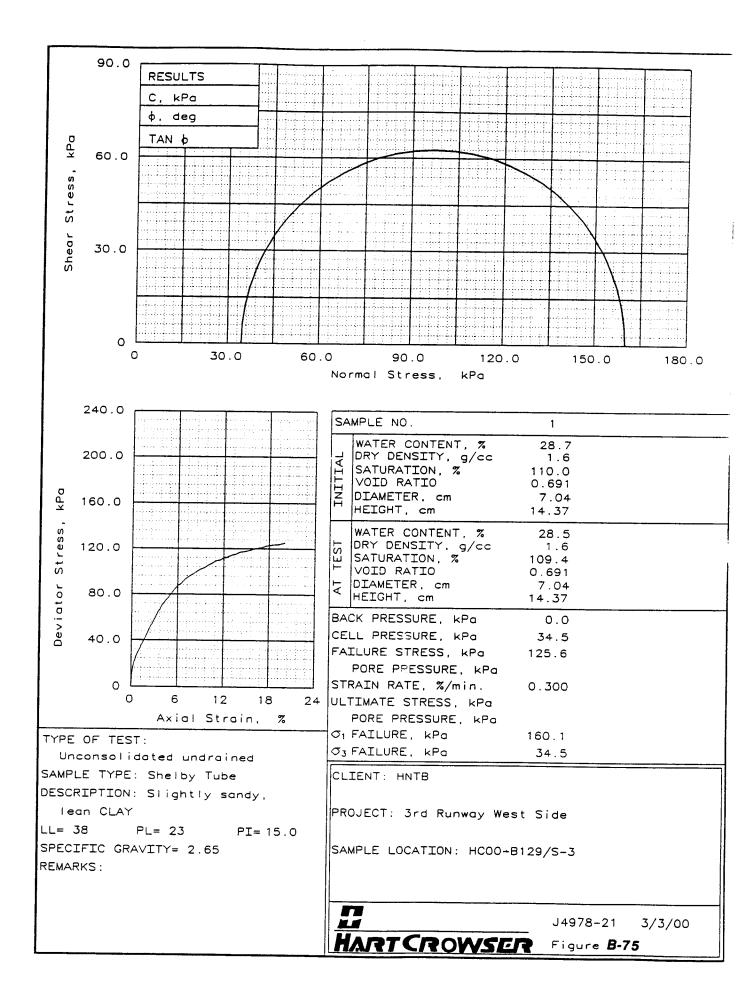


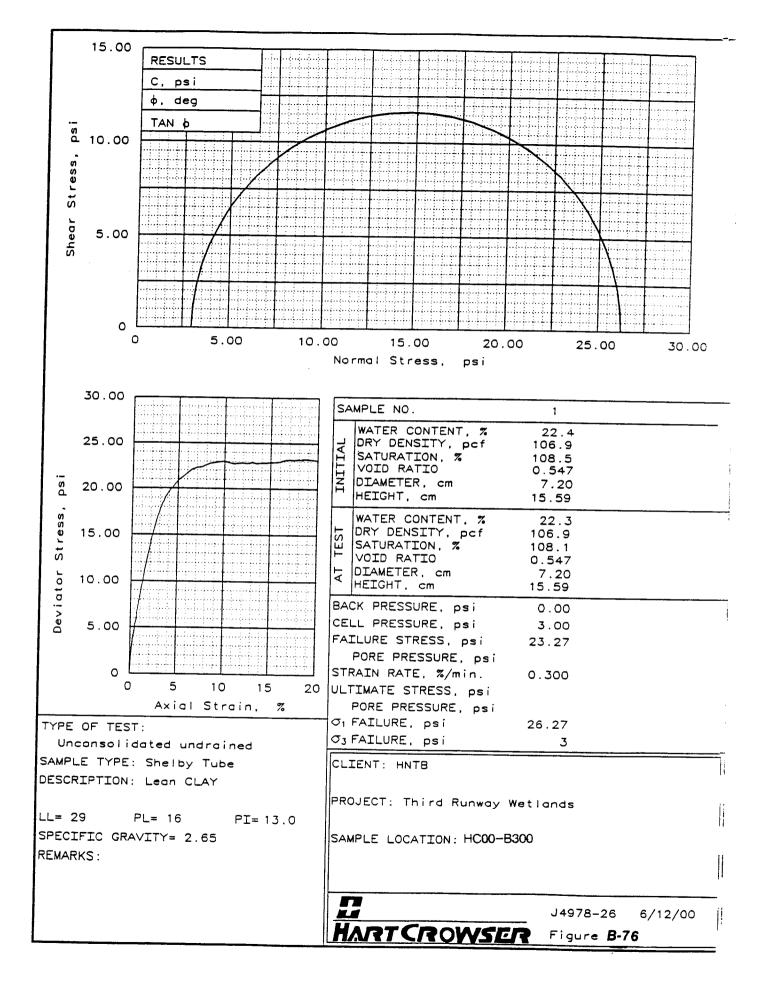


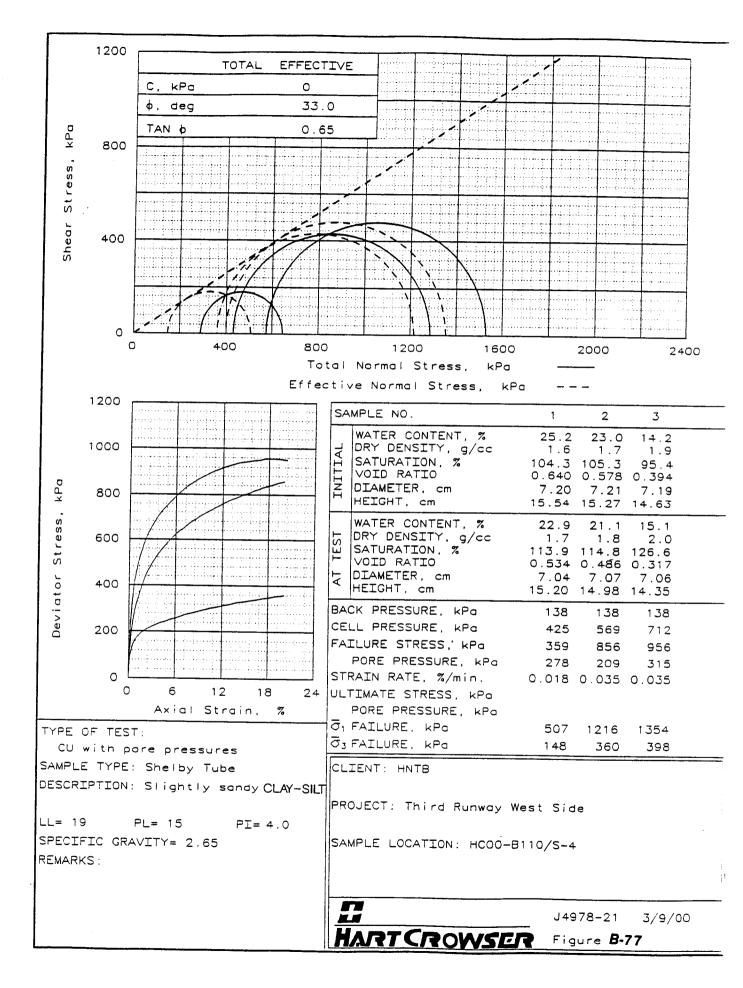




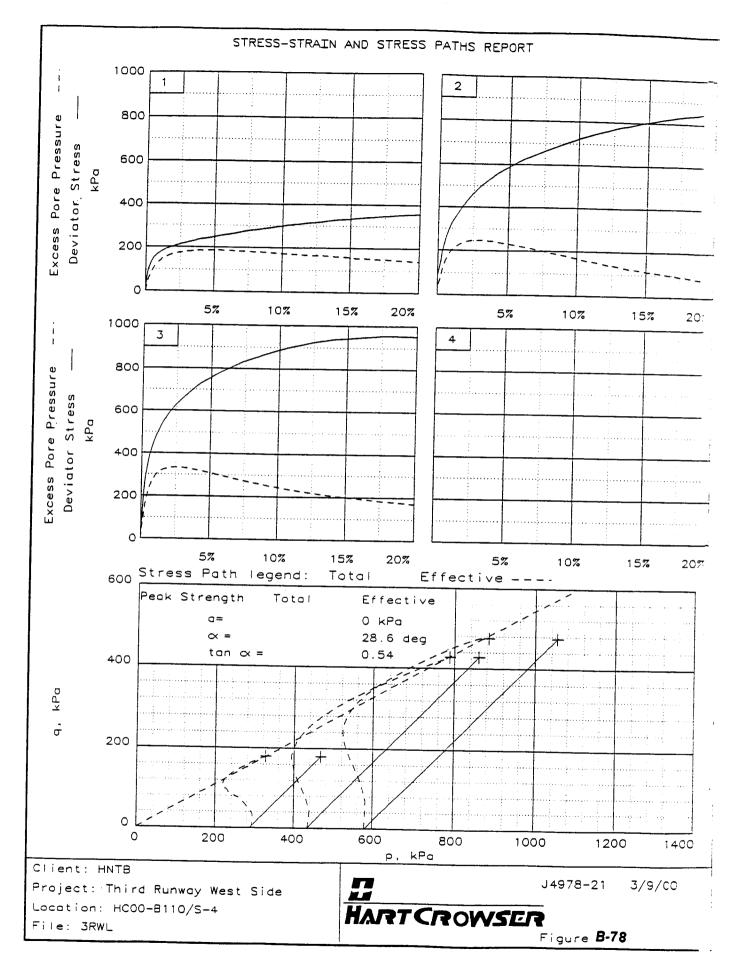


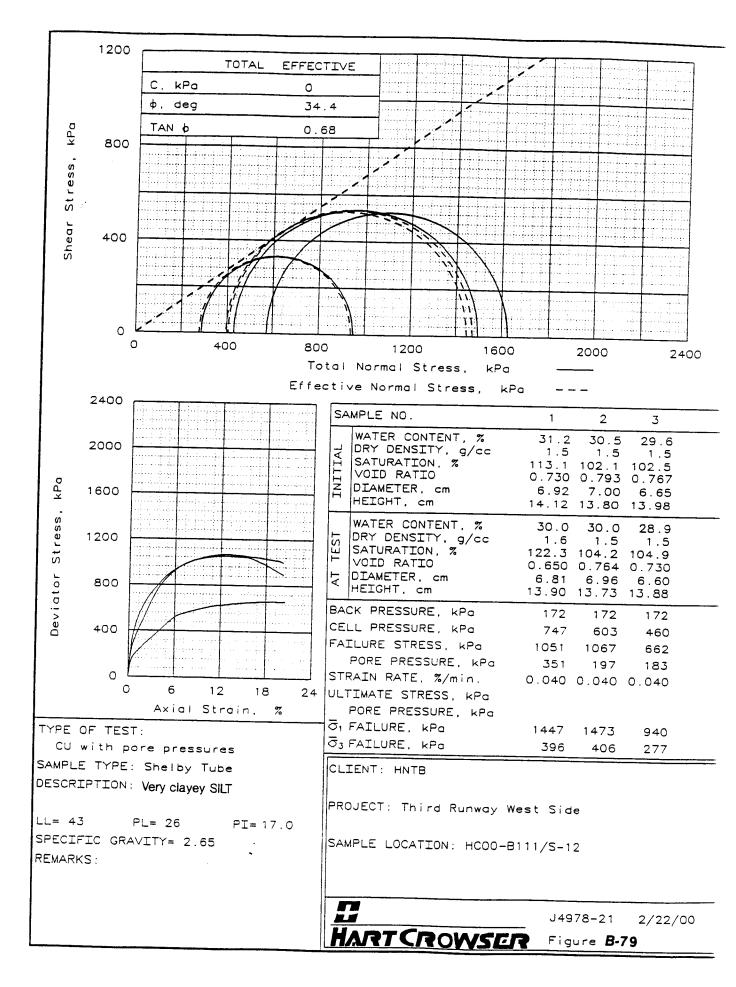




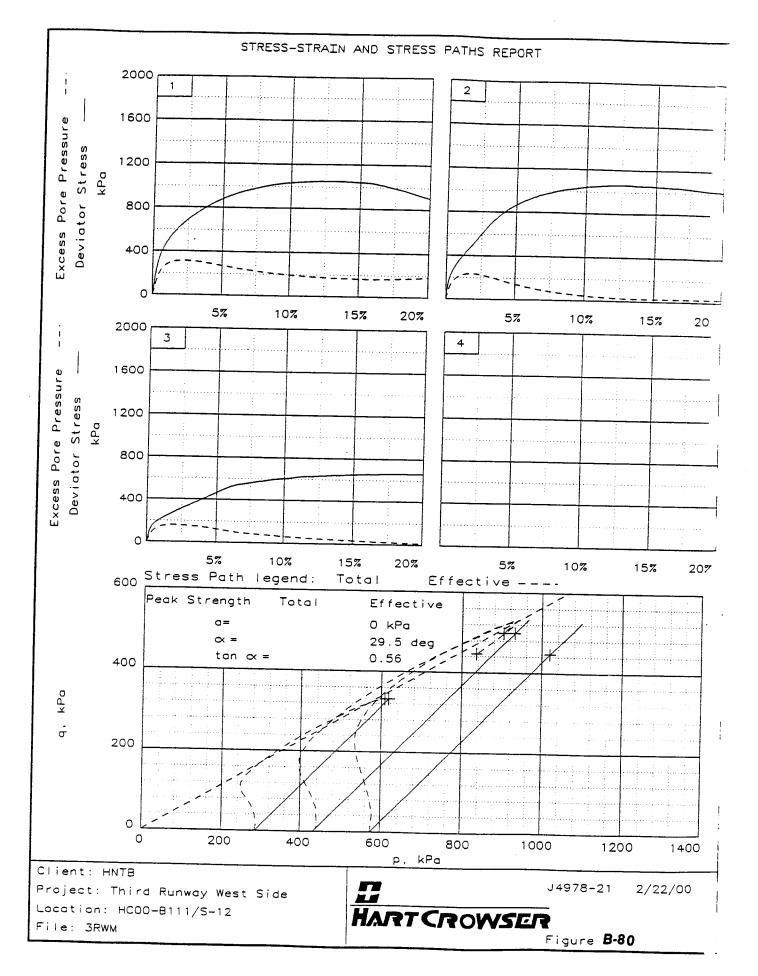


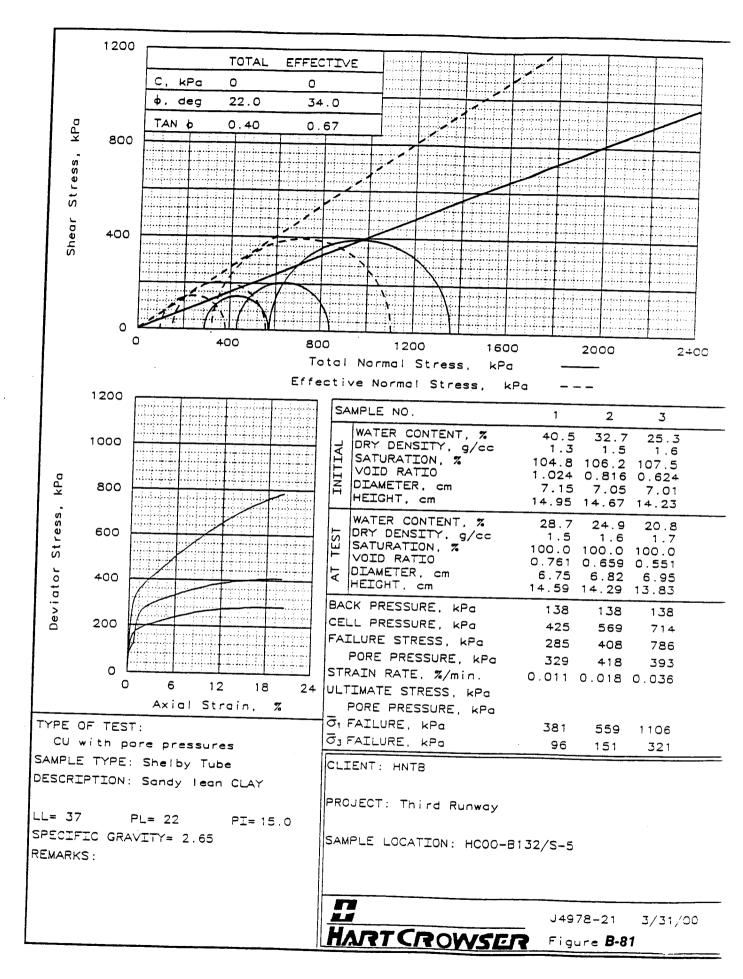
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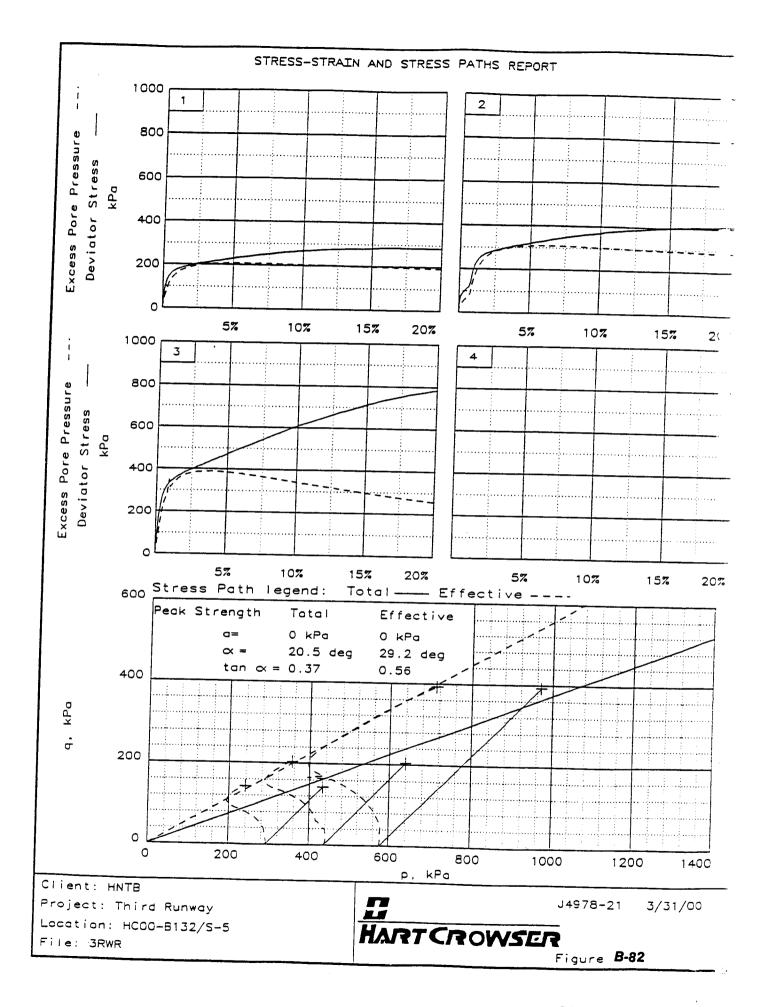


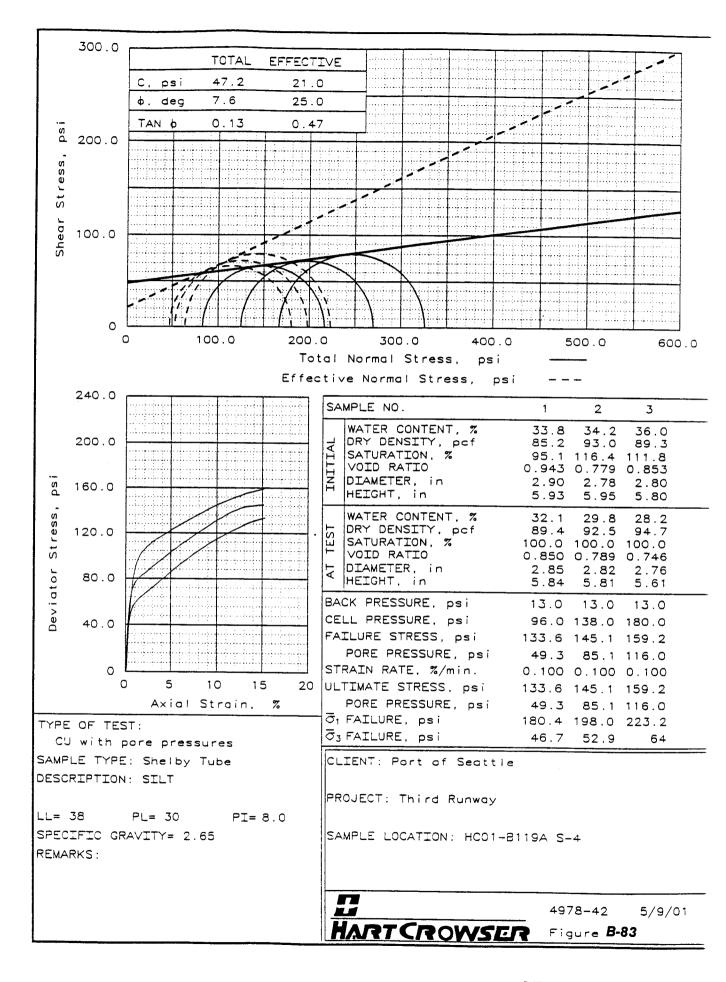


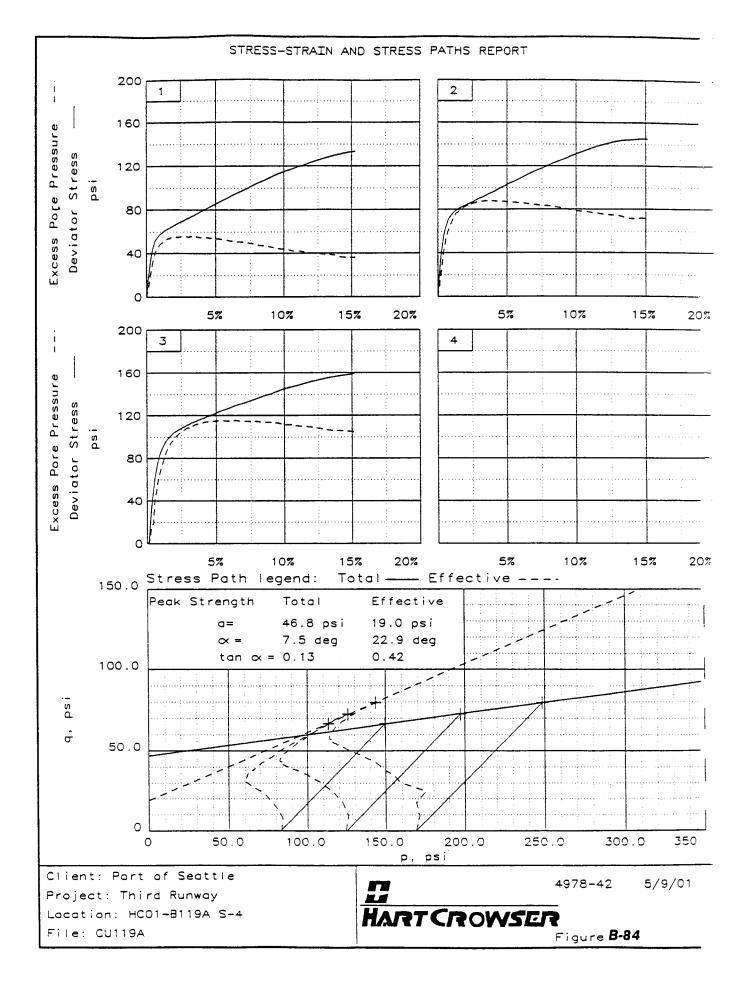
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APPENDIX C FIELD EXPLORATIONS AND LAB TEST RESULTS BY OTHERS

Hart Crowser 4978-28 September 21, 2001

APPENDIX C FIELD EXPLORATIONS AND LAB TEST RESULTS BY OTHERS

This appendix presents data collected by Applied Geotechnology, Inc. (AGI) and Dames & Moore. Boring logs and test pit logs are presented on Figures C-3 through Figures C-49. Figure C-1 presents the soil classification system that was used by AGI and a legend to their exploration logs. An explanatory figure illustrating piezometer construction is provided on Figure C-2. Laboratory test data consisting of grain-size data and direct shear test results are presented on Figures C-50 through C-53. The exploration logs indicate that more laboratory tests were conducted than presented in this data report. We included the data that are available to us at this time and that are relevant to the Phase 5 work areas.

<u>NOTE</u>: Laboratory test results presented in this data report were compiled from earlier reports. These reports should be consulted in the interpretation of the laboratory test data.

This appendix includes the following subsurface explorations:

Explorations by AGI:

Borings:

AT94A-B3, AT94B-B2, AT94B-B6, AT96-B2 through AT96-B4, AT97-B2, AT97-B4 through AT97-B11, AT97-B13 through AT97-B20, AT97-B22, AT97-B23, AT97-B29 through AT97-B31, AT97-B33, AT97-B36, AT97-B37, AT97-B52, AT97-B56, AT97-B66 through AT97-B68, AT97-B72, AT97-B73.

Test Pits:

AT94B-TP5 through AT94B-TP7, AT94B-TP9, AT94B-TP10, AT94B-TP15, AT94B-TP16, AT97-TP5 through AT97-TP11.

Borings by Dames & Moore:

DM68A-B1 and DM68A-B2.

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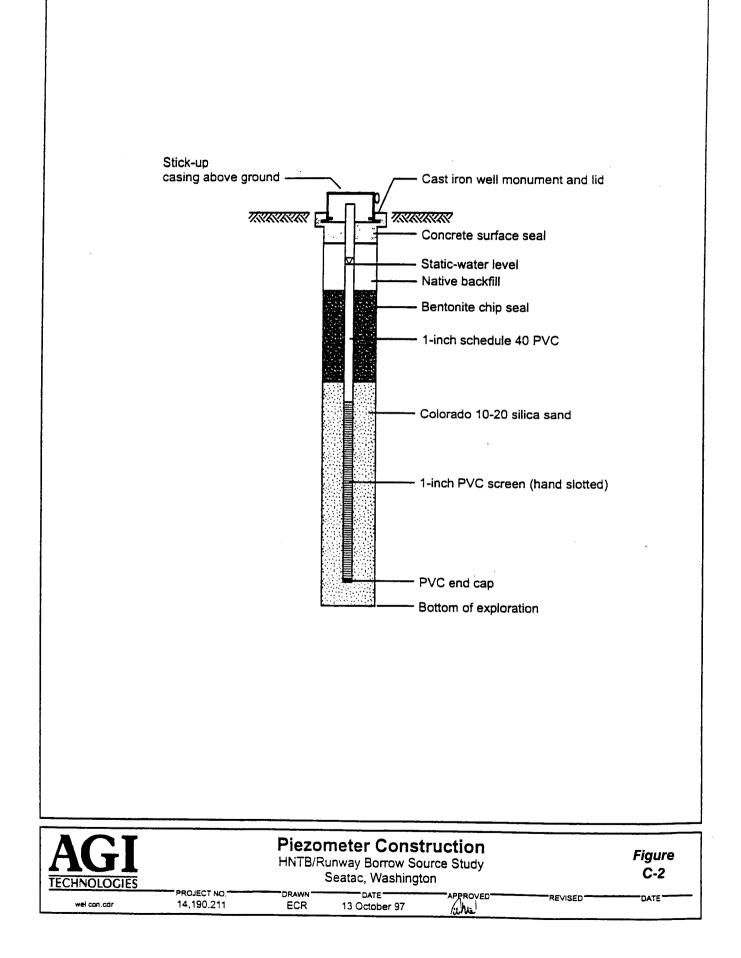
UNIFIED SOIL CLASSIFICATIONS SYSTEM								
	MAJOR DI	ISIONS				TYPICAL NAMES		
COARSE GRAINED SOILS More than half is larger than No. 200 Sieve	GRAVELS	Clean gravels with little or no fines	GW	000	Well graded	gravels, gravel-sand mixtures		
	More than half		GP		Poorly grade	ed gravels, gravel-sand mixtures		
	coarse fraction is larger than No. 4 sieve size	Gravels with over 12% fines	GM		Silty Gravels mixtures	ls, poorly graded gravel-sand-silt		
			GC			els, poorly graded -clay mixtures		
	SANDS More than half coarse fraction is larger than No. 4 sieve size	Clean sands with little or no fines	sw		Well graded	sands, gravelly sands		
			SP			ed sands, gravelly sands		
		Sands with over 12% fines	SM			boorly graded sand-silt mixtures as, poorly graded sand-clay ts and very fine sands, rock flour, silty or ands, or clayey silts with slight plasticity		
			SC		mixtures			
ILS er	SILTS AND		ML		clayey fine s			
D SOILS s smaller Sieve	Liquid limit le		CL		gravelly clay	ays of low to medium plasticity, s, sandy clays, silty clays, lean clays		
half is 200			OL			s and organic silty clays of low plasticity		
FINE GRAINED SOIL More than half is smaller than No. 200 Sieve	SILTS AND CLAYS Liquid limit greater than 50		MH		sandy or silt	y soils, elastic silts ays of high plasticity, fat clays		
			СН					
HIGHLY ORGANIC SOILS			ОН		organic silts Peat and other highly organic soils			
			PT			5,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		
	sturbed"		CONTACT BETWEEN UNITS Well Defined Change			PHYSICAL PROPERTY TESTS		
Bulk/C		Gradational Change			Consol - Consolidation			
D Not R	ecovered	Obscure Change			5	LL - Liquid Limit PL - Plastic Limit		
🖽 Recov	Recovered, Not Retained End			oloratior	1	Gs - Specific Gravity		
Hammer S - SF T - Th	PER FOOT is 300 pounds with 30 PT Sampler (2.0-Inch bin Wall Sampler (2.8- blit Barrel Sampler (2.9	O.D.) Inch Sample)	SA - Size Analysis TxS - Triaxial Shear TxP - Triaxial Permeability Perm - Permeability Po - Porosity MD - Moisture/Density DS - Direct Shear					
Dry Moist Wet	RE DESCRIPTION - Considerably less - Near optimum moi - Over optimum moi - Below water table,	roundwater	VS - Vane Shear VS - Vane Shear Comp - Compaction UU - Unconsolidated, Undrained CU - Consolidated, Undrained CD - Consolidated, Drained					
AGT Soil Classification/Legend								

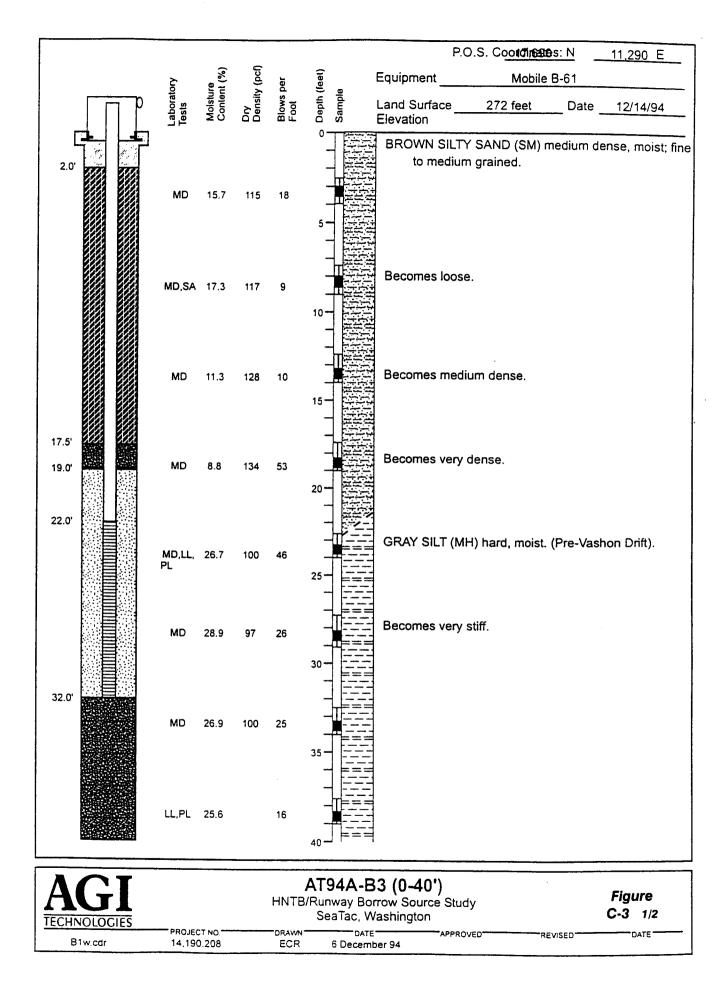
 AG51
 HNTB/SeaTac 1997 - Runway Investigation
 Figure

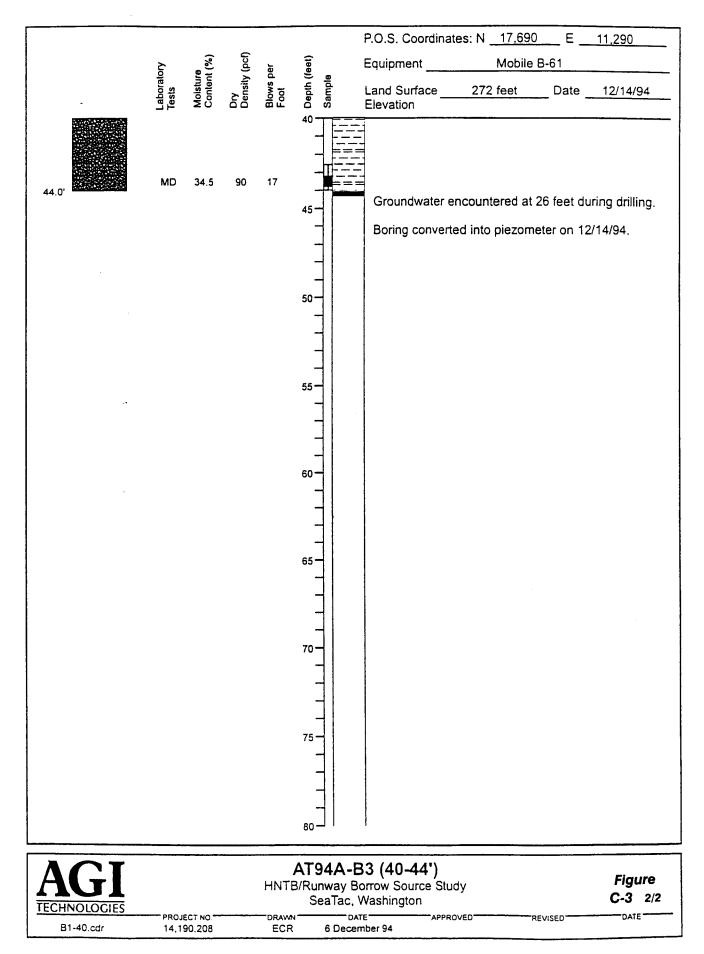
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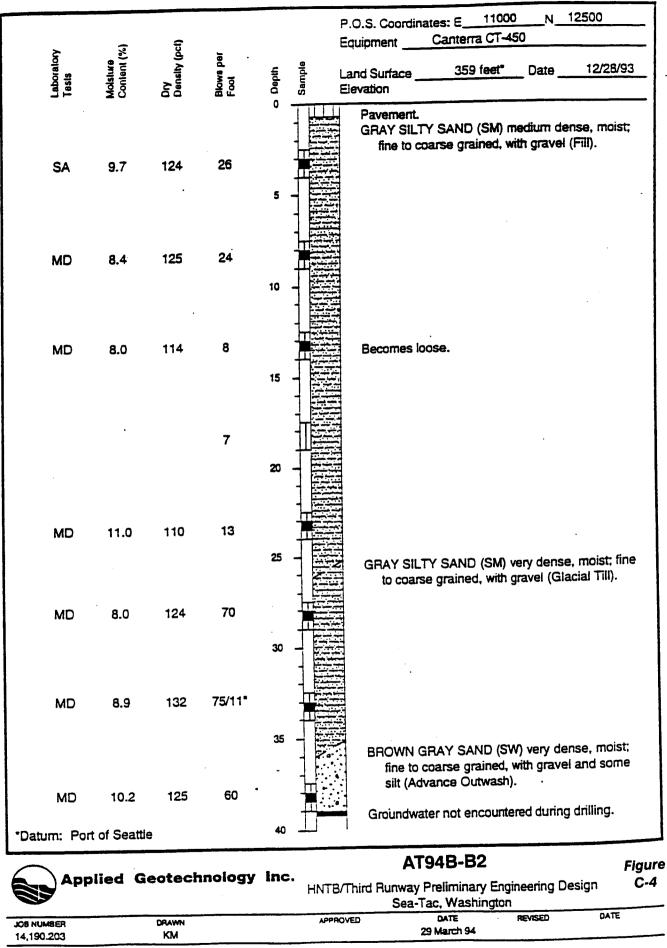
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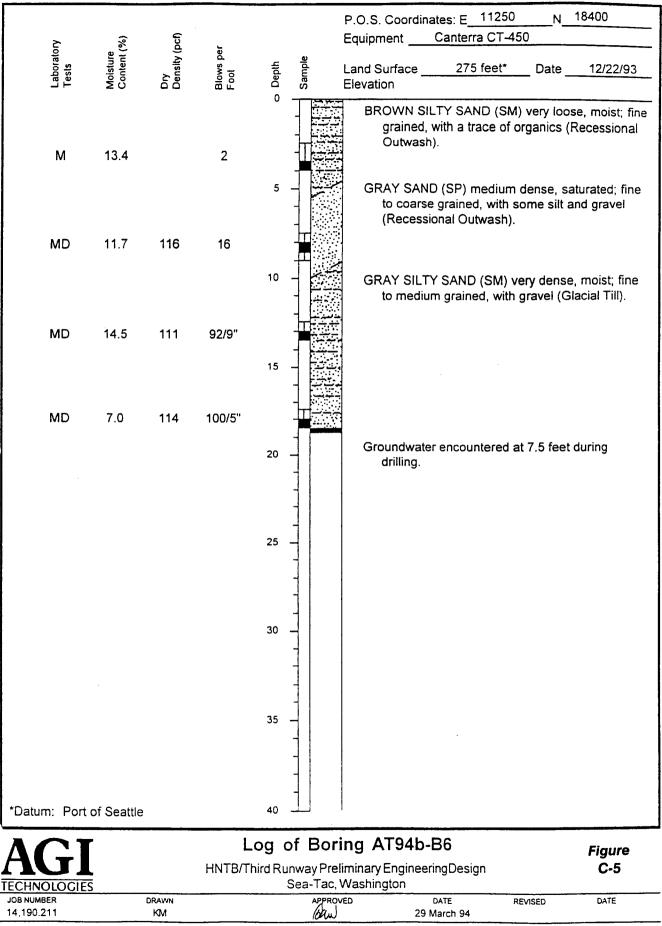




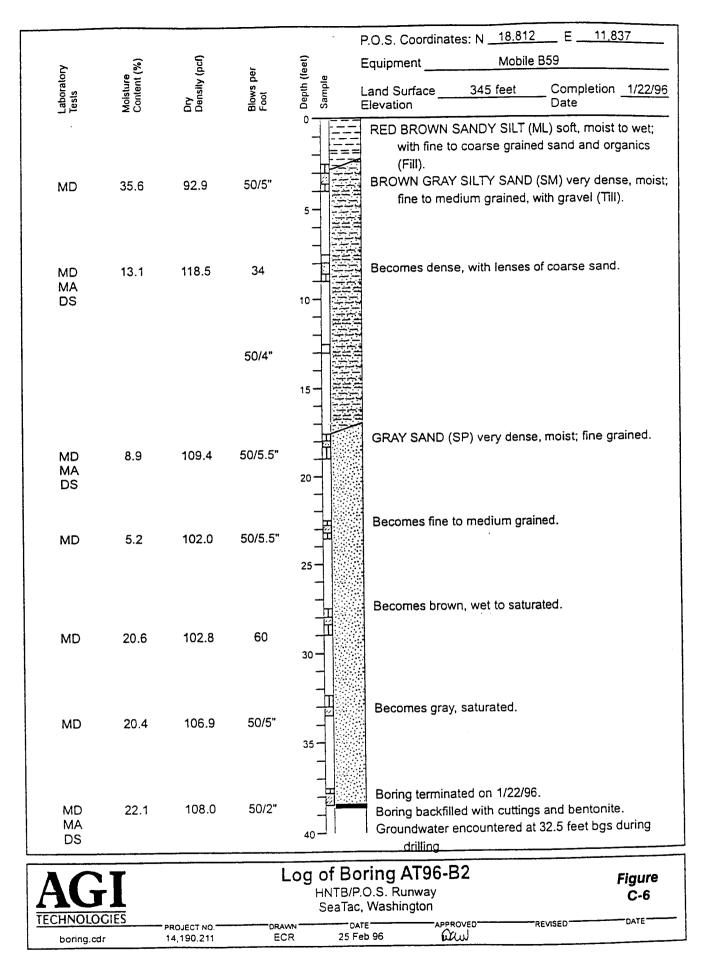


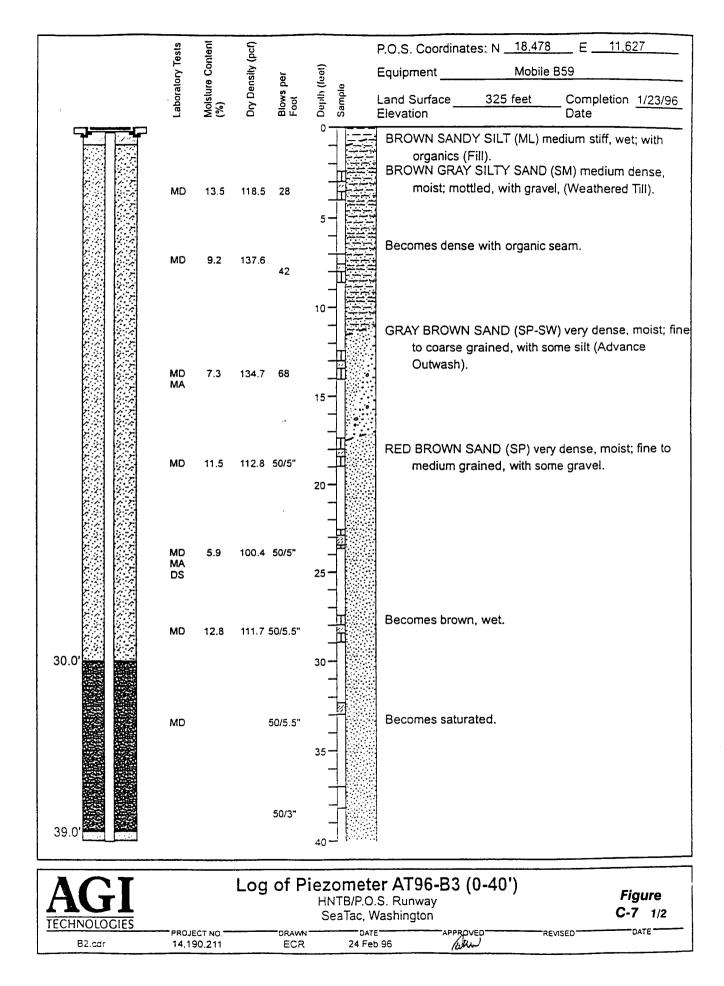


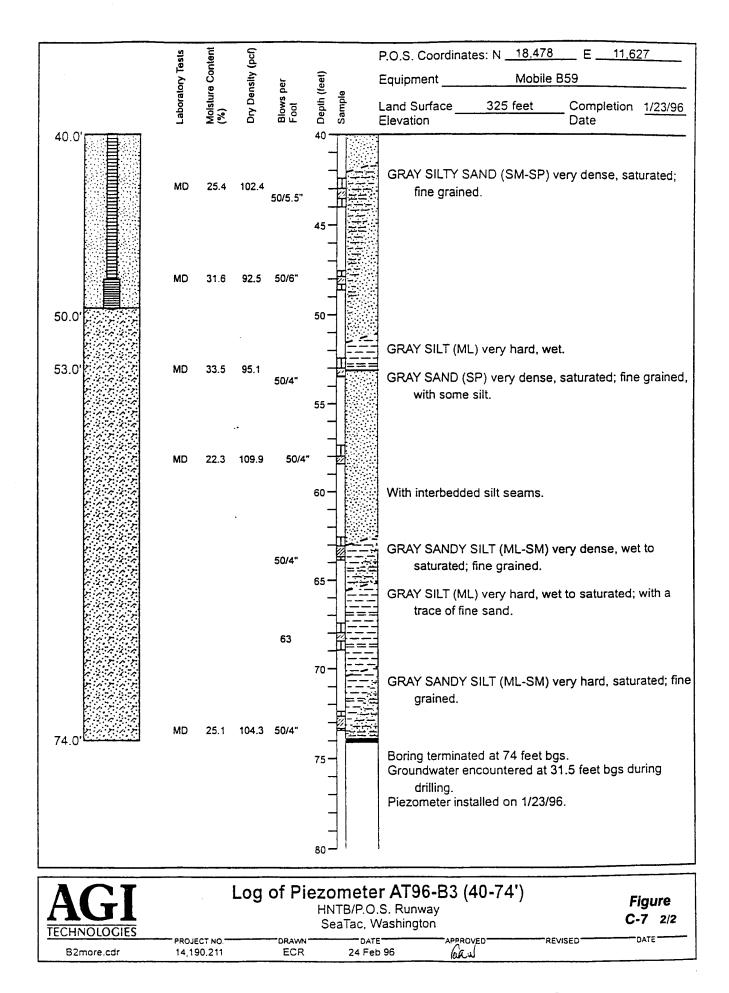
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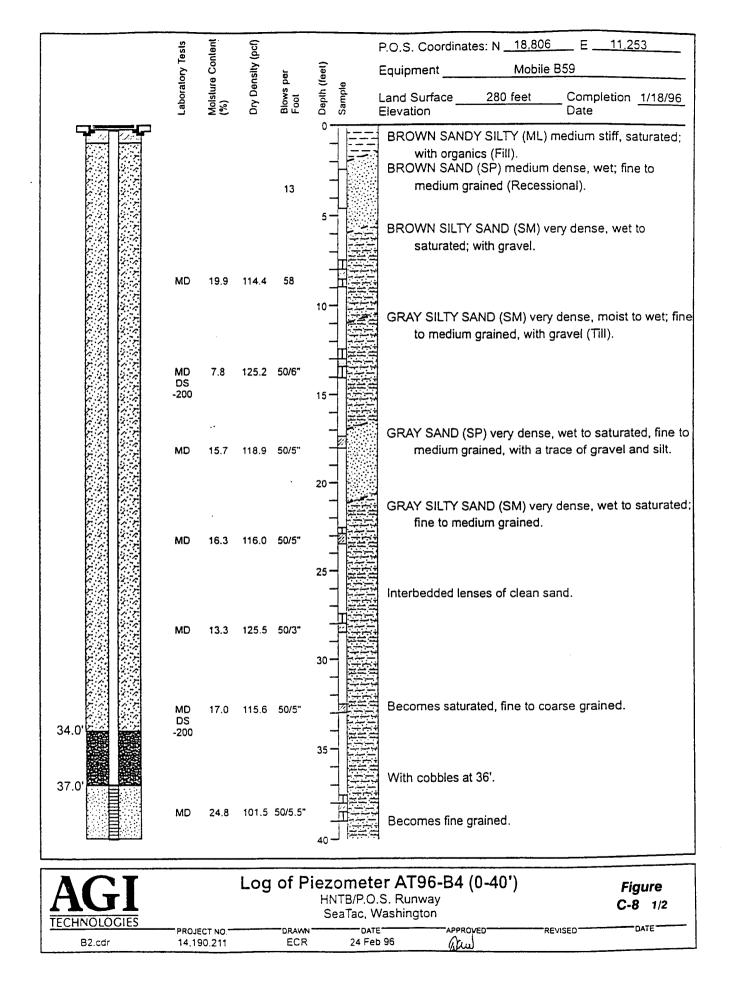


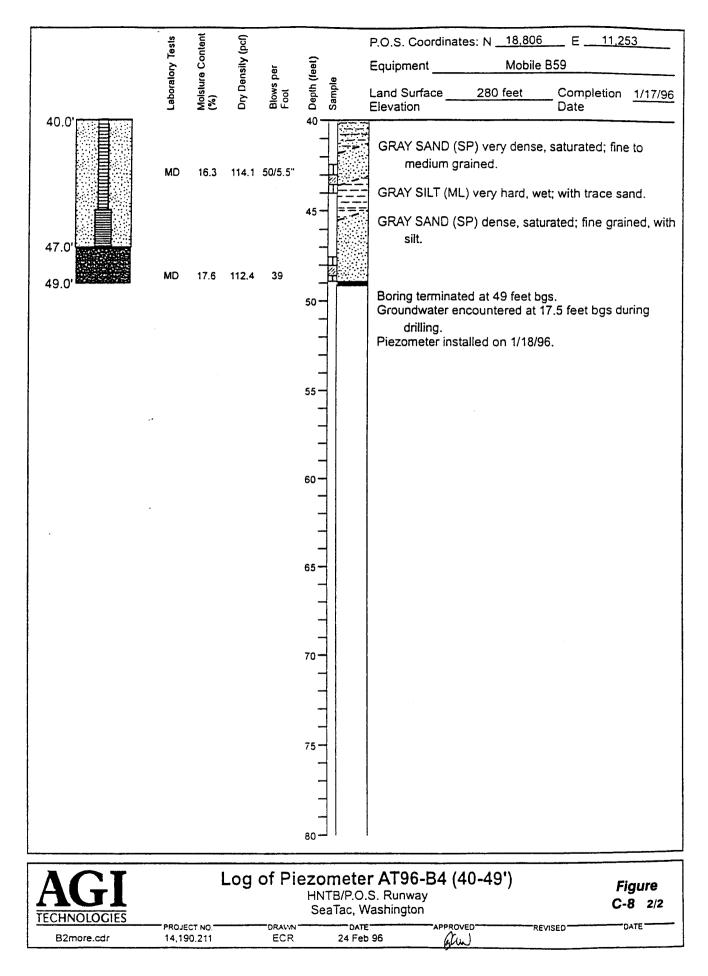
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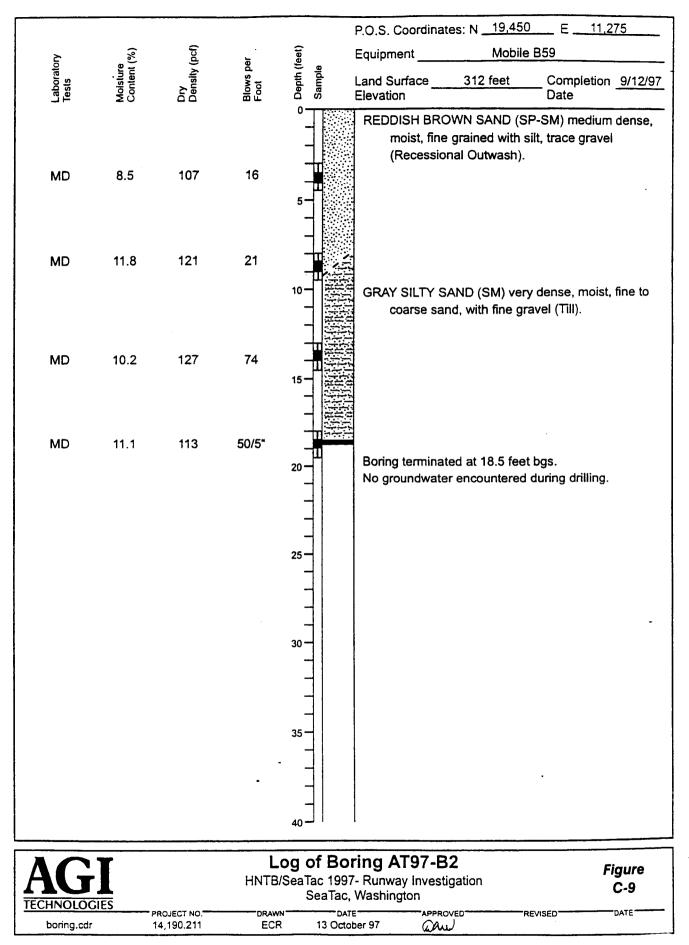




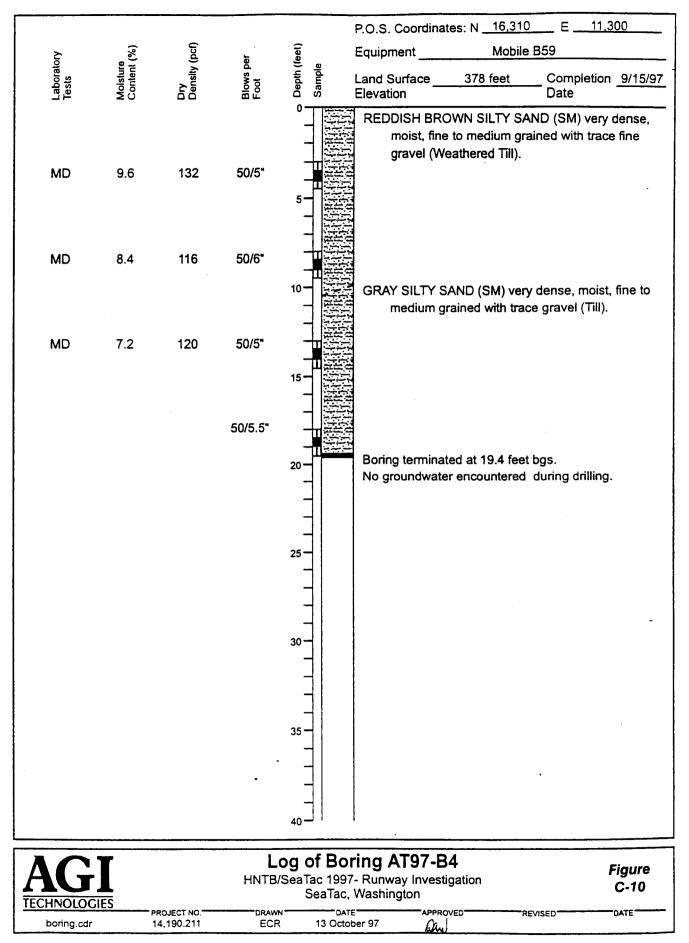


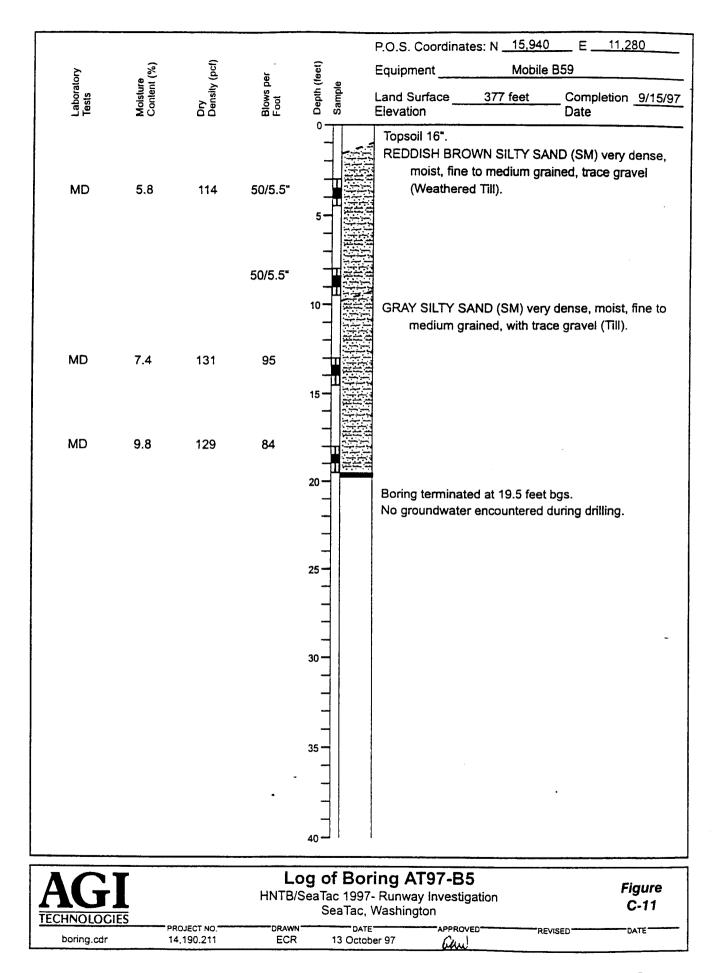


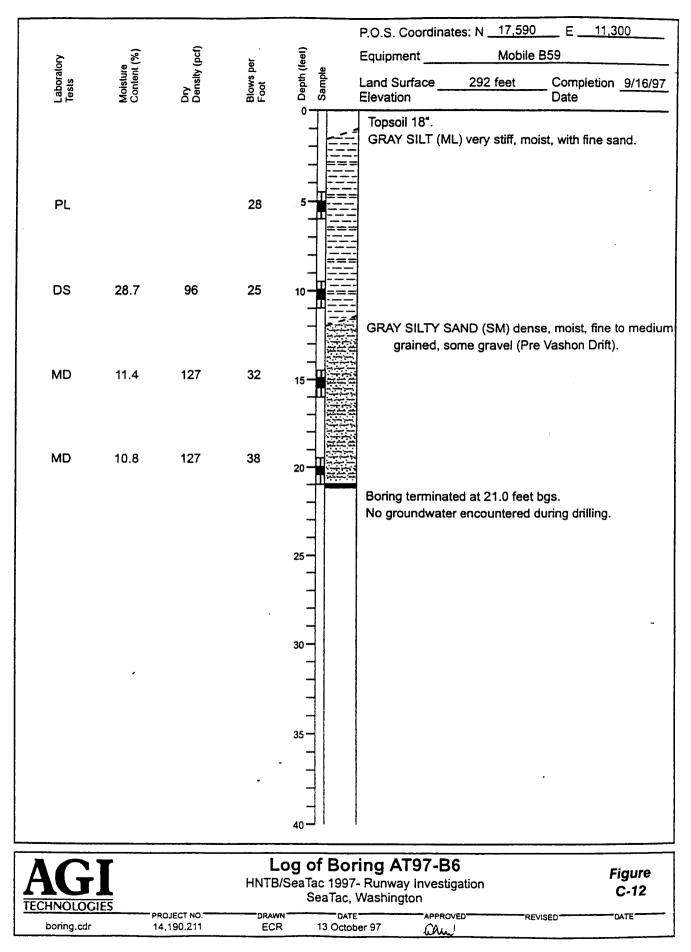


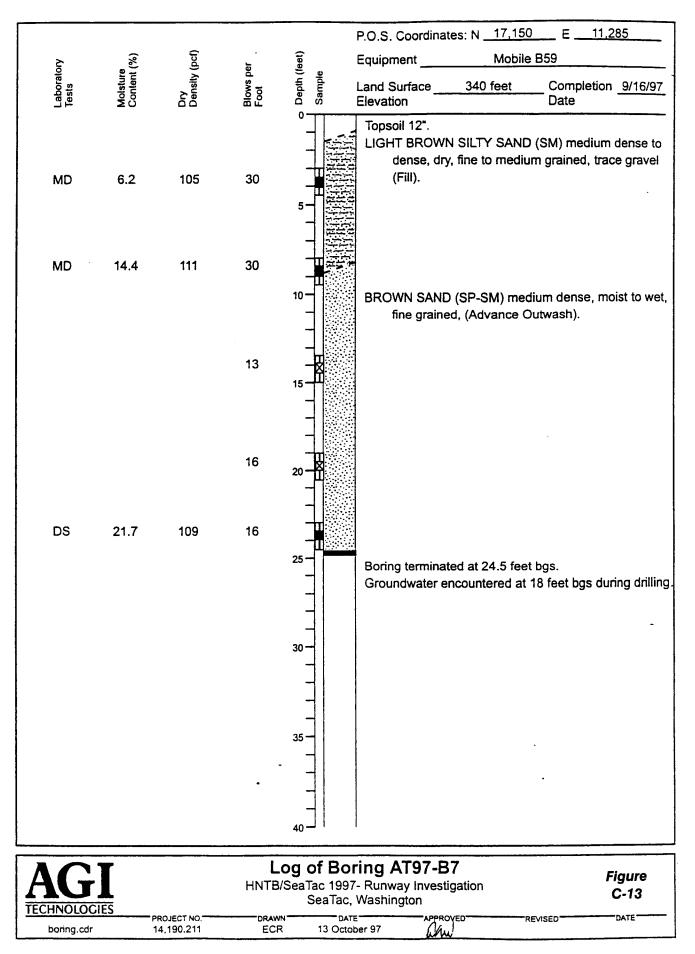


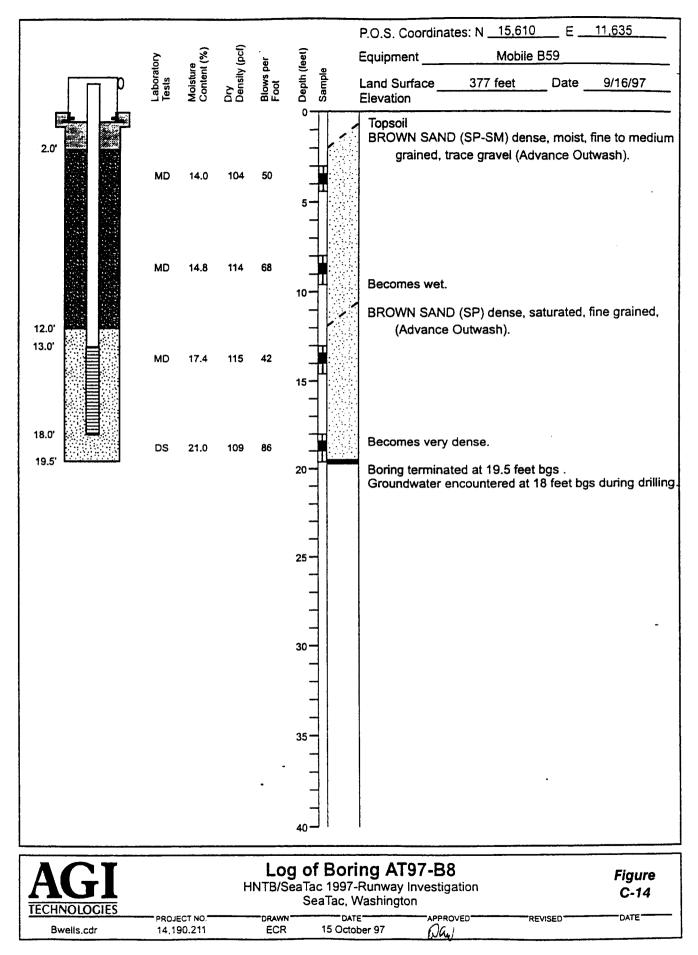
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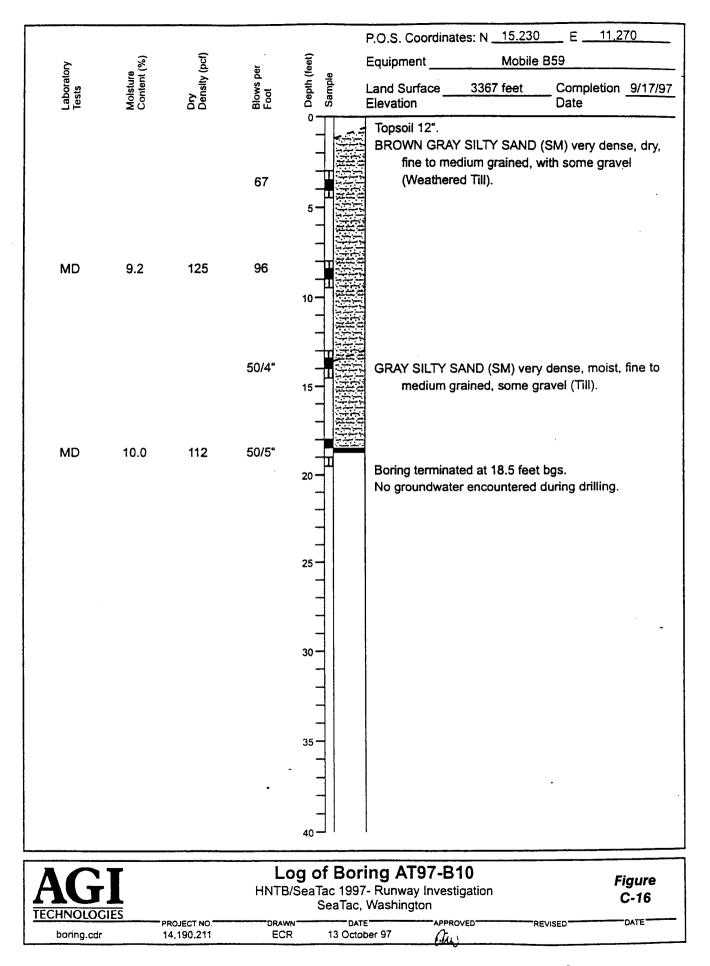


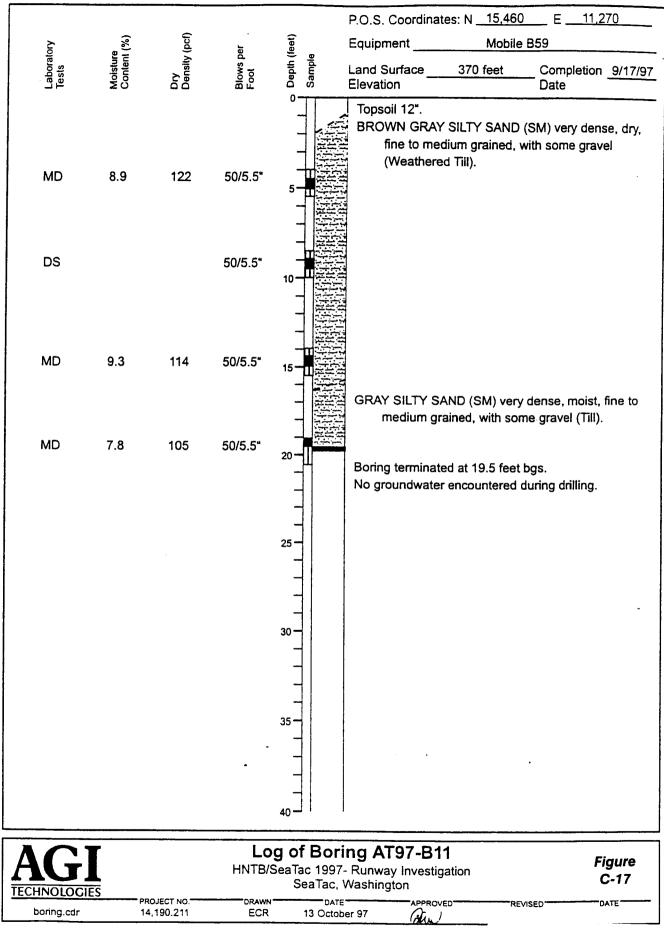


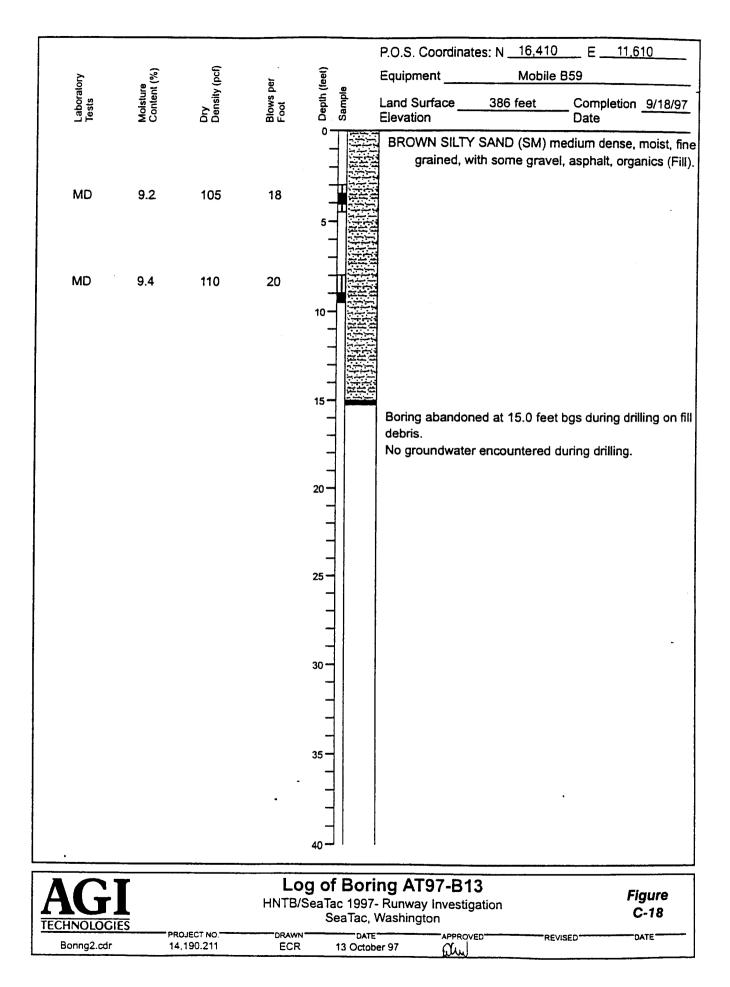
MD R.4 122 50/5* Provide and the second seco		<u> </u>			P.O.S. Coordinates: N <u>15,170</u> E <u>11,570</u>			
MD 7.5 118 50/5" Image: status of the	~	(%	oc()					
MD 7.5 118 50/5" Image: status of the	Laborator Tests	Moisture Content ('	Dry Density (r	Blows per Foot				
MD 10.9 124 50/5' MD 23.7 105 44 MD 23.7 105 44 Series grained, with gravel (Till). BROWN SAND (SP) dense, saturated, fine grained (Advance Outwash). MD 23.7 105 44 Series grained, with gravel (Till). Boring terminated at 19.5 feet bgs. Groundwater encountered at 18 feet bgs during drilling. 35 36 40 35 40 35 40 56 Figure Series (Mashington Se		7.5		50/5*	BROWN GRAY SILTY SAND (SM) very dense, moist, fine to coarse grained, with gravel, mottled			
MD 10.9 124 50/5" 15 16 BROWN SAND (SP) dense, saturated, fine grained (Advance Outwash). MD 23.7 105 44 10 Boring terminated at 19.5 feet bgs. Groundwater encountered at 18 feet bgs during drilling. 30 10 25 10 <t< td=""><td>MD</td><td>8.4</td><td>122</td><td>50/5.5"</td><td></td></t<>	MD	8.4	122	50/5.5"				
MD 23.7 105 44 20 40 40 40 40 40 40 40 40 40 4	MD	10.9	124	50/5"	- coarse grained, with gravel (Till).			
25 30 30 40 5 5 5 5 5 5 5 5 5 5 5 5 5	MD	23.7	105	44	20 - Boring terminated at 19.5 feet bgs.			
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ACJI HNTB/SeaTac 1997- Runway Investigation C-15 TECHNOLOGIES DRAWN Date Date								
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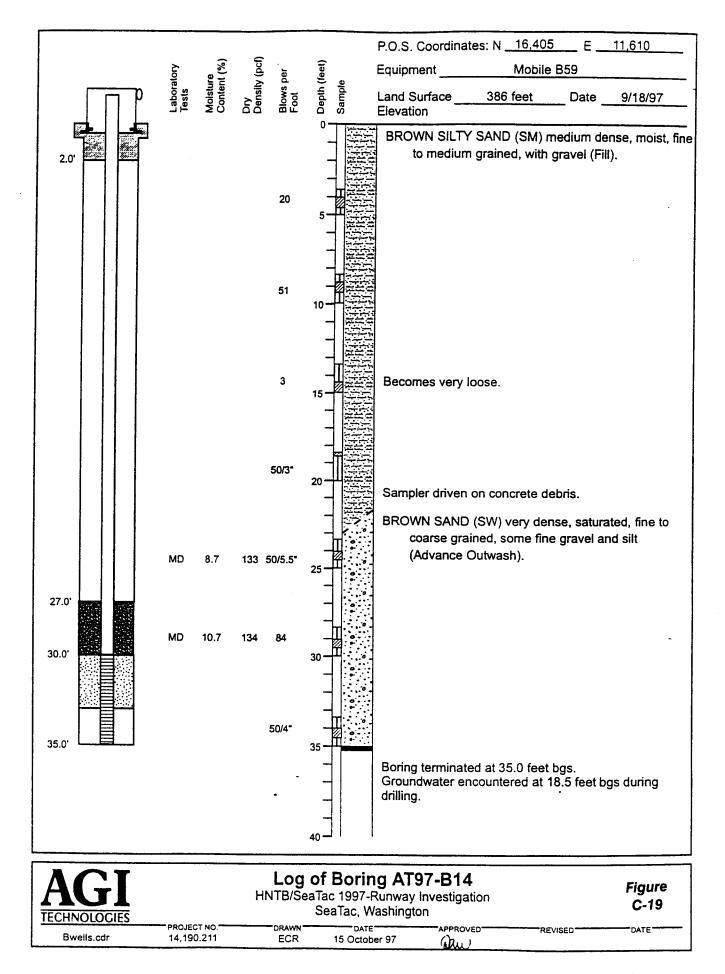
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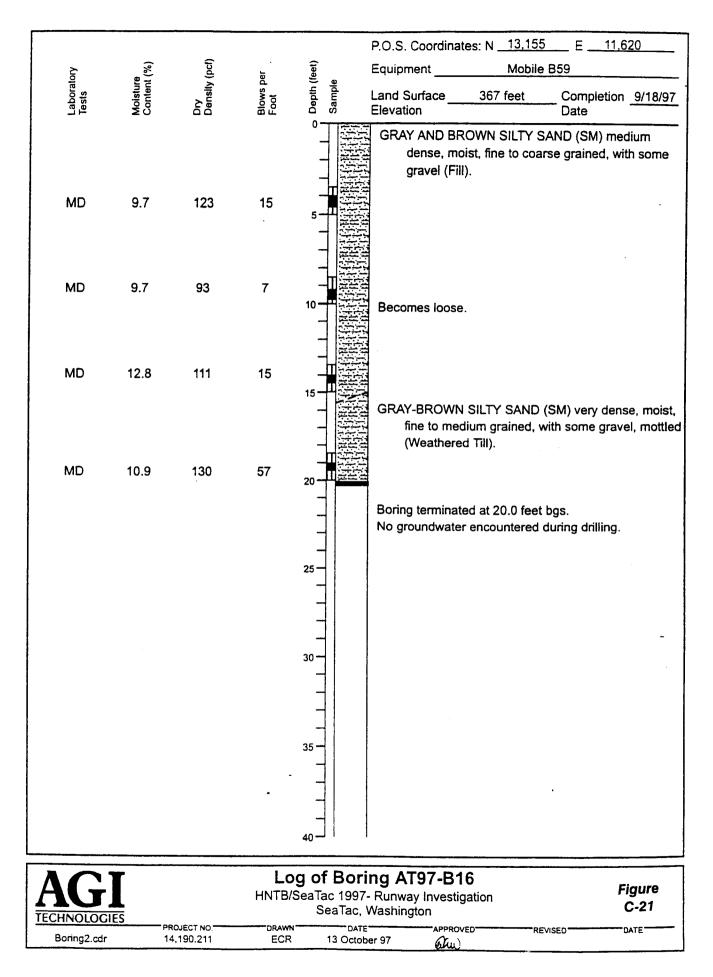


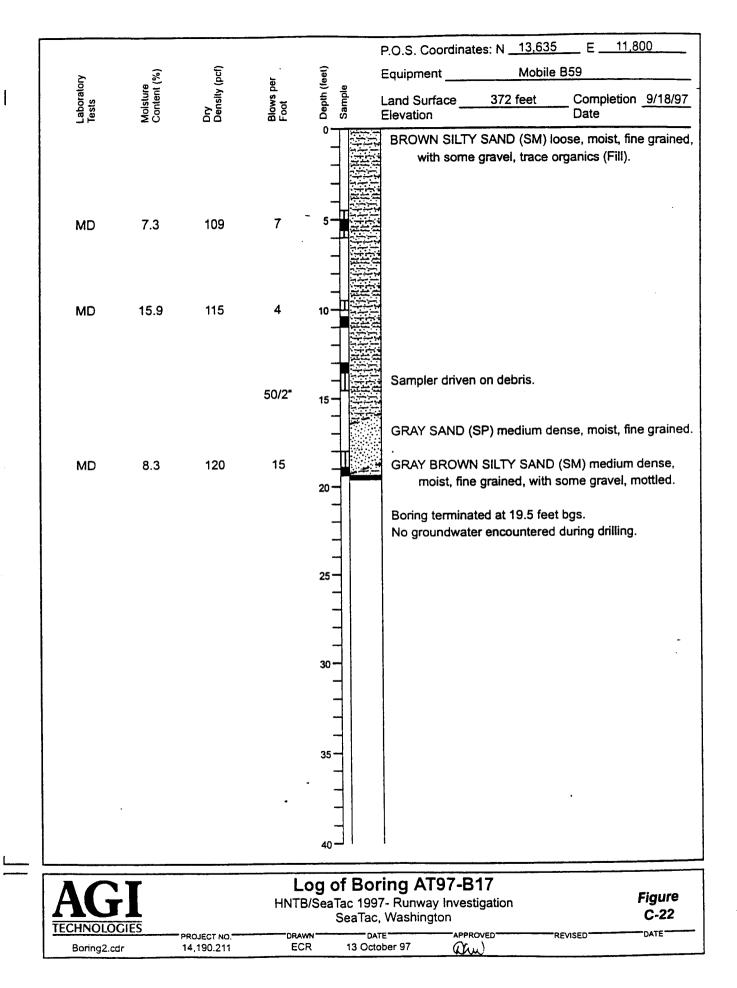


					P.O.S. Coordinates: N 13,010 E 11,200
Ž	(%)	pc()	5	eet)	Equipment Mobile 859
Laboratory Tests	Moisture Content (%)	Dry Density (pcf)	Blows per Foot	Depth (feet) Sample	Land Surface <u>359 feet</u> Completion <u>9/18/97</u> Elevation Date
MD	11.8	116	15	0 	Topsoil 12". BROWN SILTY SAND (SM) medium dense, moist, fine grained, with some gravel and asphalt (Fill).
			5		Becomes loose.
MD	11.4	120	3		Becomes very loose.
MD	13.9	105	4		
MD	8.2	112	34	25 - 1	 BROWN SAND (SP) dense, moist, fine grained, with some silt (Advance Outwash). Boring terminated at 25.5 feet bgs.
					No groundwater encountered during drilling.
			-		
AG	IES		HNTB	/SeaTac 199 SeaTac,	ing AT97-B15Figure7- Runway InvestigationC-20WashingtonC-20
Boring2.cdr		ROJECT NO. 4,190.211	DRAW ECF		

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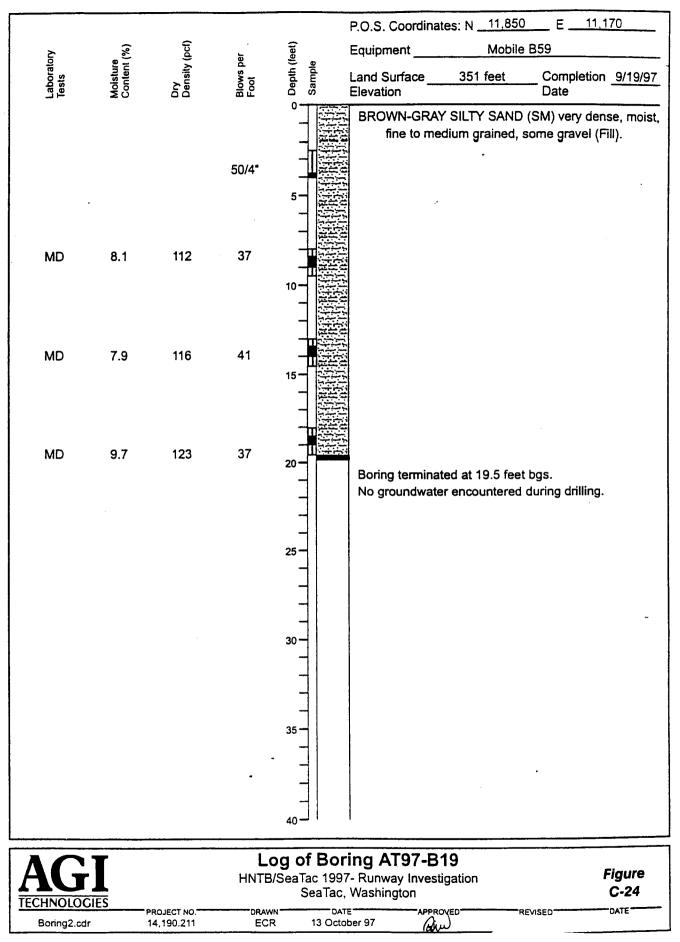
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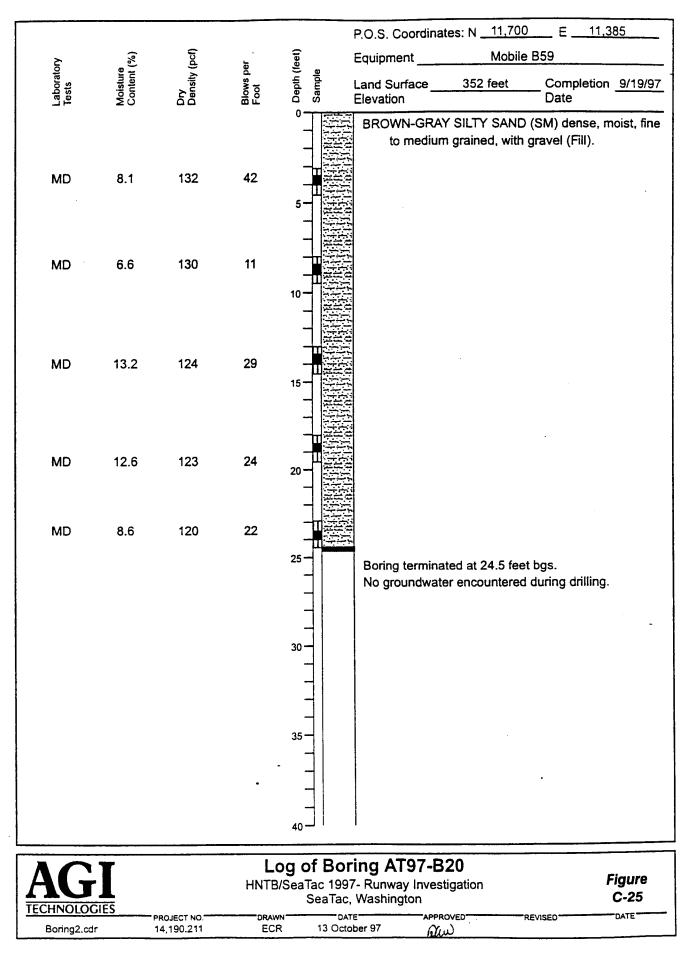


	-				P.O.S. Coordinates: N 11,605 E 11,250
Σ	(%)	(pcf)	*	aet)	Equipment Mobile B59
L aboratory Tests	Molsture Content (%)	Dry Density (pcf)	Blows per Foot	Sa De	Land Surface <u>350 feet</u> Completion <u>9/19/97</u> Elevation Date
MD	9.4	122	24		BROWN GRAY SILTY SAND (SM) medium dense, dry, fine to medium grained, some gravel (Fill).
MD	8.5	122	31		With coarse gravel.
MD	12.3	113	24		GRAY SAND (SP) medium dense, moist, fine grained.
MD	9.0	121	31		GRAY AND BROWN SILTY SAND (SP) dense, moist, fine grained, trace gravel.
MD	16.2	113	24		
MD	12	120	24		-
MD	9.4	131	21	35 - - -	Boring terminated at 34.5 feet bgs. Groundwater encountered at 23.0 feet bgs during drilling.
AG]	<u> </u>	<u> </u>		/SeaTac 1997 SeaTac,	ng AT97-B18 Runway Investigation Figure Vashington C-23
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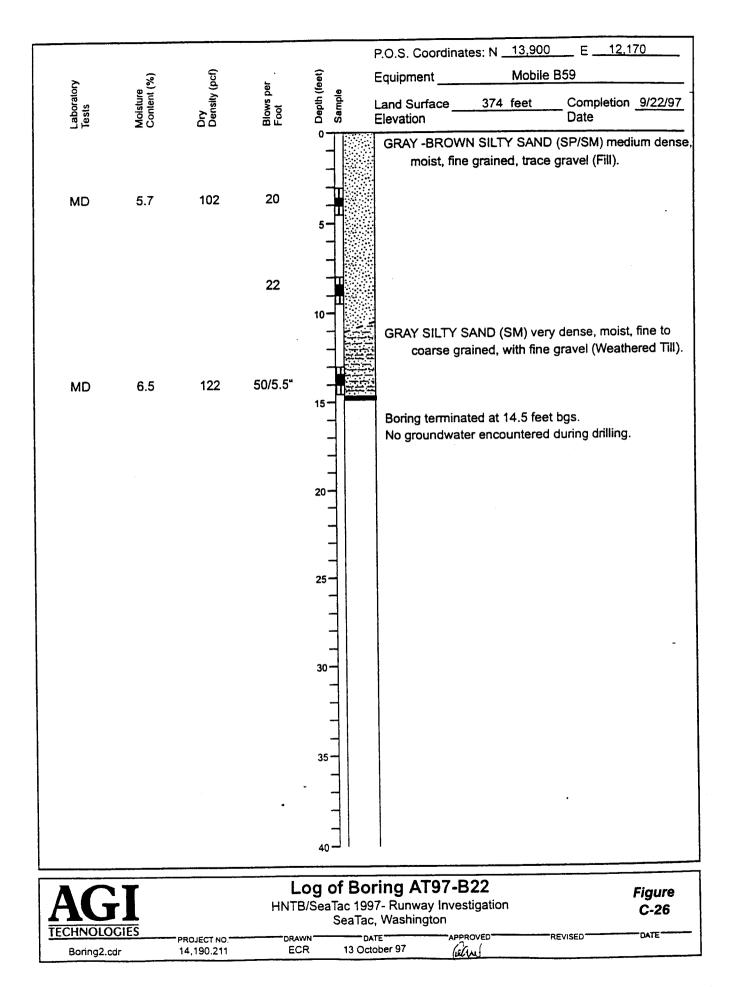
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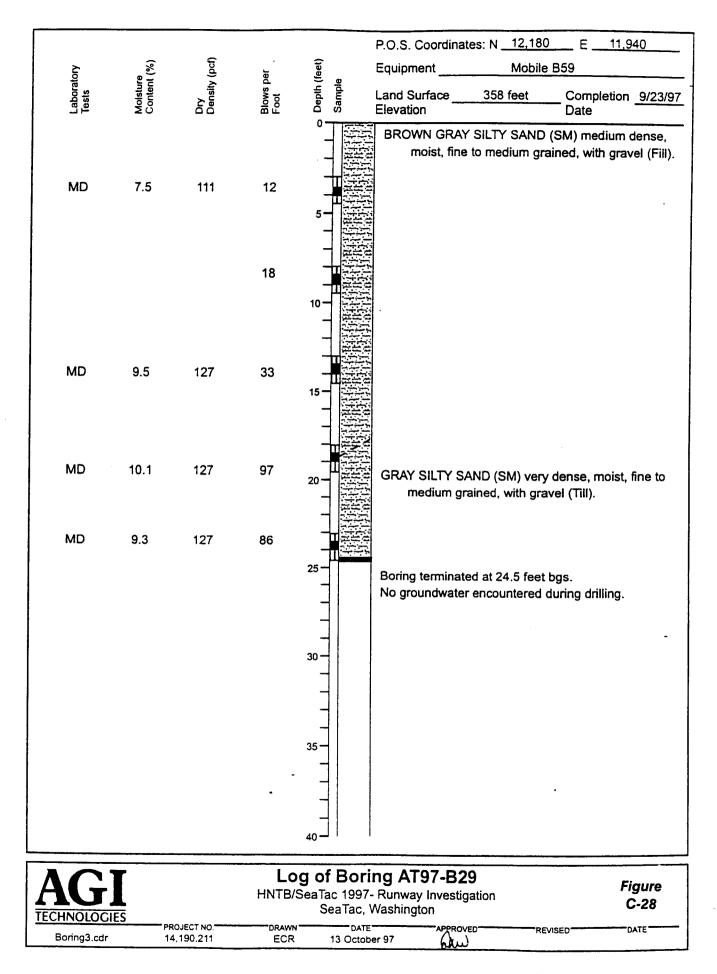
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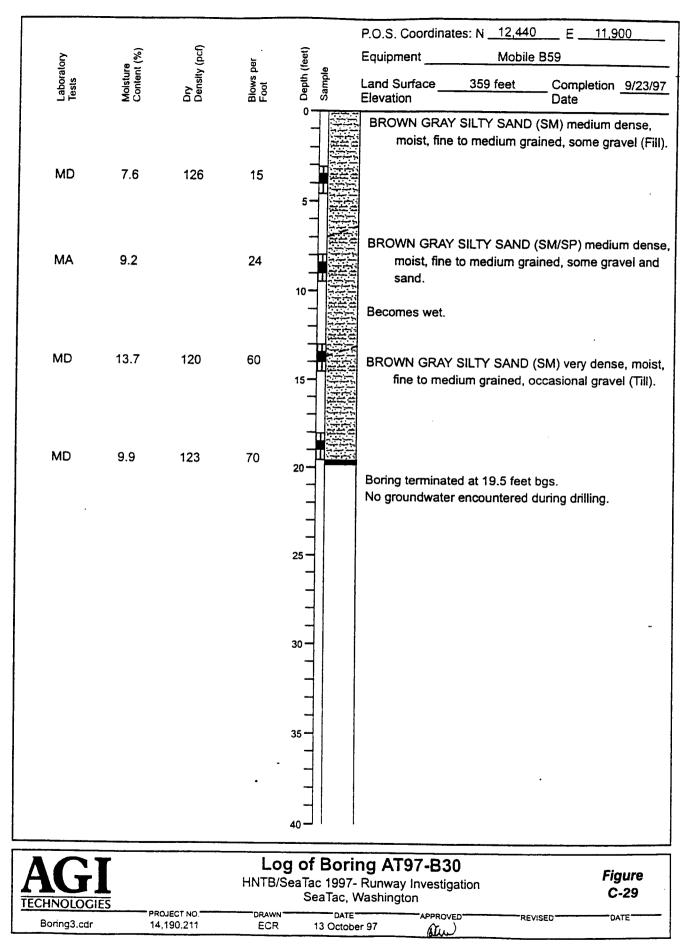


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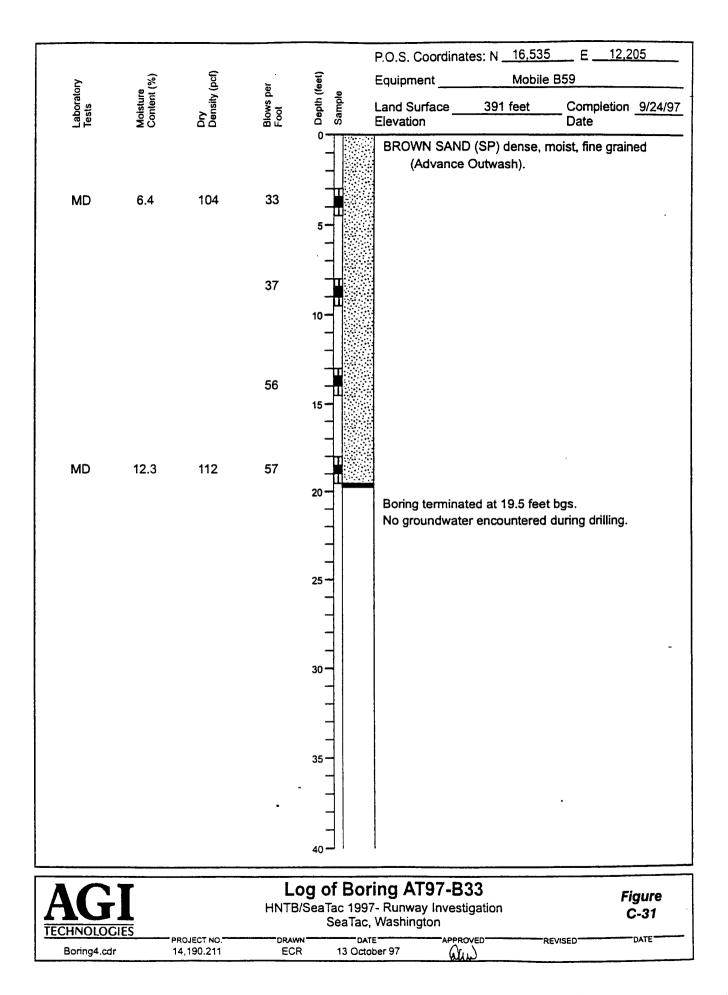


P.O.S. Coordinates: N 13,300 E	12,025
Land Surface 367 feet Comple	etion <u>9/22/97</u>
MD 9.6 127 20	
MD 10.4 121 21 1 10-1 	
MD 7.2 122 37 -	
MD 8.7 127 57	
MD 7.8 131 50/5" CRAY SILTY SAND (SM) very dense, m medium grained, some gravel (Till). Boring terminated at 24.5 feet bgs. No groundwater encountered during dril 30 40 40 40 40 40 40 40 40 40 4	
Log of Boring AT97-B23	Flaure
HNTB/SeaTac 1997- Runway Investigation	Figure C-27
TECHNOLOGIES SeaTac, Washington	

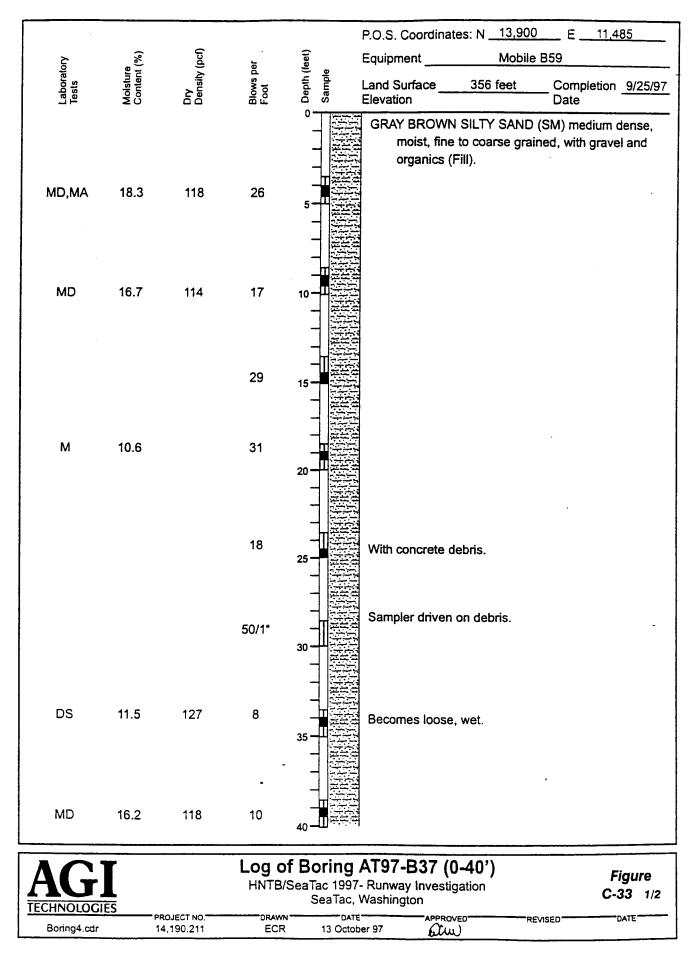


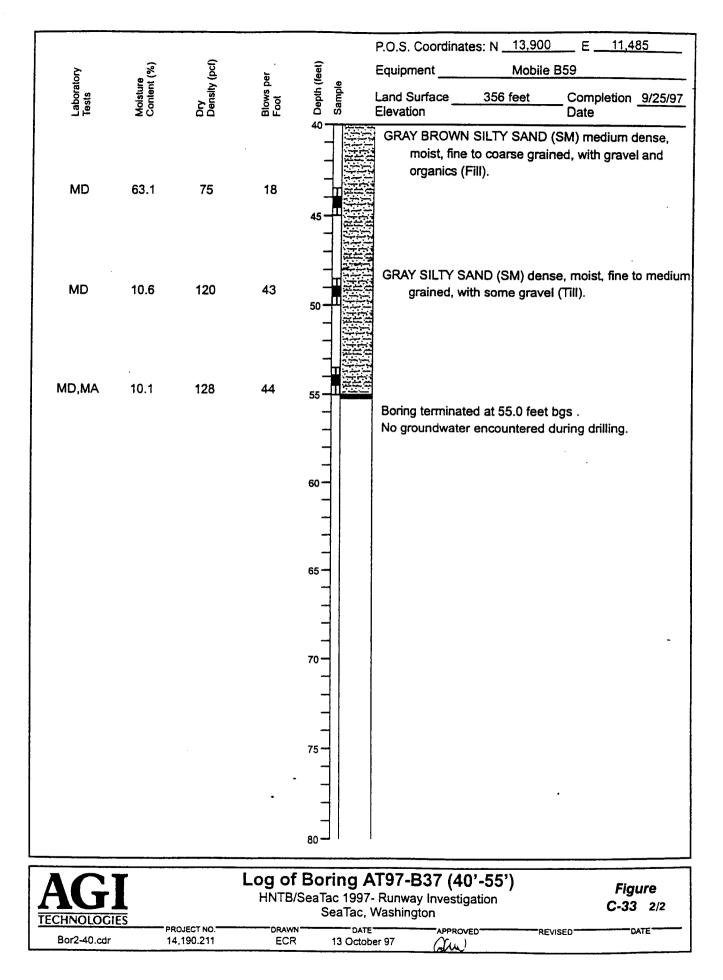


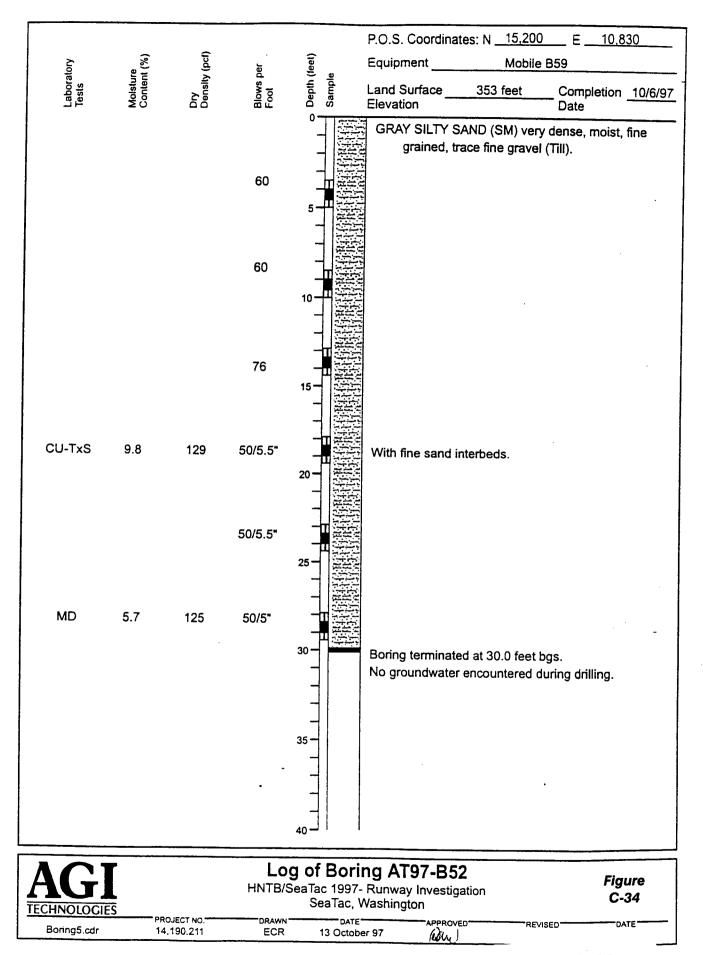
	_	~		-	P.O.S. Coordinates: N <u>12,875</u> E <u>11,370</u>
tory	re it (%)	/ (pcf	ber	(feet) ª	Equipment Mobile B59
Laboratory Tests	Moisture Content (%)	Dry Density (pcf)	Blows per Foot	Depth (feet) Sample	Land Surface 362 feet Completion 9/23/97
	Σŏ	۵ă		ດັ່ທັ 0 11=:55	Elevation Date
					BROWN SILTY SAND (SM) medium dense, moist, fine
					to medium grained (Fill).
MD,MA	5.9	115	24		
				5-1	
				-	
MD	10.1	90	4		
				10	Contains organics and wood debris, becomes loose.
MD	25.2	91	5		
				15-	
					GRAY BROWN SILTY SAND (SM) very dense, moist,
MD	9.6	92	72		fine to medium grained, some gravel (Till).
				20-	Boring terminated at 19.5 feet bgs.
					No groundwater encountered during drilling.
				_	
				25 -	
	i				
					-
				30-	
:					
				35 -	
			•		
				₄0 <u> </u>	
	T		L	og of Bor	ing AT97-B31
AL			HNTB	/SeaTac 1991	- Runway Investigation
TECHNOLOGI	ES				
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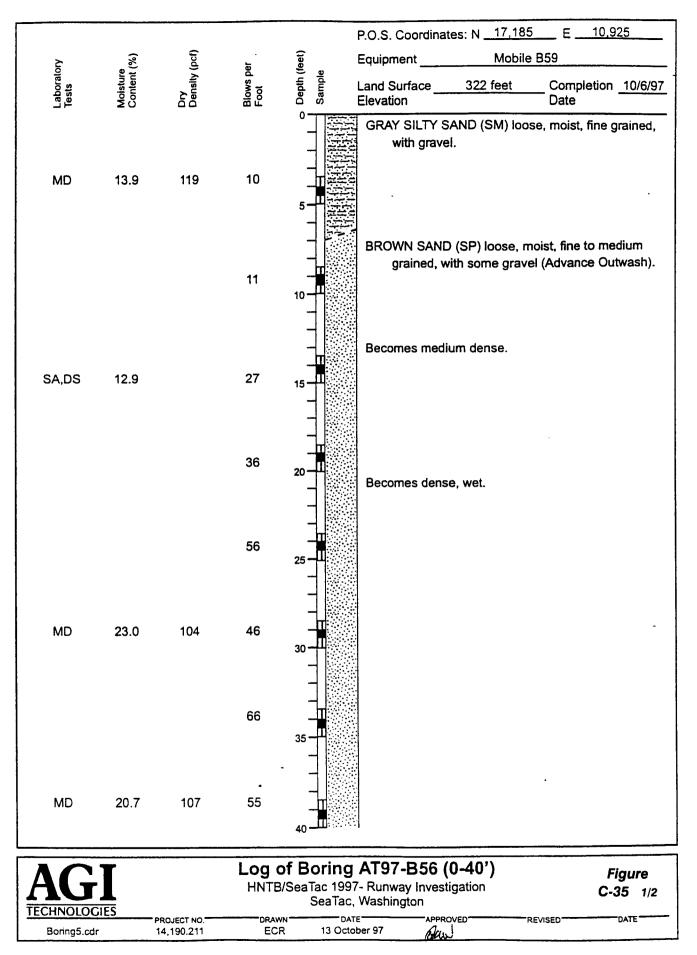


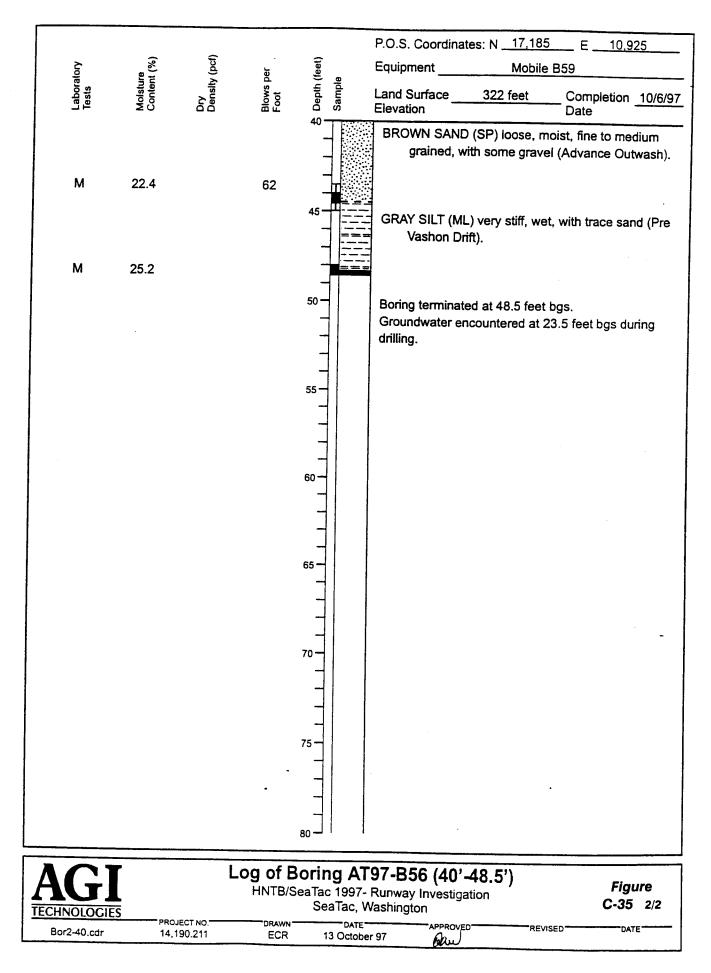
					P.O.S. Coordinates: N 11,870 E 10,870
	•	Ë		Q.	
atory	ure nt (%	ty (pc	s per	lee Je	Equipment Mobile B59
Laboratory Tests	Moisture Content (%)	Dry Denslly (pcf)	Blows per Foot	Depth (feet) Sample	Land Surface 350 feet Completion 9/25/97 Elevation Date
	20			0	BROWN GRAY SILTY SAND (SM) dense, moist, fine
					to medium grained, with gravel (Fill).
			38		
			30	5-1	
MD	14.4	122	31		CRAX SAND (SR) dense major with some silt (Fill)
	17.7	122	51	10-11	GRAY SAND (SP) dense, moist, with some silt (Fill).
				- 2	DARK GRAY BROWN SILTY SAND (SM) medium
					dense, moist, fine to medium grained, with grave
MD	18.2	118	24		and organics (Fill).
	10.2			15-11-22	
			13		
				20 - 11	
					GRAY SILTY SAND (SM) very dense, moist, fine to
	- <i>i</i>				coarse sand, some gravel (Till).
MD,MA	8.4	122	57	25-11	
				-	
				-	
MD	11.7	120	50/6"		
NVID	11.7	120	50/0	30 - 11	
					Boring terminated at 30.0 feet bgs.
					No groundwater encountered during drilling.
				35 -	
				41	
			-		
				40 J	
L	······			·•	
	T				ing AT97-B36 Figure
	1		HNTB		7- Runway Investigation C-32
TECHNOLOG		PROJECT NO.	DRAW		-
Boring4.cdr		14,190.211	ECI		

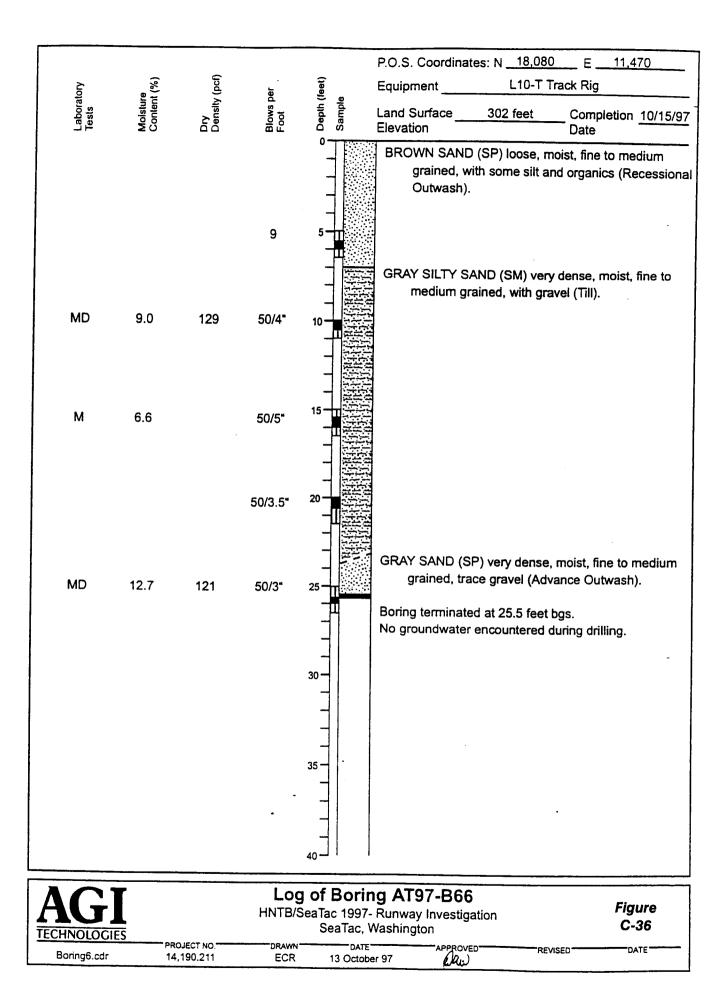


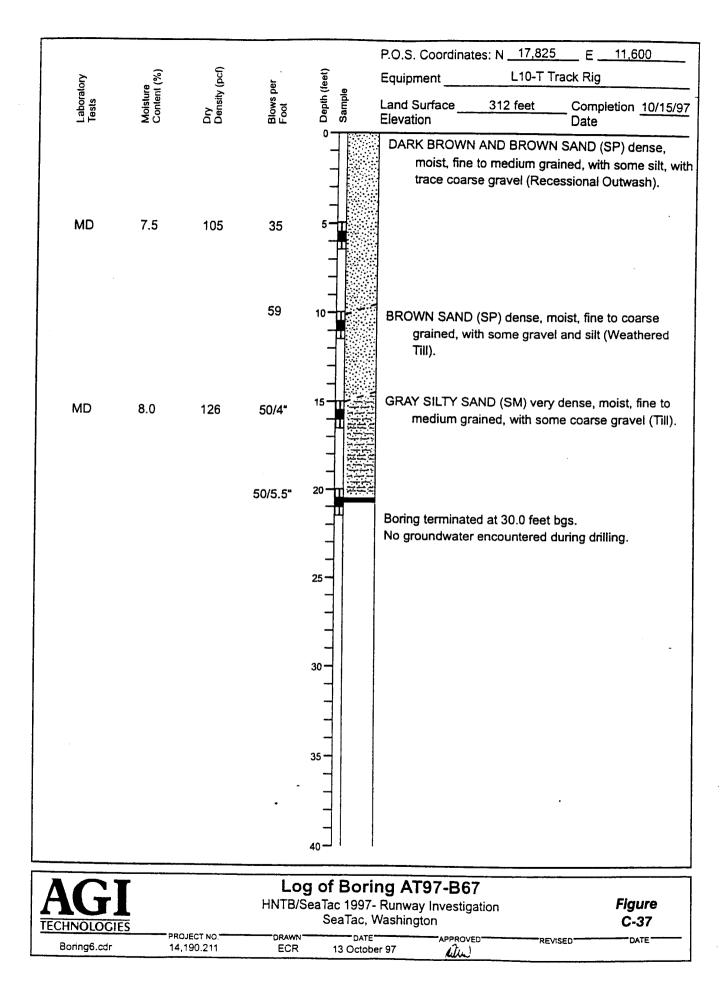




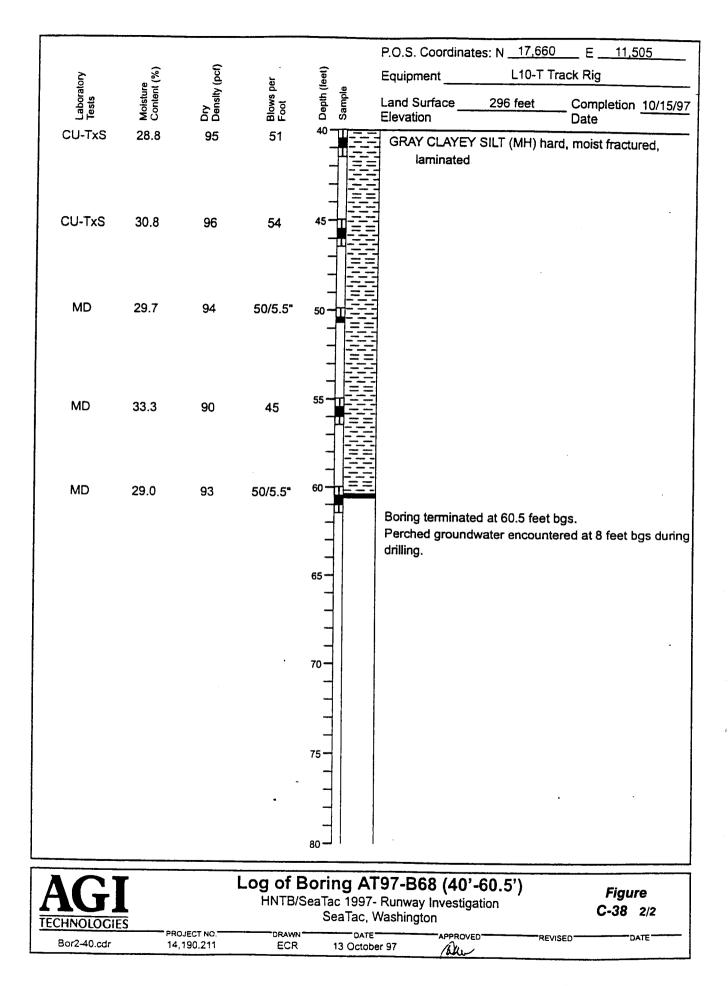


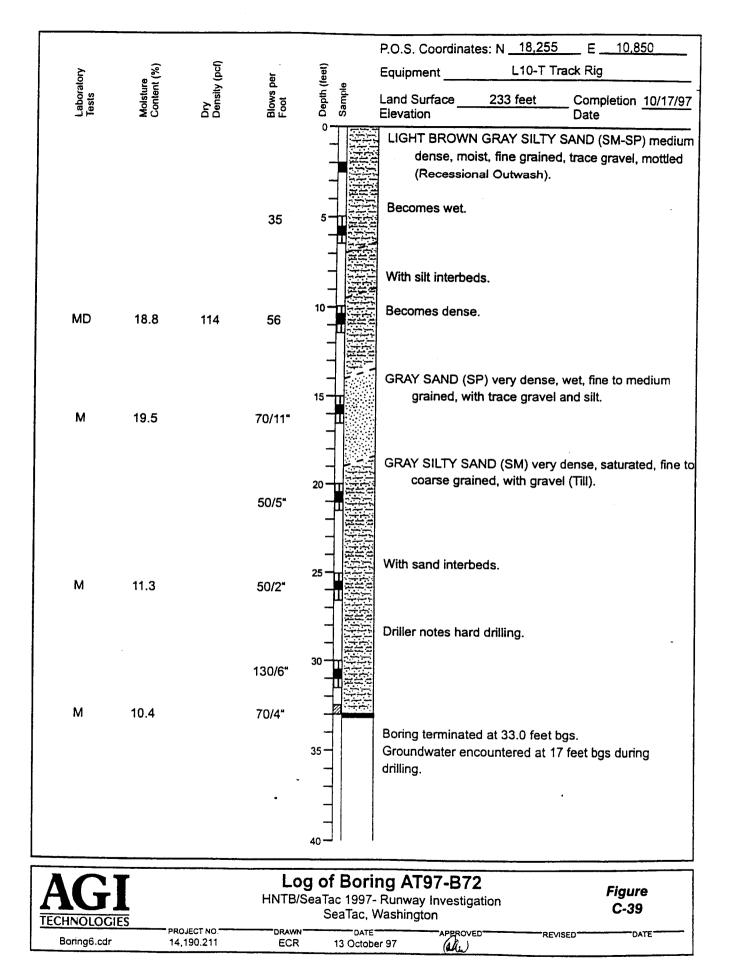




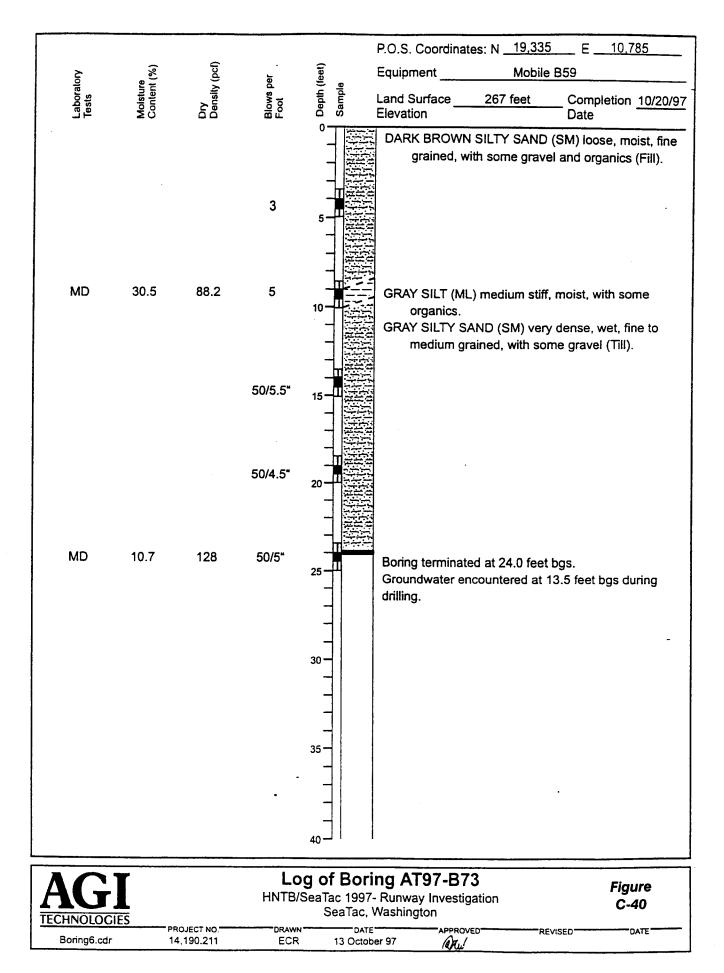


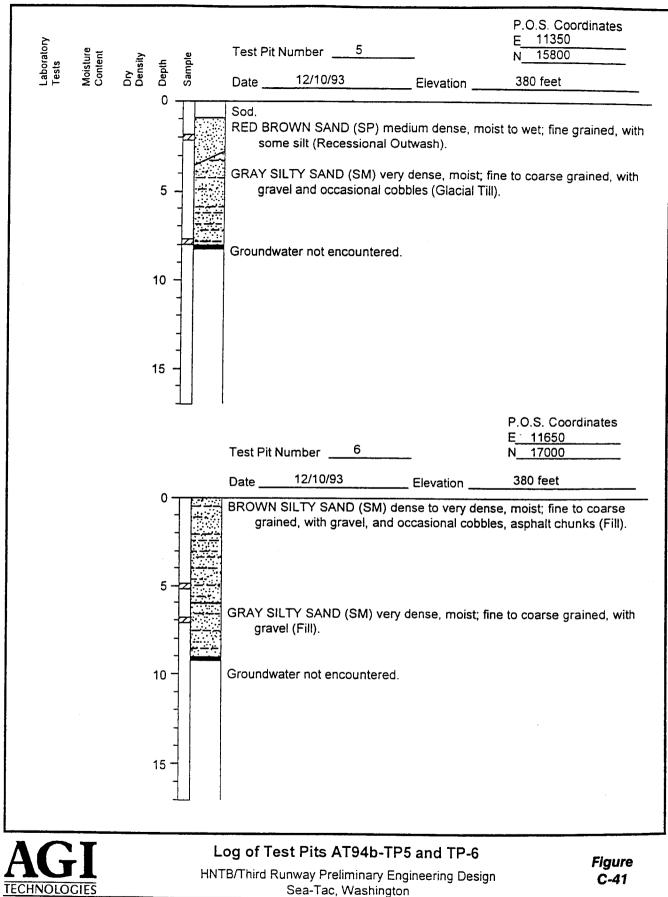
				P.O.S. Coordinates: N <u>17,660</u> E <u>11,505</u>
	(%	cl)		
Laboratory Tests	Molsture Content (%)	Dry Density (pcf)	Blows per Foot	Elevation 296 feet Date
				BROWN SILTY SAND (SM) dense, moist, fine to medium grained (Advance Outwash).
MD	13.5	120	47	5 - Transfer - Transfer - Transfer - Transfer - Transfer - Becomes saturated.
MD	16.9	111	28	GRAY SILT (ML) hard, moist, with some fine sand interbeds (Pre Vashon Drift).
MD	17.8	111	44	
			66	20 II II GRAY CLAYEY SILT (MH) hard, moist, fractured, Iaminated.
MD	27.1	95	54	
			50/4"	
CU-TxS	32.1	89	46	
AG	I			of Boring AT97-B68 (0-40') Figure B/SeaTac 1997- Runway Investigation C-38 1/2 SeaTac, Washington
TECHNOLOC Boring6.cdr		PROJECT NO.		AWN DATE APPROVED REVISED DATE DATE





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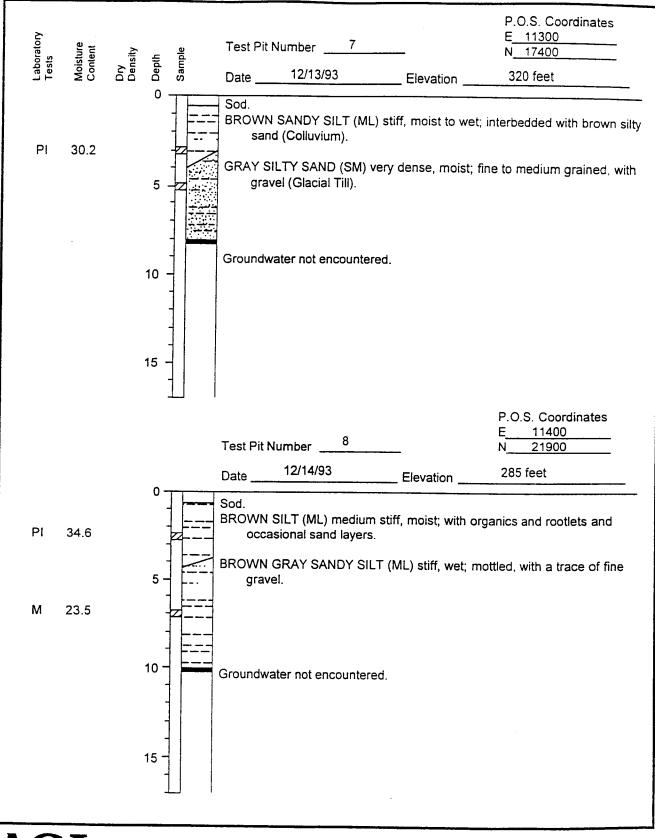
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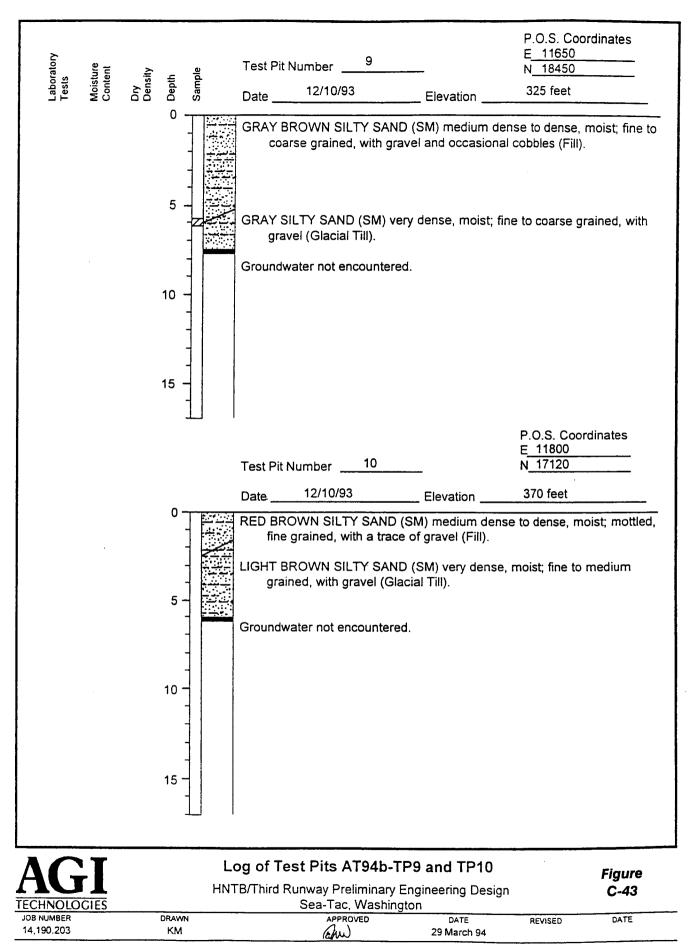
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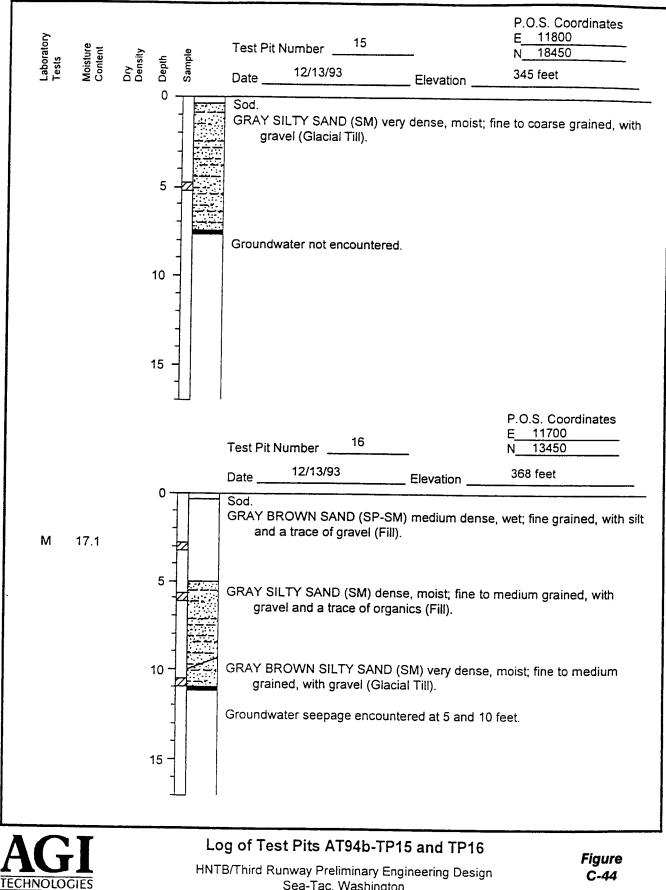
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Log of Test Pits AT94b-TP7 and TP8 Figure HNTB/Third Runway Preliminary Engineering Design C-42 TECHNOLOGIES Sea-Tac, Washington JOB NUMBER DRAWN APPROVED DATE REVISED DATE 14,190.203 KM 29 March 94 anu 190203TP.p65



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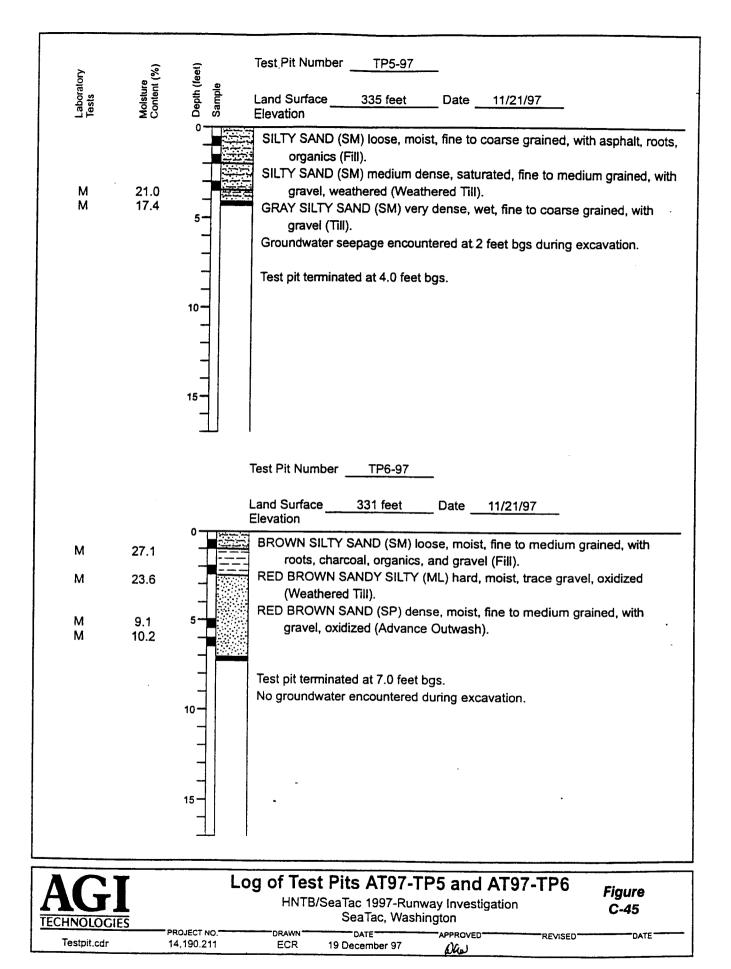
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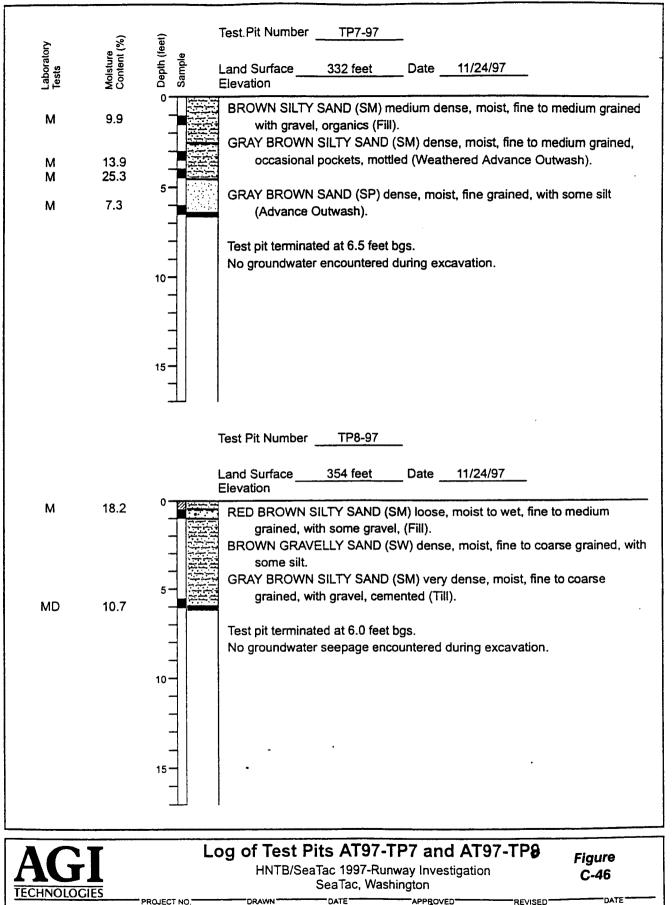
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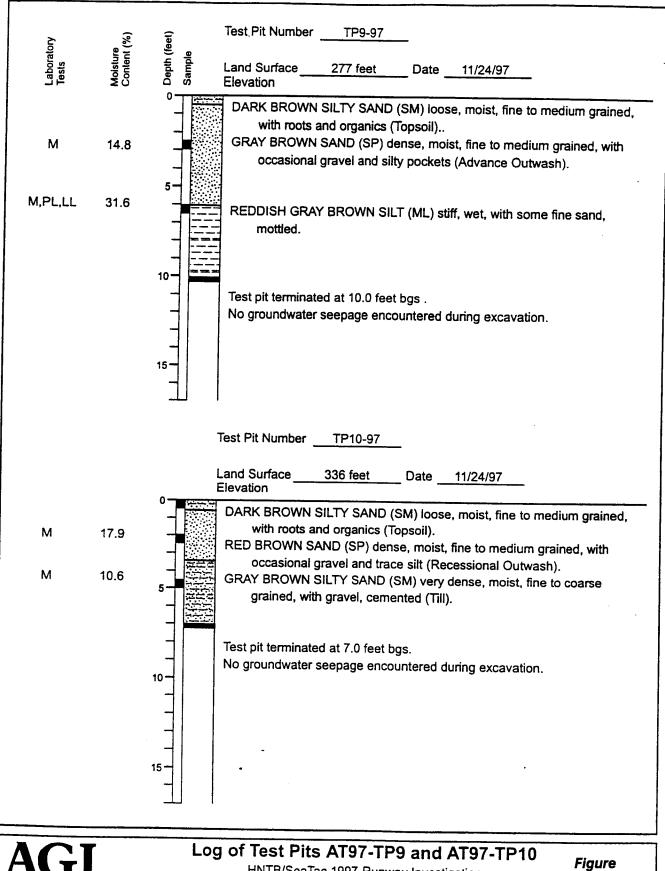
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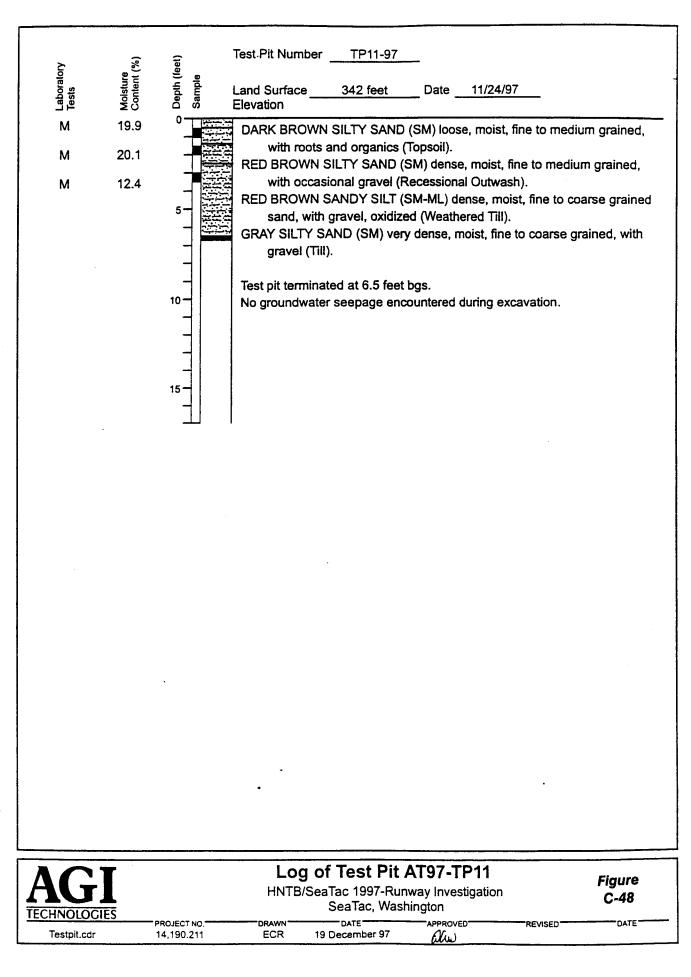
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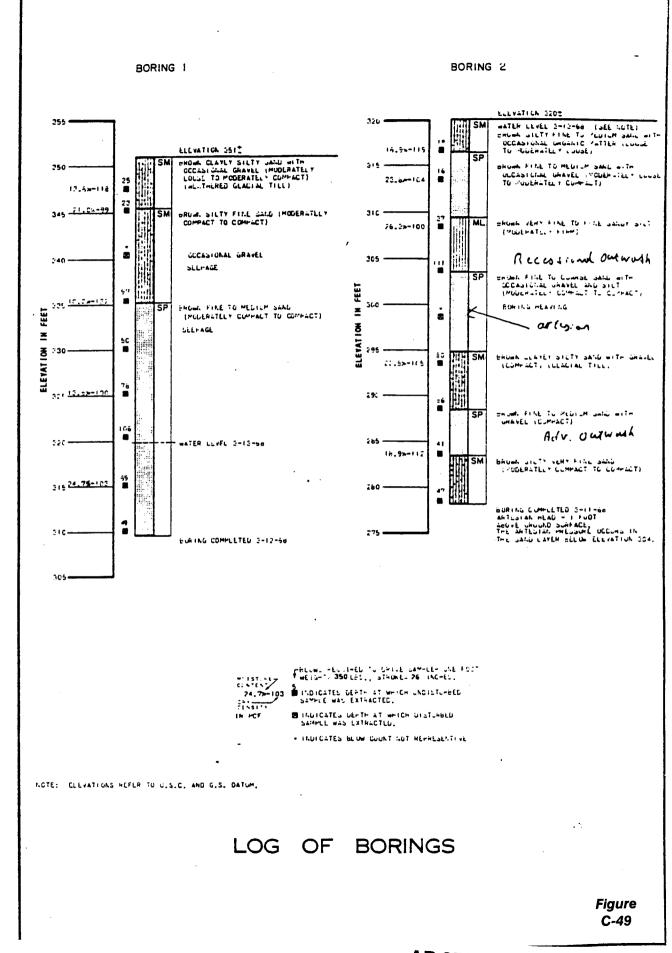
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TECHNOLOGIES		HNTB	Figure C-47		
Testpit.cdr	PROJECT NO. 14,190.211	ECR	19 December 97	APPROVED REVISED	DATE





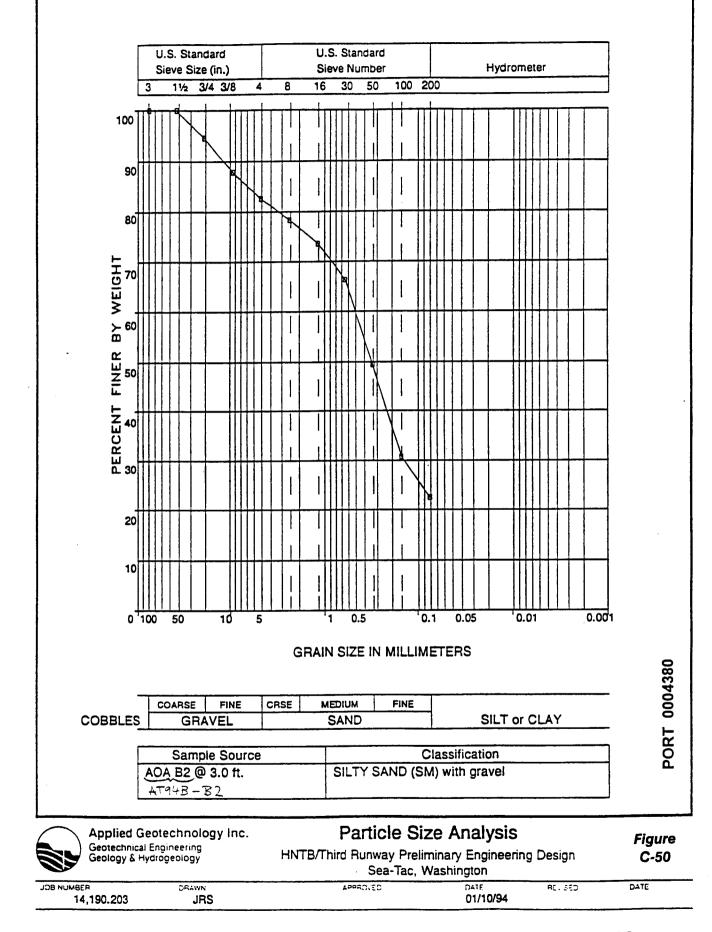
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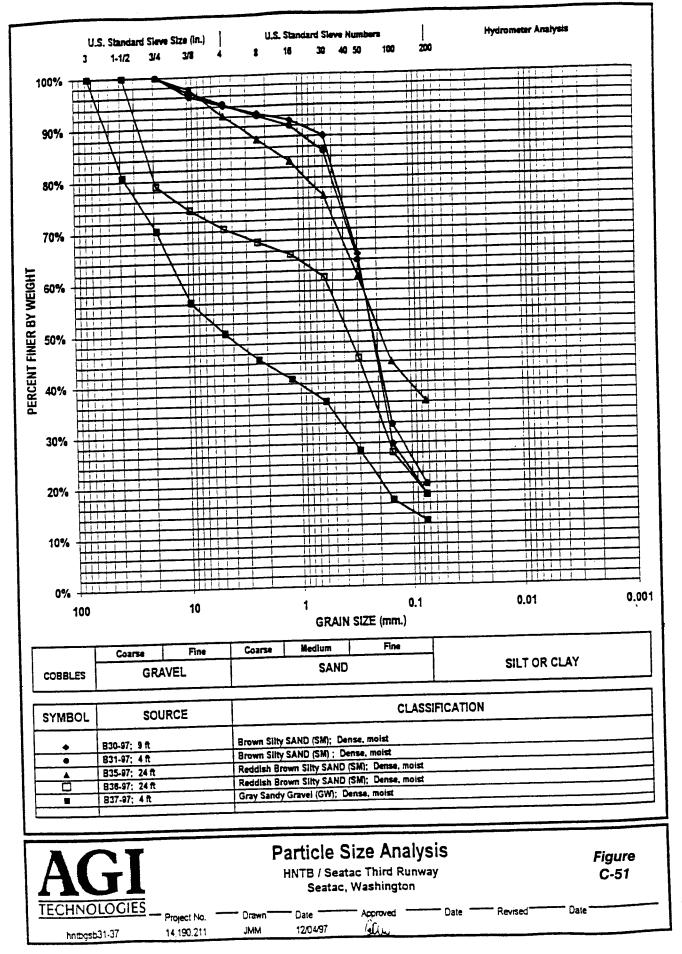
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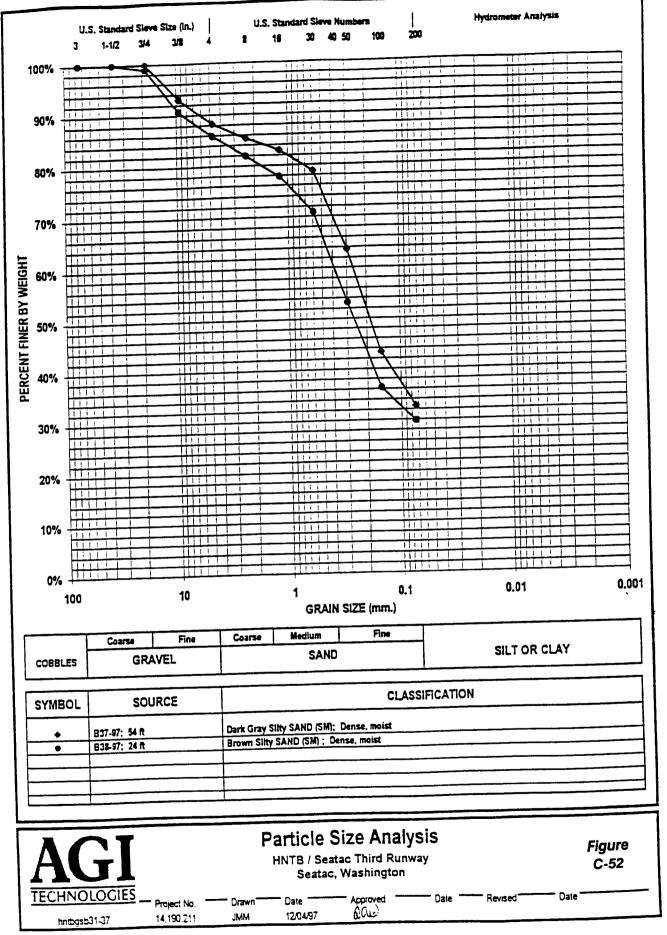
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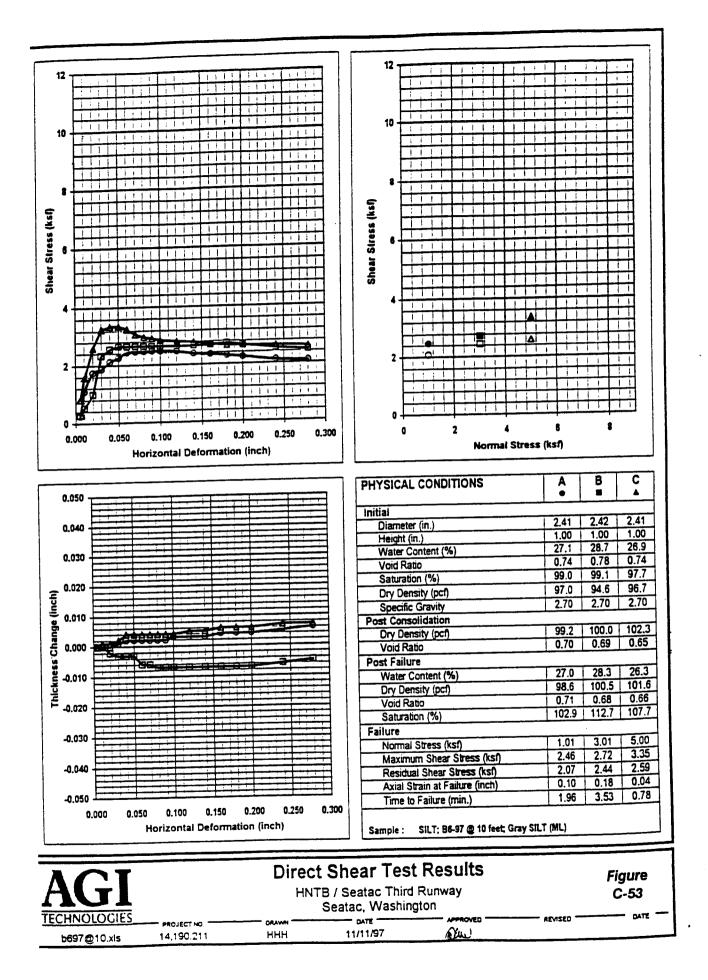
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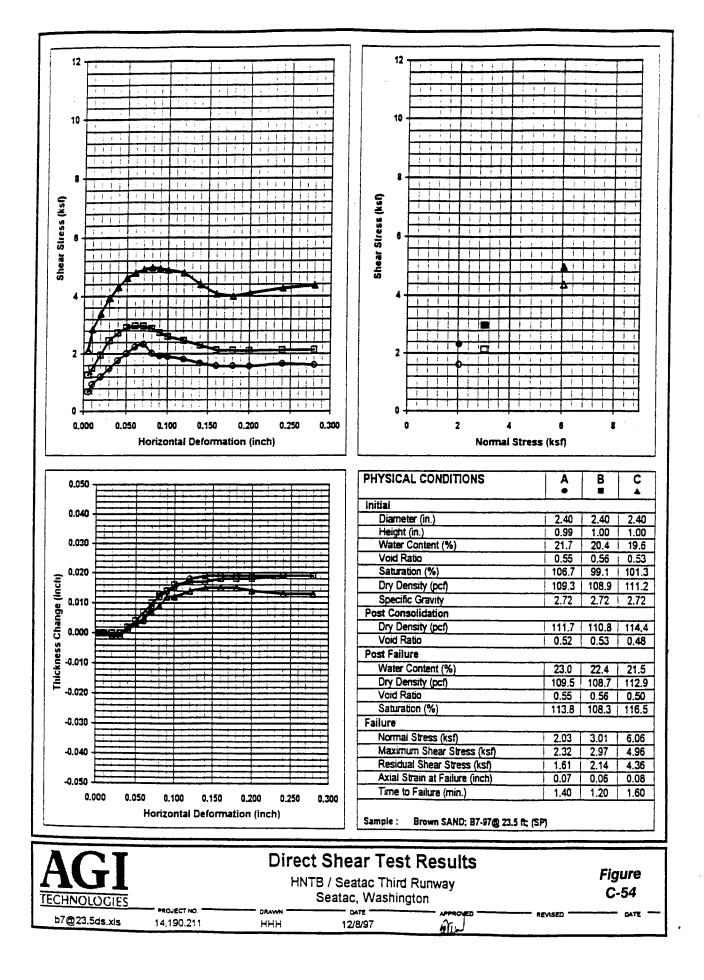


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APPENDIX D EXTENT AND CHARACTERISTICS OF PEAT IN THE NORTH SAFETY AREA AND PHASE 5 WORK AREA

Hart Crowser 4978-28 September 21, 2001



MEMORA	NDUM	Anchorage
DATE:	July 10, 2001	
TO:	Third Runway File	Boston
FROM:	Michael Bailey, P.E., Carsten Becker, E.I.T., Hart Crowser, Inc.	
RE:	Extent and Characteristics of Peat in the North Safety Area and Phase 5 Work Area (West Wall) 4978-42	Chicago

This memo summarizes information on peat soils that will be encountered during construction of the Third Runway embankment at Seattle-Tacoma International Airport. Exploration data from borings, test pits, hand-auger holes, and cone penetration tests have been used to identify and locate peat deposits along the proposed Third Runway MSE walls and embankments. Classification tests, field vane tests, direct shear, and consolidation tests were conducted to better characterize the peat in the Phase 5 Work Area (West Wall) and North Safety Areas.

EXTENT OF EXISTING PEAT

Peat soils observed range from relatively fibrous residue of more or less decomposed plant material intermixed with sand particles ranging from gravel and sand to clay in size, to predominantly silt (or clay) soils that have non-fibrous, highly decomposed residual organics. Classification testing along the West Wall and the North Safety Area revealed some differences in composition. In general, the peat samples obtained in the NSA have higher organic and fiber contents than the peat from the West Wall area. The buried West Wall peat appears to have a significant sand fraction ranging from sandy to very sandy, while the peats in the NSA are practically non-sandy.

Deposits of peat were encountered in explorations along the proposed West Wall and in the North Safety Area. The following paragraphs summarize the location and thickness of the peat deposits. In some areas, shallow peat deposits were encountered. In other areas, layer(s) of peat is interbedded with other soil layers. The locations of the peat and organic silt deposits in the North Safety Area and West Wall Area are shown on Figures D-1 and

1910 Fairview Avenue East Seattle, Washington 98102-3699 Fax 206.328.5581 Tel 206.324.9530

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Seattle

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Portland

Denver

Fairbanks

Jersev City

Juneau

Long Beach



D-2, respectively. Data concerning peat deposits in these same areas are presented in Tables D-1 and D-2, respectively.

West Wall

Peat deposits along the West Wall ranged from soft to very stiff. Peat was generally observed around Station 180+00 where a depression in the topography coincides with the area designated as Wetland 37a. Runoff water from higher elevations to the east collects in a small unpaved drainage channel that runs along the bottom of the depression and is transported to Miller Creek downslope.

Buried Peat Deposits along the West Wall

Peat deposits buried underneath deposits of sands were encountered in explorations near Station 180+00 (west of the new runway). These deposits were encountered at depths ranging from 3.5 to 9.5 feet and varied in thickness between 1.5 and 5.5 feet. Based on observations in explorations in this area, it appears that buried peat deposits might be located in an area up to about 200 feet wide, between Stations 179+20 and 181+20.

SPT blow counts in the buried peat deposits near Station 180+00 typically indicate consistencies ranging from medium stiff to very stiff (N=10 to 30). To physically measure undrained shear strengths, Hart Crowser conducted *in situ* field vane tests. Undrained shear strengths of 1,044 and 1,607 psf were measured, which correlates with a consistency range of upper-bound medium stiff to stiff cohesive material.

In two borings (HC 00-B123 and HC00-B132) located below the proposed MSE wall near Station 179+00, consistency of the buried peat deviates from conditions encountered in adjacent borings. In these borings, the top 9 to 11 feet of soil consists of loose sand with interbedded layers of soft peat and silt.

Shallow Peat Deposits along the West Wall

The majority of peat deposits near the ground surface were encountered between Runway Stations 178+40 and 183+30 with deposit thickness typically varying between a few inches and about 2 feet. As mentioned in the previous section, in boring HC00-B132 soft peat layers were encountered to a depth of 9 feet.

While in most areas along the West Wall shallow peat deposits appear to be scattered and localized, there is an increased abundance of shallow peat in Wetland 37a near Station



179+00, just north and south of the unnamed drainage that carries water toward Miller Creek.

The shallow peat deposits in the wetland areas generally appear to be soft and highly compressible. Field vane tests were conducted in two locations in the wetlands near Station 179+00. The undrained shear strengths were determined to range from 167 to 459 psf.

Peat also was encountered in five explorations along the West Wall north of Station 183+30. No peat was encountered in explorations along the West Wall north of Station 192+80. The peat deposits scattered between Stations 183+30 and 192+80 generally appear to be isolated deposits. In these deposits, the thickness of the peat ranges from a few inches to about 3 feet.

North Safety Area

Peat deposits in the North Safety Area were encountered in the footprint of the proposed embankment and MSE Wall area, as well as to the west and north of the proposed fill in the area of the proposed Miller Creek floodplain improvements. These deposits were generally characterized as very soft to soft peat, based on SPT blow counts typically ranging from 1 to 2.

Peat Deposits in the Miller Creek Floodplain

In the North Safety Area, peat extending to depths greater than 9 feet was encountered to the west and north of the proposed embankment fill. In most of these areas, 1 to 4 feet of silty sands overlie the peat. The only structure that is proposed in this area is a sewer that will cross underneath Miller Creek. This work will require trenching in the peat

Hart Crowser conducted field vane tests to measure undrained shear strengths of the peat. The *in situ* undrained shear strength of the peat near the proposed sewer ranges from 626 psf at a depth of 3.5 feet to values of 313 and 375 psf at greater depths of up to about 8 feet. The undrained shear strength of the peat generally decreased with depth, possibly as a result of dessication and surface compaction associated with previous farming in this area.

Hong West also conducted field vane tests in the Miller Creek floodplain (Hong West 1998). However, their report does not clearly state what soil types the tests were performed on. Many of the adjacent explorations encountered organic silts at the depths the tests were performed. Field vane tests were performed at depths between 8 and 10



inches. Reported peak undrained shear strengths ranged from 426 to 576 psf. Residual undrained strengths varied between 108 and 150 psf.

Peat Deposits below the Proposed Embankment and MSE Wall

Peat deposits to depths of up to 10 feet below ground surface were encountered in borings in the area of the proposed embankment and MSE wall. The *in situ* undrained shear strength of the peat measured in this area ranged from 292 to 542 psf. A test conducted on a shallow organic silt deposit exhibited an undrained strength of 835 psf. The undrained shear strength of the peat generally decreased with depth.

PEAT CLASSIFICATION

Peat samples were visually classified using the Unified Soil Classification System for exploration log descriptions. Selected representative samples were also classified in general accordance with ASTM D 4427. We determined the following characteristics: Moisture content (ASTM D 2974), fiber content (ASTM D 1997), ash/organic content (ASTM D 2974), and acidity (ASTM D 2976); botanical composition and absorbency were not determined.

West Wall Area

Buried Peat Deposits along the West Wall

Peat samples obtained in the West Wall area at depths between 3.8 to 7 feet were visually classified as stiff, moist, dark brown, sandy peat (see Table D-4). Two of three samples were slightly gravelly.

Laboratory classification testing revealed moisture contents varying between 72 and 132 percent, fiber contents ranging from 4 to 12 percent, and organic contents between 9 and 18 percent. In general accordance with ASTM D 4427, the samples were classified as sapric, high ash, slightly to moderately acidic peat (see Table D-3).

Shallow Peat Deposits along the West Wall

Peat samples obtained near the ground surface at depths between 4 inches and 2.1 feet were significantly softer than the samples from the buried deposits. The shallow samples

also contained less sand and gravel than the deeper deposits. The shallow samples were visually classified as very soft to soft, moist to wet, dark brown peat (see Table D-4).

In general accordance with ASTM D 4427, the samples were classified as sapric, high ash, slightly acidic peat with moisture contents of 132 to 168 percent, fiber contents of 11 percent, and organic contents ranging from 18 to 24 percent (see Table D-3).

North Safety Area

In this area, samples were obtained in the Miller Creek floodplain and beneath the proposed embankment. The peat deposits were visually classified as soft to medium stiff, moist to wet, dark brown, fibrous peat (see Table D-4). One medium stiff organic silt sample was also obtained at a depth of about 10 inches. Organic silt was encountered at shallow depth only.

Generally, the peat obtained in the North Safety Area is more fibrous and has significantly higher organic contents and moisture contents than the peat from the West Wall area. Fiber contents ranged from 22 to 52 percent. Organic contents varied between 54 and 90 percent. Moisture contents ranged from 561 to 981 percent. In general accordance with ASTM D 4427, the peat in this area obtained at depths between 4.0 and 7.8 feet may be described as sapric to hemic, medium to high ash, slightly to moderately acidic peat (see Table D-3). The peat appears to become softer with depth, which may be attributed to varying water levels and higher degrees of desiccation near the ground surface.

STRENGTH CHARACTERISTICS

Hart Crowser conducted field vane tests in the West Wall and North Safety Areas this year to obtain *in situ* undrained shear strength values. To obtain drained ϕ -angles of the existing peat, two sets of direct shear tests were performed on undisturbed samples from the North Safety Area.

Field Vane Testing

Test results of the field vane testing in the North Safety Area and along the West Wall are shown in Table D-4. Table D-4 shows field vane results and classification test results along with additional information. It appears that the peat deposits in the West Wall area generally have higher strengths than the peat in the NSA, which might be related to the relatively lower organic content or lower moisture contents in the West Wall area.

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Direct Shear Testing

Two sets of direct shear tests were performed on samples from the North Safety Area at borings HC01-B33A/B and HC01-HB3A.

The direct shear tests were conducted on soft peat samples with fiber contents of 22 and 25 percent and organic contents of 54 and 61 percent. Both tests exhibited very similar drained ϕ -angles ranging from about 15 degrees at 2 percent strain to about 25 degrees at 8 percent strain (see Figures D-3 and D-4). Somewhat lower values (ϕ approximately 7 degrees) are observed in one sample at very low confining pressures.

Consolidation Testing

Consolidation test results for samples obtained in borings HC01-B33B and HC01-HB3A are shown on Figures D-5 and D-6, respectively. Results obtained from the data shown on the figures are summarized in the table below. The consolidation tests were conducted in accordance with ASTM D 2435 Test Method B.

Consolidation Properties

	Sample Depth in Feet	p _c ' in psf	σ _{v0} ' in psf	OCR	C _{CE}	Cc
HC01-B33B	6.5	660	307	2.1	0.58	5.0
НС01-НВЗА	7.2	480	396	1.2	0.51	5.8

Consolidation ratios (i.e., modified compression indexes) and compression indexes were determined in the upper portion of the virgin compression curves. A specific gravity of 1.6 was assumed to convert from consolidation ratios to compression indexes. At high strains, the virgin compression curves are no longer straight lines when plotted in the semi-log space. Consolidation ratios and compression indexes decrease at strains greater than about 50 to 60 percent.

Compression indexes for the samples from HC01-B33B and HC01-HB3A were estimated to be 5 and 5.8, respectively. The samples were estimated to be in the normally to slightly overconsolidated range with OCRs of 2.1 and 1.2

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REFERENCES

Hong West 1998. Geotechnical and Dewatering Evaluation, Miller Creek Relocation, SeaTac Airport, Washington, June 8, 1998.

Civiltech 1997. Geotechnical Report, South 154th Street/156th Way Relocation, Sea-Tac International Airport, SeaTac, Washington, October 27, 1997.

Attachments:

- Table D-1 Peat and Organic Silt in the North Safety Area
- Table D-2 Peat and Organic Silt along the West Wall
- Table D-3 Peat Classification Summary
- Table D-4 Summary Table Field Vane Results, Classification Testing Results, and

 SPT Blow Counts in Adjacent Borings
- Figure D-1 Peat and Organic Silt North Safety Area
- Figure D-2 Peat and Organic Silt West Wall Area
- Figure D-3 Direct Shear Test Results HC01-B33A/B
- Figure D-4 Direct Shear Test Results HC01-HB3A
- Figure D-5 Consolidation Test Results HC01-B33B
- Figure D-6 Consolidation Test Results HC01-HB3A

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(soft) (soft)

organic SILT organic SILT

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P-B1

CT98-B4 CT98-B5 CT98-B6 CT98-B8

HW98-TP7

1 soft

0 v.soft 0 v.soft 2 v.soft

- 7

0 0

260.5

264 264

3.5 2

3.5 2 2.5 3.5

2.5 3.5

261.5 260.5

264

264

262

304.5

297.5 273

359

344

248.7

265

280.5

298 307

300 308 306 264

300 306 306

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Layer

Exploration Number	Soil Description	N Value (consistency/density based on N values)	Depth of Top of Layer in Feet	Depth of Bottom of Layer in Feet	Layer Thickness in Feet	Ground Surface Elevation in Feet	Top of Layer Elevation in Feet	
NORTH SAFETY	NORTH SAFETY AREA - EXPLORATIONS IN MILLER CREEK FLOODPLAIN	LER CREEK FLOODPLAIN						
Shallow PEAT an	Shallow PEAT and Organic SILT							
HC99-B48	siSw/org (PEAT)	grab (loose)	0	ę	m	268	26.8	
HC99-B31	PEAT	1 v.soft	0	15	15	263.7	263.7	
HW98-TP1	organic SILT	(soft)	0	2	5	346	346	
HW98-TP2	organic SILT	(soft)	0	2	2	361	361	
HW98-TP3	organic SILT	(v.soft-soft)	0	1.5	1.5	500	000	
HW98-TP4	organic SILT	(soft)	0	-		274	274	
HW98-TP5	organic SILT	(soft)	0	2.5	2.5	283	283	
HW98-TP6	organic SILT	(soft)	0	7	7	300	300	

Table D-1 - Peat and Organic Silt in North Safety Area

(250.5) (250.5)

260 259 259

263 263 262

> >8.5 >8.5

> B0B B0B

-

4

6 4 6

(v.soft-soft) (v.soft-soft)

(soft)

259

254.6 252.1

262.6 254.6

264.6 264.6

8 2.5

10 12.5

9 2

push (m.dense & soft)

Buried PEAT and Organic SILT Deposits

HC00-B301 HC00-B301

11 stiff

siS & PEAT SILT & PEAT

organic SILT PEAT PEAT

HW98-HB1

HW98-HB1 HW98-HB2 Notes: BOB = Bottom of boring. Boring was not drilled through layer. Layer is thicker than distance between BOB and top of layer.

Sheet 2 of 3

Table D-1 - Peat and Organic Silt in North Safety Area

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2 3 3	Exploration Number	Soil Description	N Value (consistency/density based on N values)	Depth of Top of Layer in Feet	Depth of Bottom of Layer In Feet	Layer Thickness in Feet	Ground Surface Elevation in Feet	Top of Layer Elevation in Feet	Bottom of Layer Elevation In Feet
PEAT (vsoft-soft) 3.5 BOB >9 263 3 1 2 263 3 6 PEAT (soft) 2 3 5 1.5 263 263 3 5 1.5 263 263 3 5 1.5 263 <td< td=""><td>Buried PEAT and</td><td>l d Organic SILT Deposits</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></td<>	Buried PEAT and	l d Organic SILT Deposits							
4 organic SILT (soft) 2 35 908 7 283 5 organic SILT (soft) 3 808 7 283 7 283 6 PEAT (soft) 3 808 7 283 7 283 7 PEAT (vsoft-soft) 3.5 B08 7 283 7 283 7 PEAT (vsoft-soft) 3.5 B08 7 283 284 283	HW98-HB3	PEAT	(v.soft-soft)	3.5	ВСЯ	9	202		
4 PEAT (soft) 3 BOB -1 283 5 PEAT (soft) 3 BOB -6.5 1.5 283 6 FEAT (v.soft-soft) 3.5 BOB -6.5 283 283 7 FEAT (v.soft-soft) 3.5 BOB >0.5 283 283 8 organic SILT (soft) 1.5 3.5 BOB >0.5 282 283 8 organic SILT (v.soft-soft) 3.5 BOB >0.5 282 283 9 PEAT (v.soft) 1.5 3.5 BOB >0.5 283 283 9 PEAT (v.soft) 1.5 3.5 1.5 283 </td <td>HW98-HB4</td> <td>organic SILT</td> <td>(soft)</td> <td>6</td> <td></td> <td>~~~</td> <td>203</td> <td>229.5</td> <td>(250.5)</td>	HW98-HB4	organic SILT	(soft)	6		~~~	203	229.5	(250.5)
5 organic SILT (soft) 2 3.5 BOB >7 283 7 PEAT (v.soft-soft) 3.5 BOB >0.5 283 283 7 PEAT (v.soft-soft) 3.5 BOB >0.5 283 283 7 PEAT (v.soft-soft) 3.5 BOB >0.5 283 283 8 organic SILT (v.soft-soft) 3.5 BOB >0.5 282 283 9 organic SILT (v.soft) 3.5 BOB >0.5 282 283 9 organic SILT (v.soft) 3.5 BOB >0.5 283 283 9 organic SILT (v.soft) 2.2 3.5 1.5 283 9 PEAT (v.soft) 2.5 8.08 >0.5 283 283 9 PEAT (v.soft) 1.5 5 3.5 283 283 283 283 283 283 283	HW98-HB4	PEAT	(100) (100)	N C	n (, –	263	261	260
FEAT (vsoft-soft) 3.5 BOB -5.5 2.63 7 PEAT (vsoft-soft) 3.5 BOB >6.5 2.63 8 PEAT (vsoft-soft) 3.5 BOB >6.5 2.63 8 organic SILT (vsoft-soft) 3.5 BOB >0.5 2.82 9 organic SILT (vsoft) 1.5 3.5 BOB >0.5 2.82 9 organic SILT (vsoft) 3.5 BOB >0.5 2.83 9 PEAT (vsoft) 3.5 BOB >0.5 2.83 9 PEAT (vsoft) 2 3.5 BOB >0.5 2.83 1 PEAT (vsoft) 1.5 5 2.83 2.63 1 PEAT (vsoft) 1.5 5 2.83 2.83 1 PEAT (vsoft) 1.5 5 2.83 2.83 1 PEAT (vsoft) 1.5	HW98-HB5	organic SII T	(aut)	m (BOB	>7	263	260	(253)
PEAT (vsoft-soft) 3.5 BOB >6.5 263 PEAT (vsoft-soft) 3.5 BOB >0.5 263 263 PEAT (vsoft-soft) 3.5 BOB >0.5 263 263 PEAT (vsoft) 1.5 3.5 BOB >0.5 263 263 PEAT (vsoft) 1.5 3.5 BOB >0.5 262 263 PEAT (vsoft) 2.5 BOB >0.5 263 263 263 PEAT (vsoft) 2.5 BOB >0.5 263 263 PEAT (vsoft) 1.5 5 3.5 1.5 263 PEAT (vsoft) 1.5 5 3.5 263 263 PEAT (vsoft) 1.5 5 3.5 263 263 PEAT (vsoft) 1.5 5 263 263 263 PEAT (vsoft) 1.5 5 <td< td=""><td>HW98-HB5</td><td></td><td>(solt)</td><td>N </td><td>3.5</td><td>1.5</td><td>263</td><td>261</td><td>259.5</td></td<>	HW98-HB5		(solt)	N	3.5	1.5	263	261	259.5
TCAI (v.soft-soft) 3 BOB >0.5 282 8 organic SILT (v.soft-soft) 3.5 BOB >0.5 282 9 organic SILT (v.soft) 1.5 3.5 BOB >0.5 282 9 organic SILT (v.soft) 1.5 3.5 BOB >0.5 282 9 organic SILT (v.soft) 3.5 BOB >0.5 283 9 organic SILT (v.soft) 3.5 BOB >0.5 283 9 PEAT (v.soft) 2 3.5 BOB >0.5 283 9 PEAT (v.soft) 1.5 5 3.5 283 9 PEAT (v.soft) 1.5 5 283 283 9 PEAT (v.soft) 1.5 5 283 283 9 PEAT (v.soft) 1.5 5 283 283 9 PEAT (v.soft) <	HW98-HB6		(V.SOR-SOff)	3.5	BOB	>6.5	263	259.5	(253)
TEAT (v.soft-soft) 3.5 BOB >0.5 262 9 organic SILT (soft) 1.5 3.5 BOB >0.5 262 9 organic SILT (soft) 1.5 3.5 BOB >0.5 262 9 organic SILT (soft) 3.5 BOB >0.5 262 1 FEAT (v.soft) 3.5 BOB >0.5 263 1 FEAT (v.soft) 2 3.5 BOB >0.5 263 1 FEAT (v.soft) 2 8 6 263 263 1 FEAT (v.soft) 1.5 5 3.5 263 263 1 FEAT (v.soft) 1.5 5 3.5 263 263 1 FEAT (v.soft) 1.5 5 3.5 263 263 1 FEAT (v.soft) 1.5 5 2.63 263 263	HW98-HB7	DEAT	(v.soft-soft)	ო	BOB	>0.5	262	259	(255.5)
PEAT (soft) 1.5 3.5 BOB >0.5 262 PEAT (v.soft-soft) 3.5 BOB >0.5 283 283 PEAT (v.soft) 3.5 BOB >0.5 283 283 PEAT (v.soft) 3.5 BOB >0.5 283 283 PEAT (v.soft) 2 3.5 BOB >0.5 283 PEAT (v.soft) 2 8 6 263 263 PEAT (v.soft) 1.5 5 3.5 263 263 PEAT (v.soft) 1.5 5 3.5 263 263 PEAT (v.soft) 1.1 BOB >7 263 263 PEAT (v.soft) 2.5 8 5.5 263 263 PEAT (v.soft) 2.5 8 7 263 263 PEAT (soft) 10 2 800B >7 263	HW08-HB8		(v.soft-soft)	3.5	BOB	>0.5	262	258.5	(258)
PEAI (v.soft soft) 3.5 BOB >0.5 262 9 organic SILT (soft) 2 3.5 BOB >0.5 263 9 PEAT (v.soft) 3.5 BOB >0.5 263 263 9 PEAT (v.soft) 2 3.5 BOB >0.5 263 1 PEAT (v.soft) 2 8 6 263 263 1 PEAT (v.soft) 1.5 5 3.5 263 263 1 PEAT (v.soft) 1.5 5 3.5 263 263 1 PEAT (v.soft) 1 BOB >7 263 263 PEAT (v.soft) 1 2.5 8 5.5 263 263 PEAT (soft) 10 2 80B >7 263 263 PEAT (soft) 1 10 2 263 263 263			(soft)	1.5	3.5	2	262	260.5	258.5
organic SiLi (soft) 2 3.5 1.5 263 PEAT (v.soft) 3.5 BOB >0.5 263 PEAT (v.soft) 2 3.5 BOB >0.5 263 PEAT (v.soft) 2 8 6 263 263 PEAT (v.soft) 1.5 5 3.5 263 263 PEAT (v.soft) 1.5 8 6 263 263 PEAT (v.soft) 1.5 8 7.0 263 263 PEAT (soft) 10 2 8 6 5.5 263 PEAT (soft) 1 10 2 263 264 PEAT		PEAL	(v.soft-soft)	3.5	BOB	>0.5	262	258.5	(258)
FEAT (v.soft) 3.5 BOB >0.5 263 PEAT (v.soft) 2 8 6 263 PEAT (v.soft) 2 8 6 263 PEAT (v.soft) 2 8 6 263 PEAT (v.soft) 1.5 5 3.5 267 PEAT (v.soft) 1.5 5 3.5 263 PEAT (v.soft) 1.5 5 3.5 263 PEAT (v.soft) 1.5 8 6 263 PEAT (v.soft) 2.5 8 5.5 263 PEAT (v.soft) 1 10 2 263 PEAT (soft) 10 2 263 263 PEAT (soft) 1 80B 2 263 PEAT (soft) 1 80B 2 263 PEAT (soft) 1 80B 2 263 <td></td> <td>organic SiL F</td> <td>(soft)</td> <td>7</td> <td>3.5</td> <td>1.5</td> <td>263</td> <td>261</td> <td>259.5</td>		organic SiL F	(soft)	7	3.5	1.5	263	261	259.5
PEAI (v.soft) 2 8 6 263 PEAT (v.soft) 2 8 6 263 PEAT (v.soft) 1.5 5 3.5 261 PEAT (v.soft) 1.5 5 3.5 263 PEAT (v.soft) 1.5 5 3.5 263 PEAT (v.soft) 1.5 5 3.5 263 PEAT (v.soft) 1.1 BOB >7 263 PEAT (soft) 1 10 2 263 PEAT (soft) 10 BOB >10 2 263 PEAT (soft) 1 BOB >2 263 PEAT (soft) 1 BOB >7 2 263 PEAT (soft) 1 BOB >7 2 263 PEAT no description 2.25 BOB >7 264 564 PEAT		PEAL	(v.soft)	3.5	BOB	>0.5	263	259.5	(259)
PEAI (v.soft) 2 8 6 266 PEAT (v.soft) 1.5 5 3.5 267 PEAT (v.soft) 1.5 5 3.5 263 PEAT (v.soft) 1.5 5 3.5 263 PEAT (v.soft) 1 BOB >7 263 PEAT (v.soft) 2.5 8 5.5 263 PEAT (v.soft) 2.5 8 5.5 263 PEAT (soft) 10 BOB >10 2 263 PEAT (soft) 1 1 BOB >2 263 PEAT (soft) 1 1 BOB >7 263 PEAT (soft) 1 1 BOB >7 263 PEAT no description 2.25 BOB >7.75 264 PEAT no description 2.25 BOB >7.55 264 PEA		PEAL	(v.soft)	7	8	9	263	261	255
PEAT (v.soft) 1.5 5 3.5 267 PEAT (v.soft) 1 BOB >7 263 PEAT (v.soft) 1 BOB >7 263 PEAT (v.soft) 1 BOB >7 263 PEAT (v.soft) 2.5 8 5.5 263 SILT w/PEAT (soft) 10 2.5 263 263 PEAT (soft) 10 BOB >10 2 263 PEAT (soft) 10 BOB >10 2 263 PEAT (soft) 1 BOB >7 10 263 PEAT (soft) 1 BOB >7 10 263 PEAT no description 2.75 BOB >7 7.5 264 PEAT no description 2.75 1 25 264 564 PEAT no description 2.75 BOB >7.75 <td></td> <td>PEAL</td> <td>(v.soft)</td> <td>2</td> <td>8</td> <td>9</td> <td>266</td> <td>264</td> <td>258</td>		PEAL	(v.soft)	2	8	9	266	264	258
PEAT (v.soft) 1 BOB >7 263 PEAT (v.soft) 2.5 8 5.5 263 PEAT (v.soft) 2.5 8 5.5 263 SILTwPEAT (soft) 2.5 8 5.5 263 PEAT (soft) 10 BOB >2 263 PEAT (soft) 10 BOB >2 263 PEAT (soft) 2 BOB >10 263 PEAT (soft) 1 BOB >2 263 PEAT no description 1.25 BOB >7.75 264 PEAT no description 2.75 7.25 4.5 264 PEAT no description 2.75 80B >7.55 264 PEAT no description 2.25 BOB >7.55 264 PEAT no description 2.25 BOB >7.55 264 PEAT no description <td></td> <td>PEAT</td> <td>(v.soft)</td> <td>1.5</td> <td>S</td> <td>3.5</td> <td>267</td> <td>265.5</td> <td>262</td>		PEAT	(v.soft)	1.5	S	3.5	267	265.5	262
PEAI (v.soft) 2.5 8 5.5 263 SILT w/PEAT (soft) 8 10 2 263 PEAT (soft) 10 80B >2 263 PEAT (soft) 10 80B >2 263 PEAT (soft) 10 80B >10 2 263 PEAT (soft) 2 80B >10 263 263 PEAT (soft) 1 80B >10 263 263 PEAT no description 1.25 80B >7.75 264 564 PEAT no description 2.75 7.25 4.5 264 564 PEAT no description 2.25 80B >7.75 264 564 PEAT no description 2.25 80B >7.55 264 562 PEAT no description 3 80B >0 262 262		PEAL	(v.soft)	-	BOB	>7	263	262	(255)
Sill Wireal (soft) 8 10 2 263 PEAT (soft) 10 80B >2 263 PEAT (soft) 10 BOB >2 263 PEAT (soft) 2 BOB >10 263 PEAT (soft) 1 BOB >10 263 PEAT (soft) 1 BOB >10 263 PEAT no description 1.25 BOB >7.75 264 PEAT no description 2.75 7.25 4.5 264 PEAT no description 2.25 BOB >7.55 264 PEAT no description 2.25 BOB >7.55 262 PEAT no description 3 BOB >7.3 262		PEAI Sult	(v.soft)	2.5	80	5.5	263	260.5	255
FEAT (soft) 10 BOB >2 263 PEAT (soft) 2 BOB >10 263 263 PEAT (soft) 2 BOB >10 263 263 PEAT (soft) 1 BOB >10 263 263 PEAT no description 1.25 BOB >7.75 264 PEAT no description 2.75 7.25 4.5 264.5 PEAT no description 2.75 7.25 4.5 264.5 PEAT no description 2.25 BOB >7.55 264.5 PEAT no description 2.25 BOB >7.35 262	HW98-TP5		(soft)	8	10	2	263	255	253
FEAT (soft) 2 BOB >10 263 PEAT (soft) 1 BOB >10 263 PEAT (soft) 1 BOB >9 263 PEAT no description 1.25 BOB >7.75 264 PEAT no description 2.75 7.25 4.5 264.5 PEAT no description 2.25 BOB >7.55 264.5 PEAT no description 2.25 BOB >7.55 262.5 PEAT no description 3 BOR >7.3 262	HW98-TP6	DEAT	(soft)	10	BOB	>2	263	253	(251)
FEAL (soft) 1 BOB >9 263 PEAT no description 1.25 BOB >7.75 264 PEAT no description 1.25 BOB >7.75 264 PEAT no description 2.75 7.25 4.5 264.5 PEAT no description 2.25 BOB >7.55 264.5 PEAT no description 2.25 BOB >7.55 262 PEAT no description 3 BOB >7.3 262		PEAL	(soft)	7	BOB	>10	263	261	(251)
PEAT no description 1.25 BOB >7.75 264 PEAT no description 2.75 7.25 4.5 264.5 PEAT no description 2.75 7.25 4.5 264.5 PEAT no description 2.25 BOB >7.55 262.5 PEAT no description 3 BOB >7.3 262		real	(soft)		BOB	6^	263	262	(253)
PEAT no description 2.75 7.25 4.5 264.5 PEAT no description 2.25 BOB >7.55 264.5 PEAT no description 2.25 BOB >7.55 262 PEAT no description 3 BOB >7.55 262	P-B1	PEAT	no description	1.25	BOB	~7 76	1.00		
PEAT Not description 2.13 7.25 4.5 264.5 PEAT no description 2.25 BOB >7.55 262 PEAT no description 3 BOB >0.3 262	P-82	PEAT	no description	7		61.12	204	262.75	(255)
PEAT no description 2.25 BOB >7.55 262 PEAT no description 3 BOR >n 3 2.25	P-B3	PEAT		C/.7	1.25	4.5	264.5	261.75	257.25
no description 3 BOB >0.3 Acc	P.SB1		no description	2.25	BOB	>7.55	262	259.75	(252.2)
		LEAL	no description	3	BOB	>0.3	266	263	(262.7)

Notes: BOB = Bottom of boring. Boring was not drilled through layer. Layer is thicker than distance between BOB and top of layer.

497828\TablesD-1&D-2.xls - Table D-1 (NSA) Hart Crowser

Sheet 3 of 3

Table D-1 - Peat and Organic Silt in North Safety Area

																						_	_	
Bottom of Layer Elevation in Feet		()EF.	(cc2) 250.75	2.004	256	251.5	254.5	256.5	255				203.1	1.202	259./ 251	1		260.5	2/6.3	271.5	264.5	258.5	258.5	256.5
Top of Layer Elevation In Feet		260 TE	262.75		262	260.5	262	261.5	260.5			1 636	203.2	202.1	255		000	203	2/0.0	276	266	265	264	264
Ground Surface Elevation in Feet		264	265	1	264	264	264	264	264			263.2	7 002	265.2	264		763	278.8	0.012	5/6	267	265	264	264
Layer Thickness in Feet		>5.75	<i>.</i> 0		9	თ	7.5	5	5.5		<u></u>	9.5	10	5.5	4		ол Л	2.5		0.4	1.5	6.5	5.5	7.5
Depth of Bottom of Layer in Feet		BOB	5.25		Ð	12.5	9.5	7.5	0			9.5	10	5.5	13		2.5	2.5	2 2	; i	2.5	6.5	5.5	7.5
Depth of Top of Layer in Feet		3.25	2.25	c	V	3.5	7	2.5	3.5	SE WALL AREA		0	0	0	5		0	0	c	, -		0	0	0
N Value (consistency/density based on N values)		no description	no description	for soft		U & push v.soft	1 v.soft	0 v.soft	1 v.soft	DPOSED EMBANKMENT / MS		v soft	1 v.soft	2 very soft	(v.soft)		GRAB (soft)	grab (soft)	grab (loose)	(cott)	gidu (suit)	4 10 13 501	Z V.SOT	2 v.soft
Soil Description	Buried PEAT and Organic SILT Deposits	PEAT	PEAT	SILT w/ora (PFAT)				PEAL	PEAT	NORTH SAFETY AREA - EXPLORATIONS IN PROPOSED EMBANKMENT / MSE WALL AREA	Area of Proposed Overexcavation (10 Feet)	PEAT	PEAT	SILT & PEAT interbedded	organic SILT (PEAT)	Area Outside of Proposed Overexcavation	organic CLAY	PEAT	SAND & PEAT	PEAT	Ornanic SII T (DEAT)			VIGAILIC OLLI (PEAL)
Exploration Number	Buried PEAT and	P-SB2	P-SB3	CT97-B4A	CT98-R4	CT08_B5		0190-00	01-00-D0	NORTH SAFETY	Area of Proposed	HC99-B32	HC99-B33	HC99-B34	CT98-B7	Area Outside of P	HC99-B42	HC99-B47	HC00-B167	HC00-B174	CT98-B3	CT98-B9	CT98-B10	

Hart Crowser 497828\TablesD-1&D-2.xis - Table D-1 (NSA) Notes: BOB = Bottom of boring. Boring was not drilled through layer. Layer is thicker than distance between BOB and top of layer.

Exploration Number	Soll Description	N Value (consistency/density based on N values)	Depth of Top of Layer in Feet	Depth of Bottom of Layer In Feet	Layer Thickness In Feet	Ground Surface Elevation In Feet	Top of Layer Elevation in Feet	Bottom of Layer Elevation
WEST WALL - E	WEST WALL - BURIED PEAT DEPOSITS AT STATION 180+00	00+(
HC99-B37	scPEAT	30 v.stiff	9.5	15	5.5	234.6	225.1	219.6
HC99-B38	sPEAT	15 stiff	4.5	9.5	5	227.6	223.1	218.1
HC99-B39	PEAT	(3 soft - only part. in peat)	3.5	ŝ	1.5	231.1	227.6	226.1
HC00-B124	sPEAT	10 stiff	7.5	10	2.5	231	223.5	221
WEST WALL - P	WEST WALL - PEAT DEPOSITS NEAR STATION 180+00 RANGIN	LANGING FROM SHALLOW TO DEEP	DEEP					
HC00-B123	sSILT w/PEAT & siS layers	3 to 6 soft-m.stiff	0	11	7	235	235	224
HC00-B132	siSAND w/sl.sSILT & PEAT layers	2 loose	0	6	თ	227	227	218
WEST WALL - S	WEST WALL - SHALLOW PEAT DEPOSITS BETWEEN STATIONS	ATIONS 178+40 AND 183+30						
HC00-B121	SILT w/S & PEAT interbedded	grab (v.soft)	0	2	7	230	230	228
HC00-B122	siSAND & PEAT	grab (v.loose)	0	2	2	240	240	238
HC00-1P113	sl.g,siSAND w/PEAT	(loose)	0	2	2	239	239	237
HC00-1P114	g,siSAND w/PEAT (topsoil)	(loose)	0	0.5	0.5	248	248	247.5
HC00-1P115	sl.g,siSAND w/PEAT (topsoil)	(loose)	2.5	ę	0.5	250	247.5	247
WEST WALL - S	WEST WALL - SCATTERED, LOCALIZED PEAT DEPOSITS NORT	NORTH OF STATION 183+30						
HC00-B146	SPEAT	grab (soft)	0	5	2	261	261	750
HC00-A100	sl.sPEAT w/roots	(v.soft-soft)	0	2	2	268	268	266
HC00-A105	siSAND w/PEAT	(soft)	0	-	-	291	291	062
HC00-A137	siSAND w/PEAT	(loose)	0	e	e	235	235	232
HC00-A143	organic SILT w/organics	(v.soft)	1.5	4	2.5	237	235.5	233
HC00-1P121	siSAND w/PEAT	(loose)	11	13	2	254	243	241

Table D-2 - Peat and Organic Silt along the West Wall

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Hart Crowser 497828\TablesD-1&D-2.xls - Table D-2 (West Wall)

Hart Crowser 497828\TablesD-3&D-4.xls - Table D-3

Medium Ash - Peat with between 5 and 15% ash. Hemic - Peat with between 33 and 67% fibers. High Ash - Peat with more than 15% ash. Sapric - Peat with less than 33% fibers. Low Ash - Peat with less than 5% ash. Ash Content:

Fibric - Peat with greater than 67% fibers.

Fiber Content:

Notes: ¹⁾ Definitions as per ASTM D 4427:

Acidity:

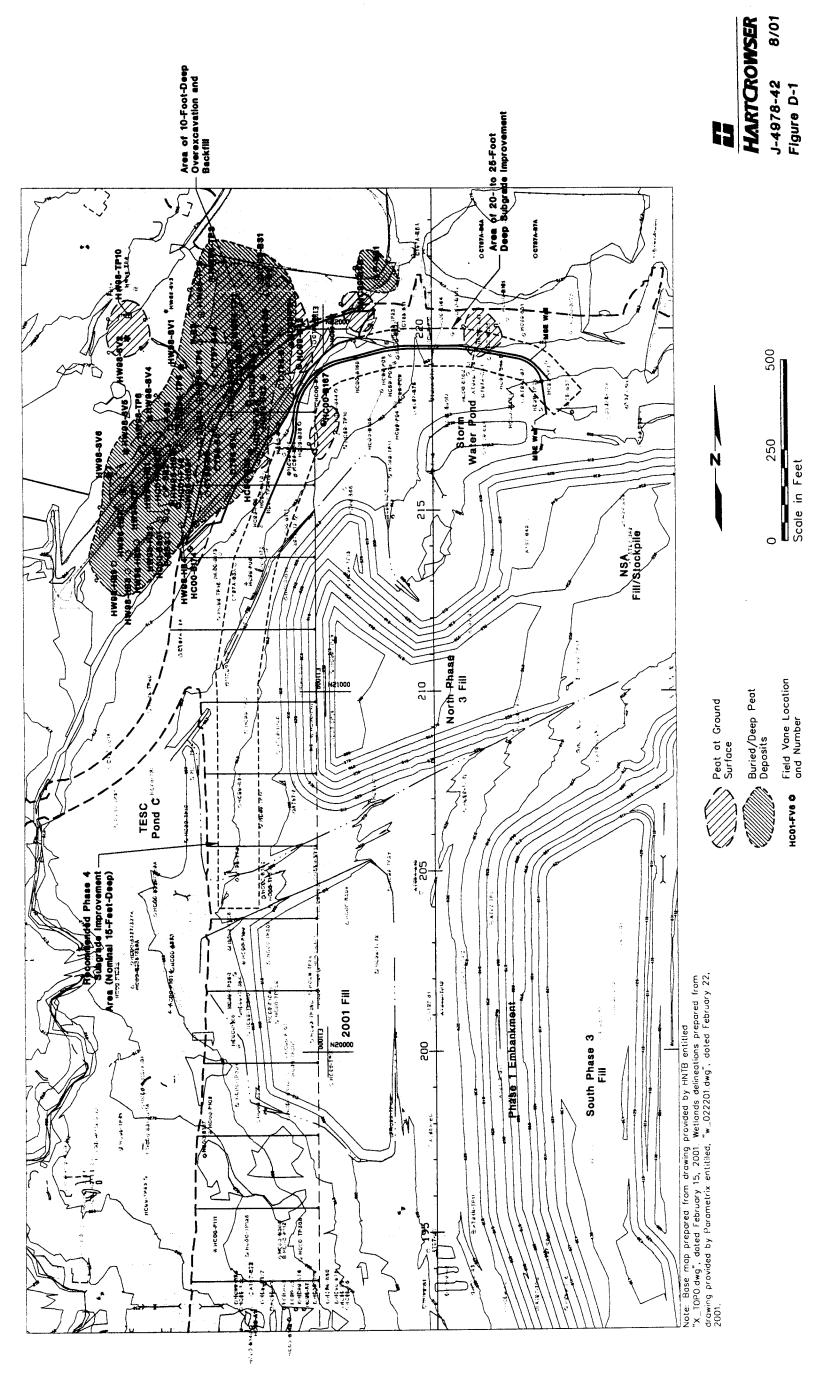
Slightly Acidic - Peat with a pH greater than 5.5 and less than 7. Moderately Acidic - Peat with a pH between 4.5 and 5.5. Basic - Peat with a pH equal or great than 7. Highly Acidic - Peat with a pH less than 4.5.

New Exploration Number	Sample Depth In Feet	Previous Exploration	General Location	Moisture Content	Fiber Content	Organic Content	pH (Water)	pH (CaCl2)	Classification (ASTM D 4427) ⁽¹⁾
HC01-FV1	0.2 to 0.4	HC00-B121	West Wall	132%	11%	18%	5.9	5.4	Sabric. high ash slightly acidic Peat
HC01-FV7B	1.8 to 2.1	HC00-B123	West Wall	168%	11%	24%	5.6	4.8	Sabric, high ash. slightly acidic Peat
HC01-FV4	3.8 to 4.3	HC99-B39	West Wall	132%	12%	18%	5.2	4.8	Sabric, high ash, moderately acidic Peat
HC01-FV8	6.8 to 7	HC99-B38	West Wail	72%	4%	%6	5.6	5.6	Sapric, high ash, slightly acidic Peat
HC-01-FV9	4.0 to 4.3	HC99-B33	NSA	981%	32%	%6 <i>L</i>	5.5	5.0	Sapric. high ash moderately acidic Peat
HC01-FV6	4.3 to 4.5	HW98-HB3	NSA	783%	51%	%06	5.5	4.9	Hemic, medium ash, moderately acidic Deat
HC01-B33B	5.0 to 5.5	HC99-B33	NSA	561%	22%	54%	6.5	6.0	Sabric, high ash, slightly acidic Peat
HC01-HB3A	7.3 to 7.8	HW98-HB3	NSA	640%	25%	61%	6.0	5.7	Sapric, high ash, slightly acidic Peat

Table D-3 - Peat Classification Summary

				Moisture	Fiher	Ornanio					
Field		Test	Undrained	Content	Content	Content		Previous.	SPT	SPT	
valle Location	Location	Depth in Feet	Strength in psf	In Percent	In Percent	ln Percent	USC Visual Classification (1)	Adjacent Boring	Depth	Blow	
HC01-FV1	West Wall	0.33	167	132		at at	Vorus and used dark traver Dark the				tenary .
HC01-FV7A	West Wall	1.6	417	.	: .	2	Very Suit, wet, uark brown reat with tine roots	HC00-B121	•	•	No shallow SPT samples.
HC01-FV7R	West Woll	<u>;</u> .			• :	•	Soft, moist, dark brown Peat	HC00-B123	•	•	No shallow SPT samples.
		7	409	168	-	24	Soft, moist, dark brown Peat	HC00-B123		•	No shallow SPT samples
HCU1-FV4	West Wall	4.2	1607	132	12	18	Stiff, moist, dark brown, sandy Peat	HC99-B39	3.5	e.	Only hottom of sample in BEAT
HC01-FVB	West Wall	6.4	1044				Stiff, moist, dark brown, stightly gravely, sandy Peat	HC00.B38		- ÷	
		7	1044	72	4	5	Stiff, moist, dark brown, slightly gravelly, sandy Peat		2	2	
HC01-FV6	NSA	3.5	626				Medium stiff, moist dark brown fibrous Paat			-	:
		4.5	459	783	51	6	Soft. moist, dark hrown fihmus Peat	COL-06441	•	,	Hand auger - no blow counts avail.
		5.8	313				No sample (boring log indicates part)				
		6.8	375				No sample (boring log indicates Peat)				
		7.8	375				No sample (horino log indicates post)				
HC01-FV9	NSA	0.8	835				Medium stiff, moist to wet, dark brown Ornanic Silt	HC00 B33			
		2.1	542				Medium stiff, wet, dark hrown fihrens Peat	559-860L		• •	No shallow SPT samples.
		4.2	292	981	32	6/	Soft. wet. dark trown fibrinis Paat		0. U	N (

(1) West Wall peat samples typically classified as sapric, high ash, slightty acidic; per ASTM D 4427. NSA peat samples typically classified as sapric, high ash, slightly to moderate acidic, per ASTM D 4427.

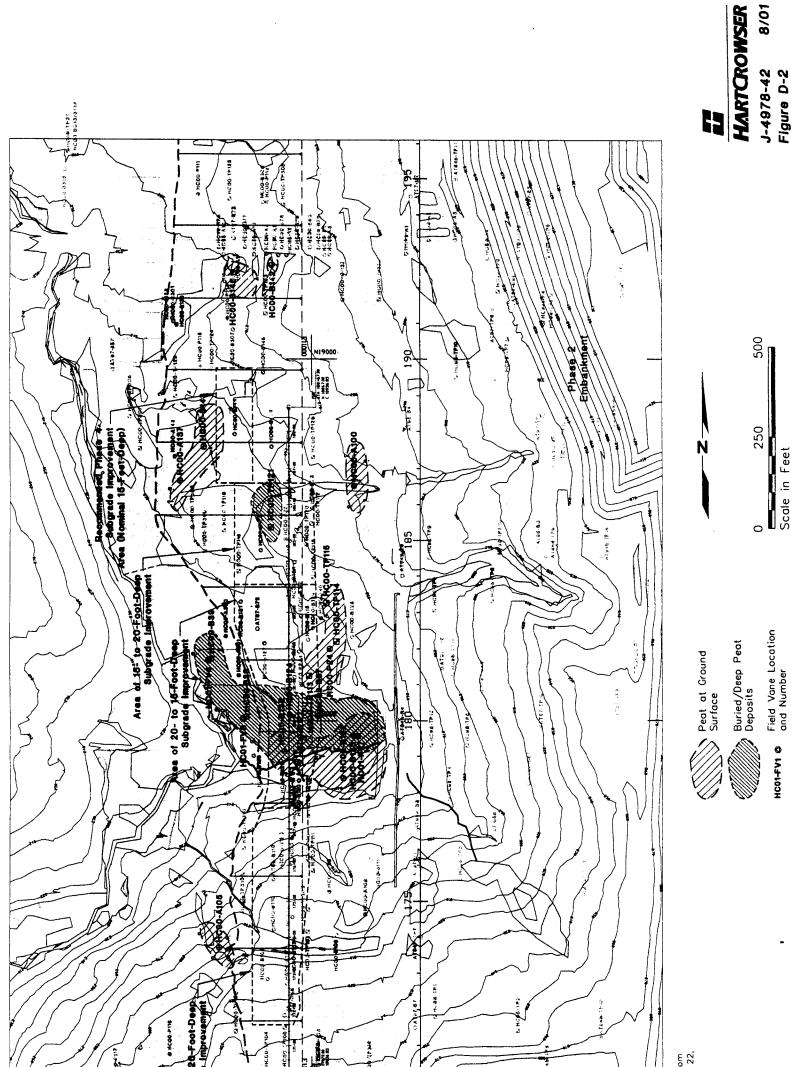


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AR 051599

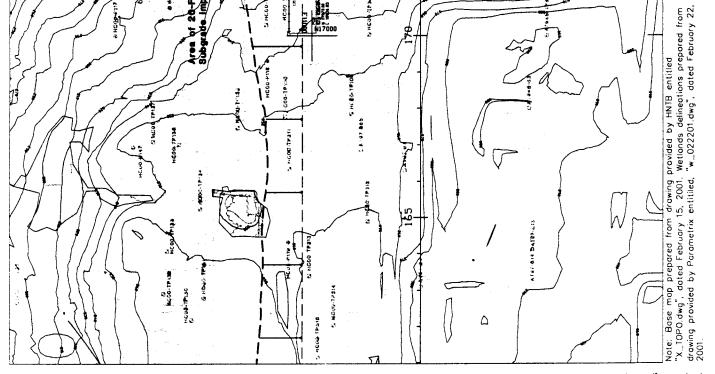
Peat and Organic Silt North Safety Area

497842001 0.0H 8/17/01 1=250 (xref) see awg/charlie.pc2



AR 051600

Peat and Organic Silt West Wall Area



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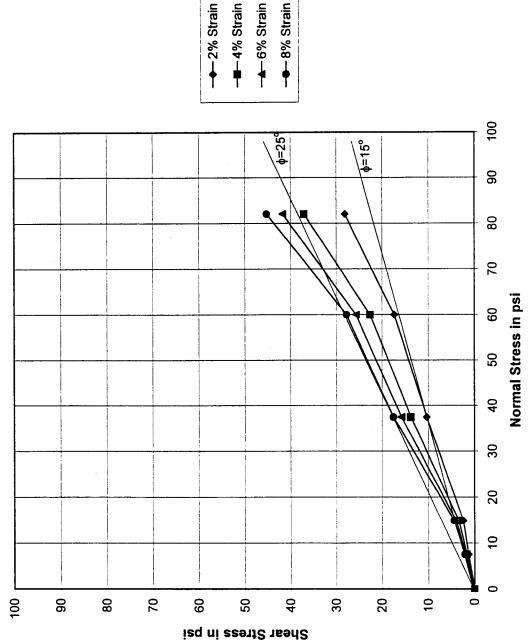
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49764202 RC 8/15/01 1=250 (xref) see dwg/chorie.pc2

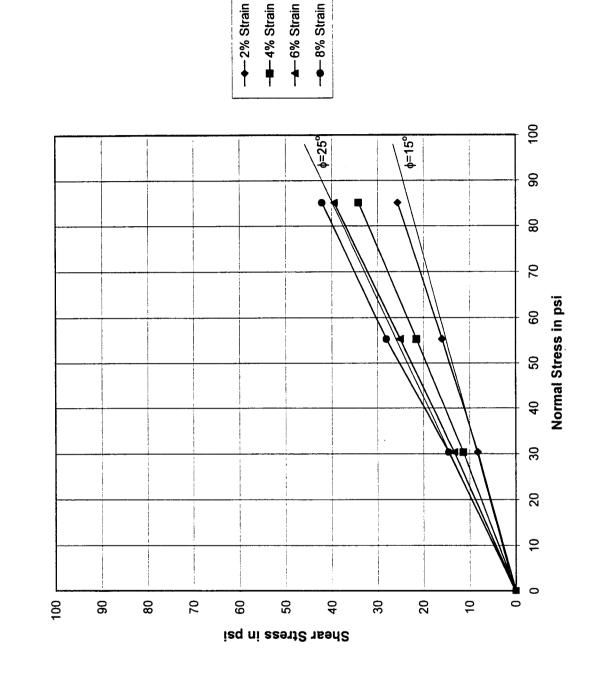




HARTCROWSER J-4978-42 8/01 Figure D-3

AR 051601

-≜--6% Strain



Direct Shear Test on sapric, high ash, slightly acidic Peat (Boring HC01-HB3A, 6.3 to 7.6 feet)



