

**PCHB NO. 01-133**

**PORT OF SEATTLE'S  
MEMO OPPOSING  
ACC'S MOTION FOR STAY**

*Declaration of Paul S. Fendt*

**Volume 2 of 3**

**AR 010824**





**PREFACE**

**Comprehensive Stormwater Management Plan  
Seattle-Tacoma International Airport  
Master Plan Update Improvements**

**July 2001**

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ENVIRONMENTAL  
HEARINGS OFFICE

This document contains replacement pages developed in response to comments received from the Washington State Department of Ecology (Ecology) on Volumes 1 through 4 of the December 2000 Comprehensive Stormwater Management Plan (SMP) for the Seattle-Tacoma International Airport Master Plan Update Improvements. A facilitated process was used to document specific revisions required by Ecology to the December 2000 SMP. Each SMP volume contains an itemized list of replacement pages, and the replacement pages are identified by a July 2001 footer.

**SMP VOLUME 3**  
**JULY 2001 REPLACEMENT PAGES**

Preface (before pg. *i*)

List of Volume 3 Replacement Pages

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Certificate of Engineer (before pg. B1-*i*)

Figure B1-2 (pg. B1-4)

Section 1.1 (pg. B1-5)

Figure B1-3 (pg. B1-6)

Figure B1-4 (pg. B1-7)

Figure B1-5 (pg. B1-8)

Figure B1-6 (pg. B1-9)

Section 1.3 (pg. B1-10)

Section 1.5.2 (pg. B1-14)

Des Moines Creek HSPF Calibration Model Input Files (21 pgs.)

Des Moines Creek Calibrated Model Hydrographs:

SDS3 Average and Peak Daily Flow (4 pgs.) *(replaces graphs previously contained on 2 pgs)*

Golf Weir Average and Peak Daily Flow (4 pgs.) *(replaces graphs previously contained on 2 pgs)*

Near Mouth March 6, 1995 through March 28, 1995 (1 pg.)

SDS3 December 28, 1995 through January 18, 1996 (4 pgs.) *(replaces graphs previously contained on 5 pgs.)*

**APPENDIX B2:**

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Section 2.1 (pg. B2-2)

Figure B2-1 (pg. B2-3)

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Table B2-3 (pg. B2-6)

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Figure B2-3b (pg. B2-27)

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Section 5.3 (pg. B2-51)

Sections 5.6 and 5.7 (pgs. B2-52 and B2-52a)

Figure B2-23 (pg. B2-53)

Table B2-9 (pg. B2-54)

Table B2-10 (pgs. B2-54a and B2-54b)

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Section 6 (pg. B2-64)

Miller Creek HSPF Calibration Model Input File (20 pgs.)

Walker Creek HSPF Calibration Model Input File (9 pgs.)

*Note: Listed page totals do not include "guidance" pages (the guidance pages do not need to be inserted as replacement pages).*

*July 2001*  
*556-2912-001 (28)*

**AR 010826**

**DES MOINES CREEK HSPF MODEL CALIBRATION REPORT**

**VOLUME 3—APPENDIX B1**

Prepared for

**PARAMETRIX, INC.**

5808 Lake Washington Blvd. N.E., Suite 200  
Kirkland, Washington 98033-7350

Prepared by

Dave Harms

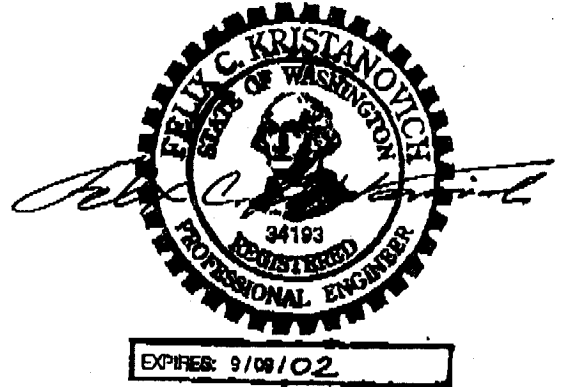
Roth Hill Engineering Partners, Inc.

**AUGUST 2000**  
**556-2912-001 (28)**

**AR 010827**

**CERTIFICATE OF ENGINEER**

The technical material and data contained in this document were prepared under the supervision and direction of the undersigned.



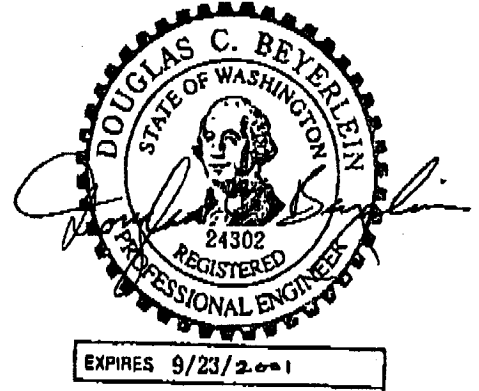
*Felix C. Kristanovich*  
Felix C. Kristanovich, Ph.D., P.E.

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**AR 010828**

**CERTIFICATE OF ENGINEER**

The technical material and data contained in this document were prepared under the supervision and direction of the undersigned.



*Douglas Beyerlein*  
 \_\_\_\_\_  
 Douglas Beyerlein, P.E., Senior Engineer

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**AR 010829**

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## APPENDIX B1

### DES MOINES CREEK HSPF MODEL INFORMATION

#### 1. INTRODUCTION

This appendix documents the modifications to and calibration of the Des Moines Creek HSPF model. The HSPF model used for the Des Moines Creek Basin Plan (King County 1997), was used as a basis for the modification of the model that was used in the preparation of the draft Preliminary Comprehensive Stormwater Management Plan (draft SMP) (Parametrix 1999). These modifications led to this current version of the model and its calibration results, presented herein. Calibration results are compared to results obtained using Basin Plan model parameters, U.S. Geological Survey parameters and the previous version of the Des Moines Creek model, used in preparation of the November 1999 draft SMP.

#### 1.1 CHANGES FROM THE KING COUNTY BASIN PLAN MODEL

##### 1.1.1 Model Origin

According to the Des Moines Creek Basin Plan, the HSPF model used to perform that analysis resulted from a modification and enhancement of the HSPF model developed by Montgomery Water Group (MWG 1995). The MWG model was also the basis for the previous version of the Des Moines Creek model used in preparation of the November 1999 draft SMP. This current version of the Des Moines Creek model is a modification of the November 1999 draft SMP model, incorporating the Basin Plan model and a number of additional modifications. A schematic representation of the model is shown in Figure B1-1. The HSPF input file for the calibrated model is included at the end of this Appendix.

##### Changes to the Model

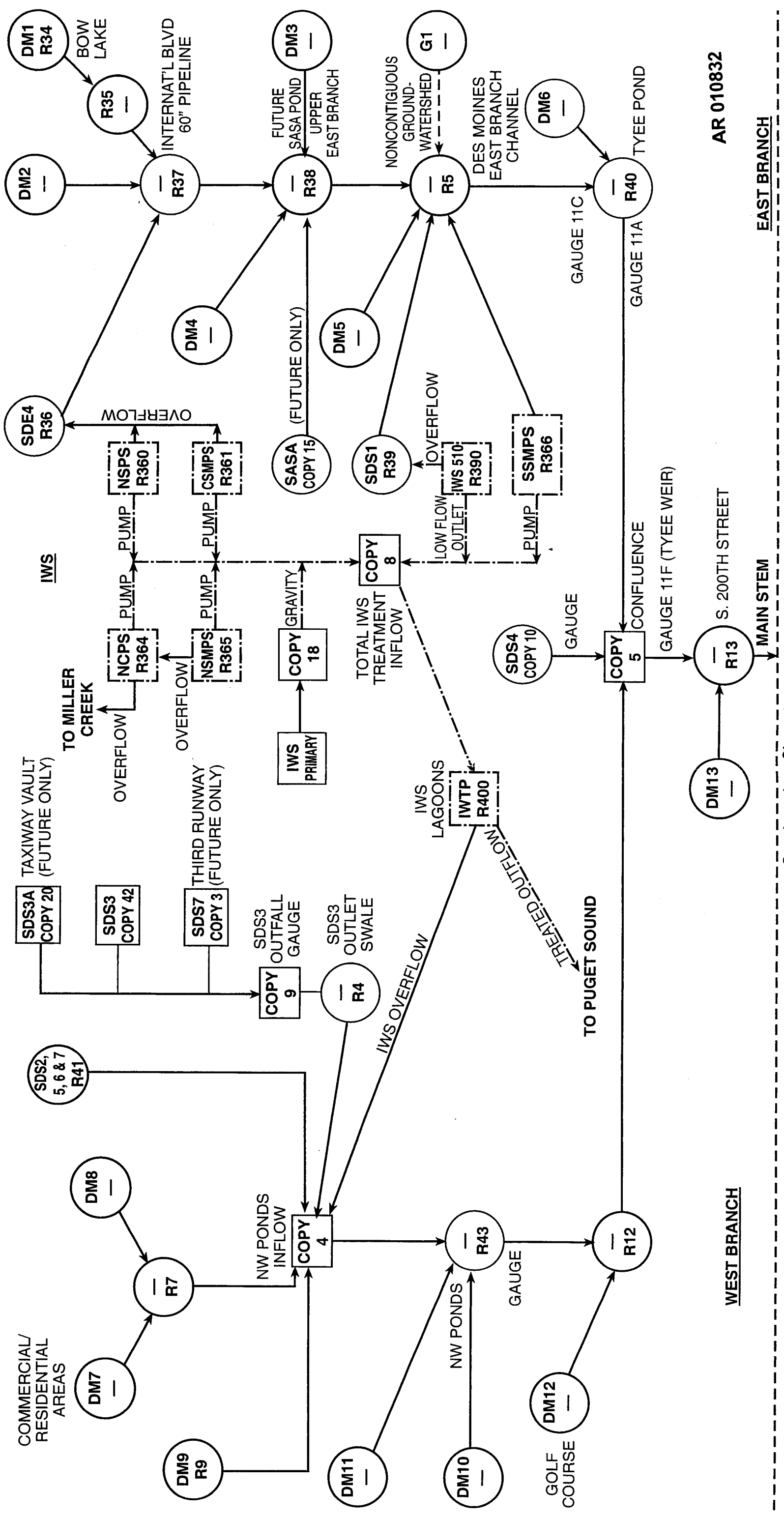
The following changes were incorporated into the November 1999 draft SMP version of the model, to produce the current Des Moines Creek model:

- The drainage basin and subbasin boundaries were modified. Changes include modification of the boundary to subbasin D1, excluding an area south of Bow Lake from discharging through the lake, modifying D16 to exclude an area to the southwest of Executel Pond from discharging through the pond and modifying the boundary of D21 to match the location of flow recording gage 11D. Boundary modifications resulted from examination of topographic mapping and review of basin boundaries determined for the City of SeaTac Surface Water Plan (Earth Tech 1997), which incorporated extensive field examination and the effects of stormwater conveyance on boundary definition. The resulting boundaries are shown in Figure B1-2.

CITY OF SEATAC

SEATAC AIRPORT

CITY OF SEATAC/BOW LAKE



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Continued on Sheet 2

Parametrix, Inc. Port of Seattle/HSPF Model/556-2912-001 (028) 500 (K)

DM# = Des Moines Creek Subbasin Number

SDS# = Storm Drainage South Number

SDE# = Storm Drainage East Number

R# = Routing Reach Number (Channel or Storage Pond)

DM1  
R34

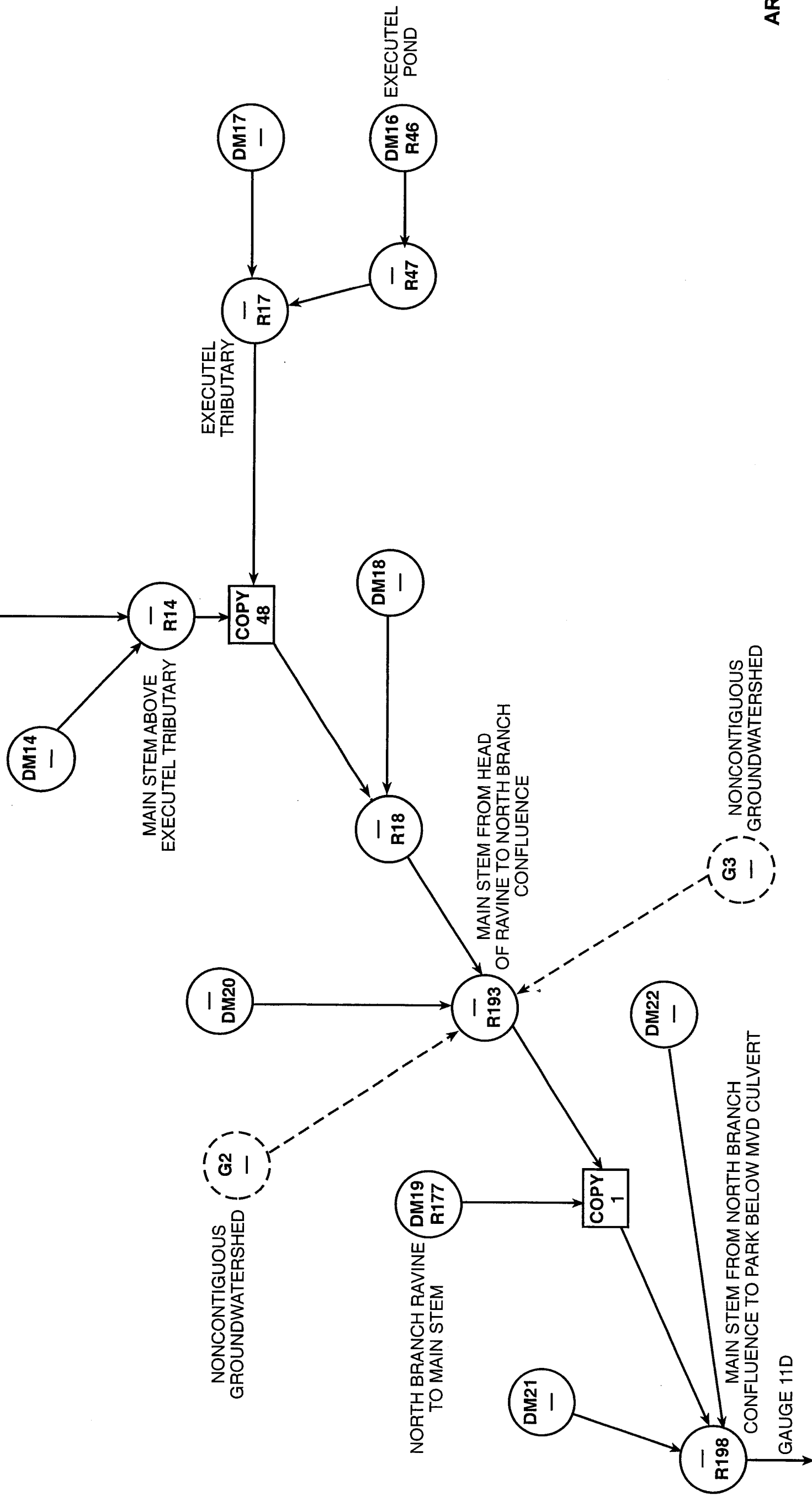
COPY  
8

Summation of  
upstream flow

Figure B1-1  
Des Moines Creek  
HSPF Model Schematic  
Sheet 1



Continued from Sheet 1



AR 010833

Parametrix, Inc. Port of Seattle/HSPF Model/556-2912-001(028) 5/00 (K)

DM# = Des Moines Creek Subbasin Number

SDS# = Storm Drainage South Number

SDE# = Storm Drainage East Number

R# = Routing Reach Number (Channel or Storage Pond)

DM1  
R34

Subbasin ID  
Routing Reach Number

COPY 8  
Summation of upstream flow

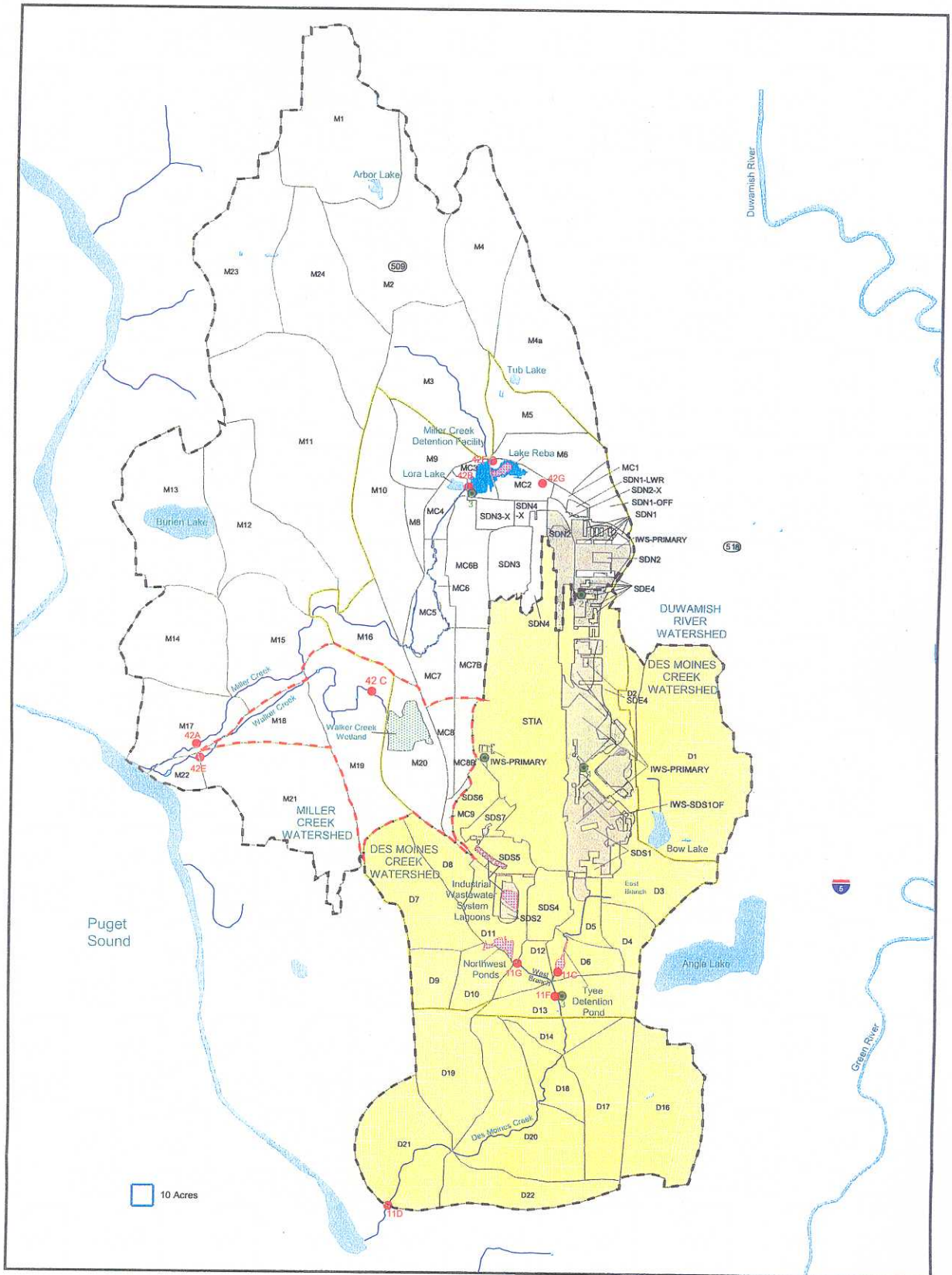
- Land use acreage has been recomputed for the airport facilities and the area south of the airport, to South 200<sup>th</sup> Street. The process for updating the land use is discussed later in this Appendix. The resulting land use is summarized in the Table B1-1 provided at the end of this Appendix. Land use for the area within the Des Moines Creek basin, east of the airport and south of South 200<sup>th</sup> Street, was modified primarily so that the percentage of impervious area within those subbasins was consistent with Basin Plan values. Those impervious percentages were subsequently reduced for the subbasins south of South 200<sup>th</sup> Street as part of the calibration process.
- Assumptions about groundwater contributions to base flow have been modified, based on the regional groundwater study, discussed below. Figure B1-3 identifies the areas contributing groundwater to Des Moines Creek and the direction of groundwater flow.
- PERLND categories were not separated by slope, consistent with the Basin Plan model.
- Only the surface flow component of runoff is routed to airport subbasin discharge points. Interflow is routed to downstream discharge points, since runoff from airport subbasins is collected in the STIA stormwater drainage system (SDS) and it is assumed that runoff that becomes interflow cannot be collected in the SDS system. Groundwater from the majority of airport subbasins is routed to the Walker Creek drainage basin, as identified in Figure B1-3.
- FTABLES were not modified since they appear to have been previously reviewed by King County, as indicated by comment lines inserted in the calibration input file for the November 1999 SMP.

## 1.2 SUBBASIN BOUNDARIES, LAND USE, AND SOILS DATA

Subbasin boundaries, land use, and soils data used to develop the previous FEIS and draft SMP basin models were checked against available information sources. Additional data representing 1994 conditions in the STIA subbasin areas were used since land use changes may have occurred since development of earlier versions of the model. The land use information is presented for the STIA subbasin areas later in this Appendix. Figure B1-4 shows the STIA subbasins and the subbasins that comprise the remainder of the Des Moines Creek watershed.

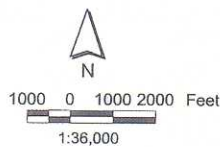
Land use data for subbasins within the STIA area of analysis (Figure B1-4) were compiled using the ArcView 3.2 GIS coverage of airport land use from 1994 aerial photography. Data were developed for the 1994 base year drainage basin boundaries, soil, vegetation, and impervious surfaces. Land use was categorized as shown in Figure B1-4. Soil type was categorized as shown in Figure B1-5. Soil classification was obtained from mapping by the U.S. Geological Survey (Waldron 1962). Soil Conservation Service soil maps do not contain coverage of the area near STIA. Impervious area for the 1994 aerial photo coverage was digitized (Figure B1-6).

The STIA subbasin land use and soil/vegetation information used in the 1994 HSPF model was calculated using ARC/INFO GIS algorithms. The map layers of 1994 impervious areas, 1994 land use codes, 1994 soil codes, existing slope codes and lake areas were combined with the 1994 drainage subbasins using the mathematical concept of identity. A tabular file was output from the



Parametrix, Inc. - Sea-Tac Airport Stormwater Management Plan/556-2912-001(28) 6/00 File: K:\GIS\2912\ArcView\mreatac-apdca\_may2001.apr  
 Source: Roads based on King County data. Water bodies derived from USGS hypsography data. Detention boundaries are approximate.  
 Note: Subbasin boundaries shown outside of STIA area are for illustration and reference only.  
 STIA subbasins assume existing (1994) conditions.

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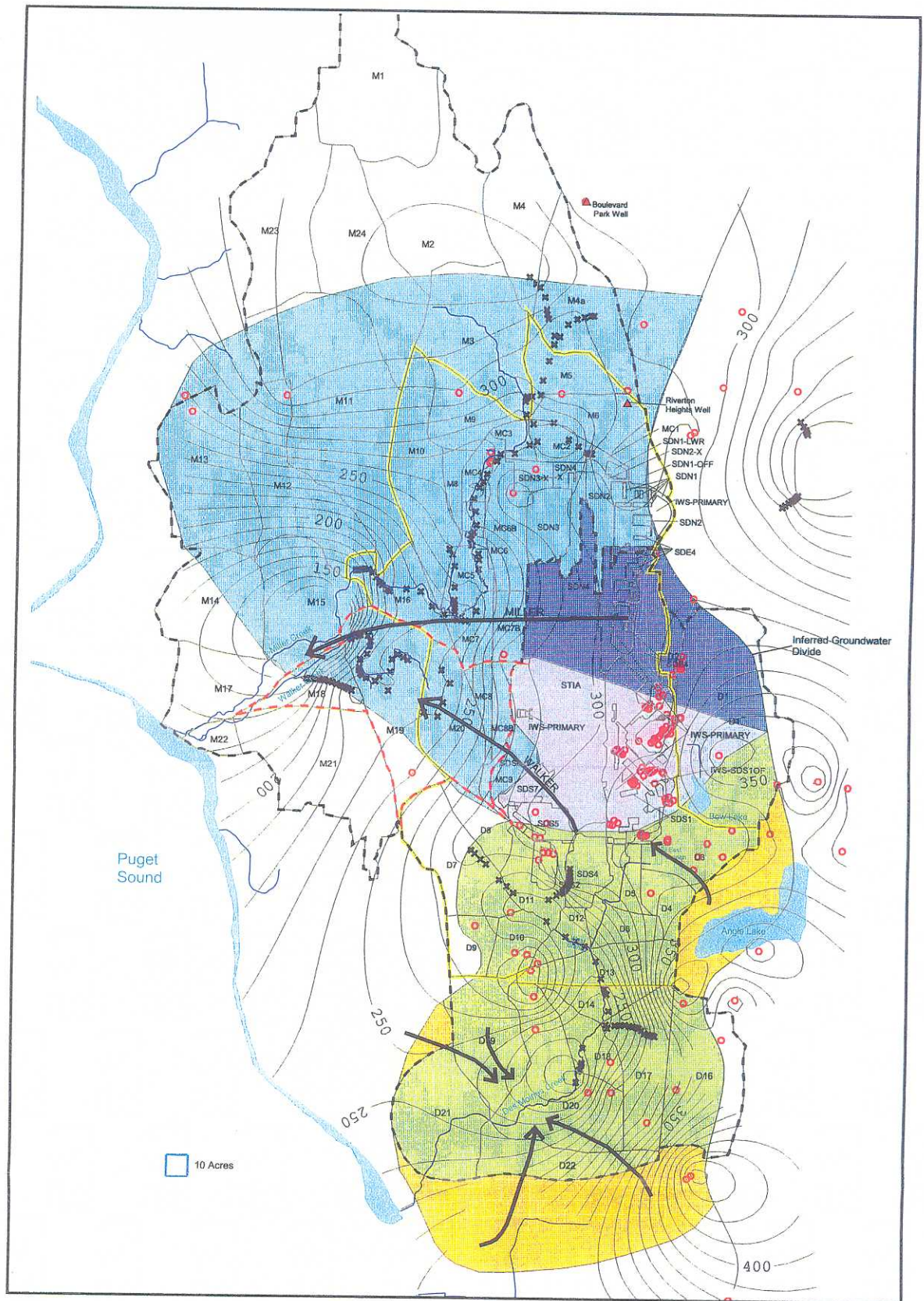
- Roads
- Existing (1994) Drainage Subbasins
- STIA Area (see note)
- Constructed Water Features
- IWS Drainage Area

- Subwatershed Boundary
- Watershed Boundary
- Rivers
- Water Bodies
- Detention Facilities (existing)
- Des Moines Creek Watershed

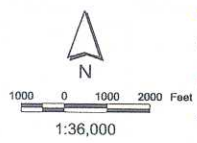
- Precipitation Gaging Stations:
  - Type 1 - National Weather Service (Gage relocated 1998)
  - Type 2 - POS Rainfall Monitoring
  - Type 3 - King County Rainfall Monitoring
- Streamflow King County Gaging Stations:
  - 42A - Miller Ck @ SW 175th Pl & 12th Ave SW
  - 42B - Miller Ck @ Lake Reba/RDF Outlet
  - 42C - Walker Ck @ 171st Pl
  - 42E - Walker Ck @ 12th Ave SW
  - 42F - Miller Ck @ SR518
  - 42G - Miller Ck @ East Branch
  - 11D - Des Moines Ck near mouth
  - 11F - Des Moines Ck @ Golf Course
  - 11C - Des Moines Ck @ Tyeo Pond
  - 11G - Des Moines Ck @ NW Ponds

**Figure B1-2  
 Des Moines  
 Creek Watershed  
 Existing Conditions**





Parametrix, Inc. Sea-Tac Airport Stormwater Management Plan/556-2912-001(28) 7/00 File: K:\GIS\2912\ArcView\rsr\stia-epd\stia\_may2001  
 Source: Water bodies derived from USGS hydrography data. Ground water contours from Seattle-Tacoma International Airport Ground Water Study. Associated Earth Sciences, Inc., and S. S. Papadopoulos & Assoc., 1999.  
 Note: STIA Subbasin GIS coverage obtained where conditions may change between 1994 and other conditions; subbasin boundaries shown outside of STIA area are for illustration and reference only.

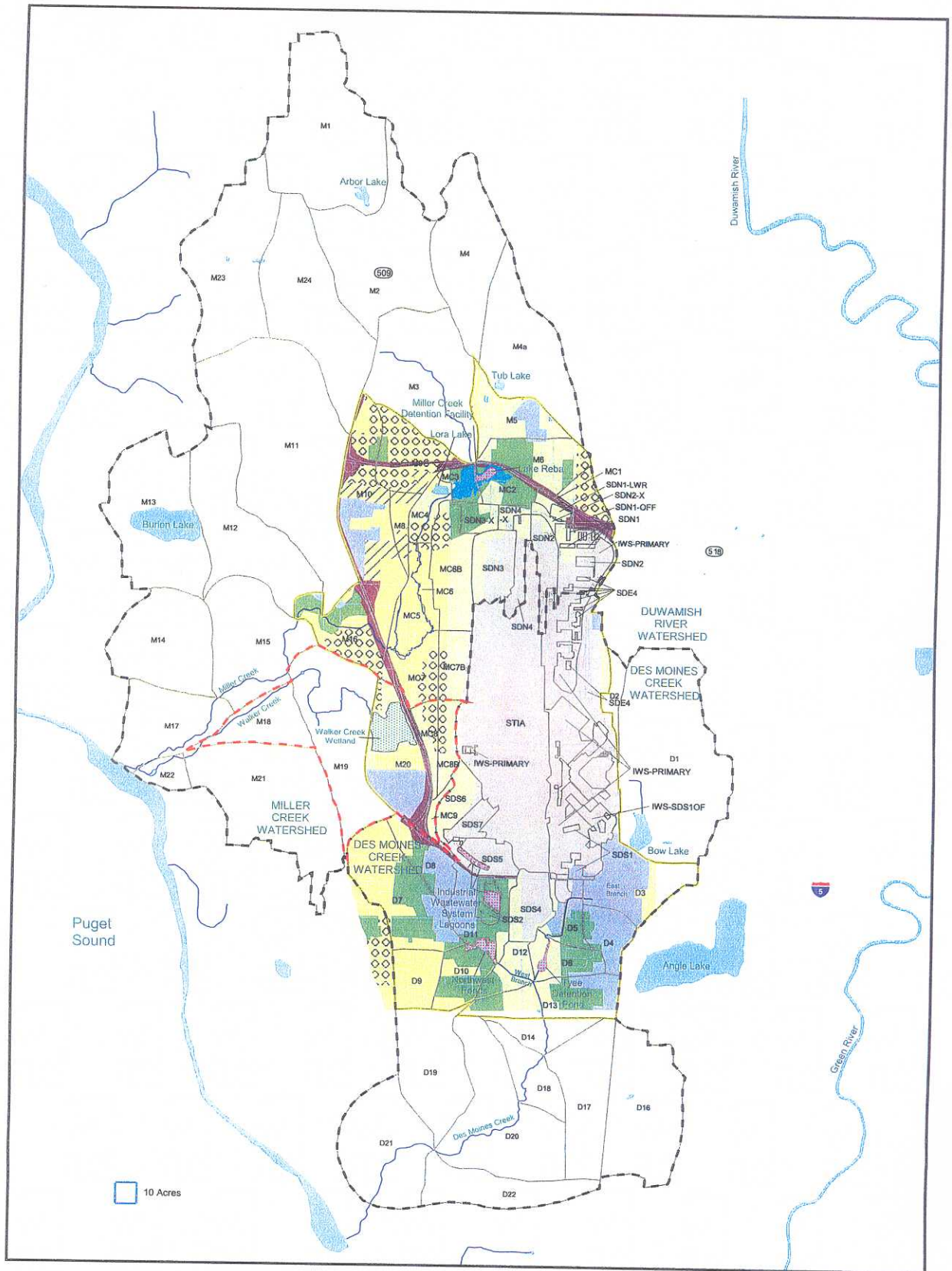


- Existing (1994) Drainage Subbasins
- STIA Area (see note)
- Watershed Boundary
- Subwatershed Boundaries
- Rivers
- Roads
- Water Bodies
- Noncontiguous Miller Creek groundwater area
- Noncontiguous Walker Creek groundwater area
- Contiguous Miller/Walker groundwater area
- Noncontiguous Des Moines Creek groundwater area
- Contiguous Des Moines Creek groundwater area
- Ground water elevation contour (10 ft. interval) for shallow (C1) aquifer
- Data Point (e.g. monitoring well)
- Wells
- Locations where ground water elevation intersects stream channel
- General ground water flow direction

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 556-2912-001 (28)

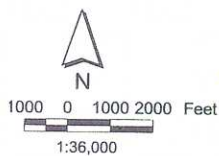
**Figure B1-3  
 Groundwater  
 Flow  
 Direction and  
 Boundaries**





Parametrix, Inc./Sea-Tac Airport Stormwater Management Plan/556-2912-001(28) 6/00 File: K:\Gis\2912\Arcview\rsseatac-apdca\_may2001.apr  
 Source: Roads based on King County data. Water bodies derived from USGS hypsography data. Estimated wetland boundaries are based on field reconnaissance by Parametrix, Inc.  
 Land use data interpreted from 1993 STIA aerial photograph (Walker and Assoc. 1993).  
 Note: Subbasin boundaries shown outside of STIA area are for illustration and reference only.

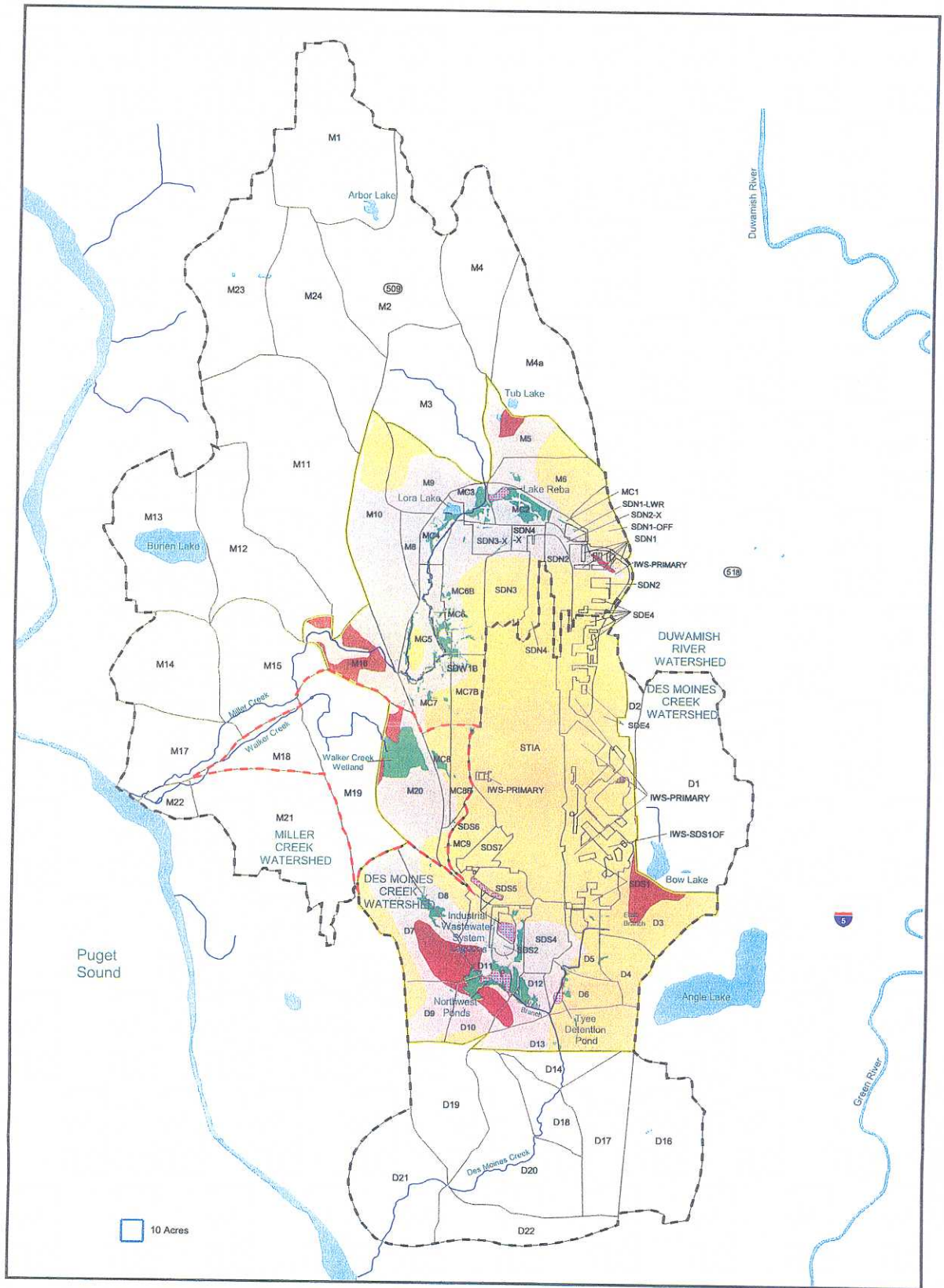
July 2001  
 556-2912-001 (28)



- |  |   |   |   |
|--|---|---|---|
| <ul style="list-style-type: none"> <li>— Existing (1994) Roads</li> <li>— Drainage Subbasins</li> <li>— STIA Area (see note)</li> <li>- - - Watershed Boundary</li> <li>- - - Subwatershed Boundary</li> </ul> | <ul style="list-style-type: none"> <li>— Rivers</li> <li>Water Bodies</li> <li>Constructed Water Features</li> <li>Detention Facilities (existing)</li> </ul> | <ul style="list-style-type: none"> <li>Sea-Tac Airport</li> <li>Commercial</li> <li>Open/Grass</li> <li>Transportation</li> </ul> | <ul style="list-style-type: none"> <li>Low Density Residential</li> <li>High Density Residential</li> <li>Multi-Family Residential</li> <li>Forest</li> </ul> |
|--|---|---|---|

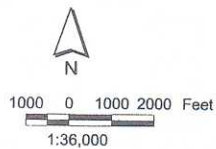
**Figure B1-4**  
**STIA Drainage**  
**Subbasins and**  
**1994 Land Use**

AR 010837



Parametrix, Inc. See-Tac Airport Stormwater Management Plan 556-2912-001(28) 6/00 File: K:\GIS\2012\Arcview\msoasac-ap01a\_may2001.apr  
 Source: Roads based on King County data. Water bodies derived from USGS hydrography data. Estimated wetland boundaries are based on field reconnaissance and interpretation of aerial photographs by Parametrix, Inc.  
 Wetlands are approximate until verified by the ACOE. Soils based on Geologic Map of the Des Moines Quadrangle Washington, (Waldran, 1962, U. S. Geological Survey).

July 2001  
 556-2912-001 (28)

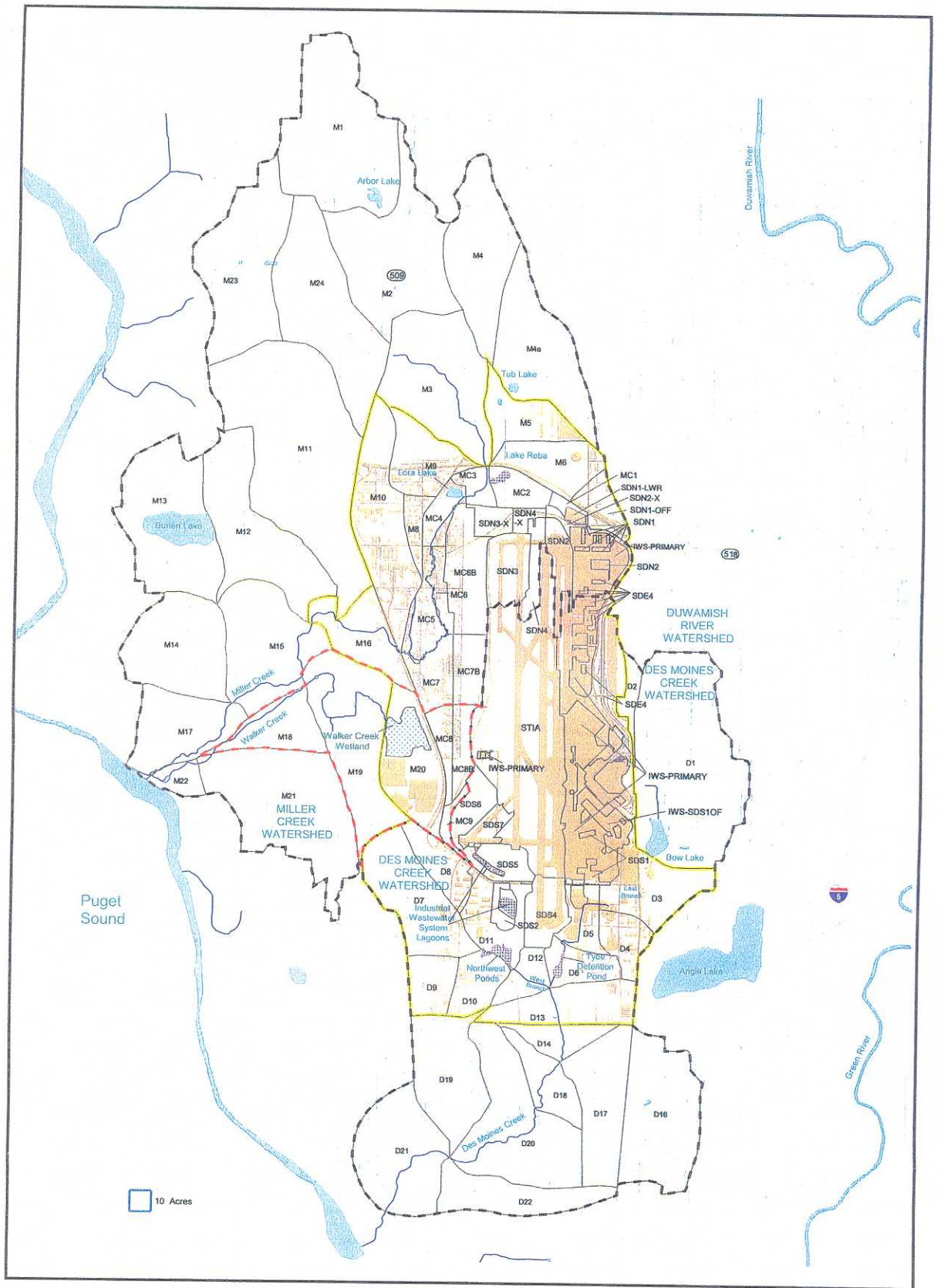


- |       |                                    |   |                            |        |   |            |
|-------|------------------------------------|---|----------------------------|--------|---|------------|
| —     | Roads                              | — | Rivers                     | Soils: | ■ | Till       |
| —     | Existing (1994) Drainage Subbasins | ■ | Water Bodies               | ■      | ■ | Wetland    |
| ■     | STIA Area (see note)               | ■ | Constructed Water Features | ■      | ■ | Outwash    |
| - - - | Watershed Boundary                 | ■ |                            | ■      | ■ | Lacustrine |
| - - - | Subwatershed Boundary              |   |                            |        |   |            |

**Figure B1-5**  
**STIA Drainage**  
**Subbasins and**  
**1994 Soils**

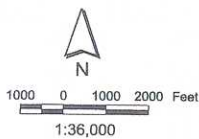
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Parametrix, Inc. Sea-Tac Airport Stormwater Management Plan/556-2912-001(28) 6/00 File: K:\GIS\2912\ArcView\rssea-tac-apdxa\_may2001.apr  
 Source: Roads based on King County data. Water bodies derived from USGS hypsography data. Impervious surface data interpreted from 1993 STIA aerial photograph (Walker and Assoc. 1993).  
 Note: Subbasin boundaries shown outside of STIA area are for illustration and reference only.

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 556-2912-001 (28)



- Roads
- Watershed Boundary
- Subwatershed Boundary
- Rivers
- STIA Area (see note)
- Existing (1994) Drainage Subbasins
- Impervious Areas
- Constructed Water Features
- Water Bodies

**Figure B1-6**  
**STIA Drainage**  
**Subbasins and**  
**1994 Impervious Areas**

resulting 1994 GIS map. This map was used to create the 1994 table used in the HSPF model. The 1994 report figures were also created using the 1994 GIS map data.

Table B1-1, at the end of this Appendix, summarizes the land use information (by subbasin) used for the model calibration based on 1994 watershed conditions. Existing impervious surfaces were assumed to be 100 percent effective for airport drainage to the SDS systems.

Subsequent to the GIS analysis examination of aerial photography identified 23.2 acres within SDS3 that should be categorized as impervious. The results primarily from construction of interconnecting taxiways, during the calibration period. The model was modified to incorporate this.

### 1.3 REGIONAL GROUNDWATER STUDY

In conjunction with an assessment of potential groundwater movement in the vicinity of STIA, the Port of Seattle commissioned a regional groundwater study to be performed by Associated Earth Sciences (AESI) and S.S. Papodopulos and Associates (AESI 1999). As part of that study, a groundwater model is being developed. At this time, preliminary results are available based on water well monitoring that indicate the transport direction of shallow and intermediate groundwater in the vicinity of Miller Creek.

Figure B1-3 shows the inferred direction of groundwater movement within the vicinity of the Des Moines Creek watershed. Note that much of the flow is directed toward a region in the lower portion of the creek just south of South 208<sup>th</sup> Street. Discharging groundwater occurs in this region as evidenced by the drawdown of the groundwater surface potentiometric lines. These form the shape of concentric ellipses in the discharge region shown in Figure B1-3. Other discharges occur upstream of this region where the stream intersects the base of the shallow aquifer layer, as identified by the green x's.

Figure B1-3 also depicts how the shallow aquifer groundwater elevation map was used to estimate the non-contiguous groundwater area contributing to Des Moines Creek. The map was superimposed with an outline of the subbasins used in the Des Moines Creek HSPF model. The approximate 512-acre area that is not contiguous with the surface water drainage topography was identified as three contributing "groundwater sheds" for Des Moines Creek.

### 1.4 CALIBRATION PROCESS AND RESULTS

Calibration of the model was performed using data from three flow gages:

- 11D – Des Moines Creek near the mouth.
- 11C – East branch of Des Moines Creek/Tyee Pond inflow.
- SDS3.

The locations of the Des Moines Creek flow monitoring gages are shown in Figure B1-1. The location of the SDS3 gage is south of subbasin SDS3, between SDS-2 and SDS4.



Parameter value adjustments were made initially to match the total simulated runoff volume with the total flow monitoring volume, within the standard criteria of 10 percent. After a good volume calibration was achieved, parameter values were adjusted to achieve proper simulation of base flows. Finally, detailed storm hydrographs were calibrated, primarily by adjustment of the IRC parameter.

The flow duration curves and hydrographs included at the end of this appendix summarize the results of the calibration at all three locations. For purposes of comparison, additional graphs are included for the following alternative parameter value sets:

- U.S. Geological Survey Regional Calibration Parameters.
- Des Moines Creek Basin Plan parameters. The results of this scenario do not reflect the actual Basin Plan model, but rather the model parameter set in the current calibration model, for comparison.
- HSPF Model Used in Preparation of the November 1999 draft SMP.

Table B1-2 lists parameter values for the calibrated model and the three scenarios mentioned above.

**Table B1-2. Parameter value sets.**

Parameter	Category	Value			
		Calibration	U.S. Geological Survey Regional <sup>1</sup>	Basin Plan	11/99 draftSMP <sup>1</sup>
LZSN	Till Forest	4.5	4.5	4.5	4.5
	Till Grass	4.5	4.5	4.5	4.5
	Outwash Forest	5.0	5.0	5.0	5.0
	Outwash Grass	5.0	5.0	5.0	5.0
INFILT	Till Forest	0.2	0.08	0.2	0.2
	Till Grass	0.075	0.03	0.075	0.075
	Outwash Forest	2.0	2.0	2.0	2.0
	Outwash Grass	0.8	0.8	0.8	0.8
KVARY	Till Forest	0.5	0.5	0.5	0.5
	Till Grass	0.5	0.5	0.5	0.5
	Outwash Forest	0.3	0.3	0.3	0.3
	Outwash Grass	0.3	0.3	0.3	0.3
INFEXP	Till Forest	2.0	2.0	2.0	2.0
	Till Grass	2.0	2.0	2.0	2.0
	Outwash Forest	2.0	2.0	2.0	2.0
	Outwash Grass	2.0	2.0	2.0	2.0
DEEPPFR	Till Forest	0.55	-	0.7	0.6
	Till Grass	0.55	-	0.7	0.6
	Outwash Forest	0.55	-	0.7	0.6
	Outwash Grass	0.55	-	0.7	0.6
BASETP	Till Forest	0.0	0.0	0.0	0.0
	Till Grass	0.0	0.0	0.0	0.0

**Table B1-2. Parameter value sets (continued).**

Parameter	Category	Value			
		Calibration	U.S. Geological Survey Regional <sup>1</sup>	Basin Plan	11/99 draftSMP <sup>1</sup>
UZSN	Outwash Forest	0.0	0.0	0.0	0.0
	Outwash Grass	0.0	0.0	0.0	0.0
	Till Forest	0.50	0.50	0.50	0.50
	Till Grass	0.25	0.25	0.25	0.25
AGWRC	Outwash Forest	0.50	0.50	0.50	0.50
	Outwash Grass	0.50	0.50	0.50	0.50
	Till Forest	0.996	0.996	0.996	0.996
	Till Grass	0.996	0.996	0.996	0.996
INTFW	Outwash Forest	0.996	0.996	0.996	0.996
	Outwash Grass	0.996	0.996	0.996	0.996
	Till Forest	3.0	6.0	3.0	3.0
	Till Grass	3.0	6.0	3.0	3.0
LZETP	Outwash Forest	0.0	0.0	0.0	0.0
	Outwash Grass	0.0	0.0	0.0	0.0
	Till Forest	0.70	0.70	0.70	0.70
	Till Grass	0.25	0.25	0.25	0.25
IRC	Outwash Forest	0.70	0.70	0.70	0.70
	Outwash Grass	0.25	0.25	0.25	0.25
	Till Forest	0.50	0.50	0.25	0.25
	Till Grass	0.50	0.50	0.25	0.25
AGWETP	Outwash Forest	0.70	0.70	0.35	0.35
	Outwash Grass	0.70	0.70	0.35	0.35
	Till Forest	0.0	0.0	0.0	0.0
	Till Grass	0.0	0.0	0.0	0.0
	Outwash Forest	0.0	0.0	0.0	0.0
	Outwash Grass	0.0	0.0	0.0	0.0

<sup>1</sup> Since the calibration model does not distinguish between slope categories, parameter values for moderate slopes were input for comparison.

Parameter values summarized in Table B1-2 generally show consistency among the four parameter value sets. Calibration INFILT values for till soils are higher than USGS values but are consistent with Basin Plan values.

Table B1-3 compares monitored runoff to simulated total runoff volume near the mouth of the Creek for all four scenarios. The results indicate that the best overall volume match at this location is achieved using the calibration and Basin Plan model.

**Table B1-3. Comparison of monitored and simulated runoff volume at the mouth of Des Moines Creek.**

Scenario	Simulated Volume % Difference vs Monitored Volume
Calibrated Model	3%
U.S. Geological Survey Parameters	15%
Basin Plan Parameters	-3%
11/99 draft SMP Model	4%

Comparison of the flow duration curves and hydrographs for the four parameter value sets, near the mouth of Des Moines Creek shows that the calibration file provides the best overall match of peak flows. The other three parameter value sets over simulate peak flows at this location. The calibrated model and the Basin Plan parameters provide the best overall match of base flows near the mouth.

Comparison of hydrographs at SDS3 also shows that the calibration file provides the best overall match of peak flows. The other three parameter value sets over simulate peak flows at this location as well. Examination of the hydrographs indicates that a much better overall match occurs—assuming that only surface runoff is collected in the SDS and conveyed to the monitored discharge point.

Comparison of flow duration curves and hydrographs at the Tyee Pond inflow indicate that all four parameter value sets underestimate both peaks and base flow. Repeated efforts at parameter value and impervious percentage adjustment failed to result in significant improvement of the calibration at this location. Attempts at increasing Tyee Pond simulated peaks and base flow consistently resulted in a poor calibration near the mouth of the creek. Refer to Calibration Issues, below, for further discussion about flow monitoring at this location.

## 1.5 CALIBRATION ISSUES

### 1.5.1 Tyee Pond Inflow Calibration

Although a good calibration was achieved at the mouth, the Tyee pond inflow is under simulating both volume and large peaks. However, a reasonable match is achieved for smaller peak flows. INFILT & IRC were lowered in an attempt to match peaks, with only limited improvement at Tyee Pond, but resulting in a poor calibration near the mouth. Parameters were therefore left at their previous values. This appears to be a hydraulic routing or flow monitoring data problem that can not necessarily be solved by parameter adjustment. Comparison of monitored peak flows at Tyee Pond inflow versus the creek mouth, for the March 23, 1995 storm show that the Tyee Pond inflow peak (~ 62 cfs) is actually higher than the peak flow near the mouth (~ 57 cfs).

According to the Basin Plan, Des Moines Creek has a base flow of approximately 0.55 cfs near the mouth. Flow monitoring data at the Tyee Pond inflow location indicates a base flow of approximately 0.35 cfs. The land use contributing to the east branch of the creek at Tyee Pond is so heavily urbanized that it seems unlikely that enough rainfall can get into groundwater to support 0.35 cfs base flow. If the flow monitoring data is correct at both locations, then 18 percent of the basin pervious area is contributing 64 percent of the creek base flow near the mouth. Additionally,

much more outwash soil is located in the west and downstream subbasins which contributes a much larger portion of rainfall to the groundwater supply and subsequently the base flow.

### **1.5.2 SDS3 Calibration**

For SDS3/airport subbasins, preexisting airport fill is modeled as till soil since the underlying soil is till. Results using Basin Plan model parameters indicate that total simulated runoff volumes are 11 percent greater than recorded volumes. Receding limbs of the hydrographs did not decrease nearly as rapidly as recorded data. Since runoff from SDS3 is collected in a piped system, simulations were performed assuming that only surface runoff discharges from the subbasin through the SDS piping. This produced a much better match with receding limbs.

Since only surface runoff is routed to airport subbasin discharge points, interflow is routed to Des Moines Creek, downstream of the SDS3 collection system. Mapping provided as part of the regional groundwater study referenced in Section 3.4 was utilized for determination of interflow routing. Des Moines Creek subbasins located in the eastern portion of the airport, including interflow from IWS subbasin pervious areas, were routed to Bow Lake. Interflow from SDS3 was routed to Northwest Ponds inflow.

Flow monitoring data from the SDS3 gage has not been observed to exceed approximately 61 cfs for the three largest storms during the calibration period. Since the flow-monitoring gage is at the end of a long pipe with a tee outlet, it is possible that the capacity of that structure is approximately 61 cfs. Parametrix staff observed flow from the discharge pipe to be pulsating during a large storm event, a potential indicator of air binding. Additionally, the upstream manholes on the discharge pipe had to be sealed due to surcharging; further evidence that capacity is reached.

Total simulated volume is only 78 percent of monitored volume;<sup>1</sup> however, SDS3 flow monitoring also indicates a consistent base flow of approximately 0.5 cfs. Since runoff is collected and discharged through a piped system, it is unknown what phenomenon could produce this base flow. One explanation is that the flow monitoring device will not register zero flow. This could account for the undersimulation of total volume at the gage.

### **Gage 11D (Near Mouth)**

According to a discussion with King County staff gage 11D, is believed to be accurate up to approximately 200 cfs. Above 200 cfs a downstream flow restriction creates a backwater at the gage location. Under this condition, small increases in stream flow create larger increases in stage at the gage location. As a result the gage is believed to overestimate flows above 200 cfs.

---

<sup>1</sup> Calibration results for the period October 1, 1989 through September 20, 1996 showed the total simulated volume to be 97 percent of the monitored volume.

## 2. REFERENCES

- Associated Earth Sciences, Inc., and S.S. Papadopulos and Associates, Inc. 1999. Seattle-Tacoma International Airport groundwater study. Conceptual flow model boundary presentation. Prepared for Port of Seattle.
- Earth Tech, Inc. 1997. City of SeaTac Surface Water Plan. Earth Tech, Inc., Bellevue, Washington. Prepared for City of SeaTac, SeaTac, Washington.
- King County Department of Natural Resources. 1997. Des Moines Creek Basin Plan.
- Montgomery Water Group, Inc. 1995. Hydrologic modeling study for Sea-Tac Airport Master Plan Update EIS. Appendix G.
- Parametrix, Inc. 1999. Preliminary comprehensive stormwater management master plan. Prepared for Port of Seattle.

**DES MOINES CREEK HSPF CALIBRATION MODEL INPUT FILE**

**AR 010846**

```

RUN
GLOBAL
*** FILE: DM94RLF2.INP
*** HSPF MODEL OF DES MOINES CREEK
*** PARAMETRIX JUNE, 2000
*** Revised to include King County comments (March 2001)
*** 1994 BASE CONDITION/CALIBRATION MODEL
*** REVISE AIRPORT PERLND = 30% IMP. ASSUMPTION
*** ADD 23.2 AC OF IMPERVIOUS TO SDS 3 THAT WERE LEFT OUT
***
DES MOINES CREEK BASIN HSPF MODEL
START      1948/10/01 00:00  END      1996/09/30 24:00
START      1992/01/01 00:00  END      1996/09/30 24:00***
RUN INTERP OUTPUT LEVEL  0
RESUME     0 RUN         1
END GLOBAL

```

```

FILES
MESSU      24    DM94R.MES
WDM        25    Dmcalib.WDM
END FILES

```

```

OPN SEQUENCE
INGRP                      INDELT 01:00
  PERLND                    16
  PERLND                    26
  PERLND                    34
  PERLND                    44
  PERLND                    54
  PERLND                    65
  PERLND                    66
  PERLND                    67
  PERLND                    68
  IMPLND                    14
  COPY                      50
  COPY                      51
  COPY                      52
  COPY                      53
  COPY                      54
  RCHRES                    360
  RCHRES                    361
  RCHRES                    36
  RCHRES                    390
  RCHRES                    39
  *** COPY                  20
  *** COPY                  3
  COPY                      42
  COPY                      41
  COPY                      10
  COPY                      9
  RCHRES                    4
  RCHRES                    364
  RCHRES                    365
  RCHRES                    366
  COPY                      18
  COPY                      8
  RCHRES                    400
  RCHRES                    41
  RCHRES                    7
  RCHRES                    9
  COPY                      4
  RCHRES                    43
  *** COPY                  15
  RCHRES                    34
  RCHRES                    35
  RCHRES                    37
  RCHRES                    38
  RCHRES                    5
  RCHRES                    40
  RCHRES                    12
  COPY                      5
  RCHRES                    13
  RCHRES                    14
  RCHRES                    177
  RCHRES                    46
  RCHRES                    47

```

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```

RCHRES      17
COPY        48
RCHRES      18
RCHRES      193
COPY         1
RCHRES      198
COPY        100
END INGRP
END OPN SEQUENCE

```

```

COPY
TIMESERIES
Copy-opn
# - # NPT NMN
1 54 1
100 100 4
END TIMESERIES
END COPY

```

```

PERLND
GEN-INFO
<PLS >
# - #
Name NBLKS Unit-systems Printer
User t-series Engr Metr
in. out
16 IFM- TILL FOR MOD 1 1 1 1 60 0
26 TGM- TILL GR MOD 1 1 1 1 60 0
34 OF - OUTWASH FOR 1 1 1 1 60 0
44 OG - OUTWASH GR 1 1 1 1 60 0
54 SA - WETLANDS 1 1 1 1 60 0
65 GW - TILL FOR MOD 1 1 1 1 60 0
66 GW - TILL GR MOD 1 1 1 1 60 0
67 GW - OUTWASH FOR 1 1 1 1 60 0
68 GW - OUTWASH FOR 1 1 1 1 60 0

```

```

END GEN-INFO
ACTIVITY
<PLS > ***** Active Sections *****
# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC
14 68 0 0 1 0 0 0 0 0 0 0 0 0
END ACTIVITY

```

```

PRINT-INFO
<PLS > ***** Print-flags ***** PIVL PYR
# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC *****
14 68 0 0 6 0 0 0 0 0 0 0 0 0 1 9
END PRINT-INFO

```

```

PWAT-PARM1
<PLS > ***** Flags *****
# - # CSNO RTOP UZFG VCS VUZ VNN VIFW VIRC VLE
14 68 0 0 0 0 0 0 0 0 0 0
END PWAT-PARM1

```

```

PWAT-PARM2
<PLS > *****
# - # ***FOREST LZSN INFILT LSUR SLSUR KVARV AGWRC
16 4.5000 0.2000 200.00 0.1000 0.5000 0.9960
26 4.5000 0.0750 400.00 0.1000 0.5000 0.9960
34 5.0000 2.0000 200.00 0.0500 0.3000 0.9960
44 5.0000 0.8000 200.00 0.0500 0.3000 0.9960
54 4.0000 2.0000 200.00 0.0010 0.5000 0.9960
65 4.5000 0.2000 200.00 0.1000 0.5000 0.9960
66 4.5000 0.0750 400.00 0.1000 0.5000 0.9960
67 5.0000 2.0000 200.00 0.0500 0.3000 0.9960
68 5.0000 0.8000 200.00 0.0500 0.3000 0.9960

```

```

END PWAT-PARM2
PWAT-PARM3
<PLS > *****
# - #*** PETMAX PETMIN INFEXP INFILD DEEPFR BASETP AGWETP
16 2.0000 2.0000 2.0000 0.55 0.00 0.0
26 2.0000 2.0000 2.0000 0.55 0.00 0.0
34 2.0000 2.0000 2.0000 0.55 0.00 0.0
44 2.0000 2.0000 2.0000 0.55 0.00 0.0
54 10.0000 2.0000 2.0000 0.55 0.00 0.7
65 2.0000 2.0000 2.0000 0.55 0.00 0.0
66 2.0000 2.0000 2.0000 0.55 0.00 0.0
67 2.0000 2.0000 2.0000 0.55 0.00 0.0

```

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68 2.0000 2.0000 0.55 0.00 0.0

END PWAT-PARM3

PWAT-PARM4

<PLS >

# - #	CEPSC	UZSN	NSUR	INTFW	IRC	LZETP***
16	0.2000	0.5000	0.3500	3.000	0.5000	0.7000
26	0.1000	0.2500	0.2500	3.000	0.5000	0.2500
34	0.2000	0.5000	0.3500	0.000	0.7000	0.7000
44	0.1000	0.5000	0.2500	0.000	0.7000	0.2500
54	0.2000	3.0000	0.5000	1.000	0.7000	0.8000
65	0.2000	0.5000	0.3500	3.000	0.5000	0.7000
66	0.1000	0.2500	0.2500	3.000	0.5000	0.2500
67	0.2000	0.5000	0.3500	0.000	0.7000	0.7000
68	0.1000	0.5000	0.2500	0.000	0.7000	0.2500

END PWAT-PARM4

PWAT-STATE1

<PLS > PWATER state variables\*\*\*

# - #***	CEPS	SURS	UZS	IFWS	LZS	AGWS	GWVS
16	0.078	0.	0.0010	0.	0.075	0.267	0.026
26	0.051	0.	0.0350	0.	1.928	0.680	0.049
34	0.078	0.	0.0010	0.	0.090	0.676	0.038
44	0.051	0.	0.0040	0.	1.127	0.614	0.152
54	0.051	0.	0.3330	0.	0.622	0.000	0.000
65	0.078	0.	0.0010	0.	0.075	0.267	0.026
66	0.051	0.	0.0350	0.	1.928	0.680	0.049
67	0.078	0.	0.0010	0.	0.090	0.676	0.038
68	0.051	0.	0.0040	0.	1.127	0.614	0.152

END PWAT-STATE1

END PERLND

IMPLND

GEN-INFO

<ILS >

# - #	Name	Unit-systems	Printer	***
		User t-series	Engl Metr	***
		in out		***
13 140	IMPERVIOUS	1 1 1	60 0	

END GEN-INFO

ACTIVITY

<ILS >

# - #	ATMP	SNOW	IWAT	SLD	IWG	IQAL	Active Sections	****
13 140	0	0	1	0	0	0		

END ACTIVITY

PRINT-INFO

<ILS >

# - #	ATMP	SNOW	IWAT	SLD	IWG	IQAL	Print-flags	PIVL	PYR	*****
13 140	0	0	6	0	0	0		1	9	

END PRINT-INFO

IWAT-PARM1

<ILS >

# - #	CSNO	RTOP	VRS	VNN	RTLI	Flags	***
13 140	0	0	0	0	0		

END IWAT-PARM1

IWAT-PARM2

<ILS >

# - #	LSUR	SLSUR	NSUR	RETSC	***
14	500.0	0.0100	0.1000	0.100	
140	100.00	0.0500	0.1000	0.0500	

END IWAT-PARM2

IWAT-PARM3

<ILS >

# - #	PETMAX	PETMIN	***
13 140			

END IWAT-PARM3

IWAT-STATE1

<ILS >

# - #	RETS	SURS	IWATER state variables	***
13 140	1.0000E-3	1.0000E-3		

END IWAT-STATE1

END IMPLND

RCHRES

GEN-INFO

RCHRES

# - #	Name	Nexits	Unit Systems	Printer	***
-------	------	--------	--------------	---------	-----

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```

# - #<-----><----> User T-series Engr Metr LKFG          ***
                                     in out
4   SDS-3 Outlet Swale      1 1 1 1 0 0 0
5   E.Branch above TyeeP   1 1 1 1 0 0 0
7   DM7 & DM8 Conveyance   1 1 1 1 0 0 0
9   DM9 Conveyance         1 1 1 1 0 0 0
12  Lower W. Branch        1 1 1 1 0 0 0
13  Confl. to 200th St.    1 1 1 1 0 0 0
14  200th to Exec. Trib.   1 1 1 1 0 0 0
17  Executel Tributary     1 1 1 1 0 0 0
18  Exec.Confl. to 208th   2 1 1 1 0 0 0
34  Bow Lake                2 1 1 1 0 0 1
35  Pipe A Bow LK Outlet    1 1 1 1 0 0 0
36  SDE-4 Combined Disch   1 1 1 1 0 0 0
37  Pipe B 60" Intl Blvd   1 1 1 1 0 0 0
38  Upper E. Branch        1 1 1 1 0 0 0
39  SDS-1 Storm Only       1 1 1 1 0 0 0
40  Tyee Pond              1 1 1 1 0 0 0
41  SDS-2,5,6 & 7 Disch    1 1 1 1 0 0 0
43  Northwest Ponds        2 1 1 1 0 0 1
46  Executel Pond          1 1 1 1 0 0 0
47  Pipe C Exec.Pond Dis   1 1 1 1 0 0 0
177 North Branch Ravine    1 1 1 1 0 0 0
193 Upper Ravine           1 1 1 1 0 0 0
198 Lower Ravine           1 1 1 1 0 0 0
360 SDE-4 NSPS             2 1 1 1 0 0 0
361 SDE-4 CSMPs            2 1 1 1 0 0 0
364 SDN-2 NCPS              2 1 1 1 0 0 0
365 SDN-2 NSMPs            2 1 1 1 0 0 0
366 OTF SSMPS              2 1 1 1 0 0 0
390 SDS-1 IWS Overflow     2 1 1 1 0 0 0
400 IWS Lagoons            2 1 1 1 0 0 0
END GEN-INFO

```

ACTIVITY

```

RCHRES ***** Active Sections *****
# - # HYFG ADFG CNFG HTFG SDFG GQFG OXFG NUFG PKFG PHFG          ***
1 400 1 0 0 0 0 0 0 0 0 0 0
END ACTIVITY

```

PRINT-INFO

```

RCHRES ***** Printout Flags ***** PIVL PYR
# - # HYDR ADCA CONS HEAT SED GQL OXRX NUTR PLNK PHCB *****
1 400 6 0 0 0 0 0 0 0 0 0 0 1 9
END PRINT-INFO

```

HYDR-PARM1

```

RCHRES Flags for each HYDR Section          ***
# - # VC A1 A2 A3 ODFVFG for each *** ODGTFG for each          FUNCT for each
      FG FG FG FG possible exit *** possible exit          possible exit
      * * * * * * * * * * * * * * * * * * * * * * * * * * *
4   17 0 1 1 0 4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 2 2 2 2 2
18  34 0 1 1 0 4 5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 2 2 2 2 2
35  41 0 1 1 0 4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 2 2 2 2 2
43   0 1 1 0 4 5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 2 2 2 2 2
46 198 0 1 1 0 4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 2 2 2 2 2
360 400 0 1 1 0 4 5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 2 2 2 2 2
END HYDR-PARM1

```

HYDR-PARM2

```

RCHRES
# - # FTABNO LEN DELTH STCOR KS DB50          ***
<-----><-----><-----><-----><-----><----->          ***
4   4 0.530
5   5 0.380
7   7 0.341
9   9 0.189
12  12 0.273
13  13 0.218
14  14 0.218
17  17 0.246
18  18 0.303
34  34 0.208
35  35 0.123
36  36 0.100
37  37 0.381
38  38 0.142
39  39 0.100

```

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```

40          40  0.189          0.3
41          41  0.530          0.3
43          43  0.189          0.3
46          46  0.047          0.3
47          47  0.417          0.3
177         177  0.407          0.3
193         193  0.795          0.3
198         198  0.631          0.3
360         360  0.010          0.0
361         361  0.010          0.0
364         364  0.010          0.0
365         365  0.010          0.0
366         366  0.010          0.0
390         390  0.010          0.0
400         400  0.010          0.3
END HYDR-PARM2

```

```

HYDR-INIT
RCHRES Initial conditions for each HYDR section ***
# - # *** VOL Initial value of COLIND Initial value of OUTDGT ***
*** ac-ft for each possible exit for each possible exit
<-----><-----><-----><-----><-----> *** <-----><-----><-----><----->
4 0.1 4.0
5 0.1 4.0
7 0.1 4.0
9 0.1 4.0
12 0.1 4.0
13 0.1 4.0
14 0.1 4.0
17 0.1 4.0
18 0.1 4.0 5.0
34 35. 4.0 5.0
35 0.0 4.0
36 0.0 4.0
37 0.0 4.0
38 0.0 4.0
39 0.0 4.0
40 0.0 4.0
41 0.0 4.0
43 0.7 4.0 5.0
46 0.0 4.0
47 0.0 4.0
177 0.0 4.0
193 0.0 4.0
198 0.0 4.0
360 0.0 4.0 5.0
361 0.0 4.0 5.0
364 0.0 4.0 5.0
365 0.0 4.0 5.0
366 0.0 4.0 5.0
390 0.0 4.0 5.0
400 0.0 4.0 5.0
END HYDR-INIT
END RCHRES

```

FTABLES

```

FTABLE 4
*** SDS-3 OUTLET SWALE
ROWS COLS ***
7 4
*** DEPTH AREA VOLUME OUTFLOW OUTFLOW2 ***
(F) (ACRES) (ACRE-FT) (CFS) (CFS)
.000 .000 0.0 0.0
.500 .198 0.1 9.0
1.000 .236 0.5 30.9
2.000 .306 1.0 115.8
3.000 .376 1.5 265.5
4.000 .446 5.0 491.8
5.000 .517 20.0 806.3
END FTABLE 4

```

```

FTABLE 5
*** (REACH 5: THIS TABLE LOOKS OK)
*** EAST BRANCH ABOVE TYEE POND
ROWS COLS ***

```

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```

13 4
*** DEPTH      AREA      VOLUME    OUTFLOW    OUTFLOW2
    (FT)      (ACRES)  (ACRE-FT) (CFS)      (CFS)
    .000      .000      .000      .000
    .550      .290      .100      4.900
    1.100      .543      .200      20.800
    1.650      .609      .300      46.500
    2.200      .671      .400      80.000
    2.750      .732      0.500     118.700
    3.300      .778      0.600     159.500
    3.850      .819      0.700     198.400
    4.400      .849      0.801     231.900
    4.950      .866      1.000     252.900
    5.500      .865      1.200     253.000
    8.200      .973      1.500     400.000
    10.200     1.043     2.000     520.000
END FTABLE 5

```

```

FTABLE 7
*** DM7 & DM8 CONVEYANCE
ROWS COLS ***
8 4
*** DEPTH      AREA      VOLUME    OUTFLOW    OUTFLOW2
    (FT)      (ACRES)  (ACRE-FT) (CFS)      (CFS)
    .000      .000      .000      .000
    .500      .360      .120      6.200
    1.000      .416      .276      20.800
    2.000      .520      .694      75.400
    3.000      .626      1.252     168.700
    4.000      .732      1.950     306.900
    5.000      .836      2.790     496.100
    6.000      .942      3.768     742.300
END FTABLE 7

```

```

FTABLE 9
*** DM9 CONVEYANCE
ROWS COLS ***
8 4
*** DEPTH      AREA      VOLUME    OUTFLOW    OUTFLOW2
    (FT)      (ACRES)  (ACRE-FT) (CFS)      (CFS)
    .000      .00      0.0       .0
    .500      .20      0.7       9.7
    1.000      .23      3.1       32.6
    2.000      .29      14.9      118
    3.000      .35      38.4      265
    4.000      .41      77        482
    5.000      .47      135       778
    6.000      .52      214       1165
END FTABLE 9

```

```

FTABLE 12
*** LOWER WEST BRANCH
*** REVISED BASED ON HEC-RAS MODEL
ROWS COLS ***
13 4
*** DEPTH      AREA      VOLUME    OUTFLOW    OUTFLOW2
    (FT)      (ACRES)  (ACRE-FT) (CFS)      (CFS)
    .000      .000      0.000     .000
    .500      .291      0.030     0.150
    1.000      .346      0.260     6.600
    2.000      .450      0.430     14.100
    3.000      .554      0.650     25.000
    4.000      .656      1.180     50.000
    5.000      .753      2.170     75.000
    6.000      .796      3.820     100.000
    7.000      .837      8.820     150.000
    8.000      .837      16.200    200.000
    9.000      .837      27.920    250.000
    10.000     .837      33.530    350.000
    11.000     .837      35.380    450.000
END FTABLE 12

```

```

FTABLE 13
*** CONFLUENCE TO 200TH STREET
ROWS COLS ***
9 4

```

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```

*** DEPTH      AREA      VOLUME    OUTFLOW    OUTFLOW2
    (FT)      (ACRES)  (ACRE-FT) (CFS)      (CFS)
    .000      .000     .000      .000
    .500      .153     .051      4.300
    1.000     .272     .132     14.400
    2.000     .317     .312     50.400
    3.000     .360     .544    109.600
    4.000     .404     .826    195.000
    5.000     .450     1.163   309.500
    6.000     .497     1.548   456.300
    7.000     .542     1.984   638.000
END FTABLE 13

```

```

FTABLE 14
*** 200TH STREET TO EXECUTEL TRIBUTARY
*** REACH 190 FROM TR-20/KING COUNTY BASIN PLAN MODEL:
ROWS COLS ***
  5      4
*** DEPTH      AREA      VOLUME    OUTFLOW    OUTFLOW2
    (FT)      (ACRES)  (ACRE-FT) (CFS)      (CFS)
    0.000     0.000     0.000     0.000
    0.900     0.70     0.4000    30.00
    1.800     0.80     1.1000   115.60
    2.700     1.10     2.1000   269.80
    4.200     1.30     4.3000   707.10
END FTABLE 14

```

```

FTABLE 17
*** EXECUTEL TRIBUTARY
ROWS COLS ***
 10      4
*** DEPTH      AREA      VOLUME    OUTFLOW    OUTFLOW2
    (FT)      (ACRES)  (ACRE-FT) (CFS)      (CFS)
    .000      .000     .000     .000
    .300     .169     .034     2.900
    .600     .192     .076     9.800
    .900     .215     .128    20.400
    1.200     .238     .189    35.100
    1.500     .259     .258    54.100
    1.800     .282     .336    77.700
    2.100     .303     .423   106.200
    3.100     .376     .779   245.000
    3.600     .412     .988   335.000
END FTABLE 17

```

```

FTABLE 18
*** CONFLUENCE WITH EXECUTEL TRIBUTARY TO 208TH STREET
*** REPRESENTS GW LOSS IN WETLAND BELOW 200TH
ROWS COLS ***
 14      5
*** DEPTH      AREA      VOLUME    OUTFLOW    OUTFLOW2
    (FT)      (ACRES)  (ACRE-FT) (CFS)      (CFS)
    .000      .000     .000     .000     0.00
    .500      .572     .191     7.300     0.00
    1.000     .799     .438    10.000     0.00
    2.000     .968     1.001    20.700     0.00
    3.000     1.155     1.727   100.000     0.00
    4.000     1.317     2.542   262.700     0.00
    5.000     1.478     3.475   400.300     0.00
    6.000     1.643     4.545   570.200     0.00
    7.000     1.791     5.688   774.400     0.00
    8.000     1.932     6.822  1015.100     0.00
    9.000     1.945     7.025  1294.500     0.00
   10.000     1.958     7.244  1614.500     0.00
   11.000     1.970     7.481  1977.000     0.00
   12.000     1.983     7.734  2384.700     0.00
END FTABLE 18

```

```

FTABLE 34
*** BOW LAKE
*** BASED ON ENTRANCE CONTROL FOR 36 INCH OUTLET PIPE
ROWS COLS ***
  8      5
*** DEPTH      AREA      VOLUME    OUTFLOW    OUTFLOW2
    (FT)      (ACRES)  (ACRE-FT) (CFS)      (CFS)
    0.000     14.000     0.000     0.000     0.00

```

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1.000	14.000	14.000	7.000	0.00
1.500	14.000	21.000	13.000	0.00
2.000	14.000	28.000	17.000	0.00
3.000	14.000	42.000	35.000	0.00
4.000	14.000	56.000	49.000	0.00
5.000	14.000	70.000	60.000	0.00
6.000	14.000	84.000	70.000	0.00

END FTABLE 34

FTABLE 35  
 \*\*\* 36" BOW LAKE DISCHARGE PIPELINE (A)  
 \*\*\* STORAGE ELIMINATED FROM MONTGOMERY MODEL (DMH 1/4/96)  
 ROWS COLS \*\*\*

13	4					
***	DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2	***
	(FT)	(ACRES)	(ACRE-FT)	(CFS)	(CFS)	
	.000	.000	.000	.000		
	.300	.020	.0006	1.000		
	.600	.026	.0026	4.200		
	.900	.032	.0068	9.400		
	1.200	.034	.0134	16.200		
	1.500	.037	.0226	24.000		
	1.800	.039	.0346	32.300		
	2.100	.040	.0492	40.100		
	2.400	.040	.0667	46.900		
	2.700	.039	.0857	51.200		
	3.000	.037	.1000	55.300		
***	SURCHARGING- NEXT LINES ARE FICTITIOUS					
	3.300	.038	.2500	60.300		
	4.000	.038	.4000	80.000		

END FTABLE 35

\*\*\* REACH 36 HUGELY OVER-ATTENUATED FLOWS PROVIDING  
 \*\*\* 25.+ AC-FT OF STORAGE. 1/4/96 DISCUSSION WITH K. RITLAND  
 \*\*\* OF MONTGOMERY GROUP SUGGESTS TR-20 MODEL WAS MISINTERPRETED  
 \*\*\* IN DEVELOPMENT OF ORIGINAL F-TABLE. FOLLOWING TABLE IS CON-  
 \*\*\* SISTENT WITH TR-20 MODEL AND 5700 FOOT LENGTH.  
 \*\*\* NOTE: LENGTH SHORTENED TO 0.1 MILE (HSPF UNITS). MORE CONSISTENT  
 \*\*\* WITH SDE-4 CONVEYANCE

FTABLE 36  
 \*\*\* SDE-4 COMBINED DISCHARGE  
 ROWS COLS \*\*\*

11	4					
***	DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2	***
	(FT)	(ACRES)	(ACRE-FT)	(CFS)	(CFS)	
	.000	.000	.000	.000		
	.400	.343	0.090	2.200		
	.800	.442	0.150	9.500		
	1.200	.523	0.200	21.100		
	1.600	.577	0.250	36.300		
	2.000	.618	0.300	54.000		
	2.400	.646	0.350	72.500		
	2.800	.659	0.400	90.200		
	3.200	.662	0.450	105.500		
	3.600	.649	0.550	115.000		
	4.000	.618	0.650	115.100		

END FTABLE 36

FTABLE 37  
 \*\*\* 60" INTERNATIONAL BLVD PIPELINE (B)  
 \*\*\* (THIS ONE LOOKS OK, DMH 1/4/96)  
 ROWS COLS \*\*\*

13	4					
***	DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2	***
	(FT)	(ACRES)	(ACRE-FT)	(CFS)	(CFS)	
	.000	.000	.000	.000		
	.450	.134	.045	4.800		
	.900	.190	.100	20.300		
	1.350	.225	.150	45.400		
	1.800	.249	.200	78.000		
	2.250	.266	.250	115.900		
	2.700	.271	.300	155.800		
	3.150	.264	.350	193.800		
	3.600	.251	.400	226.500		
	4.050	.238	.450	247.000		
	4.500	.234	.500	247.100		

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6.500	.185	.600	340.000
8.500	.166	.700	415.000

END FTABLE 37

FTABLE 38  
 \*\*\* UPPER EAST BRANCH  
 \*\*\* VOLUMES CORRECTED 1/4/96 (PREVIOUSLY HUGE VOLUMES IN MONTGOMERY TABLE)

ROWS COLS \*\*\*  
 9 4  
 \*\*\* DEPTH AREA VOLUME OUTFLOW OUTFLOW2 \*\*\*  
 (FT) (ACRES) (ACRE-FT) (CFS) (CFS)  
 .000 .000 .000 .000  
 .500 .176 .100 9.200  
 1.000 .194 0.150 30.400  
 2.000 .232 0.200 105.800  
 3.000 .271 0.250 228.900  
 4.000 .310 0.350 405.800  
 5.000 .349 0.450 642.700  
 6.000 .387 0.600 945.700  
 7.000 .426 0.800 1320.700  
 END FTABLE 38

\*\*\* AGAIN, IS THE STORAGE REALISTIC IN THIS TABLE, REACH 39  
 \*\*\* VOLUMES ARE WAY TOO HIGH TO BE CONSISTENT WITH TR-20  
 \*\*\* STORAGE IN MONTGOMERY MODEL HAS BEEN ELIMINATED (DMH 1/4/96)

FTABLE 39  
 \*\*\* SDS-1 DISCHARGE

ROWS COLS \*\*\*  
 11 4  
 \*\*\* DEPTH AREA VOLUME OUTFLOW OUTFLOW2 \*\*\*  
 (FT) (ACRES) (ACRE-FT) (CFS) (CFS)  
 .000 .000 .000 .000  
 .250 .020 .030 2.200  
 .500 .027 .035 9.400  
 .750 .031 .042 21.000  
 1.000 .035 .049 36.000  
 1.250 .039 0.056 53.600  
 1.500 .039 0.064 72.000  
 1.750 .041 0.074 89.500  
 2.000 .041 0.084 104.700  
 2.250 .041 0.094 114.200  
 2.500 .038 0.100 114.300  
 END FTABLE 39

FTABLE 40  
 \*\*\* TYEE POND  
 \*\*\* BASED ON TYEE POND AS-BUILTS AND AUTOMATED GATE OPERATION MANUAL  
 \*\*\* K RITLAND 2/4/98

ROWS COLS \*\*\*  
 20 4  
 \*\*\* DEPTH AREA VOLUME OUTFLOW OUTFLOW2 \*\*\*  
 (FT) (ACRES) (ACRE-FT) (CFS) (CFS)  
 0.00 0.00 0.00 0.00  
 0.90 0.01 0.01 10.00  
 1.65 0.02 0.02 20.00  
 3.11 0.07 0.07 30.00  
 4.56 0.22 0.29 40.00  
 6.02 0.63 0.89 50.00  
 7.48 0.88 2.02 60.00  
 8.62 1.06 3.18 70.00  
 9.79 1.18 4.29 80.00  
 10.88 1.34 5.83 90.00  
 11.99 1.48 7.20 100.00  
 13.12 1.69 9.17 110.00  
 15.13 2.04 12.90 120.00  
 16.10 2.20 14.92 124.10  
 16.30 2.24 15.40 129.65  
 16.57 2.28 15.88 150.36  
 16.64 2.32 16.36 155.00  
 16.80 2.36 16.84 208.74  
 17.03 2.40 17.32 293.59  
 17.26 2.43 17.79 428.11  
 END FTABLE 40

FTABLE 41  
 \*\*\* SDS-2,5,6 & 7 DISCHARGE

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ROWS COLS \*\*\*  
7 4

DEPTH (FT)	AREA (ACRES)	VOLUME (ACRE-FT)	OUTFLOW (CFS)	OUTFLOW2 (CFS)	***
.000	.000	0.0	0.0		
.500	.198	0.1	9.0		
1.000	.236	0.5	30.9		
2.000	.306	1.0	115.8		
3.000	.376	1.5	265.5		
4.000	.446	5.0	491.8		
5.000	.517	20.0	806.3		

END FTABLE 41

FTABLE 43  
\*\*\* NORTHWEST PONDS  
\*\*\* BASED ON KING COUNTY BASIN PLANNING MODEL (DAVID HARTLEY, 12/16/98)

ROWS COLS \*\*\*  
17 5

DEPTH (FT)	AREA (ACRES)	VOLUME (ACRE-FT)	OUTFLOW (CFS)	OUTFLOW2 (CFS)	***
0.000	12.000	0.000	0.000	0.00	
0.100	12.000	18.800	0.000	0.00	
1.000	12.000	24.000	0.200	0.00	
2.000	12.000	30.000	0.500	0.00	
3.000	12.000	37.000	1.000	0.00	
3.500	13.000	41.000	5.000	0.00	
4.000	13.000	45.700	15.000	0.00	
4.500	13.000	51.000	35.000	0.00	
5.000	14.000	56.500	150.000	0.00	
5.500	14.000	62.800	200.000	0.00	
6.000	14.000	69.000	300.000	0.00	
6.500	14.000	83.500	350.000	0.00	
7.000	15.000	99.900	400.000	0.00	
8.000	17.000	119.00	500.000	0.00	
9.000	20.000	141.50	550.000	0.00	
10.000	23.000	180.00	600.000	0.00	
11.000	27.000	200.00	650.000	0.00	

END FTABLE 43

FTABLE 46  
\*\*\* EXECUTEL POND  
ROWS COLS \*\*\*

DEPTH (FT)	AREA (ACRES)	VOLUME (ACRE-FT)	OUTFLOW (CFS)	OUTFLOW2 (CFS)	***
.000	.000	.000	.000		
1.000	.080	.080	24.420		
2.000	.230	.310	34.540		
3.000	.393	.703	42.300		
3.500	.494	.950	45.690		
4.000	.508	1.204	48.850		
4.500	.532	1.470	51.810		
5.000	.540	1.740	54.610		
5.500	.540	2.010	57.280		
6.000	.580	2.300	59.820		
6.500	.600	2.600	62.270		
7.000	.600	2.900	64.620		
7.500	.600	3.200	66.900		
8.000	.620	3.510	69.100		
8.500	.640	3.830	71.200		
9.000	.740	4.200	82.220		
10.000	.650	4.850	119.830		
11.000	.720	5.570	169.000		
12.000	.750	6.320	250.900		
13.000	1.000	7.320	500.900		

END FTABLE 46

FTABLE 47  
\*\*\* EXECUTEL POND DISCHARGE PIPELINE (C)  
ROWS COLS \*\*\*

DEPTH (FT)	AREA (ACRES)	VOLUME (ACRE-FT)	OUTFLOW (CFS)	OUTFLOW2 (CFS)	***
.000	.000	.000	.000		
.350	.069	.020	4.600		
.700	.096	.056	19.200		

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1.050	.112	.099	42.800
1.400	.124	.150	73.400
1.750	.125	.203	109.000
2.100	.121	.240	146.600
2.450	.110	.264	182.400
2.800	.096	.284	213.200
3.150	.090	.290	232.400
3.500	.088	.293	232.600

END FTABLE 47

FTABLE 177  
 \*\*\* NORTH BRANCH RAVINE  
 ROWS COLS \*\*\*

14	4				
***	DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2
	(FT)	(ACRES)	(ACRE-FT)	(CFS)	(CFS)
	.000	.000	.000	.0	
	.500	.572	.191	7.3	
	1.000	.799	.438	23.2	
	2.000	.968	1.001	75.7	
	3.000	1.155	1.727	155.1	
	4.000	1.317	2.542	262.7	
	5.000	1.478	3.475	400.3	
	6.000	1.643	4.545	570.2	
	7.000	1.791	5.688	774.4	
	8.000	1.932	6.822	1015.1	
	9.000	1.945	7.025	1294.5	
	10.000	1.958	7.244	1614.5	
	11.000	1.970	7.481	1977.0	
	12.000	1.983	7.734	2384.7	

END FTABLE177

FTABLE 193  
 \*\*\* UPPER RAVINE  
 ROWS COLS \*\*\*

14	4				
***	DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2
	(FT)	(ACRES)	(ACRE-FT)	(CFS)	(CFS)
	0.00	0.00	0.00	0.0	
	0.35	0.72	0.75	7.8	
	0.70	0.72	1.51	23.5	
	1.05	0.72	2.28	44.3	
	1.40	0.72	3.03	68.2	
	1.75	0.72	3.81	95.8	
	2.10	0.72	4.56	125.2	
	2.45	0.75	5.36	169.0	
	2.80	0.89	6.30	171.5	
	3.15	1.00	7.35	247.6	
	3.50	1.08	8.49	332.7	
	3.85	1.21	9.75	396.5	
	4.20	1.32	11.13	521.2	
	4.55	1.41	12.60	655.5	

END FTABLE193

FTABLE 198  
 ROWS COLS \*\*\*

\*\*\* LOWER RAVINE  
 \*\*\* ROUGH ESTIMATE BASED ON FIELD VISIT OF 12/20/95  
 \*\*\* NEED A 2 FT CONTOUR SURVEY TO IMPROVE DEPTH STORAGE INFO  
 \*\*\* FLOW WAS 6 TO 7 CFS WITH DEPTH OF 8"  
 \*\*\* NEAR OUTLET.  
 \*\*\* DRIVE WHICH REPRESENTS A RESTRICTION ACCORDING TO OBSERVATION  
 \*\*\* CULVERT HYDRAULICS SHOULD BE ANALYSED ALSO.

8	4				
***	DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2
	(FT)	(ACRES)	(ACRE-FT)	(CFS)	(CFS)
	0.00	0.00	0.00	0.0	
	1.00	0.50	0.80	10.0	
	2.00	0.55	1.30	25.0	
	3.00	0.60	1.80	50.0	
	5.00	0.70	2.50	100.0	
***	SUBMERGENCE OF CULVERT				
	10.00	2.50	12.00	245.0	
***	OVERBANK STORAGE				
***	FLOWS BASED ON 243', .03 D-W FACTOR, PLUS LOSS OF 1. VELOCITY HEAD				
	15.00	10.00	40.00	325.0	

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20.00 11.00 90.00 390.0  
 END FTABLE198

FTABLE 360  
 \*\*\* NORTH SATELLITE PUMP STATION (SDE-4) (INSTALLED IN 1995) \*\*\*  
 ROWS COLS \*\*\*  
 5 5 \*\*\*  
 DEPTH AREA VOLUME (IWS) (SDS) \*\*\*  
 (FT) (ACRES) (ACRE-FT) (CFS) (CFS) \*\*\*  
 .0 1.0 .00 0.00 0.00  
 1.00 1.0 .01 4.79 0.00  
 2.00 1.0 .02 4.79 0.00  
 3.00 1.0 .03 4.79 25.00  
 4.00 1.0 .04 4.79 50.00  
 END FTABLE360

FTABLE 361  
 \*\*\* CENTRAL SNOWMELT PUMP STATION (SDE-4) (INSTALLED IN 1998) \*\*\*  
 ROWS COLS \*\*\*  
 5 5 \*\*\*  
 DEPTH AREA VOLUME (IWS) (SDS) \*\*\*  
 (FT) (ACRES) (ACRE-FT) ( FT3/S) ( FT3/S) \*\*\*  
 .0 1.0 .00 0.00 0.00  
 1.00 1.0 .01 1.67 0.00  
 2.00 1.0 .02 1.67 0.00  
 3.00 1.0 .03 1.67 25.00  
 4.00 1.0 .04 1.67 50.00  
 END FTABLE361

FTABLE 364  
 \*\*\* NORTH CARGO PUMP STATION (SDN-2) (INSTALLED IN OCTOBER 1997) \*\*\*  
 ROWS COLS \*\*\*  
 5 5 \*\*\*  
 DEPTH AREA VOLUME (IWS) (SDN) \*\*\*  
 (FT) (ACRES) (ACRE-FT) (CFS) (CFS) \*\*\*  
 .0 1.00 .00 0.00 0.00  
 1.00 1.00 .01 6.13 0.00  
 2.00 1.00 .02 6.13 0.00  
 3.00 1.00 .03 6.13 25.00  
 4.00 1.00 .04 6.13 50.00  
 END FTABLE364

FTABLE 365  
 \*\*\* NORTH SNOWMELT PUMP STATION (SDN-2) (INSTALLED IN LATE 1997/1998) \*\*\*  
 ROWS COLS \*\*\*  
 5 5 \*\*\*  
 DEPTH AREA VOLUME (IWS) (SDN) \*\*\*  
 (FT) (ACRES) (ACRE-FT) (CFS) (CFS) \*\*\*  
 .0 1.00 .00 0.00 0.00  
 1.00 1.00 .01 1.67 0.00  
 2.00 1.00 .02 1.67 0.00  
 3.00 1.00 .03 1.67 25.00  
 4.00 1.00 .04 1.67 50.00  
 END FTABLE365

FTABLE 366  
 \*\*\* SOUTH SNOWMELT (OLYMPIC TANK FARM) PUMP STATION (INSTALLED IN LATE 1997/1998) \*\*\*  
 ROWS COLS \*\*\*  
 5 5 \*\*\*  
 DEPTH AREA VOLUME (IWS) (SDS) \*\*\*  
 (FT) (ACRES) (ACRE-FT) (CFS) (CFS) \*\*\*  
 .0 1.00 .00 0.00 0.00  
 1.00 1.00 .01 1.67 0.00  
 2.00 1.00 .02 1.67 0.00  
 3.00 1.00 .03 1.67 25.00  
 4.00 1.00 .04 1.67 50.00  
 END FTABLE366

FTABLE 390  
 \*\*\* IWS OVERFLOW TO SDS-1 \*\*\*  
 \*\*\* BASED ON SYMONDS MEMO 1/6/98, FROM WATERWORKS MODELING \*\*\*  
 ROWS COLS \*\*\*  
 7 5 \*\*\*  
 DEPTH AREA VOLUME (IWS) (SDS) \*\*\*  
 (FT) (ACRES) (ACRE-FT) (CFS) (CFS) \*\*\*  
 .0 .00 .000 .00 .00

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1.00	.20	.001	4.70	0.00
1.50	.20	.002	4.90	0.50
2.00	.20	.003	5.60	3.00
2.50	.23	.004	5.65	4.40
3.00	.23	.005	5.70	6.30
4.00	.23	.006	6.00	12.00

END FTABLE390

```

FTABLE      400
ROWS COLS   IWS LAGOONS
*** OUTFLOW1 = 2/4 MGD OUTPUT OF TREATED EFFLUENT TO SOUND
*** OUTFLOW2 = OVERFLOW TO NORTHWEST PONDS
9           5
DEPTH       AREA      VOLUME  IWS - PS  OVERFLOW
(FEET)      (ACRES)   (ACRE-FT) (CFS)     (CFS)
.0           0.0       .00      0.00     .00
2.0          7.0       20.0     3.09     0.00
4.0          7.0       34.0     3.09     0.00
6.0          7.5       48.0     3.09     0.00
8.0          8.0       63.0     6.19     0.00
10.0         8.0       79.0     6.19     0.00
*** THREAT OF OVERFLOW, THEN PLANT INCREASES TREATMENT RATE, BUT
*** LOSES TREATMENT EFFICIENCY IN ORDER TO AVOID DISCHARGE TO
*** DES MOINES CREEK SYSTEM
11.5         8.0       90.5     11.00    0.00
11.6         8.0       91.0     11.00   100.00
11.7         8.0       91.5     11.00   300.00
END FTABLE400
END FTABLES

```

```

EXT SOURCES
<-Volume-> <Member> SsysSgap<--Mult-->Tran <-Target vols> <-Grp> <-Member-> ***
<Name> # <Name> # tem strg<-factor->strg <Name> # # <Name> # # ***
WDM 2 PREC ENGLZERO PERLND 14 68 EXTNL PREC
WDM 2 PREC ENGLZERO IMPLND 14 EXTNL PREC
WDM 2 PREC ENGLZERO RCHRES 34 EXTNL PREC
WDM 2 PREC ENGLZERO RCHRES 40 EXTNL PREC
WDM 2 PREC ENGLZERO RCHRES 43 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 PERLND 14 440 EXTNL PETINP
WDM 1 EVAP ENGLZERO 0.8 IMPLND 14 140 EXTNL PETINP
WDM 1 EVAP ENGLZERO 0.8 RCHRES 34 EXTNL POTEV
WDM 1 EVAP ENGLZERO 0.8 RCHRES 43 EXTNL POTEV
WDM 1 EVAP ENGLZERO 0.8 RCHRES 40 EXTNL POTEV
END EXT SOURCES

```

```

EXT TARGETS
<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Volume-> <Member> Tsys Tgap Amd ***
<Name> # <Name> # #<-factor->strg <Name> # <Name> tem strg strg***
*****
*** TO WDM ONLY FOR WATER BALANCE CALIBRATION
COPY 50 OUTPUT MEAN 1 12.1 WDM 50 TAET ENGL REPL
COPY 51 OUTPUT MEAN 1 12.1 WDM 51 IMPE ENGL REPL
COPY 52 OUTPUT MEAN 1 WDM 52 UZS ENGL REPL
COPY 53 OUTPUT MEAN 1 WDM 53 LZS ENGL REPL
COPY 54 OUTPUT MEAN 1 WDM 54 AGWS ENGL REPL
*****

```

```

*** SDS
*****
*** SDE-4 (TOTAL)
RCHRES 36 HYDR RO WDM 21 FLOW ENGL REPL
*****
*** SDS-1 (TOTAL)
RCHRES 39 HYDR RO WDM 22 FLOW ENGL REPL
*****
*** SDS-3 RUNWAY (NO DETENTION)
COPY 42 OUTPUT MEAN 1 12.1 WDM 23 FLOW ENGL REPL
*****
*** SDS-3A TAXIWAY VAULT
COPY 20 OUTPUT MEAN 1 12.1 *** WDM 24 FLOW ENGL REPL
*****
*** SDS-7 3RD RUNWAY VAULT
COPY 3 OUTPUT MEAN 1 12.1 *** WDM 26 FLOW ENGL REPL
*****
*** SDS-4

```

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COPY 10 OUTPUT MEAN 1 12.1 WDM 28 FLOW ENGL REPL  
\*\*\*\*\*

\*\*\* IWS  
\*\*\*\*\*  
\*\*\* IWS TREATMENT INFLOW TOTAL  
COPY 8 OUTPUT MEAN 1 12.1 WDM 32 FLOW ENGL REPL  
\*\*\*\*\*  
\*\*\* IWS LAGOON OUTFLOW TO PUGET SOUND  
RCHRES 400 HYDR 0 1 WDM 33 FLOW ENGL REPL  
\*\*\*\*\*  
\*\*\* IWS LAGOON OVERFLOW TO NORTHWEST PONDS  
RCHRES 400 HYDR 0 2 WDM 34 FLOW ENGL REPL  
\*\*\*\*\*

\*\*\* WEST BRANCH  
\*\*\*\*\*  
\*\*\* NORTHWEST PONDS  
RCHRES 43 HYDR RO WDM 31 FLOW ENGL REPL  
\*\*\*\*\*  
\*\*\* LOWER WEST BRANCH  
RCHRES 12 HYDR RO WDM 35 FLOW ENGL REPL  
\*\*\*\*\*

\*\*\* EAST BRANCH  
\*\*\*\*\*  
\*\*\* BOW LAKE OUTFLOW  
RCHRES 35 HYDR RO WDM 36 FLOW ENGL REPL  
\*\*\*\*\*  
\*\*\* EXISTING UPPER EAST BRANCH (FUTURE SASA DETENTION SITE)  
RCHRES 38 HYDR RO WDM 37 FLOW ENGL REPL  
\*\*\*\*\*  
\*\*\* TYEE INFLOW (GAUGE 11C)  
RCHRES 5 HYDR RO WDM 38 FLOW ENGL REPL  
\*\*\*\*\*  
\*\*\* TYEE OUTFLOW  
RCHRES 40 HYDR RO WDM 39 FLOW ENGL REPL  
\*\*\*\*\*

\*\*\* MAIN STEM  
\*\*\*\*\*  
\*\*\* BELOW CONFLUENCE AT TYEE GOLF COURSE WEIR (GAUGE 11F)  
COPY 5 OUTPUT MEAN 1 12.1 WDM 40 FLOW ENGL REPL  
\*\*\*\*\*  
\*\*\* BELOW CONFLUENCE AT SOUTH 200TH STREET  
RCHRES 13 HYDR RO WDM 41 FLOW ENGL REPL  
\*\*\*\*\*  
\*\*\* LOWER DES MOINES CREEK NEAR MOUTH (GAUGE 11D)  
RCHRES 198 HYDR RO WDM 42 FLOW ENGL REPL  
\*\*\*\*\*  
\*\*\* S200TH ST  
COPY 100 OUTPUT MEAN 1 0.000343 WDM 73 AGWO ENGL AGGR REPL  
COPY 100 OUTPUT MEAN 2 0.000417 WDM 74 PETM ENGL AGGR REPL  
COPY 100 OUTPUT MEAN 3 0.000417 WDM 75 SAET ENGL AGGR REPL  
COPY 100 OUTPUT MEAN 4 WDM 76 IMPE ENGL AGGR REPL

END EXT TARGETS  
\*\*\*

NETWORK  
\*\*\* <MEMBER> SSSSSGAP<--MULT-->TRAN <-TARGET VOLS> <-MEMBER->  
<NAME> # <NAME> TEM STRG<-FACTOR->STRG <NAME> # # <-GRP> <NAME> # # \*\*\*  
\*\*\*\*\*  
\*\*\* TO WDM ONLY FOR WATER BALANCE CALIBRATION  
PERLND 16 PWATER TAET 21.898 COPY 50 INPUT MEAN 1  
PERLND 26 PWATER TAET 82.595 COPY 50 INPUT MEAN 1  
PERLND 34 PWATER TAET 21.522 COPY 50 INPUT MEAN 1  
PERLND 44 PWATER TAET 53.793 COPY 50 INPUT MEAN 1  
PERLND 54 PWATER TAET 5.520 COPY 50 INPUT MEAN 1  
\*\*\*\*\*  
IMPLND 14 IWATER IMPEV 97.745 COPY 51 INPUT MEAN 1  
\*\*\*\*\*  
PERLND 16 PWATER UZS 21.898 COPY 52 INPUT MEAN 1  
PERLND 26 PWATER UZS 82.595 COPY 52 INPUT MEAN 1  
PERLND 34 PWATER UZS 21.522 COPY 52 INPUT MEAN 1  
PERLND 44 PWATER UZS 53.793 COPY 52 INPUT MEAN 1

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PERLND 54 PWATER UZS          5.520      COPY 52      INPUT MEAN 1
*****
PERLND 16 PWATER LZS          21.898      COPY 53      INPUT MEAN 1
PERLND 26 PWATER LZS          82.595      COPY 53      INPUT MEAN 1
PERLND 34 PWATER LZS          21.522      COPY 53      INPUT MEAN 1
PERLND 44 PWATER LZS          53.793      COPY 53      INPUT MEAN 1
PERLND 54 PWATER LZS          5.520      COPY 53      INPUT MEAN 1
*****
PERLND 16 PWATER AGWS          21.898      COPY 54      INPUT MEAN 1
PERLND 26 PWATER AGWS          82.595      COPY 54      INPUT MEAN 1
PERLND 34 PWATER AGWS          21.522      COPY 54      INPUT MEAN 1
PERLND 44 PWATER AGWS          53.793      COPY 54      INPUT MEAN 1
PERLND 54 PWATER AGWS          5.520      COPY 54      INPUT MEAN 1
*****
*** AIRPORT SUBBASINS
*****
*** (DM23) SDE-4
PERLND 26 PWATER SURO          4.631      RCHRES 36      EXTNL IVOL
PERLND 26 PWATER IFWO          4.631      RCHRES 34      EXTNL IVOL
IMPLND 14 IWATER SURO          10.205     RCHRES 36      EXTNL IVOL
*****
*** (DM24) SDS-1
PERLND 26 PWATER SURO          0.123      RCHRES 39      EXTNL IVOL
PERLND 26 PWATER IFWO          0.123      RCHRES 38      EXTNL IVOL
IMPLND 14 IWATER SURO          0.915      RCHRES 39      EXTNL IVOL
*****
*** (DM25) SDS-3A TAXIWAY VAULT (to wdm only)
*** 1994 TARGET FLOW FOR FUTURE DETENTION FACILITY
PERLND 26 PWATER SURO          *****    COPY 20      INPUT MEAN 1
PERLND 26 PWATER IFWO          *****    COPY 4       INPUT MEAN 1
IMPLND 14 IWATER SURO          *****    COPY 20     INPUT MEAN 1
*****
*** (DM25) SDS-7 3RD RUNWAY (to wdm only)
*** 1994 TARGET FLOW FOR FUTURE DETENTION FACILITY
PERLND 26 PWATER SURO          *****    COPY 3       INPUT MEAN 1
PERLND 26 PWATER IFWO          *****    COPY 4       INPUT MEAN 1
IMPLND 14 IWATER SURO          *****    COPY 3       INPUT MEAN 1
*****
*** (DM25) SDS-3
*** Areas OF 26 AND 14 revised to incorporate King County comments
*** 1994 LAND USE ADJUSTED TO INCLUDE 23.2 ACRES OF RECENTLY CONSTRUCTED IMPERVIOUS
PERLND 26 PWATER SURO          21.496     COPY 42      INPUT MEAN 1
PERLND 26 PWATER IFWO          21.496     RCHRES 4     EXTNL IVOL
PERLND 54 PWATER SURO          0.039     COPY 42      INPUT MEAN 1
PERLND 54 PWATER IFWO          0.039     RCHRES 4     EXTNL IVOL
IMPLND 14 IWATER SURO          15.512     COPY 42      INPUT MEAN 1
*****
*** (DM26) SDS-2,5,6,7
*** AGWO for PERLND 26 inserted to answer King County comment
PERLND 16 PWATER SURO          0.228     RCHRES 41     EXTNL IVOL
PERLND 16 PWATER IFWO          0.228     RCHRES 43     EXTNL IVOL
PERLND 16 PWATER AGWO          0.228     RCHRES 43     EXTNL IVOL
PERLND 26 PWATER SURO          8.002     RCHRES 41     EXTNL IVOL
PERLND 26 PWATER IFWO          8.002     RCHRES 43     EXTNL IVOL
PERLND 26 PWATER AGWO          8.002     RCHRES 43     EXTNL IVOL
PERLND 34 PWATER SURO          0.676     RCHRES 41     EXTNL IVOL
PERLND 34 PWATER IFWO          0.676     RCHRES 43     EXTNL IVOL
PERLND 34 PWATER AGWO          0.676     RCHRES 43     EXTNL IVOL
PERLND 44 PWATER SURO          0.042     RCHRES 41     EXTNL IVOL
PERLND 44 PWATER IFWO          0.042     RCHRES 43     EXTNL IVOL
PERLND 44 PWATER AGWO          0.042     RCHRES 43     EXTNL IVOL
PERLND 54 PWATER SURO          0.043     RCHRES 41     EXTNL IVOL
PERLND 54 PWATER IFWO          0.043     RCHRES 43     EXTNL IVOL
PERLND 54 PWATER AGWO          0.043     RCHRES 43     EXTNL IVOL
IMPLND 14 IWATER SURO          1.096     RCHRES 41     EXTNL IVOL
*****
*** (DM27) SDS-4
PERLND 26 PWATER SURO          0.755     COPY 10      INPUT MEAN 1
PERLND 26 PWATER IFWO          0.755     COPY 5       INPUT MEAN 1
PERLND 26 PWATER AGWO          0.755     COPY 5       INPUT MEAN 1
PERLND 34 PWATER SURO          0.018     COPY 10      INPUT MEAN 1
PERLND 34 PWATER IFWO          0.018     COPY 5       INPUT MEAN 1
PERLND 34 PWATER AGWO          0.018     COPY 5       INPUT MEAN 1
PERLND 44 PWATER SURO          2.406     COPY 10      INPUT MEAN 1
PERLND 44 PWATER IFWO          2.406     COPY 5       INPUT MEAN 1

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PERLND 44 PWATER AGWO          2.406      COPY    5      INPUT MEAN  1
IMPLND 14 IWATER SURO          1.614      COPY   10      INPUT MEAN  1
*****
*** SASA ROOFTOP (FUTURE)
IMPLND 14 IWATER SURO          *****      COPY   15      INPUT MEAN  1
*****

*** IWS SYSTEM PRIMARY SYSTEM AND PUMP STATIONS
*** (INCLUDING AREAS DIVERTED FROM MILLER CREEK BASIN)
*****
*** I-1: NORTH CARGO PUMP STATION TO IWS (SDN-2)
*** OVERFLOW TO MILLER BASIN
*** INSTALLED IN OCTOBER 1997
PERLND 26 PWATER SURO          *****      RCHRES 364      EXTNL  IVOL
PERLND 26 PWATER IFWO          *****      RCHRES 364      EXTNL  IVOL
IMPLND 14 IWATER SURO          *****      RCHRES 364      EXTNL  IVOL
*****
*** I-2: NORTH SNOWMELT PUMP STATION TO IWS (SDN-2)
*** INSTALLED IN LATE 1997/1998
PERLND 26 PWATER SURO          *****      RCHRES 365      EXTNL  IVOL
PERLND 26 PWATER IFWO          *****      RCHRES 365      EXTNL  IVOL
IMPLND 14 IWATER SURO          *****      RCHRES 365      EXTNL  IVOL
*****
*** I-3: NORTH SATELLITE PUMP STATION TO IWS
*** OVERFLOW TO SDE-4
*** INSTALLED IN 1995
IMPLND 14 IWATER SURO          *****      RCHRES 360      EXTNL  IVOL
*****
*** I-4: CENTRAL SNOWMELT PUMP STATION TO IWS
*** OVERFLOW TO SDE-4
*** INSTALLED IN 1998
IMPLND 14 IWATER SURO          *****      RCHRES 361      EXTNL  IVOL
*****
*** I-5: SOUTH SNOWMELT (OLYMPIC TANK FARM) PUMP STATION TO IWS
*** OVERFLOW TO DES MOINES EAST BRANCH
*** INSTALLED IN LATE 1997/1998
IMPLND 14 IWATER SURO          *****      RCHRES 366      EXTNL  IVOL
*****
*** I-6: IWS-510 DIVERSION TO IWS
*** OVERFLOW TO SDS-1
PERLND 26 PWATER SURO          0.036      RCHRES 390      EXTNL  IVOL
PERLND 26 PWATER IFWO          0.036      RCHRES  34      EXTNL  IVOL
IMPLND 14 IWATER SURO          2.738      RCHRES 390      EXTNL  IVOL
*****
*** I-7: IWS - PRIMARY DIRECT PIPED
*** NO OVERFLOW TO SDS
PERLND 16 PWATER SURO          0.222      COPY    18      INPUT MEAN  1
PERLND 16 PWATER IFWO          0.222      RCHRES  34      EXTNL  IVOL
PERLND 26 PWATER SURO          0.598      COPY    18      INPUT MEAN  1
PERLND 26 PWATER IFWO          0.598      RCHRES  34      EXTNL  IVOL
PERLND 34 PWATER SURO          0.158      COPY    18      INPUT MEAN  1
PERLND 34 PWATER IFWO          0.158      RCHRES  34      EXTNL  IVOL
PERLND 44 PWATER SURO          0.009      COPY    18      INPUT MEAN  1
PERLND 44 PWATER IFWO          0.009      RCHRES  34      EXTNL  IVOL
IMPLND 14 IWATER SURO          19.966     COPY    18      INPUT MEAN  1
*****

*** EAST BRANCH OF CREEK
*****
*** DMI
PERLND 16 PWATER SURO          0.860      RCHRES  34      EXTNL  IVOL
PERLND 16 PWATER IFWO          0.860      RCHRES  34      EXTNL  IVOL
PERLND 16 PWATER AGWO          0.238      RCHRES  34      EXTNL  IVOL
PERLND 26 PWATER SURO          11.078     RCHRES  34      EXTNL  IVOL
PERLND 26 PWATER IFWO          11.078     RCHRES  34      EXTNL  IVOL
PERLND 26 PWATER AGWO          3.069      RCHRES  34      EXTNL  IVOL
PERLND 34 PWATER SURO          0.599      RCHRES  34      EXTNL  IVOL
PERLND 34 PWATER IFWO          0.599      RCHRES  34      EXTNL  IVOL
PERLND 34 PWATER AGWO          0.166      RCHRES  34      EXTNL  IVOL
PERLND 44 PWATER SURO          7.697      RCHRES  34      EXTNL  IVOL
PERLND 44 PWATER IFWO          7.697      RCHRES  34      EXTNL  IVOL
PERLND 44 PWATER AGWO          2.132      RCHRES  34      EXTNL  IVOL
PERLND 54 PWATER SURO          1.176      RCHRES  34      EXTNL  IVOL
PERLND 54 PWATER IFWO          1.176      RCHRES  34      EXTNL  IVOL
PERLND 54 PWATER AGWO          0.326      RCHRES  34      EXTNL  IVOL
IMPLND 14 IWATER SURO          14.274     RCHRES  34      EXTNL  IVOL

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*** DM2
PERLND 26 PWATER SURO          1.232      RCHRES 37      EXTNL IVOL
PERLND 26 PWATER IFWO          1.232      RCHRES 37      EXTNL IVOL
IMPLND 14 IWATER SURO          0.821      RCHRES 37      EXTNL IVOL
*****
*** DM3
PERLND 26 PWATER PERO          4.077      RCHRES 38      EXTNL IVOL
PERLND 54 PWATER PERO          0.028      RCHRES 38      EXTNL IVOL
IMPLND 14 IWATER SURO          6.205      RCHRES 38      EXTNL IVOL
*****
*** DM4
PERLND 16 PWATER PERO          0.574      RCHRES 38      EXTNL IVOL
PERLND 26 PWATER PERO          0.571      RCHRES 38      EXTNL IVOL
PERLND 54 PWATER PERO          0.037      RCHRES 38      EXTNL IVOL
IMPLND 14 IWATER SURO          3.234      RCHRES 38      EXTNL IVOL
*****
*** DM5
PERLND 16 PWATER PERO          1.310      RCHRES 5       EXTNL IVOL
PERLND 26 PWATER PERO          0.558      RCHRES 5       EXTNL IVOL
PERLND 34 PWATER PERO          0.114      RCHRES 5       EXTNL IVOL
PERLND 44 PWATER PERO          0.384      RCHRES 5       EXTNL IVOL
PERLND 54 PWATER PERO          0.171      RCHRES 5       EXTNL IVOL
IMPLND 14 IWATER SURO          1.156      RCHRES 5       EXTNL IVOL
*****
*** DM6
PERLND 16 PWATER PERO          1.814      RCHRES 40      EXTNL IVOL
PERLND 26 PWATER PERO          1.153      RCHRES 40      EXTNL IVOL
PERLND 44 PWATER PERO          0.917      RCHRES 40      EXTNL IVOL
PERLND 54 PWATER PERO          0.337      RCHRES 40      EXTNL IVOL
IMPLND 14 IWATER SURO          0.354      RCHRES 40      EXTNL IVOL
*****
*** WEST BRANCH OF CREEK
*****
*** DM7
PERLND 16 PWATER SURO          2.189      RCHRES 7       EXTNL IVOL
PERLND 16 PWATER IFWO          2.189      RCHRES 7       EXTNL IVOL
PERLND 16 PWATER AGWO          0.792      RCHRES 7       EXTNL IVOL
PERLND 26 PWATER SURO          2.944      RCHRES 7       EXTNL IVOL
PERLND 26 PWATER IFWO          2.944      RCHRES 7       EXTNL IVOL
PERLND 26 PWATER AGWO          1.066      RCHRES 7       EXTNL IVOL
PERLND 34 PWATER SURO          1.966      RCHRES 7       EXTNL IVOL
PERLND 34 PWATER IFWO          1.966      RCHRES 7       EXTNL IVOL
PERLND 34 PWATER AGWO          0.712      RCHRES 7       EXTNL IVOL
PERLND 44 PWATER SURO          4.138      RCHRES 7       EXTNL IVOL
PERLND 44 PWATER IFWO          4.138      RCHRES 7       EXTNL IVOL
PERLND 44 PWATER AGWO          1.498      RCHRES 7       EXTNL IVOL
PERLND 54 PWATER SURO          0.551      RCHRES 7       EXTNL IVOL
PERLND 54 PWATER IFWO          0.551      RCHRES 7       EXTNL IVOL
PERLND 54 PWATER AGWO          0.199      RCHRES 7       EXTNL IVOL
IMPLND 14 IWATER SURO          2.397      RCHRES 7       EXTNL IVOL
*****
*** DM8
PERLND 16 PWATER SURO          0.202      RCHRES 7       EXTNL IVOL
PERLND 16 PWATER IFWO          0.202      RCHRES 7       EXTNL IVOL
PERLND 16 PWATER AGWO          0.077      RCHRES 7       EXTNL IVOL
PERLND 26 PWATER SURO          0.718      RCHRES 7       EXTNL IVOL
PERLND 26 PWATER IFWO          0.718      RCHRES 7       EXTNL IVOL
PERLND 26 PWATER AGWO          0.273      RCHRES 7       EXTNL IVOL
PERLND 34 PWATER SURO          0.709      RCHRES 7       EXTNL IVOL
PERLND 34 PWATER IFWO          0.709      RCHRES 7       EXTNL IVOL
PERLND 34 PWATER AGWO          0.269      RCHRES 7       EXTNL IVOL
PERLND 44 PWATER SURO          1.111      RCHRES 7       EXTNL IVOL
PERLND 44 PWATER IFWO          1.111      RCHRES 7       EXTNL IVOL
PERLND 44 PWATER AGWO          0.422      RCHRES 7       EXTNL IVOL
PERLND 54 PWATER SURO          0.162      RCHRES 7       EXTNL IVOL
PERLND 54 PWATER IFWO          0.162      RCHRES 7       EXTNL IVOL
PERLND 54 PWATER AGWO          0.062      RCHRES 7       EXTNL IVOL
IMPLND 14 IWATER SURO          2.332      RCHRES 7       EXTNL IVOL
*****
*** DM9
PERLND 16 PWATER SURO          0.002      RCHRES 9       EXTNL IVOL
PERLND 16 PWATER IFWO          0.002      RCHRES 9       EXTNL IVOL
PERLND 16 PWATER AGWO          0.001      RCHRES 9       EXTNL IVOL
PERLND 26 PWATER SURO          1.200      RCHRES 9       EXTNL IVOL

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PERLND 26 PWATER IFWO          1.200      RCHRES  9      EXTNL  IVOL
PERLND 26 PWATER AGWO          0.524      RCHRES  9      EXTNL  IVOL
PERLND 34 PWATER SURO          0.016      RCHRES  9      EXTNL  IVOL
PERLND 34 PWATER IFWO          0.016      RCHRES  9      EXTNL  IVOL
PERLND 34 PWATER AGWO          0.007      RCHRES  9      EXTNL  IVOL
PERLND 44 PWATER SURO          3.039      RCHRES  9      EXTNL  IVOL
PERLND 44 PWATER IFWO          3.039      RCHRES  9      EXTNL  IVOL
PERLND 44 PWATER AGWO          1.328      RCHRES  9      EXTNL  IVOL
PERLND 54 PWATER SURO          0.010      RCHRES  9      EXTNL  IVOL
PERLND 54 PWATER IFWO          0.010      RCHRES  9      EXTNL  IVOL
PERLND 54 PWATER AGWO          0.004      RCHRES  9      EXTNL  IVOL
IMPLND 14 IWATER SURO          0.633      RCHRES  9      EXTNL  IVOL
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\*\*\* DM10

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PERLND 16 PWATER PERO          0.943      RCHRES 43      EXTNL  IVOL
PERLND 26 PWATER PERO          0.738      RCHRES 43      EXTNL  IVOL
PERLND 34 PWATER PERO          1.934      RCHRES 43      EXTNL  IVOL
PERLND 44 PWATER PERO          0.510      RCHRES 43      EXTNL  IVOL
PERLND 54 PWATER PERO          0.710      RCHRES 43      EXTNL  IVOL
IMPLND 14 IWATER SURO          0.185      RCHRES 43      EXTNL  IVOL
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\*\*\* DM11

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PERLND 16 PWATER PERO          0.321      RCHRES 43      EXTNL  IVOL
PERLND 26 PWATER PERO          0.514      RCHRES 43      EXTNL  IVOL
PERLND 34 PWATER PERO          1.028      RCHRES 43      EXTNL  IVOL
PERLND 44 PWATER PERO          0.466      RCHRES 43      EXTNL  IVOL
PERLND 54 PWATER PERO          1.036      RCHRES 43      EXTNL  IVOL
IMPLND 14 IWATER SURO          2.618      RCHRES 43      EXTNL  IVOL
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\*\*\* DM12

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PERLND 16 PWATER PERO          0.511      RCHRES 12      EXTNL  IVOL
PERLND 26 PWATER PERO          0.001      RCHRES 12      EXTNL  IVOL
PERLND 34 PWATER PERO          0.376      RCHRES 12      EXTNL  IVOL
PERLND 44 PWATER PERO          2.114      RCHRES 12      EXTNL  IVOL
PERLND 54 PWATER PERO          0.543      RCHRES 12      EXTNL  IVOL
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\*\*\* DM13

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PERLND 16 PWATER PERO          2.684      RCHRES 13      EXTNL  IVOL
PERLND 26 PWATER PERO          1.886      RCHRES 13      EXTNL  IVOL
PERLND 34 PWATER PERO          1.204      RCHRES 13      EXTNL  IVOL
PERLND 44 PWATER PERO          1.845      RCHRES 13      EXTNL  IVOL
PERLND 54 PWATER PERO          0.068      RCHRES 13      EXTNL  IVOL
IMPLND 14 IWATER SURO          1.978      RCHRES 13      EXTNL  IVOL
*****

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\*\*\* LOWER BASIN

\*\*\* DM14

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PERLND 16 PWATER PERO          0.481      RCHRES 14      EXTNL  IVOL
PERLND 26 PWATER PERO          0.295      RCHRES 14      EXTNL  IVOL
PERLND 34 PWATER PERO          1.940      RCHRES 14      EXTNL  IVOL
PERLND 44 PWATER PERO          1.195      RCHRES 14      EXTNL  IVOL
IMPLND 14 IWATER SURO          0.340      RCHRES 14      EXTNL  IVOL
*****

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\*\*\* EXECUTEL TRIBUTARY

\*\*\* DM16 INFLOW TO EXECUTEL POND

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PERLND 16 PWATER SURO          0.647      RCHRES 46      EXTNL  IVOL
PERLND 16 PWATER IFWO          0.647      RCHRES 46      EXTNL  IVOL
PERLND 16 PWATER AGWO          0.446      RCHRES 46      EXTNL  IVOL
PERLND 26 PWATER SURO          5.573      RCHRES 46      EXTNL  IVOL
PERLND 26 PWATER IFWO          5.573      RCHRES 46      EXTNL  IVOL
PERLND 26 PWATER AGWO          3.845      RCHRES 46      EXTNL  IVOL
PERLND 34 PWATER SURO          0.639      RCHRES 46      EXTNL  IVOL
PERLND 34 PWATER IFWO          0.639      RCHRES 46      EXTNL  IVOL
PERLND 34 PWATER AGWO          0.441      RCHRES 46      EXTNL  IVOL
PERLND 44 PWATER SURO          8.023      RCHRES 46      EXTNL  IVOL
PERLND 44 PWATER IFWO          8.023      RCHRES 46      EXTNL  IVOL
PERLND 44 PWATER AGWO          5.536      RCHRES 46      EXTNL  IVOL
PERLND 54 PWATER SURO          0.183      RCHRES 46      EXTNL  IVOL
PERLND 54 PWATER IFWO          0.183      RCHRES 46      EXTNL  IVOL
PERLND 54 PWATER AGWO          0.126      RCHRES 46      EXTNL  IVOL
IMPLND 14 IWATER SURO          4.249      RCHRES 46      EXTNL  IVOL
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\*\*\* DM17

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PERLND	16	PWATER	PERO	2.078	RCHRES	17	EXTNL	IVOL
PERLND	26	PWATER	PERO	2.261	RCHRES	17	EXTNL	IVOL
PERLND	34	PWATER	PERO	3.003	RCHRES	17	EXTNL	IVOL
PERLND	44	PWATER	PERO	3.280	RCHRES	17	EXTNL	IVOL
IMPLND	14	IWATER	SURO	2.655	RCHRES	17	EXTNL	IVOL

\*\*\* MAINSTEM RAVINE

\*\*\* DM18

PERLND	16	PWATER	PERO	0.789	RCHRES	18	EXTNL	IVOL
PERLND	26	PWATER	PERO	0.277	RCHRES	18	EXTNL	IVOL
PERLND	34	PWATER	PERO	3.151	RCHRES	18	EXTNL	IVOL
PERLND	44	PWATER	PERO	1.106	RCHRES	18	EXTNL	IVOL
PERLND	54	PWATER	PERO	0.300	RCHRES	18	EXTNL	IVOL
IMPLND	14	IWATER	SURO	0.296	RCHRES	18	EXTNL	IVOL

\*\*\* NORTH BRANCH RAVINE

\*\*\* DM19

PERLND	16	PWATER	PERO	0.182	RCHRES	177	EXTNL	IVOL
PERLND	26	PWATER	PERO	6.019	RCHRES	177	EXTNL	IVOL
PERLND	34	PWATER	PERO	0.167	RCHRES	177	EXTNL	IVOL
PERLND	44	PWATER	PERO	5.552	RCHRES	177	EXTNL	IVOL
IMPLND	14	IWATER	SURO	2.617	RCHRES	177	EXTNL	IVOL

\*\*\* DM20

PERLND	16	PWATER	PERO	4.007	RCHRES	193	EXTNL	IVOL
PERLND	26	PWATER	PERO	6.624	RCHRES	193	EXTNL	IVOL
PERLND	34	PWATER	PERO	2.784	RCHRES	193	EXTNL	IVOL
PERLND	44	PWATER	PERO	4.602	RCHRES	193	EXTNL	IVOL
PERLND	54	PWATER	PERO	0.116	RCHRES	193	EXTNL	IVOL
IMPLND	14	IWATER	SURO	3.714	RCHRES	193	EXTNL	IVOL

\*\*\* LOWER MAINSTEM

\*\*\* DM21

PERLND	16	PWATER	PERO	2.143	RCHRES	198	EXTNL	IVOL
PERLND	26	PWATER	PERO	6.306	RCHRES	198	EXTNL	IVOL
PERLND	34	PWATER	PERO	1.429	RCHRES	198	EXTNL	IVOL
PERLND	44	PWATER	PERO	4.205	RCHRES	198	EXTNL	IVOL
IMPLND	14	IWATER	SURO	3.091	RCHRES	198	EXTNL	IVOL

\*\*\* DM22

PERLND	16	PWATER	PERO	0.381	RCHRES	198	EXTNL	IVOL
PERLND	26	PWATER	PERO	4.654	RCHRES	198	EXTNL	IVOL
PERLND	34	PWATER	PERO	0.218	RCHRES	198	EXTNL	IVOL
PERLND	44	PWATER	PERO	2.620	RCHRES	198	EXTNL	IVOL
PERLND	54	PWATER	PERO	0.016	RCHRES	198	EXTNL	IVOL
IMPLND	14	IWATER	SURO	1.972	RCHRES	198	EXTNL	IVOL

\*\*\* NONCONTIGUOUS GROUNDWATER BASINS

\*\*\* G1

PERLND	16	PWATER	AGWO	2.833	RCHRES	5	EXTNL	IVOL
PERLND	26	PWATER	AGWO	9.917	RCHRES	5	EXTNL	IVOL

\*\*\* G2

PERLND	16	PWATER	AGWO	0.417	RCHRES	193	EXTNL	IVOL
PERLND	26	PWATER	AGWO	1.333	RCHRES	193	EXTNL	IVOL

\*\*\* G3

PERLND	16	PWATER	AGWO	5.083	RCHRES	193	EXTNL	IVOL
PERLND	26	PWATER	AGWO	17.667	RCHRES	193	EXTNL	IVOL
PERLND	34	PWATER	AGWO	1.167	RCHRES	193	EXTNL	IVOL
PERLND	44	PWATER	AGWO	4.250	RCHRES	193	EXTNL	IVOL

\*\*\* CHANNEL NETWORK LINKAGES \*\*\*

\*\*\* DICHARGE TO IWS LAGOONS

July 2001  
556-2912-001 (28)

AR 010865

```

*****
RCHRES 360 HYDR  OVOL  1          COPY  8      INPUT  MEAN  1
RCHRES 361 HYDR  OVOL  1          COPY  8      INPUT  MEAN  1
RCHRES 364 HYDR  OVOL  1          COPY  8      INPUT  MEAN  1
RCHRES 365 HYDR  OVOL  1          COPY  8      INPUT  MEAN  1
RCHRES 366 HYDR  OVOL  1          COPY  8      INPUT  MEAN  1
RCHRES 390 HYDR  OVOL  1          COPY  8      INPUT  MEAN  1
COPY  18 OUTPUT MEAN  1          COPY  8      INPUT  MEAN  1
COPY  8  OUTPUT MEAN  1          RCHRES 400  EXTNL  IVOL
*****

```

\*\*\* PUMP STATION OVERFLOW TO SDS

```

*****
RCHRES 360 HYDR  OVOL  2          RCHRES 36      EXTNL  IVOL
RCHRES 361 HYDR  OVOL  2          RCHRES 36      EXTNL  IVOL
RCHRES 366 HYDR  OVOL  2          RCHRES 5       EXTNL  IVOL
RCHRES 390 HYDR  OVOL  2          RCHRES 39      EXTNL  IVOL
*****

```

\*\*\* OVERFLOW OF IWS LAGOONS TO SDS

```

*****
RCHRES 400 HYDR  OVOL  2          COPY  4      INPUT  MEAN  1
*****

```

\*\*\* EAST BRANCH OF CREEK

```

*****
RCHRES 34 HYDR  OVOL  1          RCHRES 35      EXTNL  IVOL
RCHRES 35 HYDR  ROVOL  1         RCHRES 37      EXTNL  IVOL
RCHRES 36 HYDR  ROVOL  1         RCHRES 37      EXTNL  IVOL
COPY  15 OUTPUT MEAN  1          ***          RCHRES 38      EXTNL  IVOL
RCHRES 37 HYDR  ROVOL  1         RCHRES 38      EXTNL  IVOL
RCHRES 38 HYDR  ROVOL  1         RCHRES 5       EXTNL  IVOL
RCHRES 39 HYDR  ROVOL  1         RCHRES 5       EXTNL  IVOL
RCHRES 5  HYDR  ROVOL  1         RCHRES 40      EXTNL  IVOL
RCHRES 40 HYDR  ROVOL  1         COPY  5       INPUT  MEAN  1
*****

```

\*\*\* WEST BRANCH OF CREEK

```

*****
COPY  42 OUTPUT MEAN  1          COPY  9      INPUT  MEAN  1
COPY  9  OUTPUT MEAN  1          RCHRES 4      EXTNL  IVOL
RCHRES 41 HYDR  ROVOL  1         COPY  4      INPUT  MEAN  1
RCHRES 4  HYDR  ROVOL  1         COPY  4      INPUT  MEAN  1
RCHRES 7  HYDR  ROVOL  1         COPY  4      INPUT  MEAN  1
RCHRES 9  HYDR  ROVOL  1         COPY  4      INPUT  MEAN  1
COPY  4  OUTPUT MEAN  1          RCHRES 43     EXTNL  IVOL
RCHRES 43 HYDR  OVOL  1          RCHRES 12     EXTNL  IVOL
COPY  10 OUTPUT MEAN  1         COPY  5      INPUT  MEAN  1
RCHRES 12 HYDR  ROVOL  1         COPY  5      INPUT  MEAN  1
*****

```

\*\*\* MAINSTEM BELOW CONFLUENCE OF E. AND W. BRANCH

```

*****
*** MAINSTEM ABOVE EXECUTEL TRIBUTARY
COPY  5  OUTPUT MEAN  1          RCHRES 13     EXTNL  IVOL
RCHRES 13 HYDR  ROVOL  1         RCHRES 14     EXTNL  IVOL
RCHRES 14 HYDR  ROVOL  1         COPY  48     INPUT  MEAN  1
*****

```

\*\*\* EXECUTEL TRIBUTARY

```

RCHRES 46 HYDR  ROVOL  1         RCHRES 47     EXTNL  IVOL
RCHRES 47 HYDR  ROVOL  1         RCHRES 17     EXTNL  IVOL
RCHRES 17 HYDR  ROVOL  1         COPY  48     INPUT  MEAN  1
*****

```

\*\*\* MAINSTEM FROM HEAD OF RAVINE TO NORTH BRANCH CONFLUENCE

```

COPY  48 OUTPUT MEAN  1          RCHRES 18     EXTNL  IVOL
RCHRES 18 HYDR  OVOL  1          RCHRES 193    EXTNL  IVOL
RCHRES 193 HYDR  ROVOL  1        COPY  1      INPUT  MEAN  1
*****

```

\*\*\* NORTH BRANCH RAVINE TO MAINSTEM

```

RCHRES 177 HYDR  ROVOL  1        COPY  1      INPUT  MEAN  1
*****

```

\*\*\* MAINSTEM FROM NORTH BRANCH CONFLUENCE TO PARK BELOW MVD CULVERT

```

COPY  1  OUTPUT MEAN  1          RCHRES 198    EXTNL  IVOL
*****

```

July 2001  
556-2912-001 (28)

AR 010866

END NETWORK

SCHEMATIC

<-Source-> <--Area--> <-Target-> MBLK \*\*\*  
<Name> # <-factor-> <Name> # Tbl# \*\*\*

\*\*\* PERLND AGWO, AET & PET TO COPY BLOCK

\*\*\* areas from HSPFmodel-MillerCreekAreas-DesMoines Landuse June19.xls, 2006basins areas

PERLND	14	34.17	COPY	100	13
PERLND	16	184.81	COPY	100	13
PERLND	18	7.44	COPY	100	13
PERLND	24	512.17	COPY	100	13
PERLND	26	597.32	COPY	100	13
PERLND	28	4.64	COPY	100	13
PERLND	34	265.34	COPY	100	13
PERLND	44	667.34	COPY	100	13
PERLND	54	66.23	COPY	100	13
PERLND	65	100.00	COPY	100	13
PERLND	66	347.00	COPY	100	13
PERLND	67	14.00	COPY	100	13
PERLND	68	51.00	COPY	100	13

\*\*\* IMPLND AET TO COPY BLOCK

\*\*\* just one IMPLND, so don't need to weight by area

IMPLND 14 COPY 100 14

END SCHEMATIC

MASS-LINK

<Volume> <-Grp> <-Member-><--Mult--> <Target> <-Grp> <-Member->\*\*\*  
<Name> <Name> # #<-factor-> <Name> <Name> # #\*\*\*

MASS-LINK	13				
PERLND	PWATER	AGWO	COPY	INPUT	MEAN 1
PERLND	PWATER	PET	COPY	INPUT	MEAN 2
PERLND	PWATER	TAET	COPY	INPUT	MEAN 3
END MASS-LINK	13				

MASS-LINK	14				
IMPLND	IWATER	IMPEV	COPY	INPUT	MEAN 4
END MASS-LINK	14				

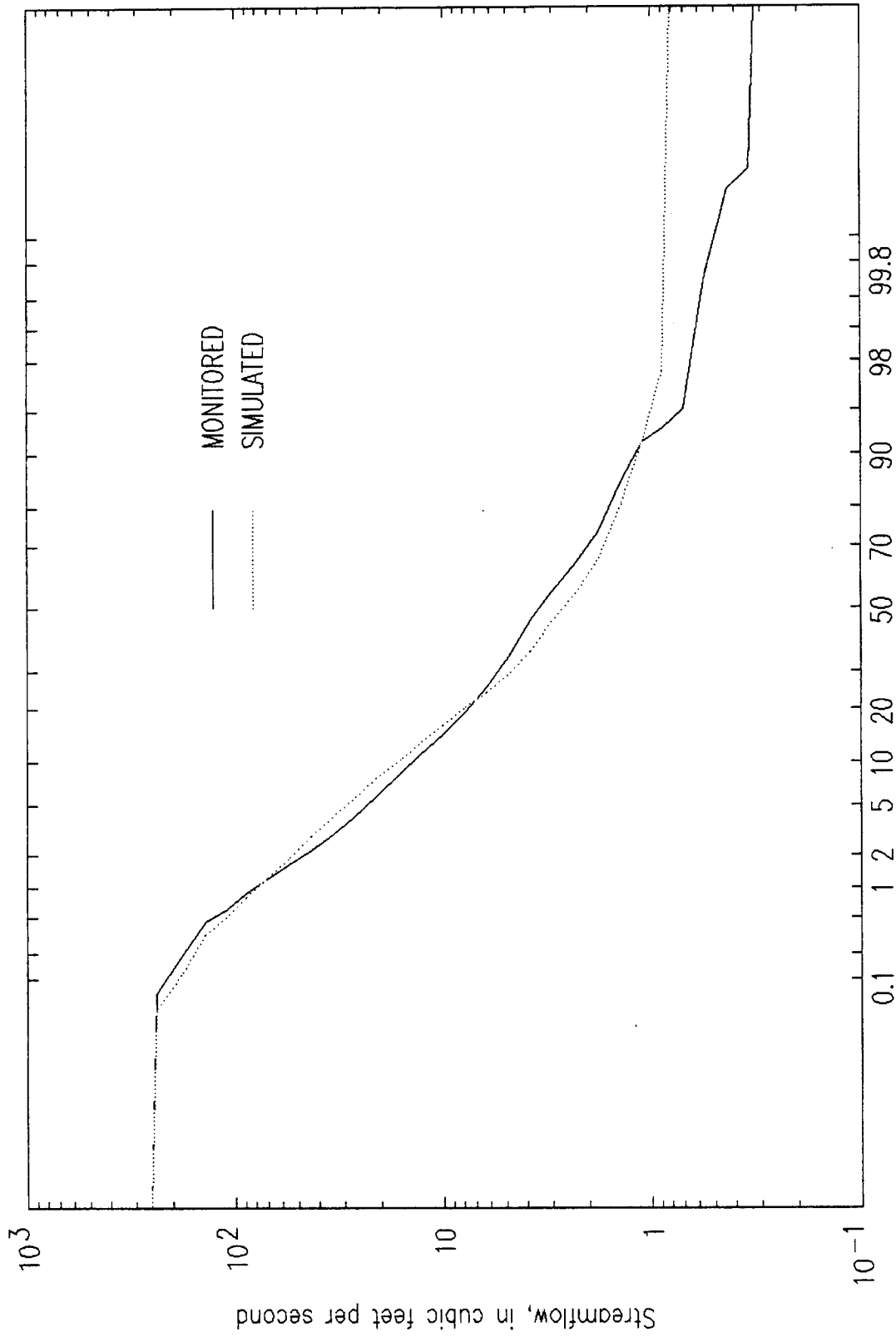
END MASS-LINK

END RUN

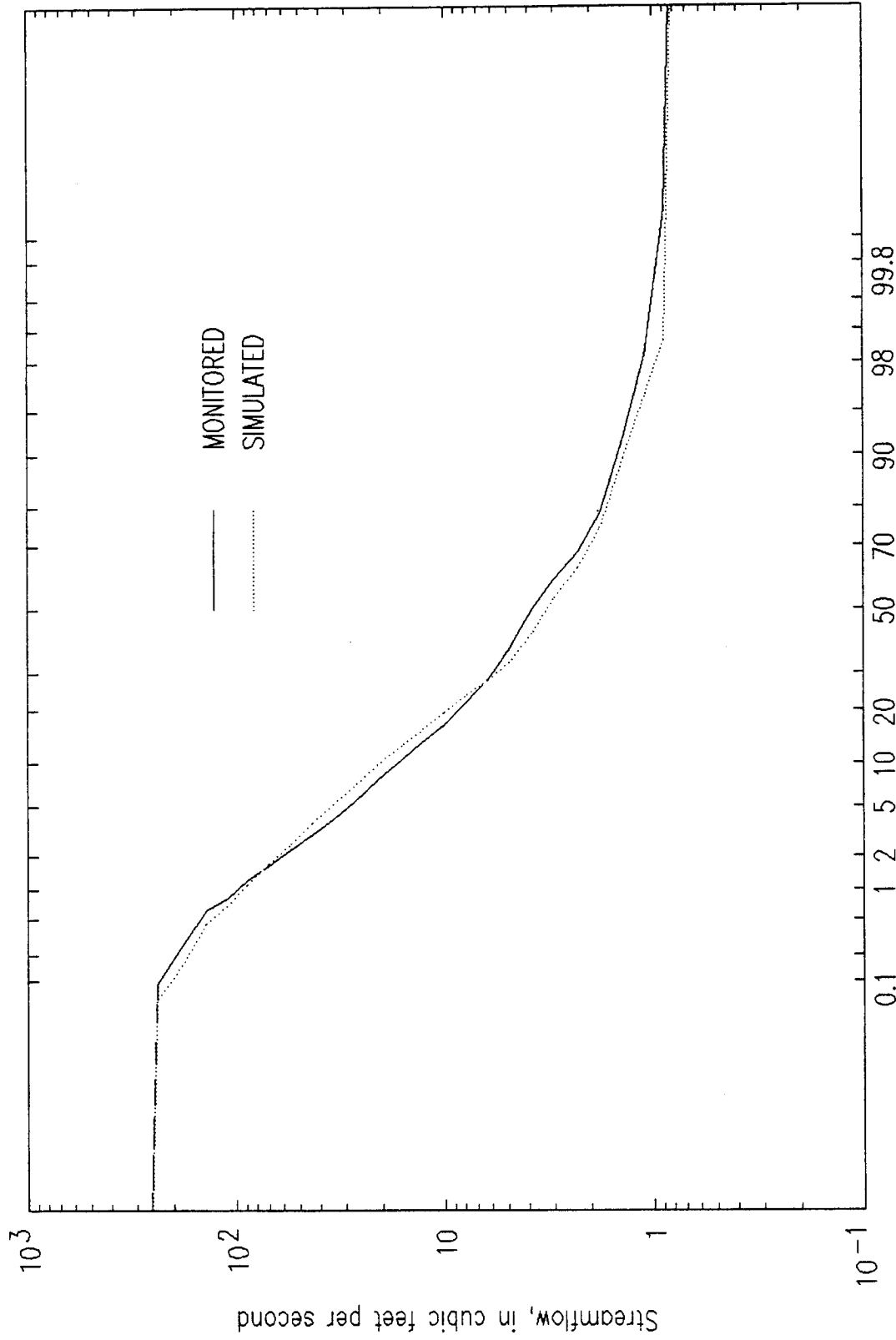
July 2001  
556-2912-001 (28)

AR 010867

**DES MOINES CREEK  
CALIBRATED MODEL HYDROGRAPHS**



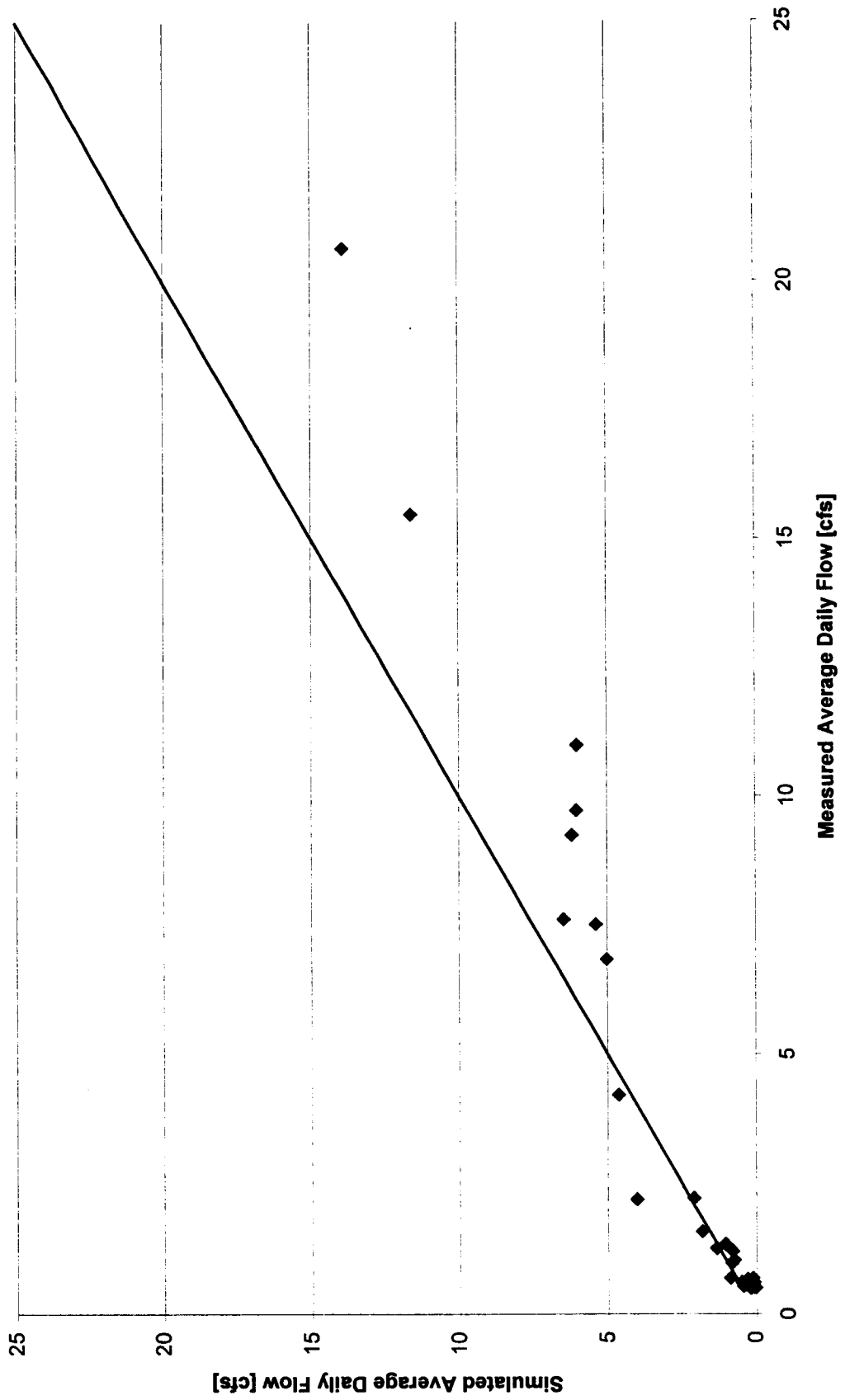
AR 010869



CALIBRATED VS MONITORED VALUES NEAR MOUTH

10-1-94-9.30-96

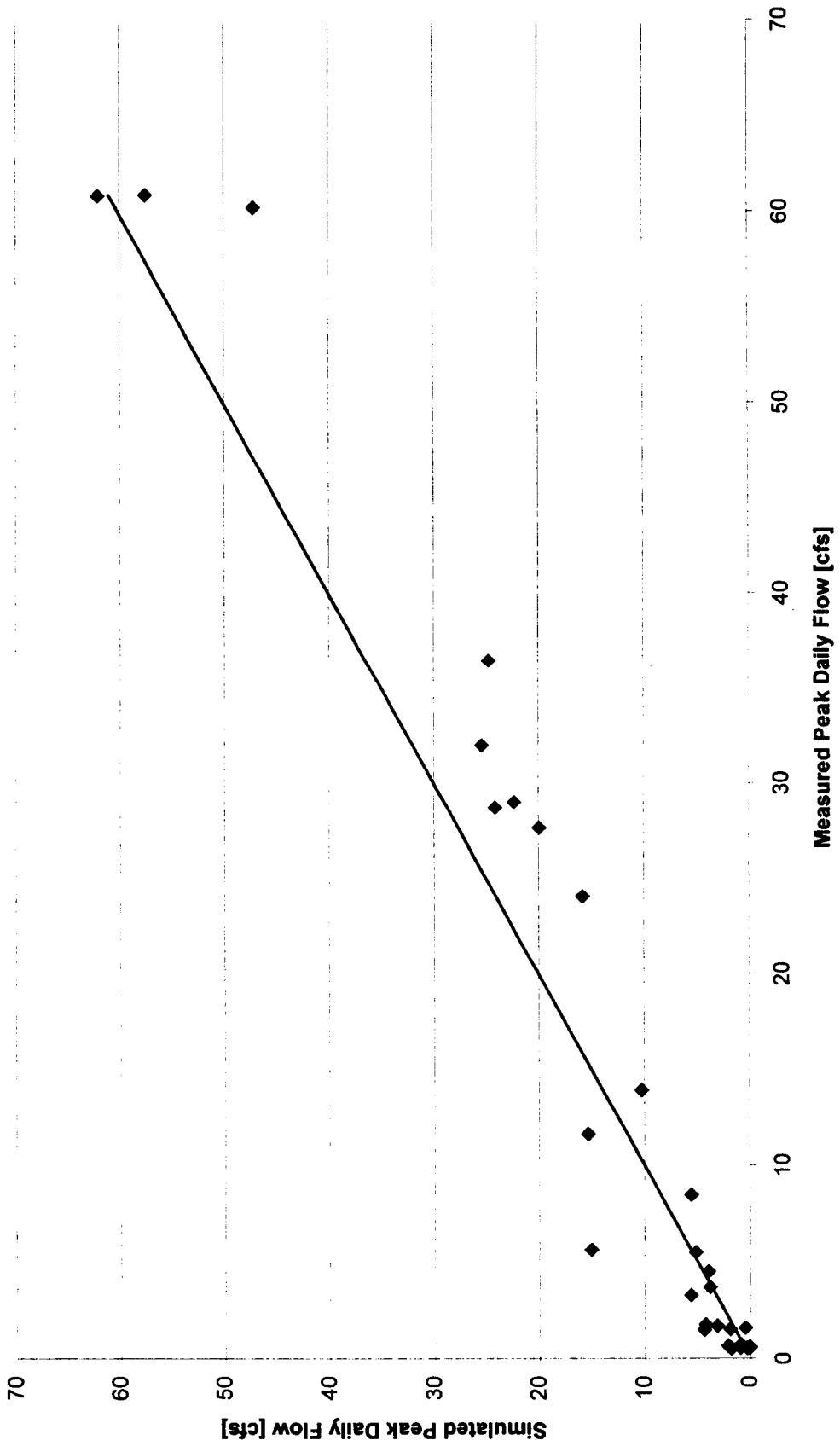
# SDS3 - Average Daily Flow



July 2001  
556-2912-001 (28)

AR 010871

**SDS3 - Daily Peak Flow**

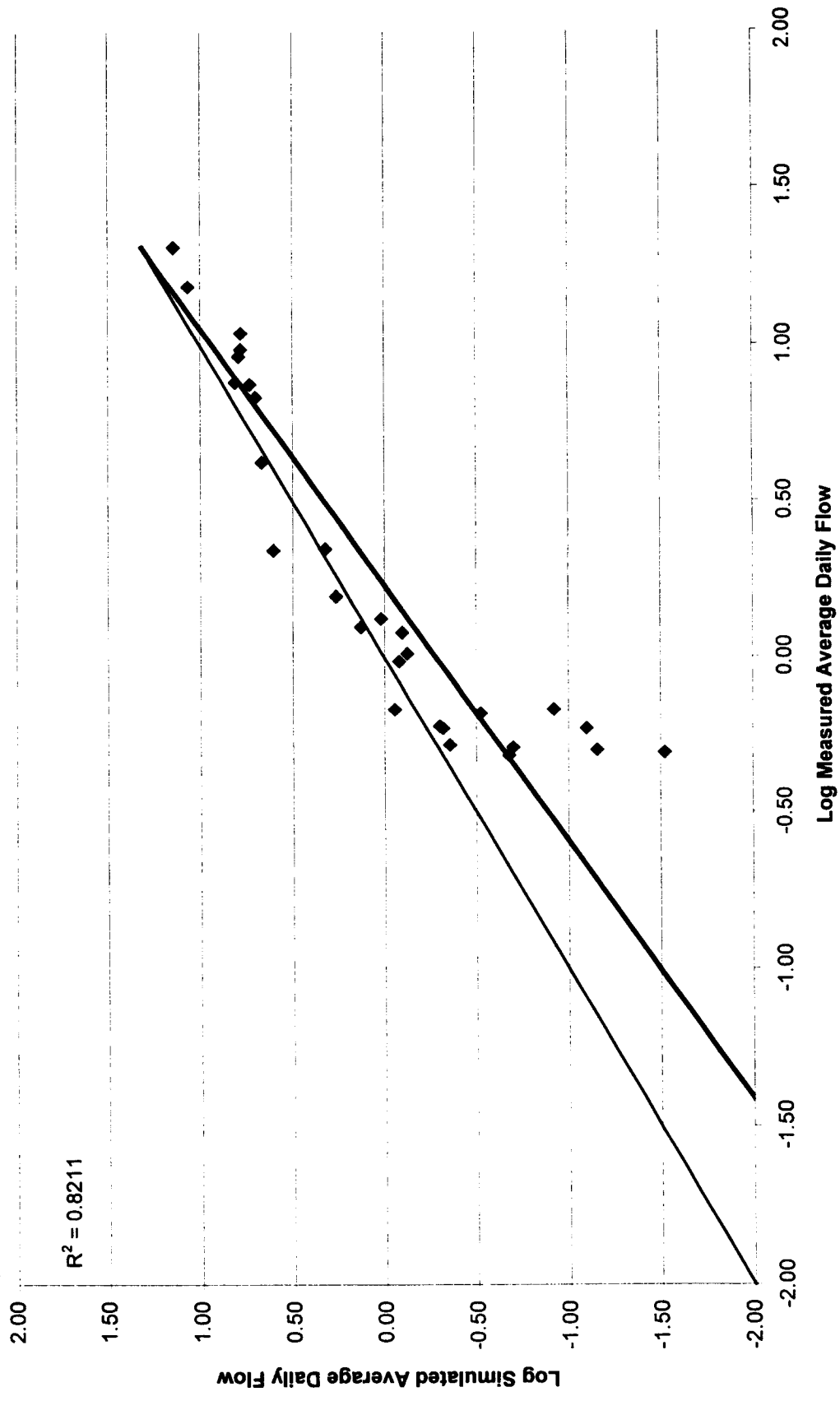


July 2001  
556-2912-001 (28)

**AR 010872**



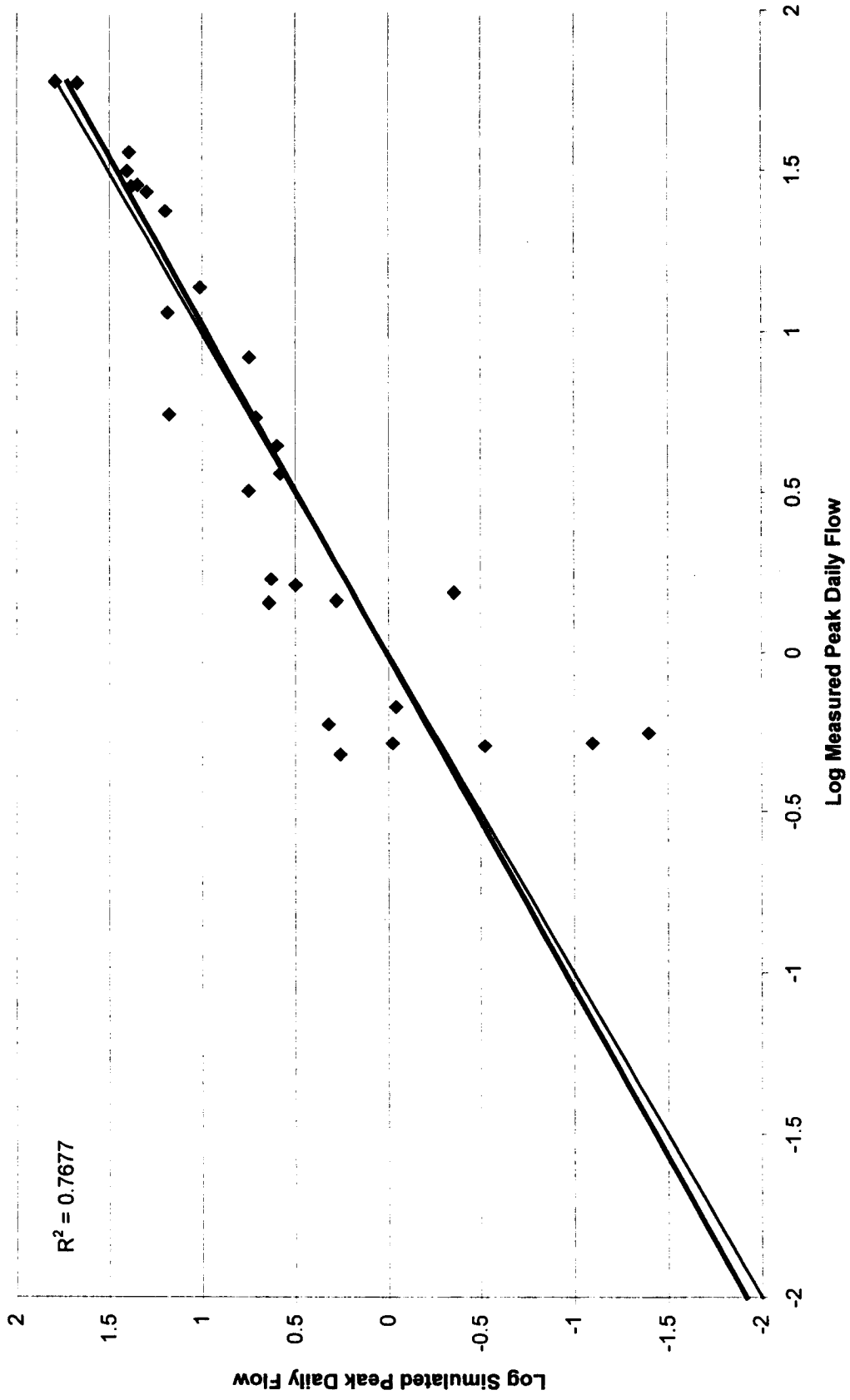
**SDS3 - Average Daily Flow**



July 2001  
556-2912-001 (28)

**AR 010873**

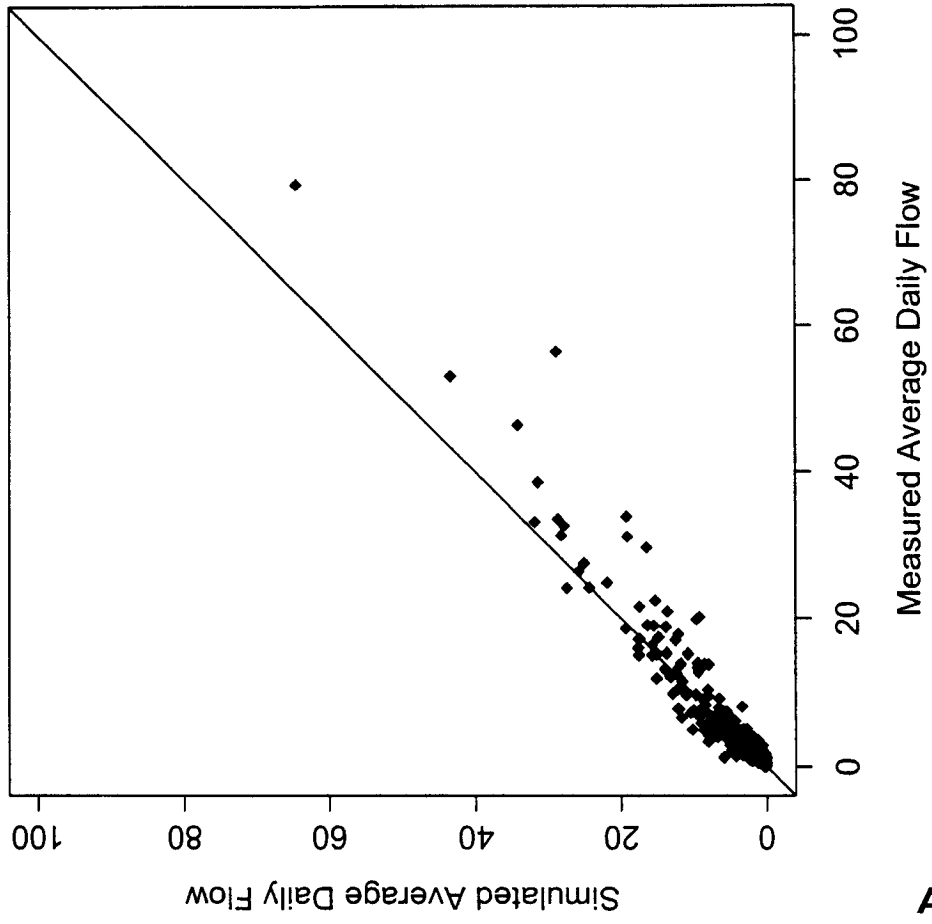
# SDS3 - Peak Daily Flow



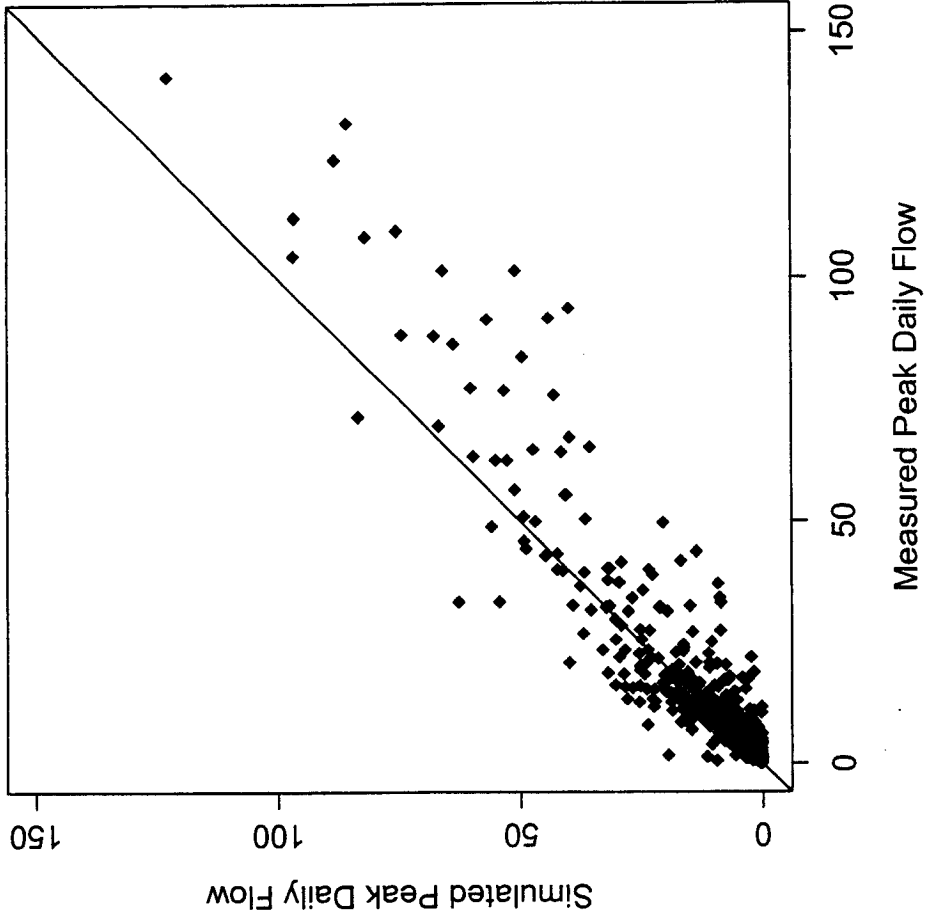
July 2001  
556-2912-001 (28)

AR 010874

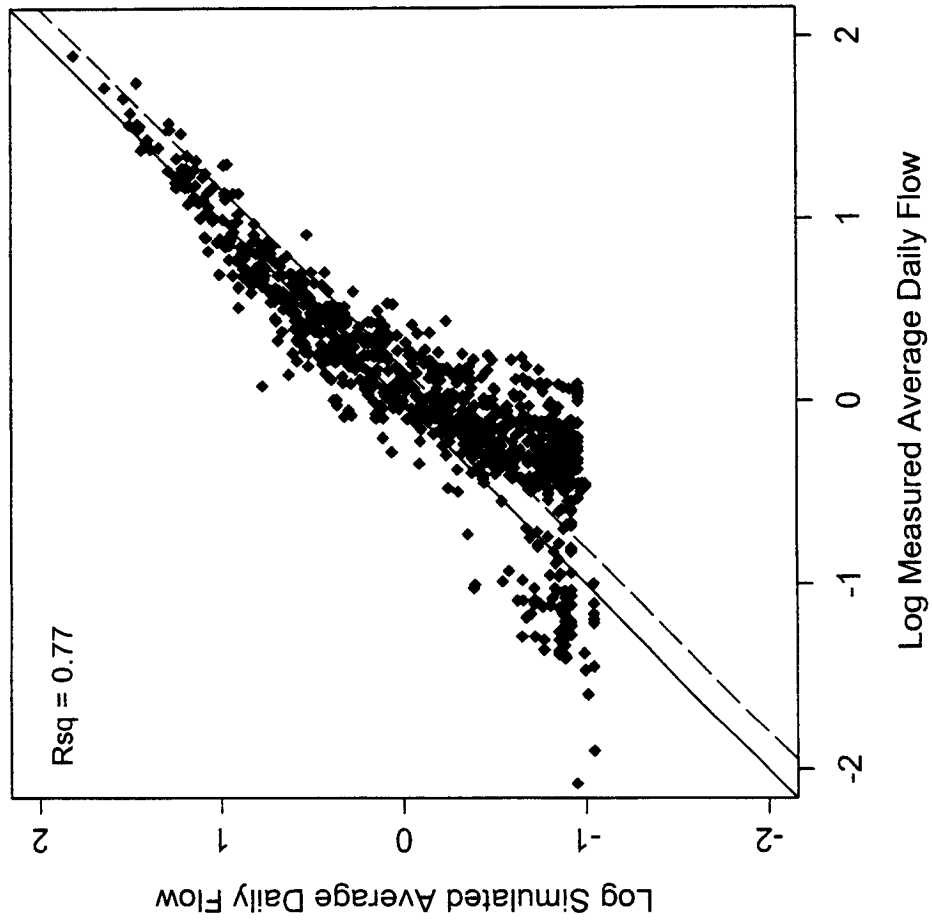
Tyee - Daily Average Flow



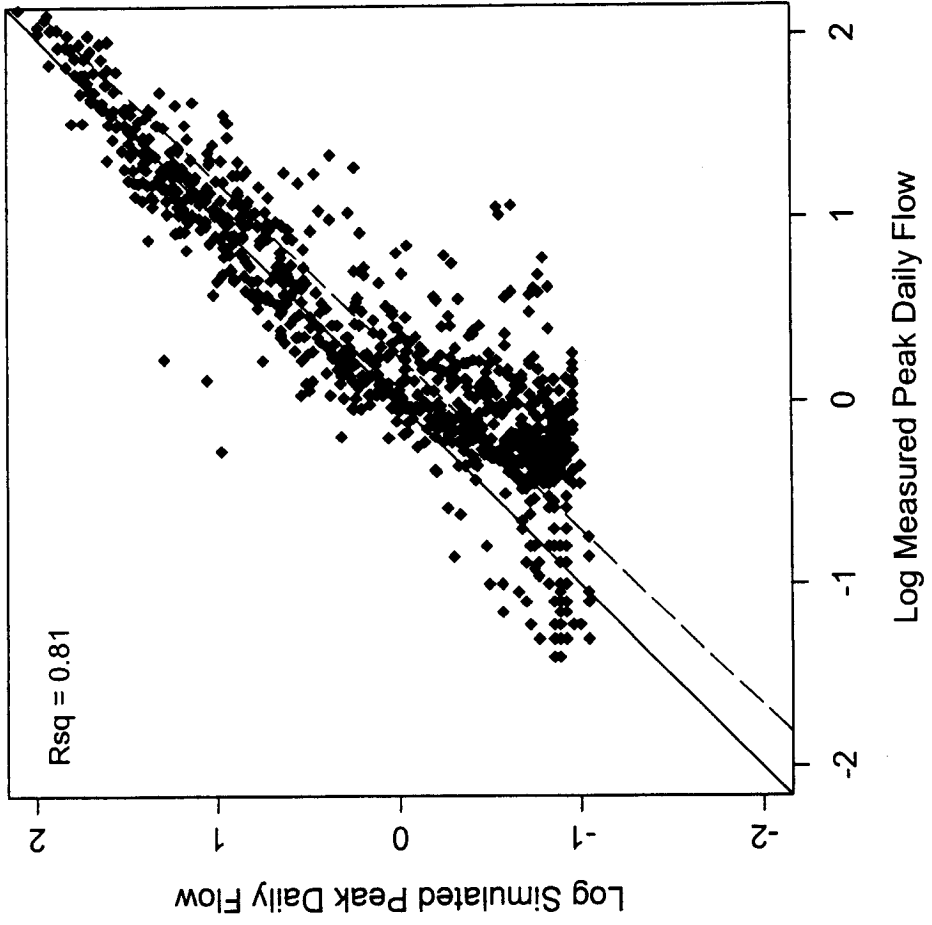
Tyee - Daily Peak Flow

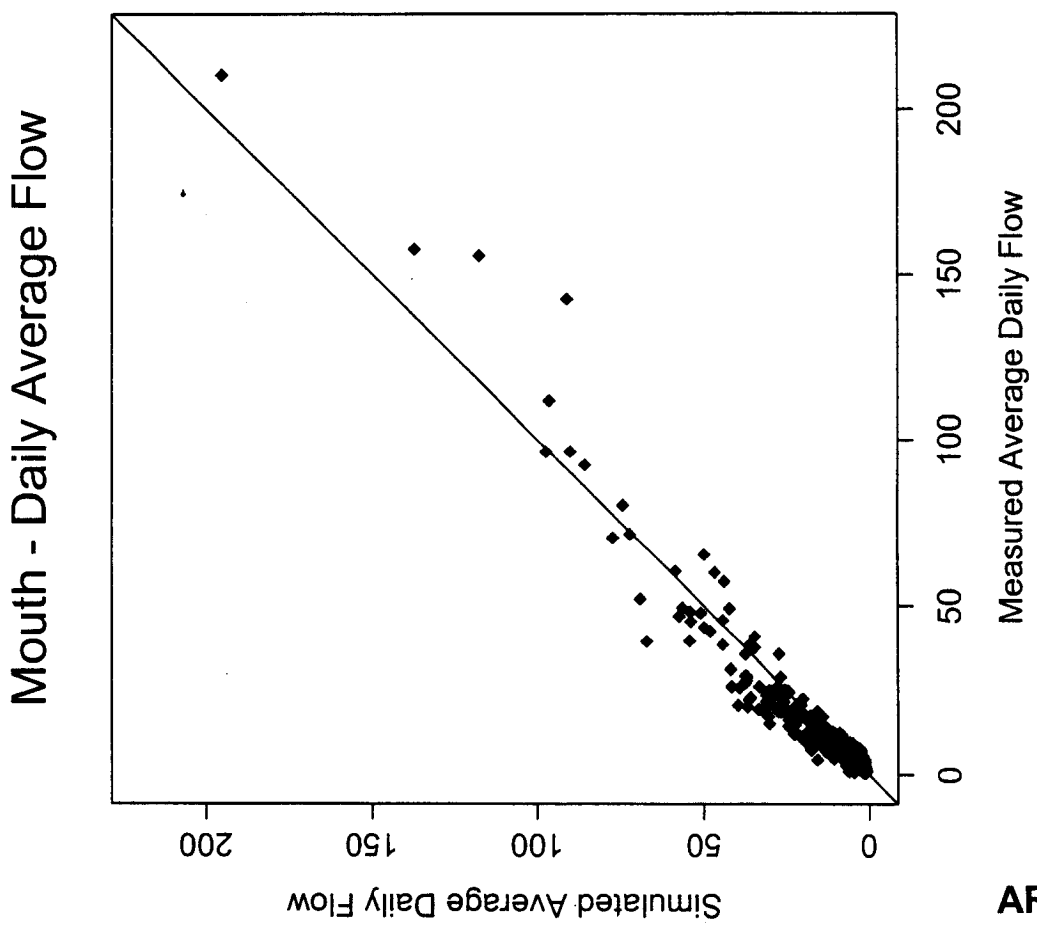
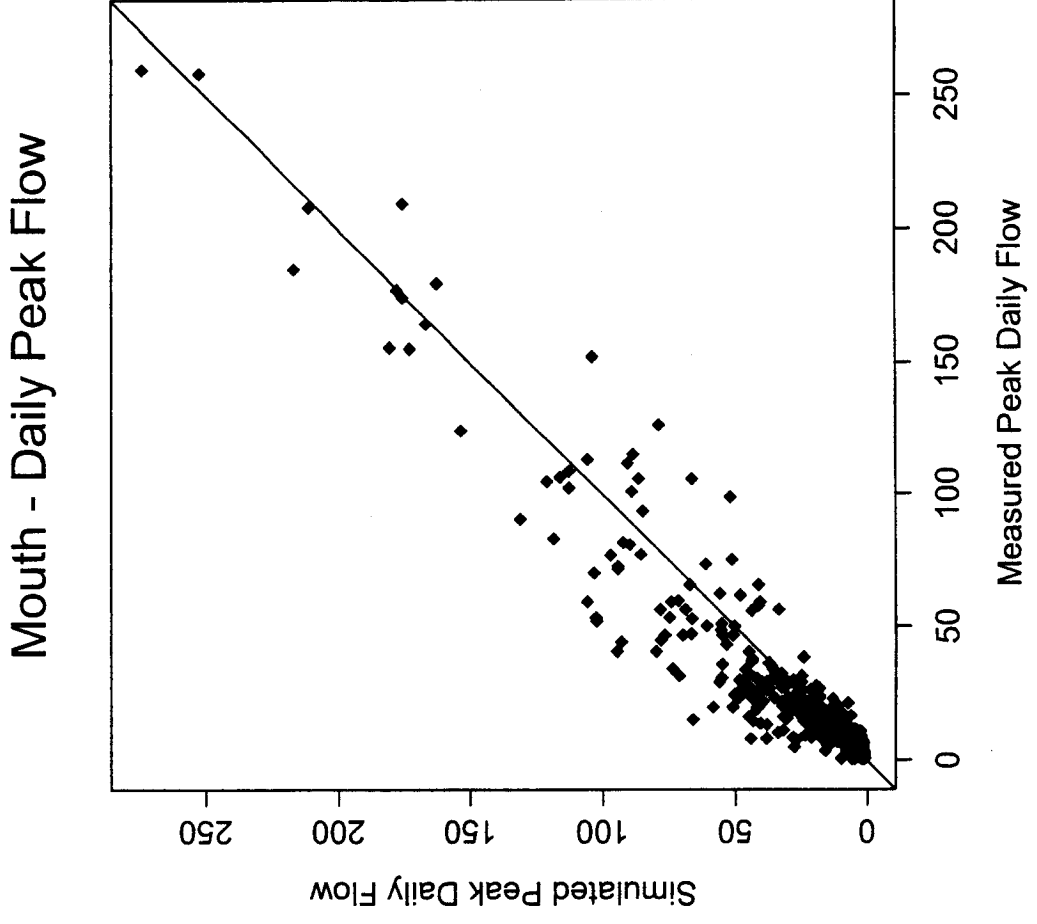


Tyee - Daily Average Flow



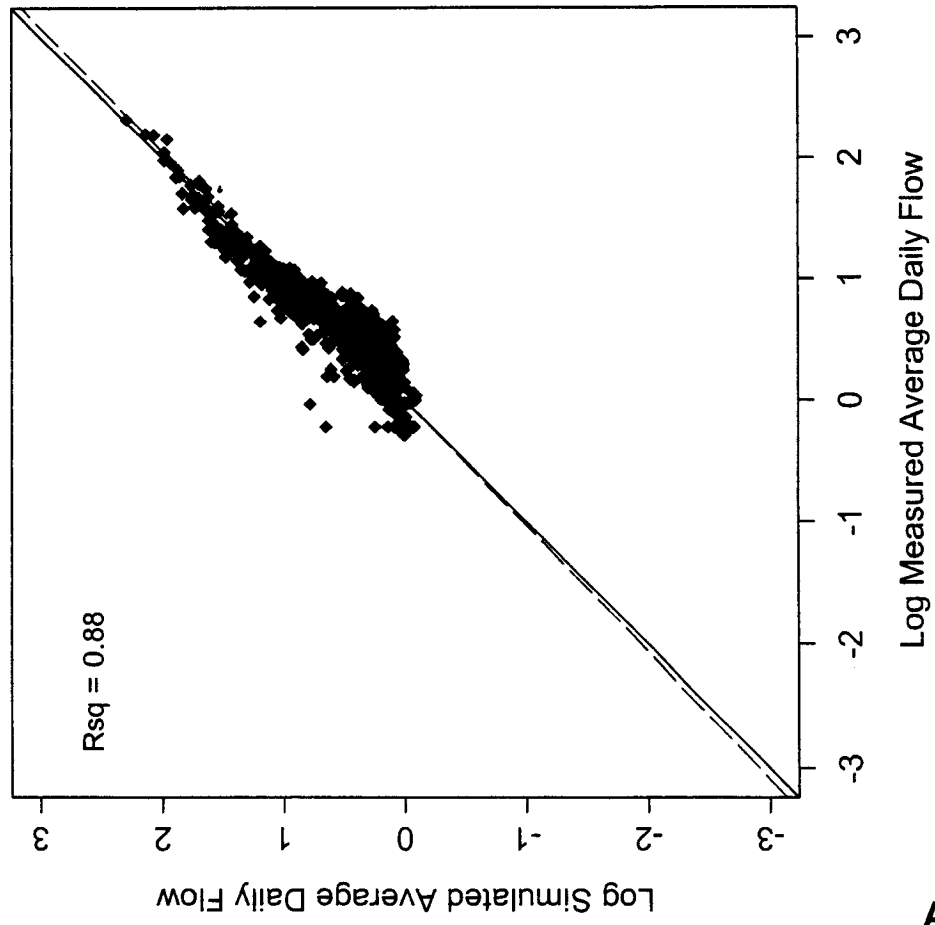
Tyee - Daily Peak Flow



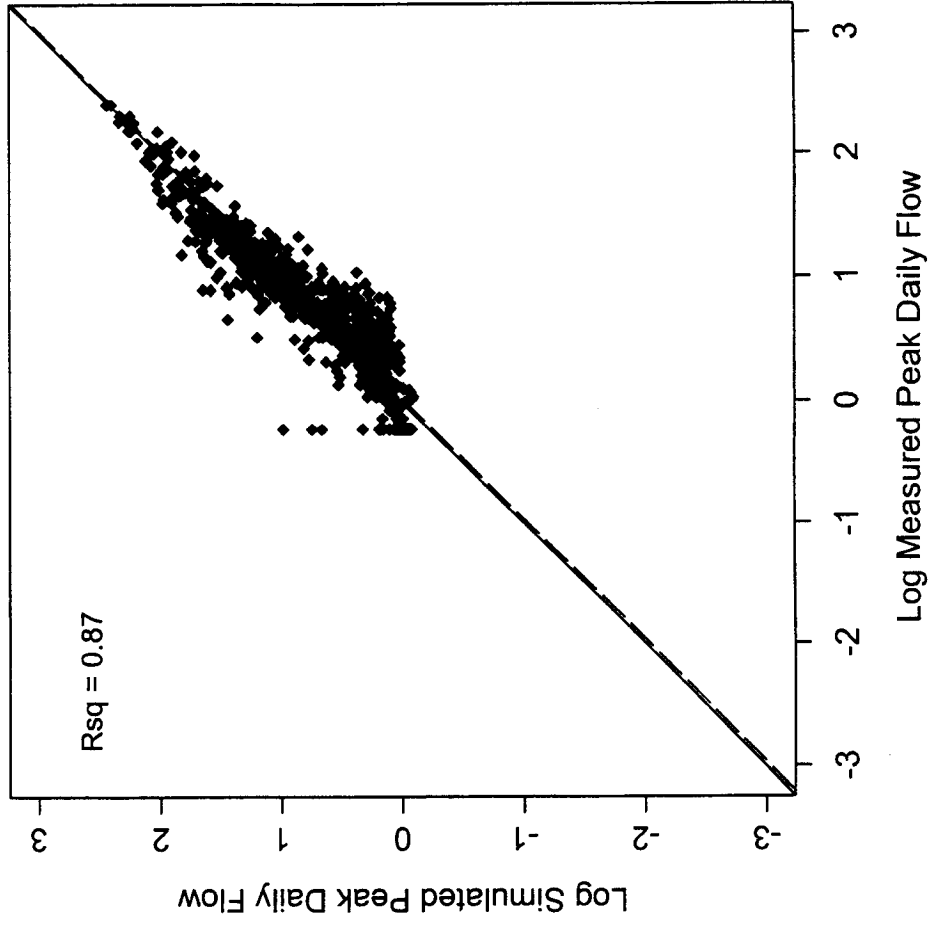


AR 010877

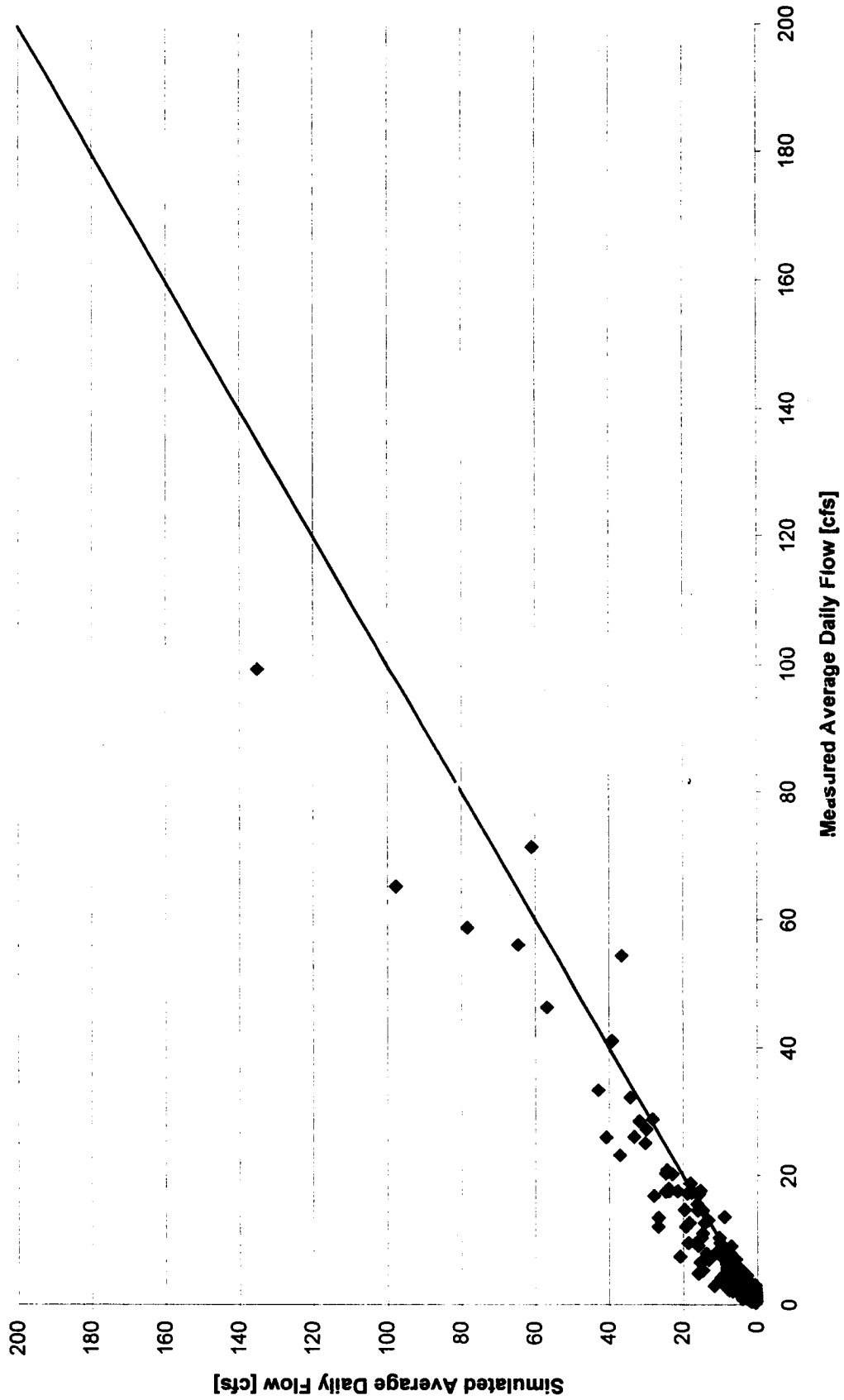
Mouth - Daily Average Flow



Mouth - Daily Peak Flow



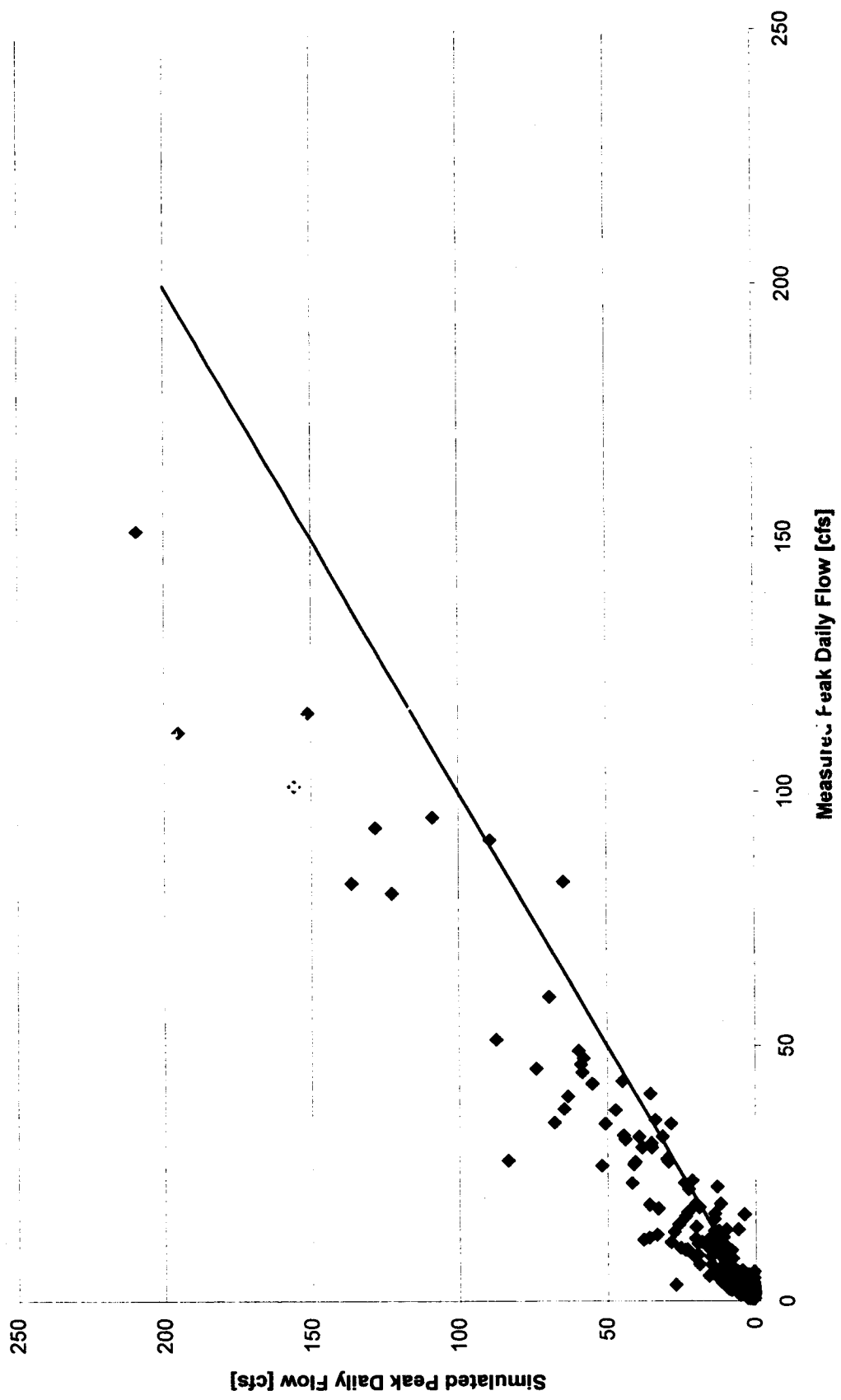
**Golf Weir (at Gage #1F) Average Daily Flow**



July 2001  
556-2912-001 (28)

AR 010879

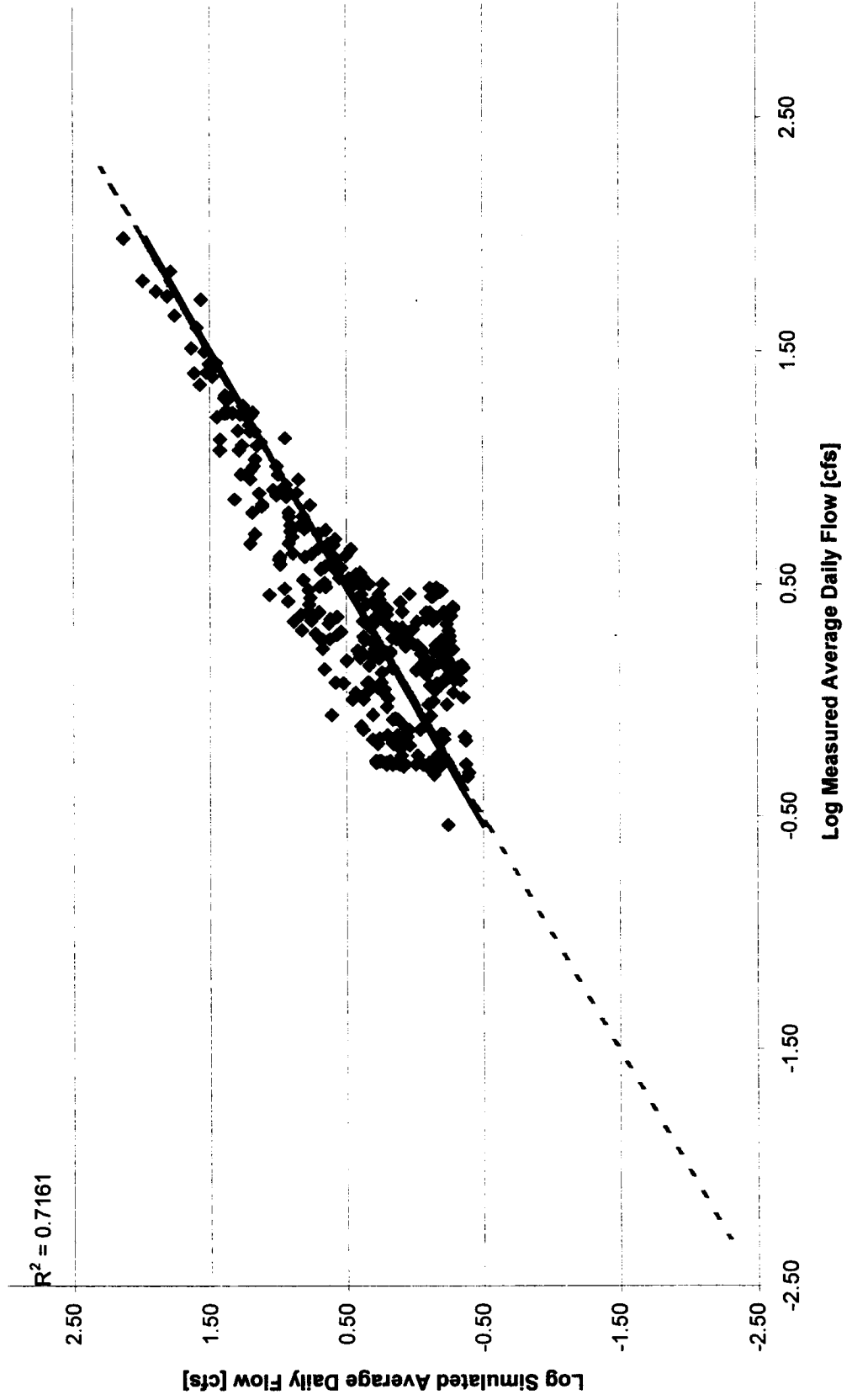
**Golf Weir (at Gage 11F) - Daily Peak Flow**



July 2001  
556-2912-001 (28)



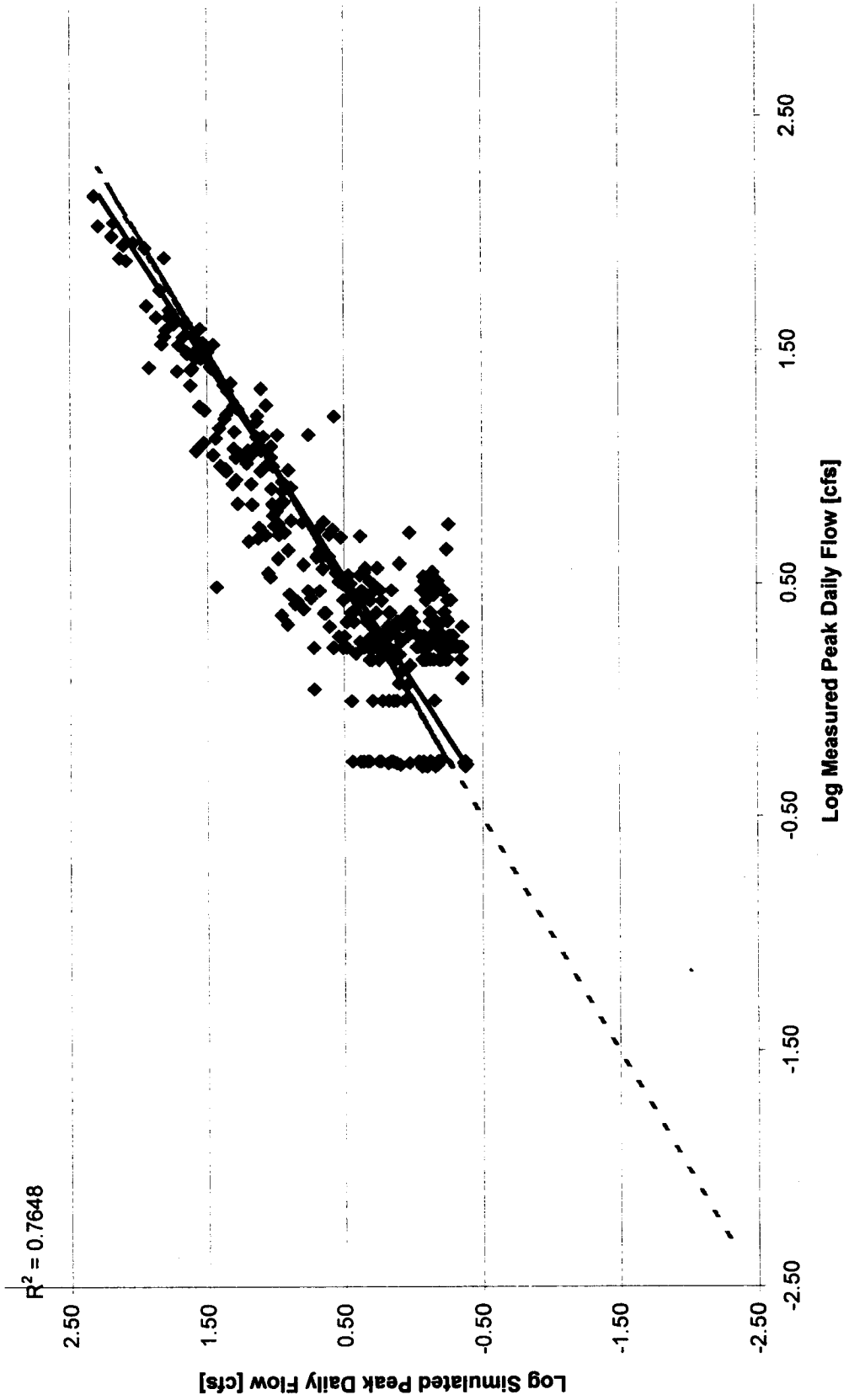
**Golf Weir (at Gage 11F) - Average Daily Flow**



July 2001  
556-2912-001 (28)

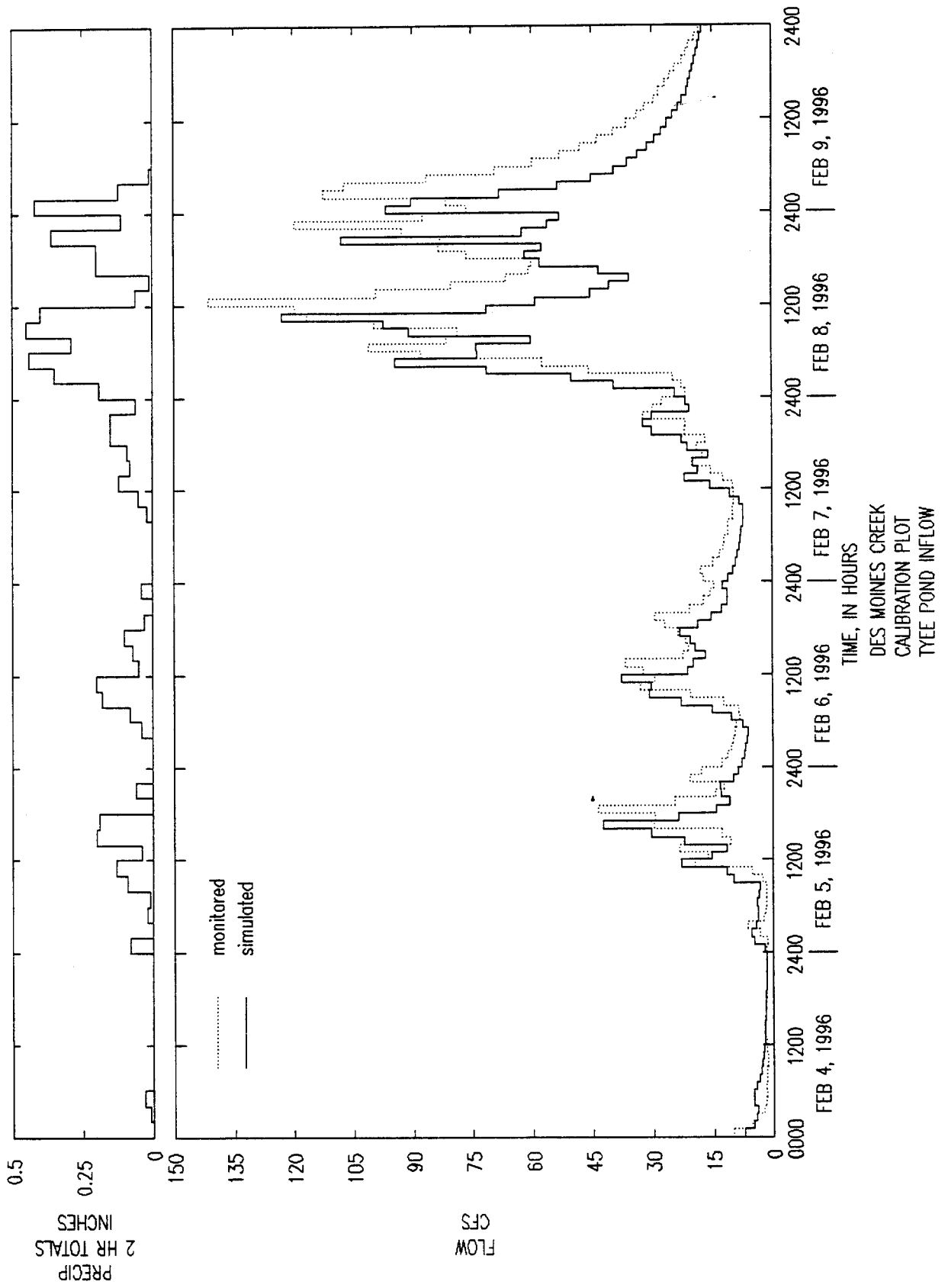
**AR 010881**

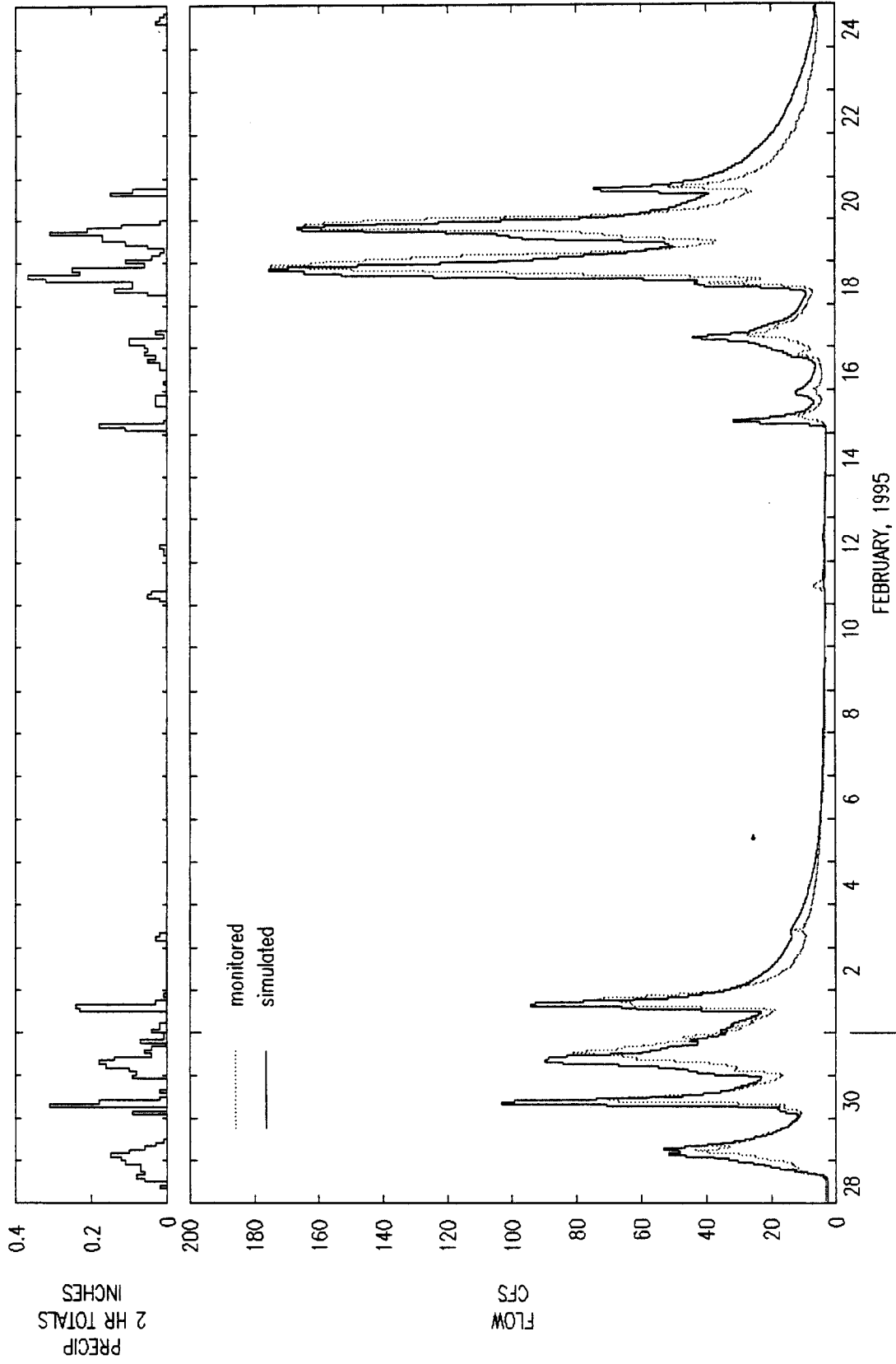
**Golf Weir (at Gage 11F) - Peak Daily Flow**



July 2001  
556-2912-001 (28)

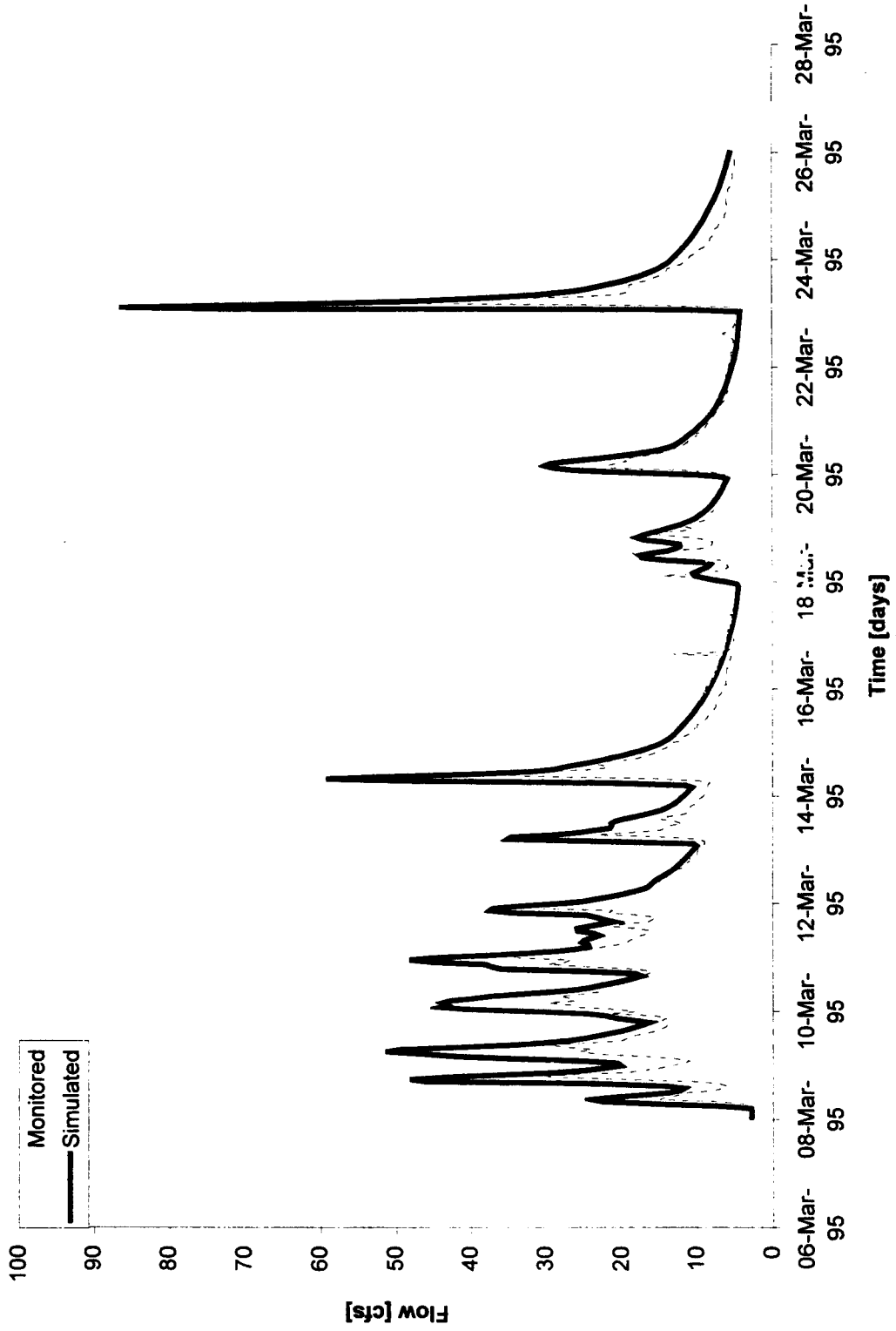
**AR 010882**





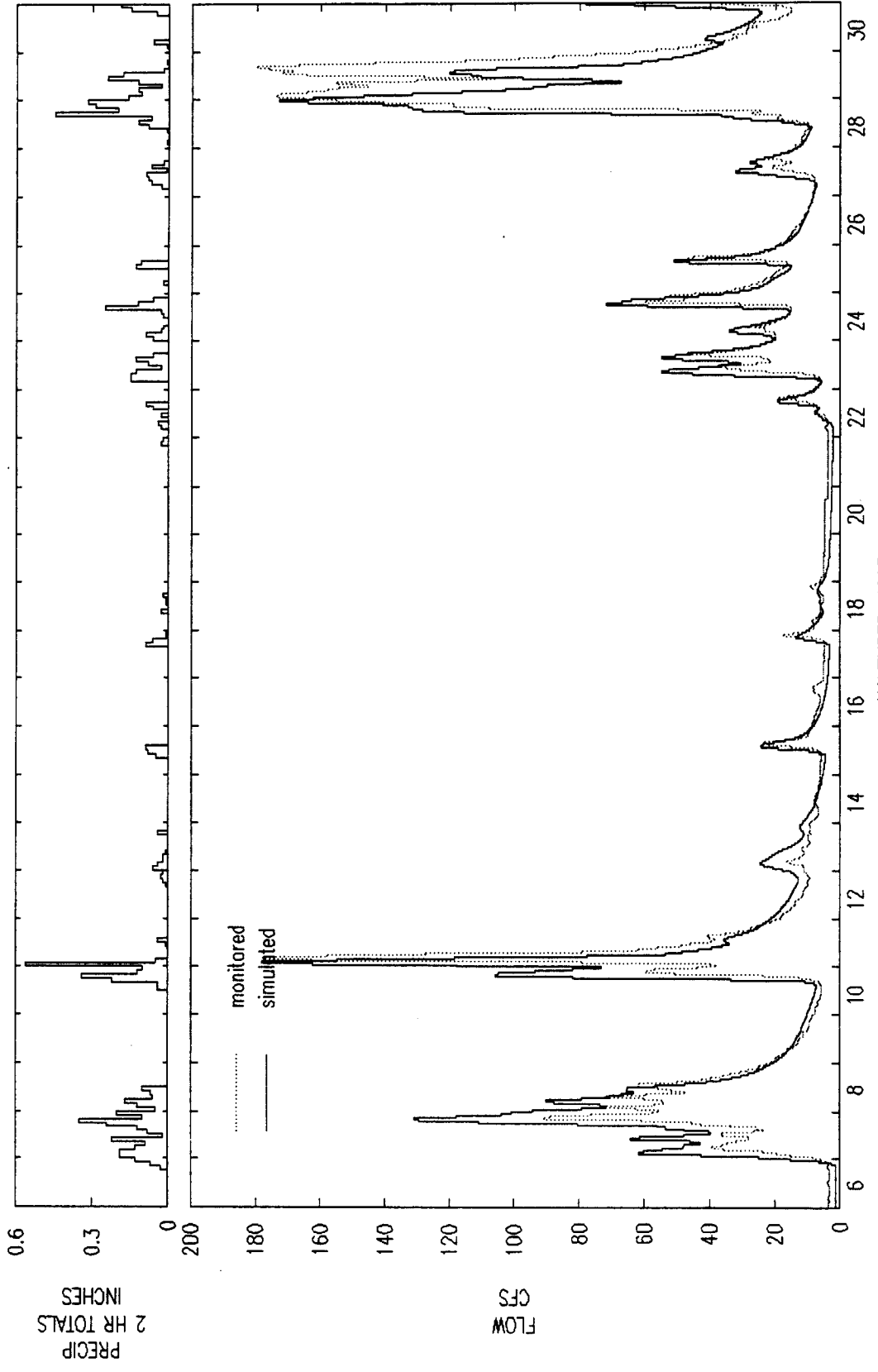
AR 010884

CALIBRATION PLOT - DES MOINES CREEK NEAR MOUTH

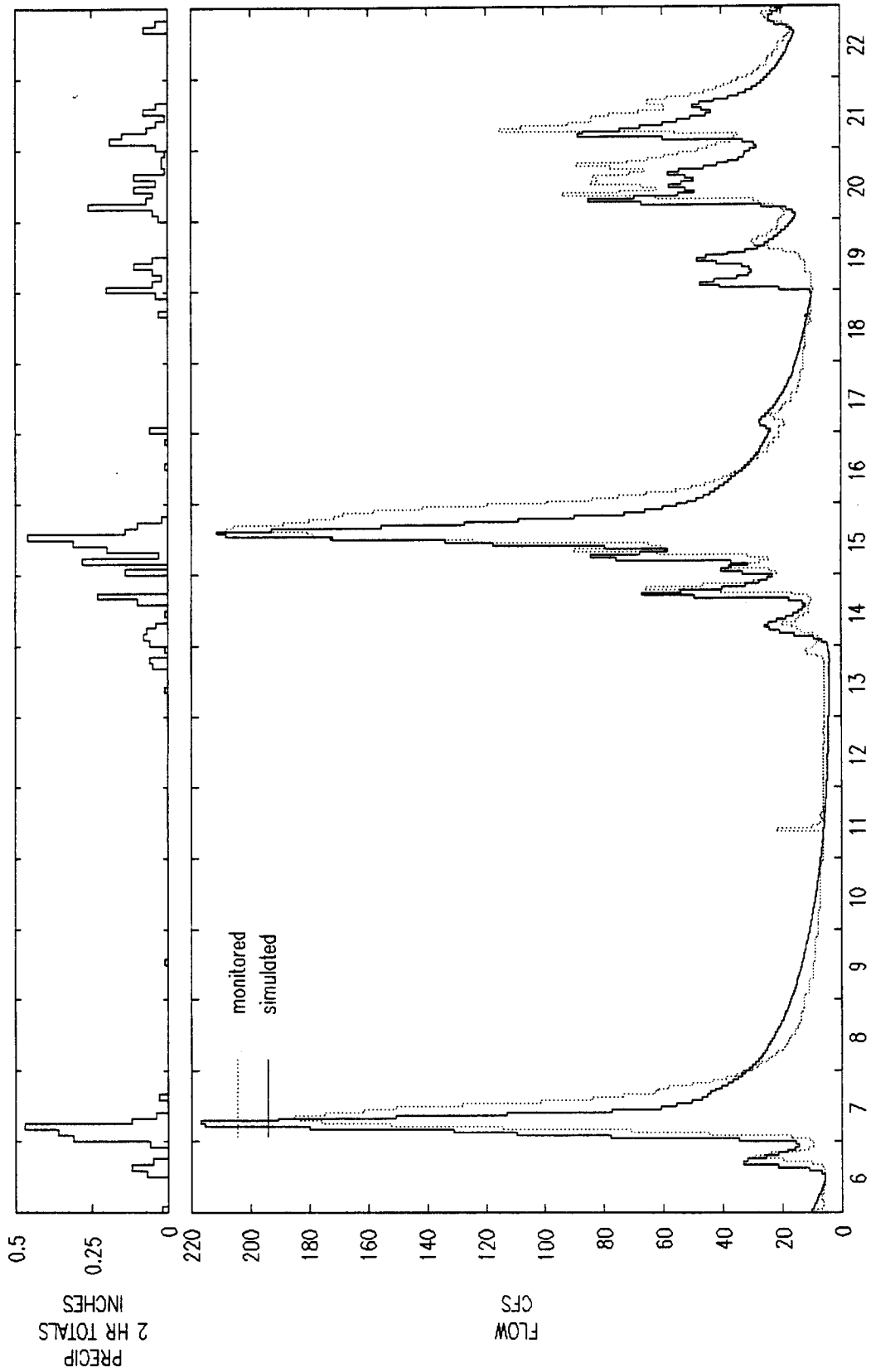


July 2001  
556-2912-001 (28)

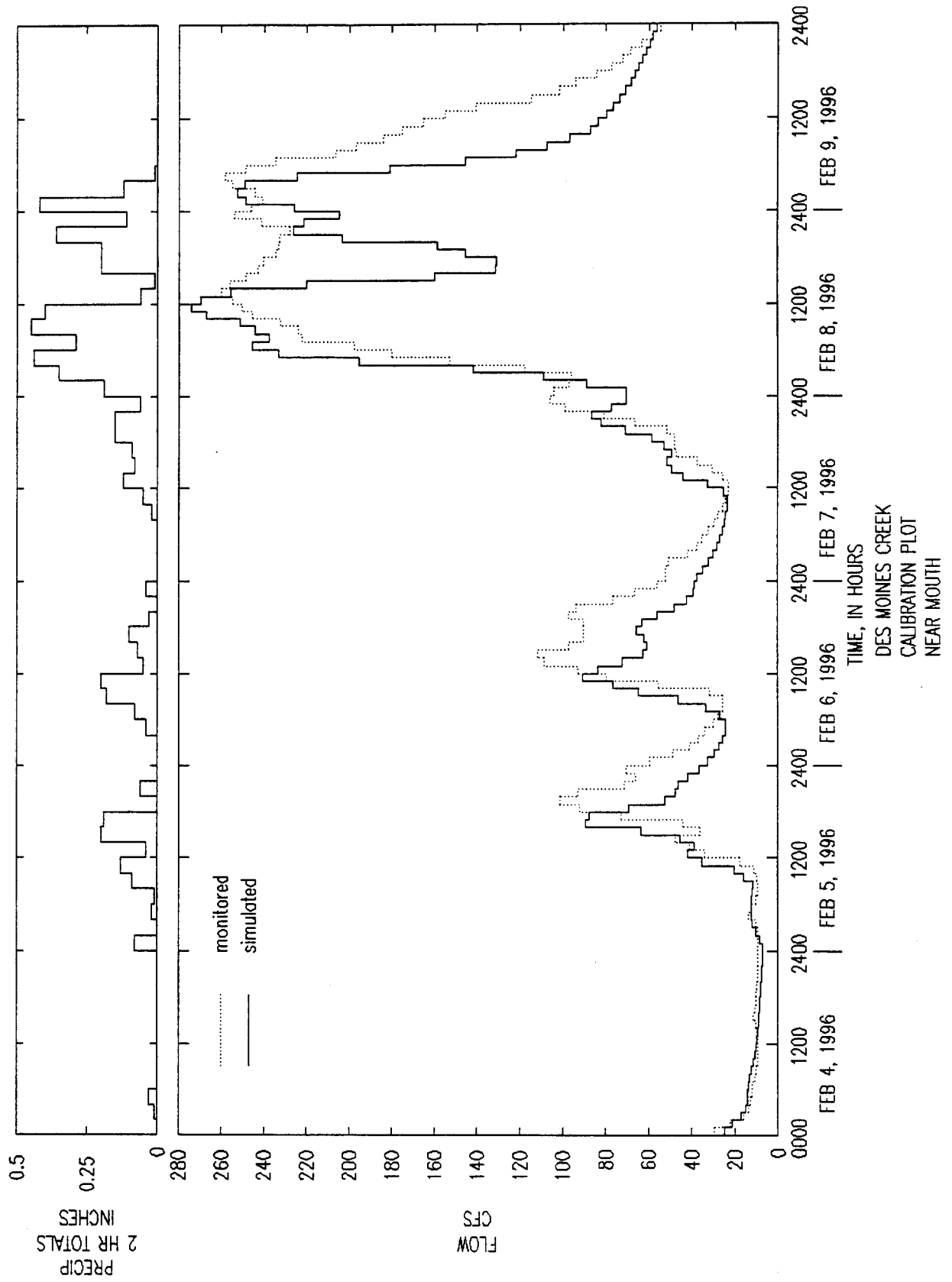
AR 010885



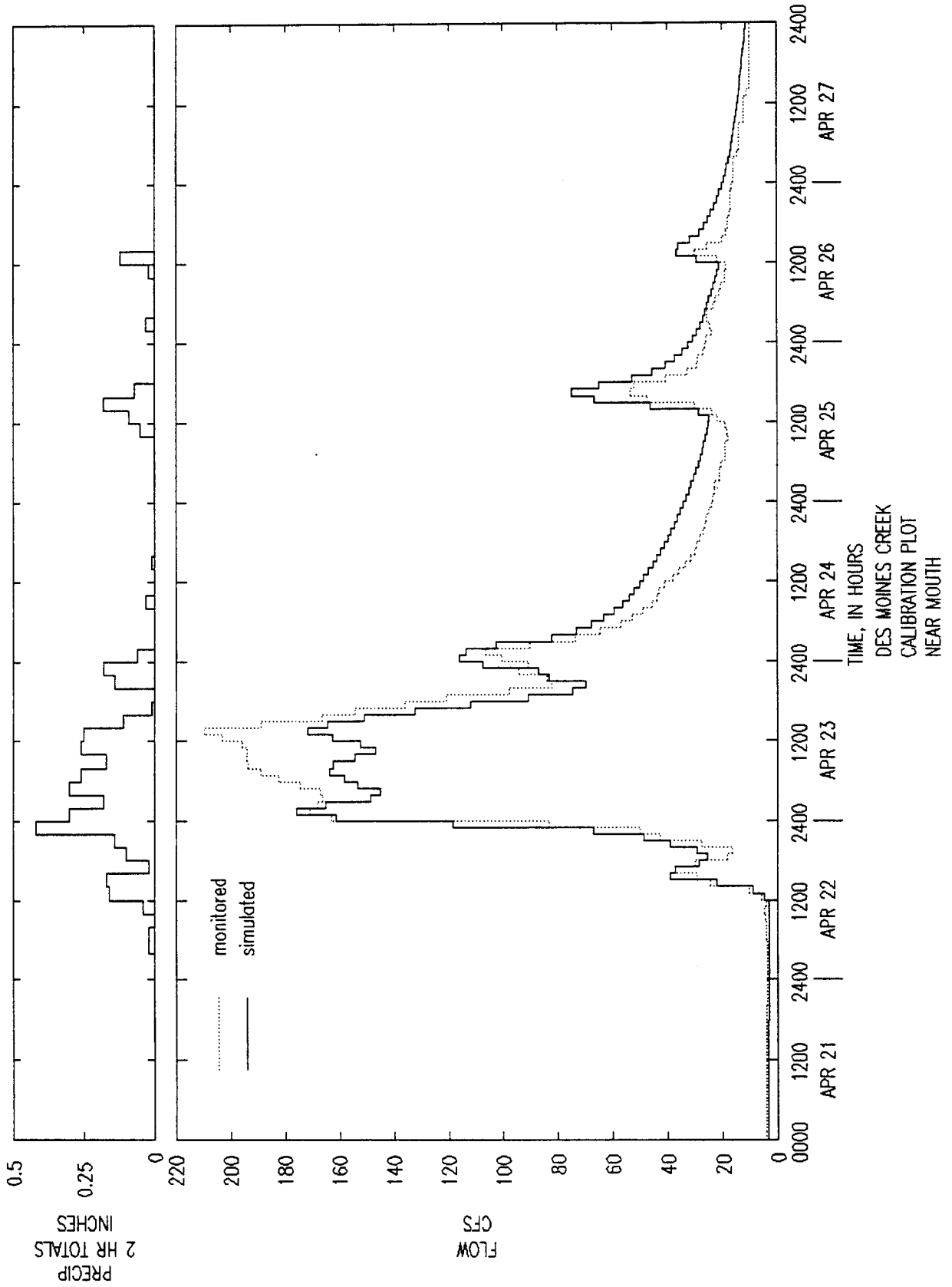
NOVEMBER, 1995  
 TIME, IN DAYS  
 DES MOINES CREEK  
 CALIBRATION PLOT  
 NEAR MOUTH



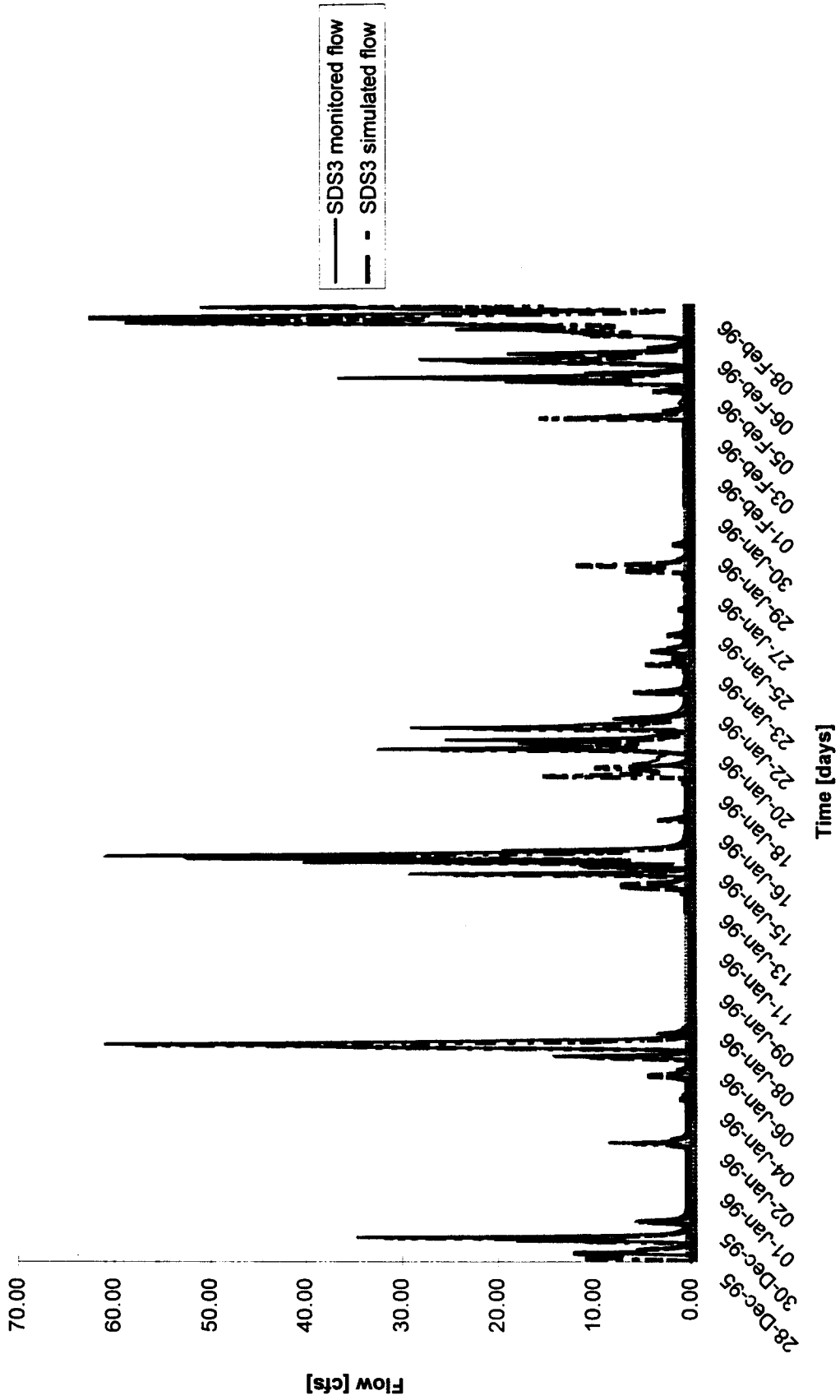
JANUARY, 1996  
 TIME, IN DAYS  
 DES MOINES CREEK  
 CALIBRATION PLOT  
 NEAR MOUTH







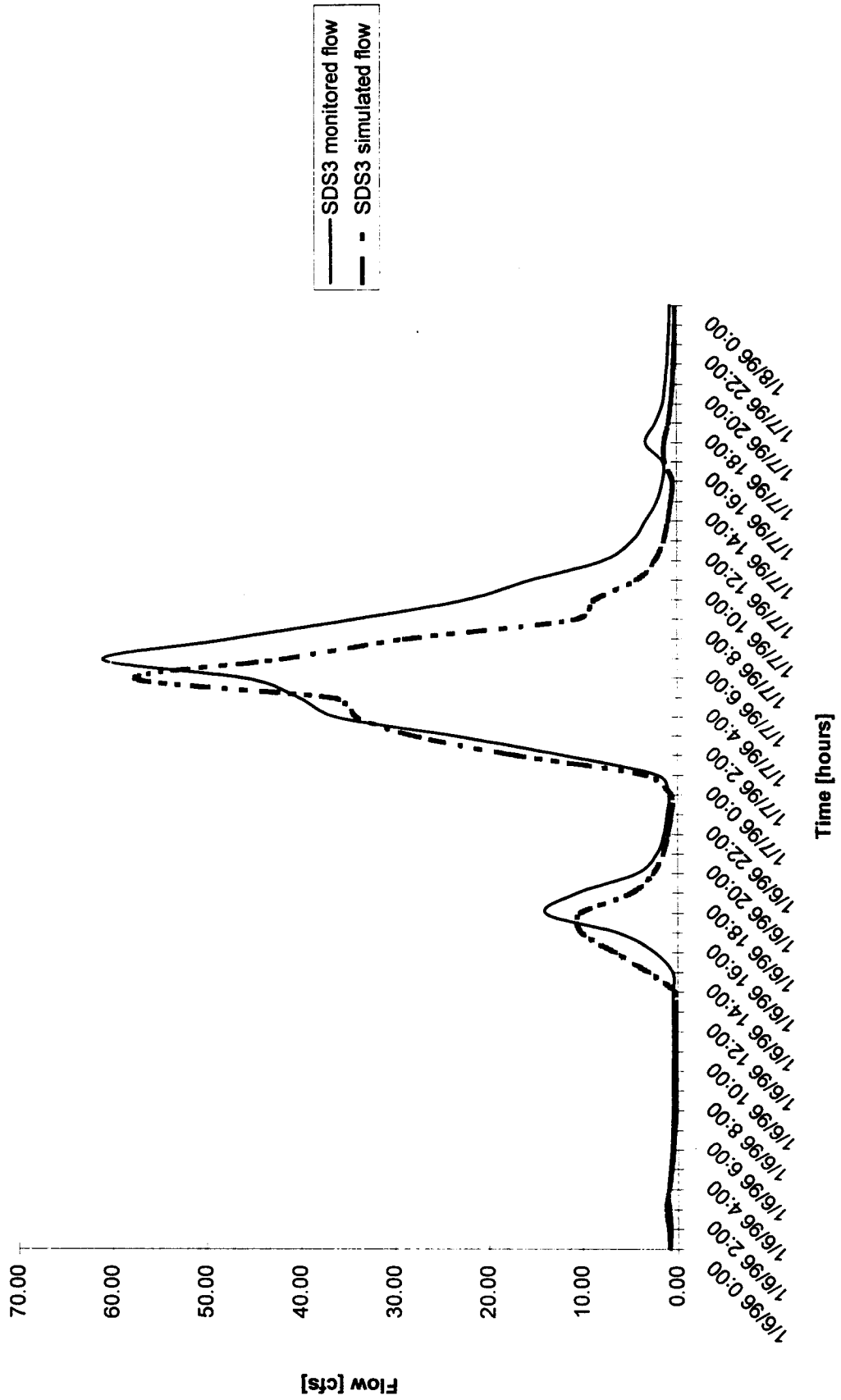
CALIBRATION PLOT - DES MOINES CREEK- SDS3



July 2001  
556-2912-001 (28)

AR 010890

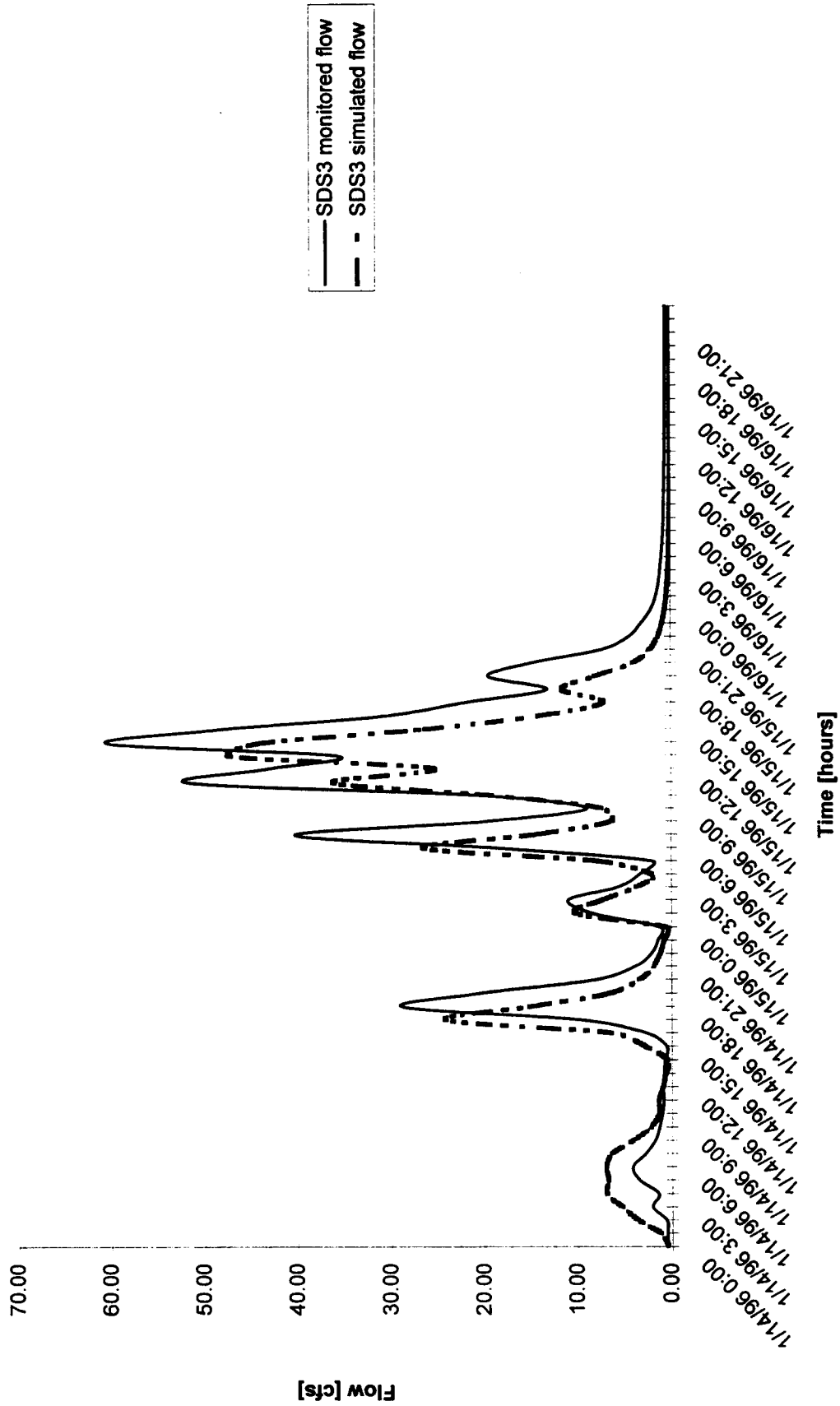
CALIBRATION PLOT - DES MOINES CREEK- SDS3



July 2001  
556-2912-001 (28)

AR 010891

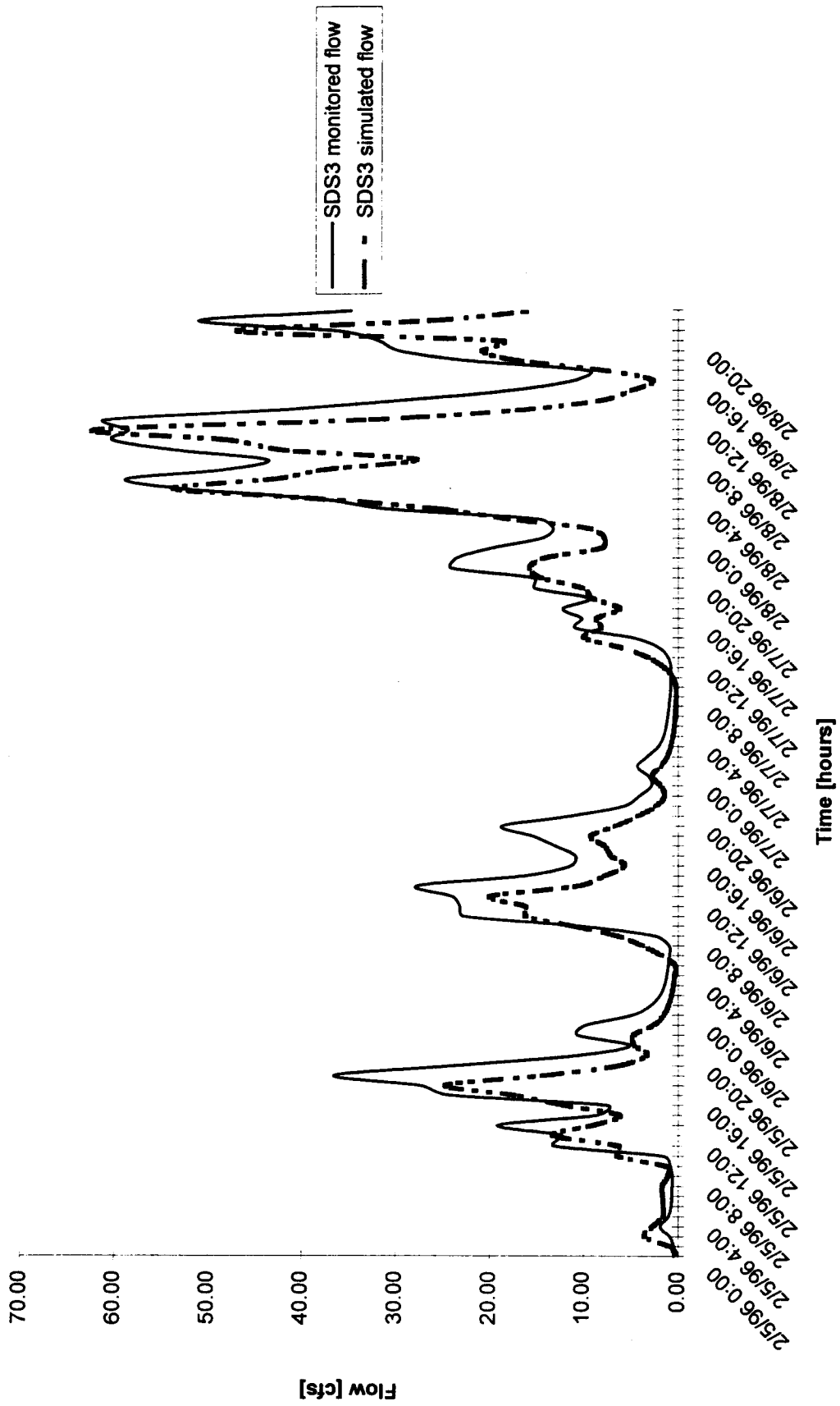
CALIBRATION PLOT - DES MOINES CREEK - SDS3



July 2001  
556-2912-001 (28)

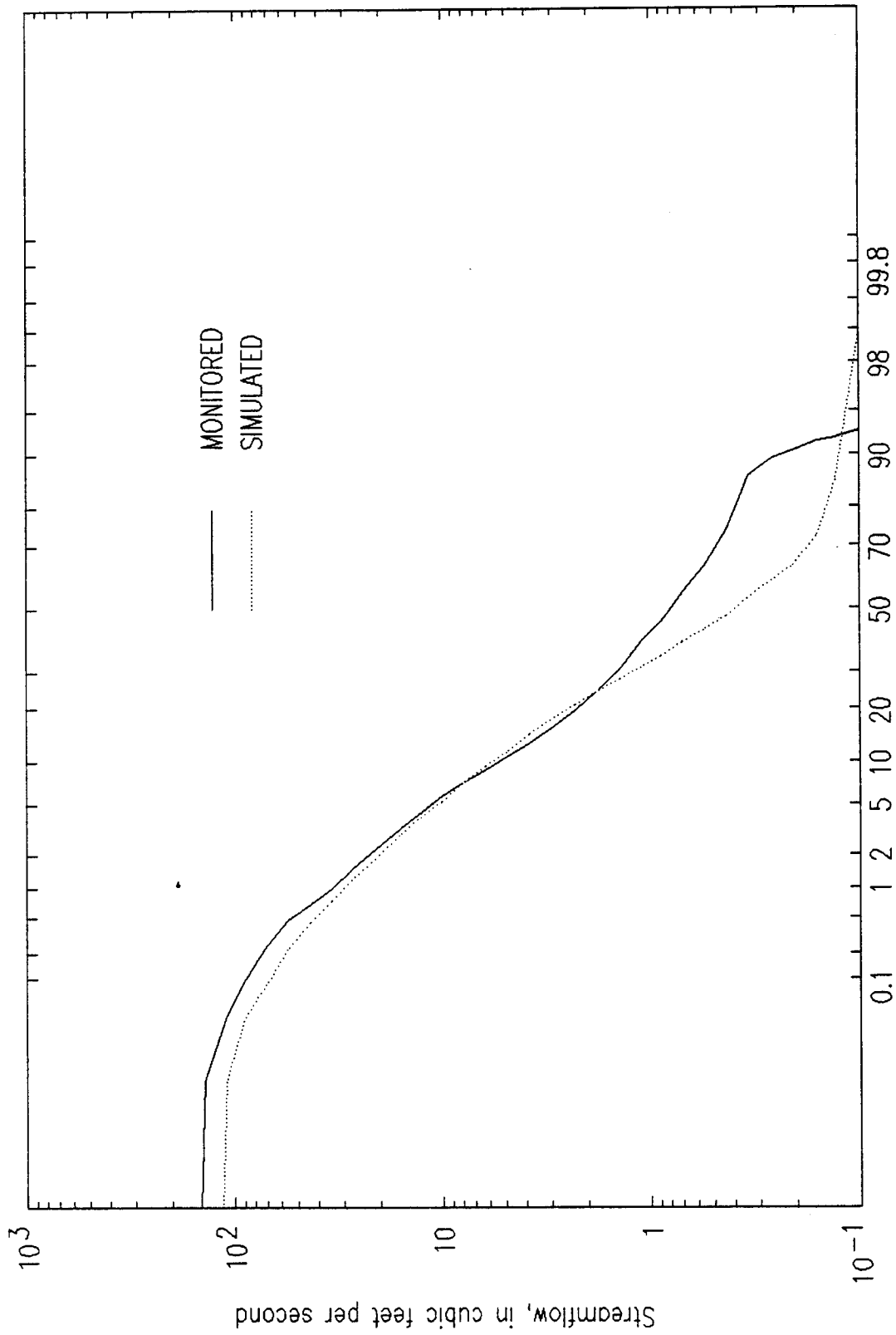
AR 010892

CALIBRATION PLOT - DES MOINES CREEK- SDS3

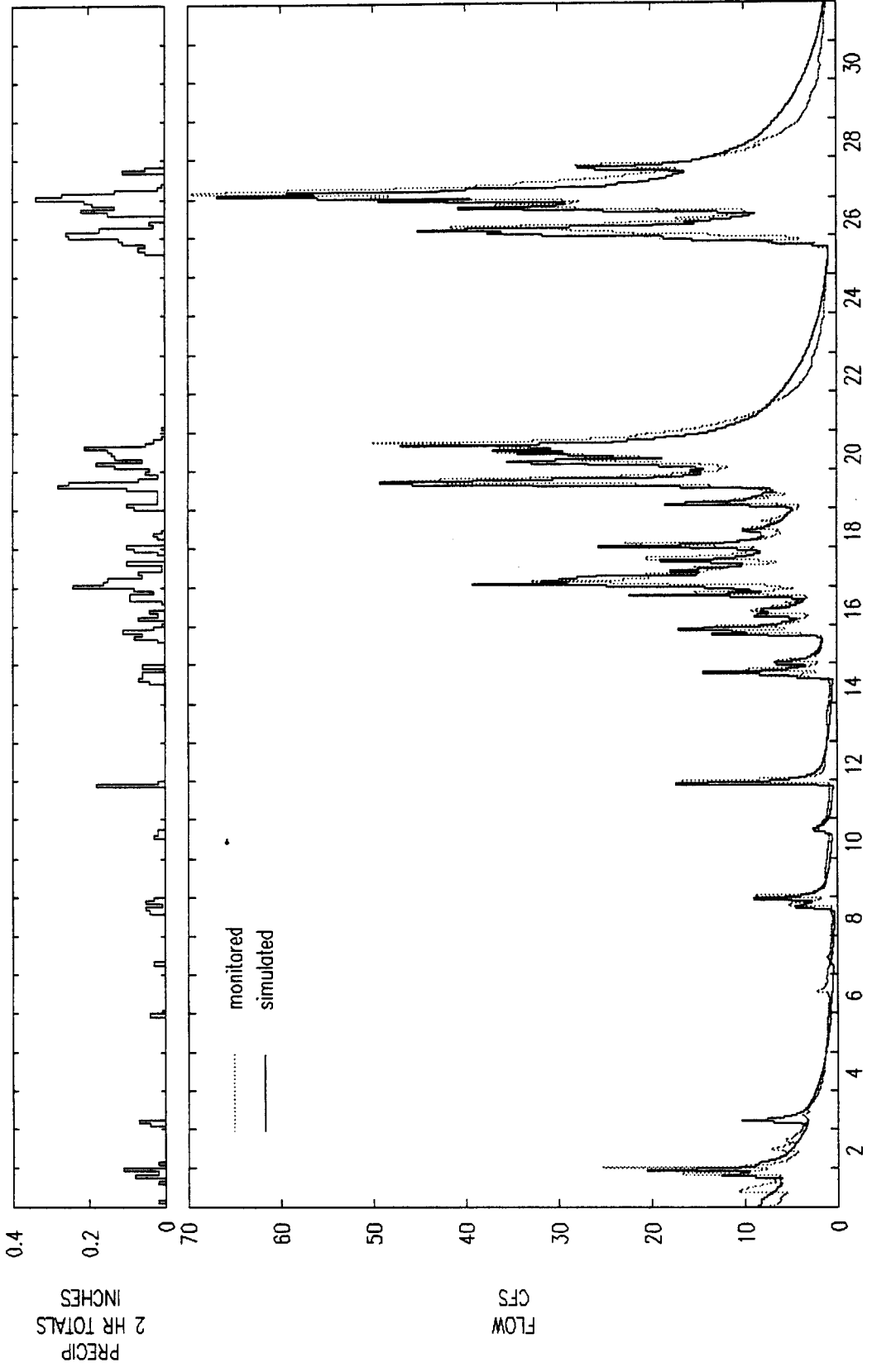


July 2001  
556-2912-001 (28)

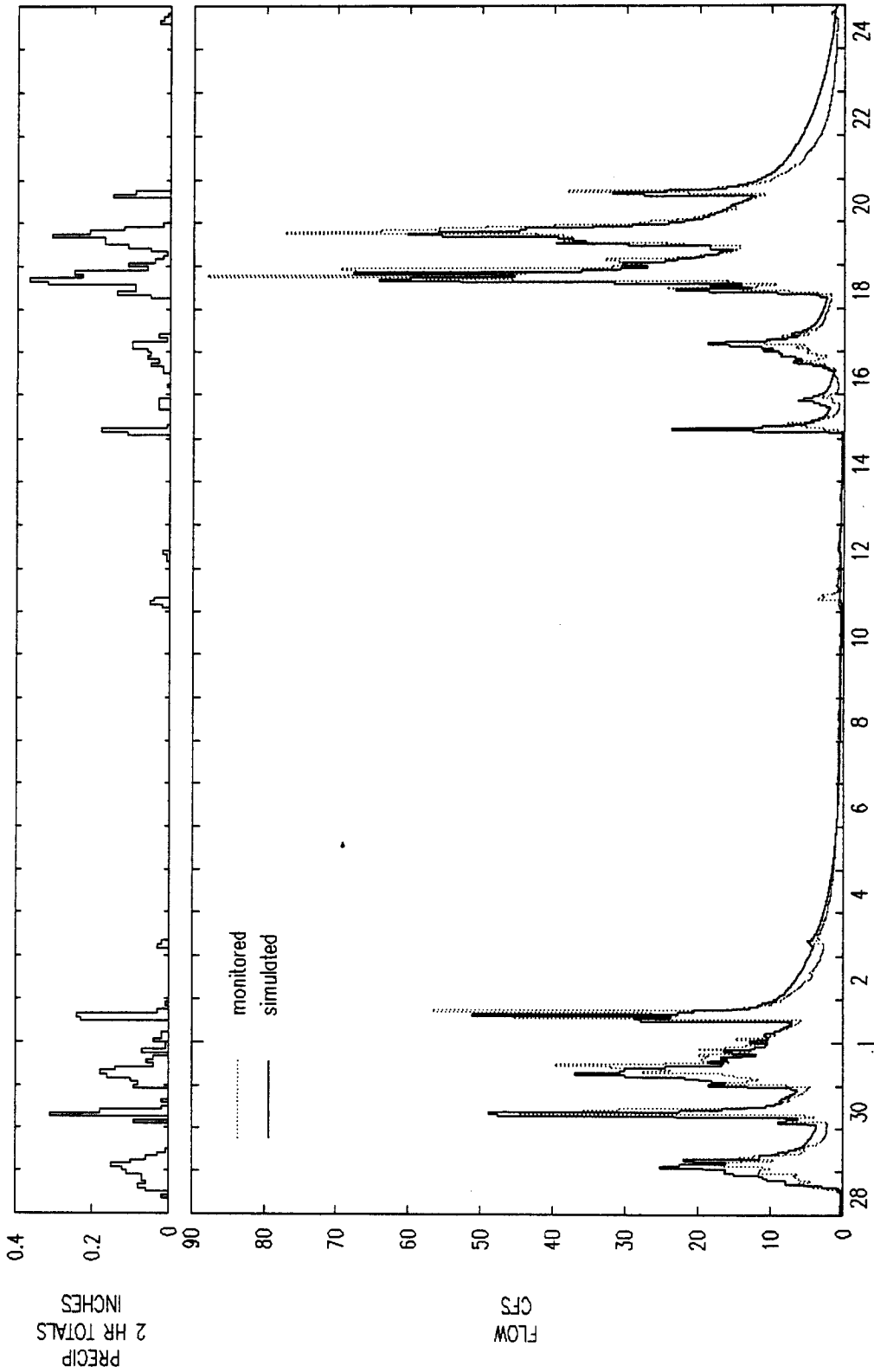
AR 010893



CALIBRATED VS MONITORED VALUES AT TEE POND INFLOW

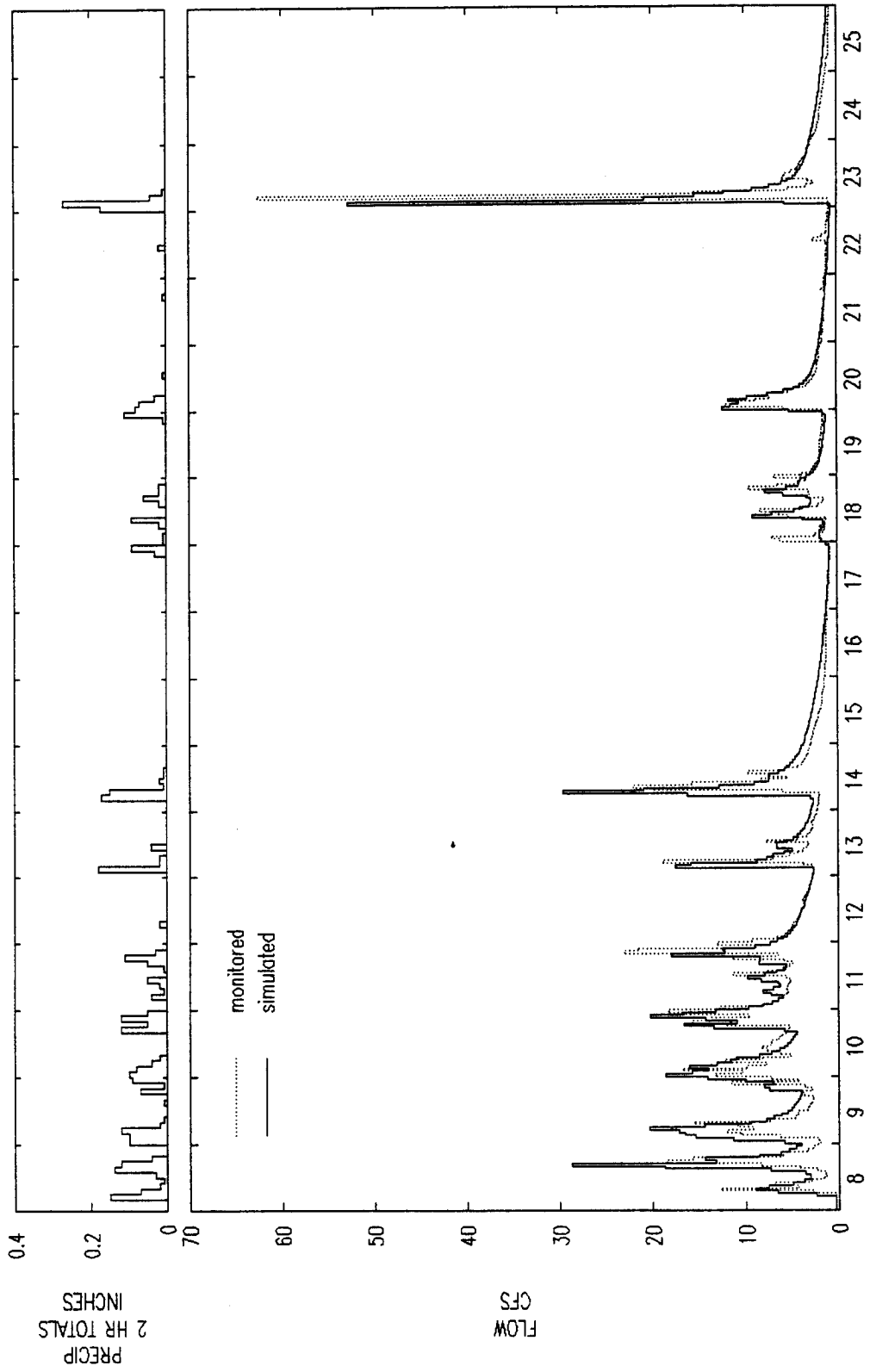


DECEMBER, 1994  
 TIME, IN DAYS  
 DES MOINES CREEK  
 CALIBRATION PLOT  
 TYEE POND INFLOW

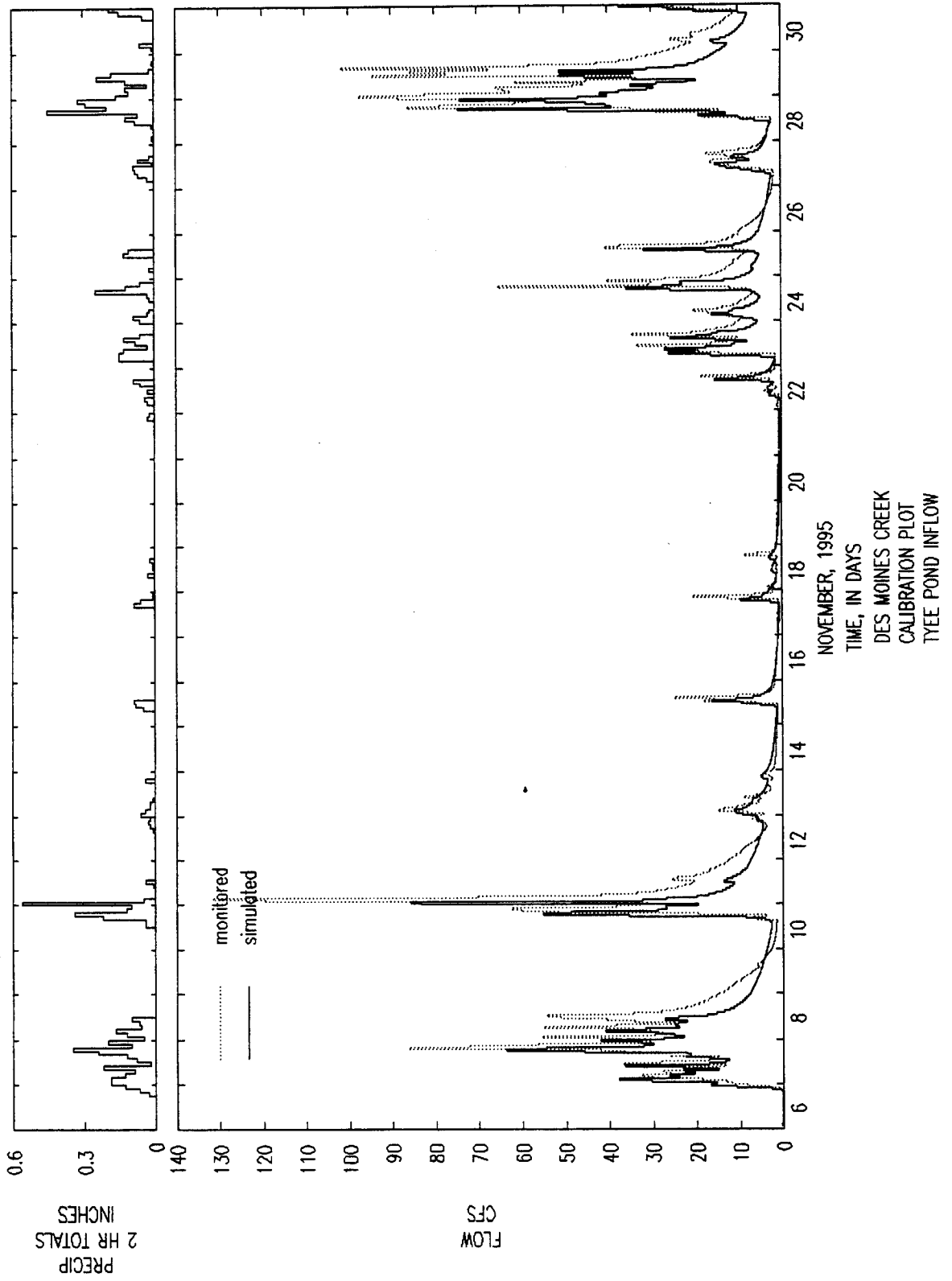


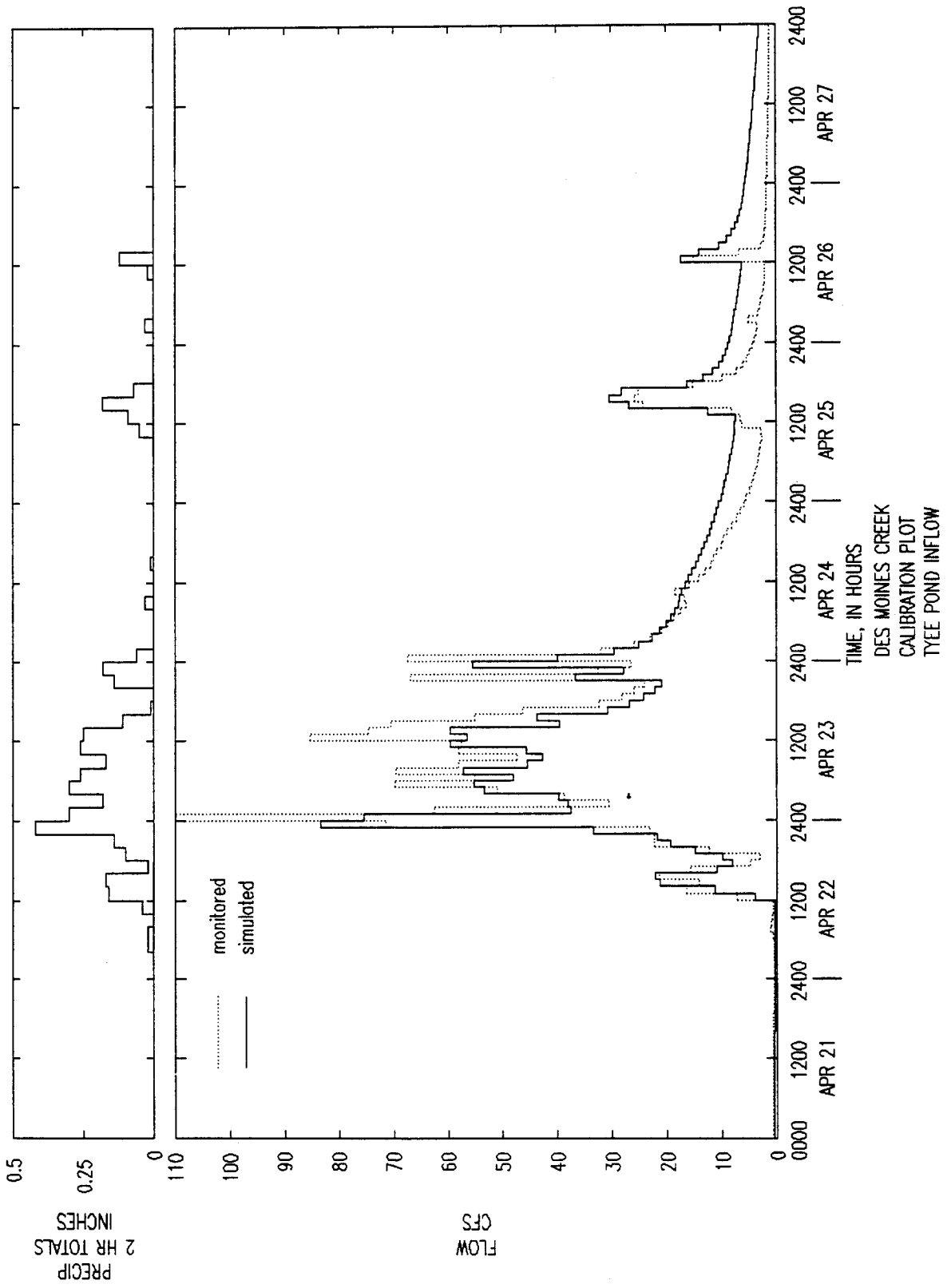
FEBRUARY, 1995  
 TIME, IN DAYS  
 DES MOINES CREEK  
 CALIBRATION PLOT  
 TYEE POND INFLOW

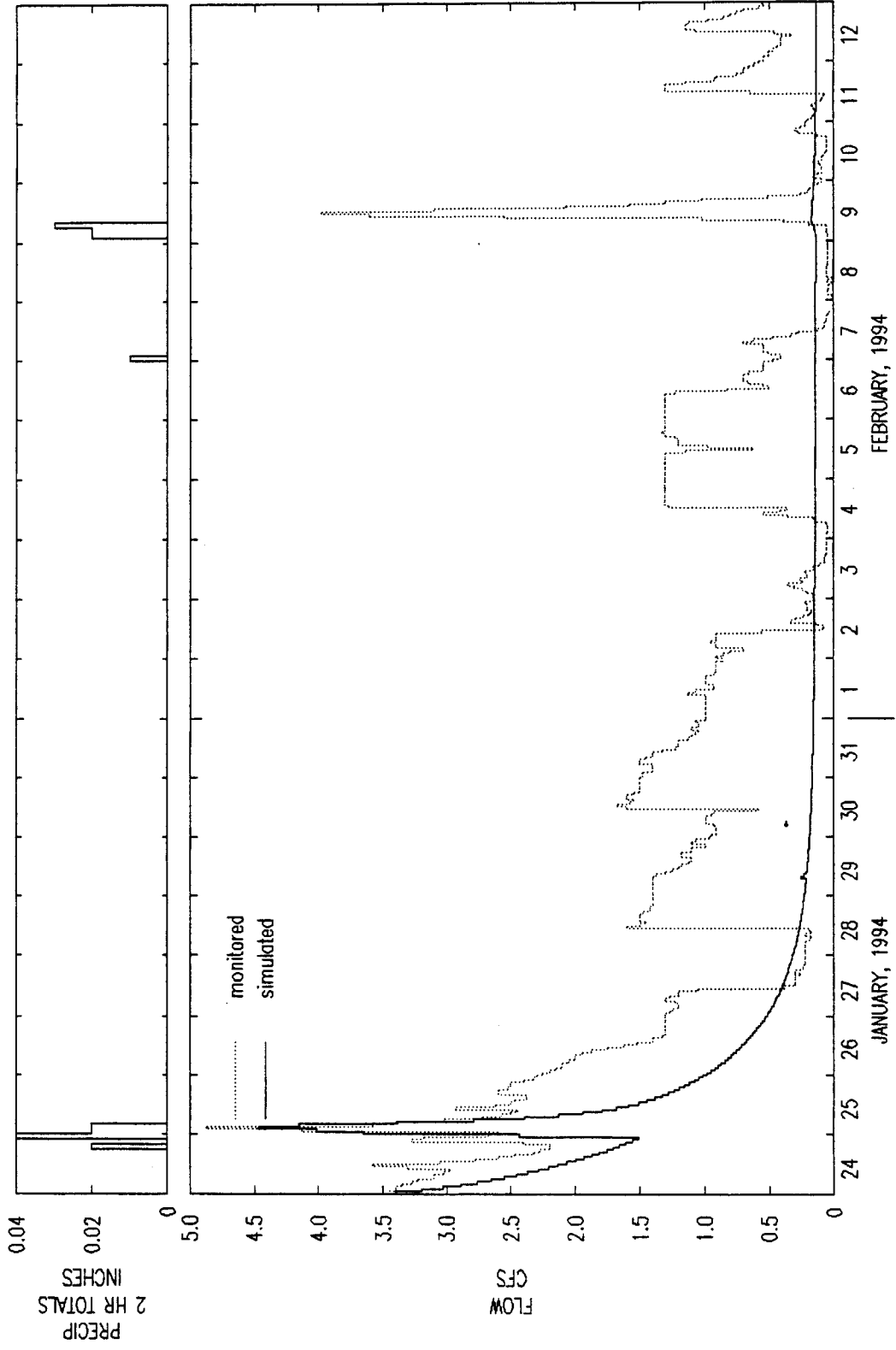




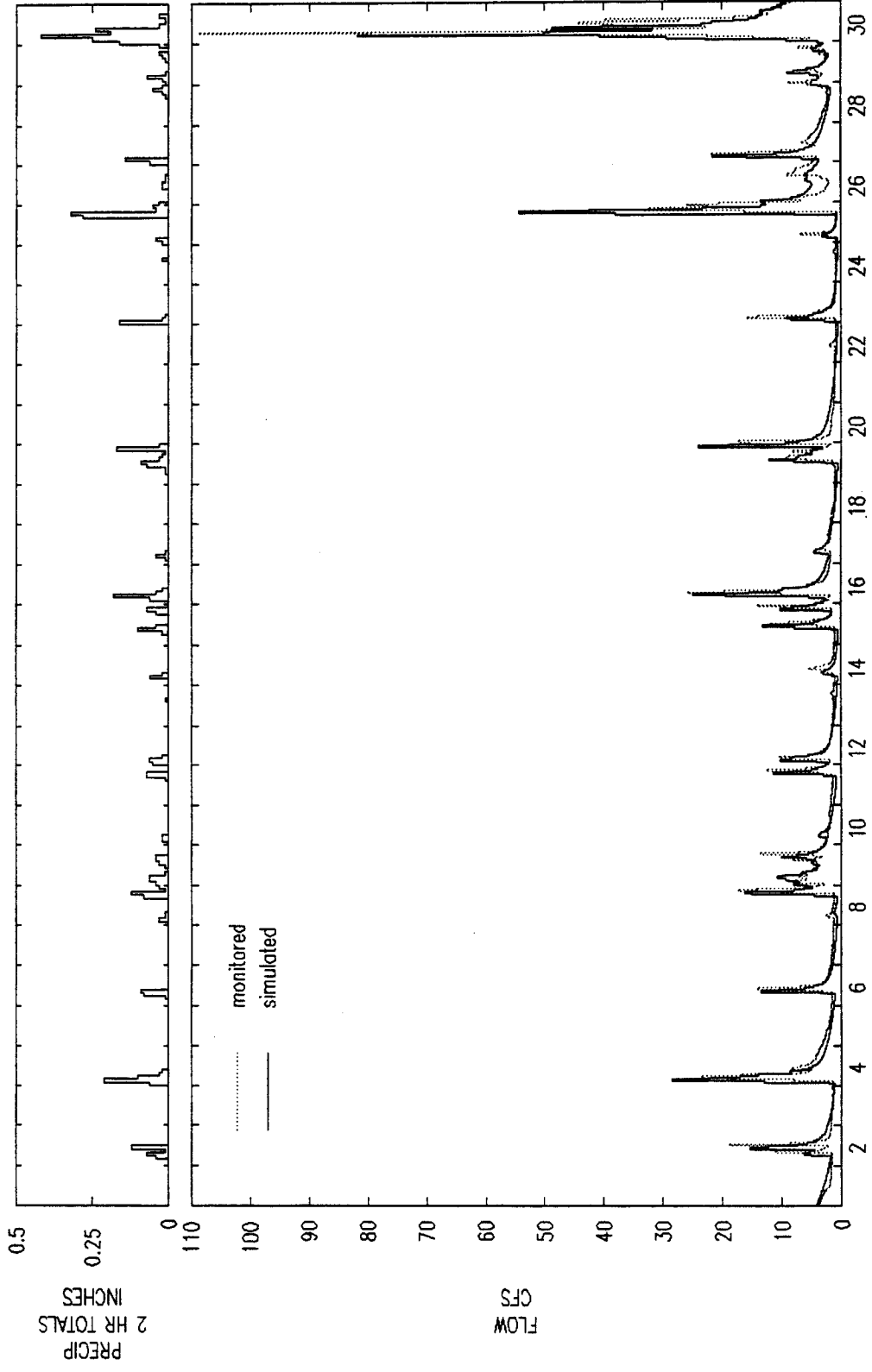
MARCH, 1995  
 TIME, IN DAYS  
 DES MOINES CREEK  
 CALIBRATION PLOT  
 TYEE POND INFLOW



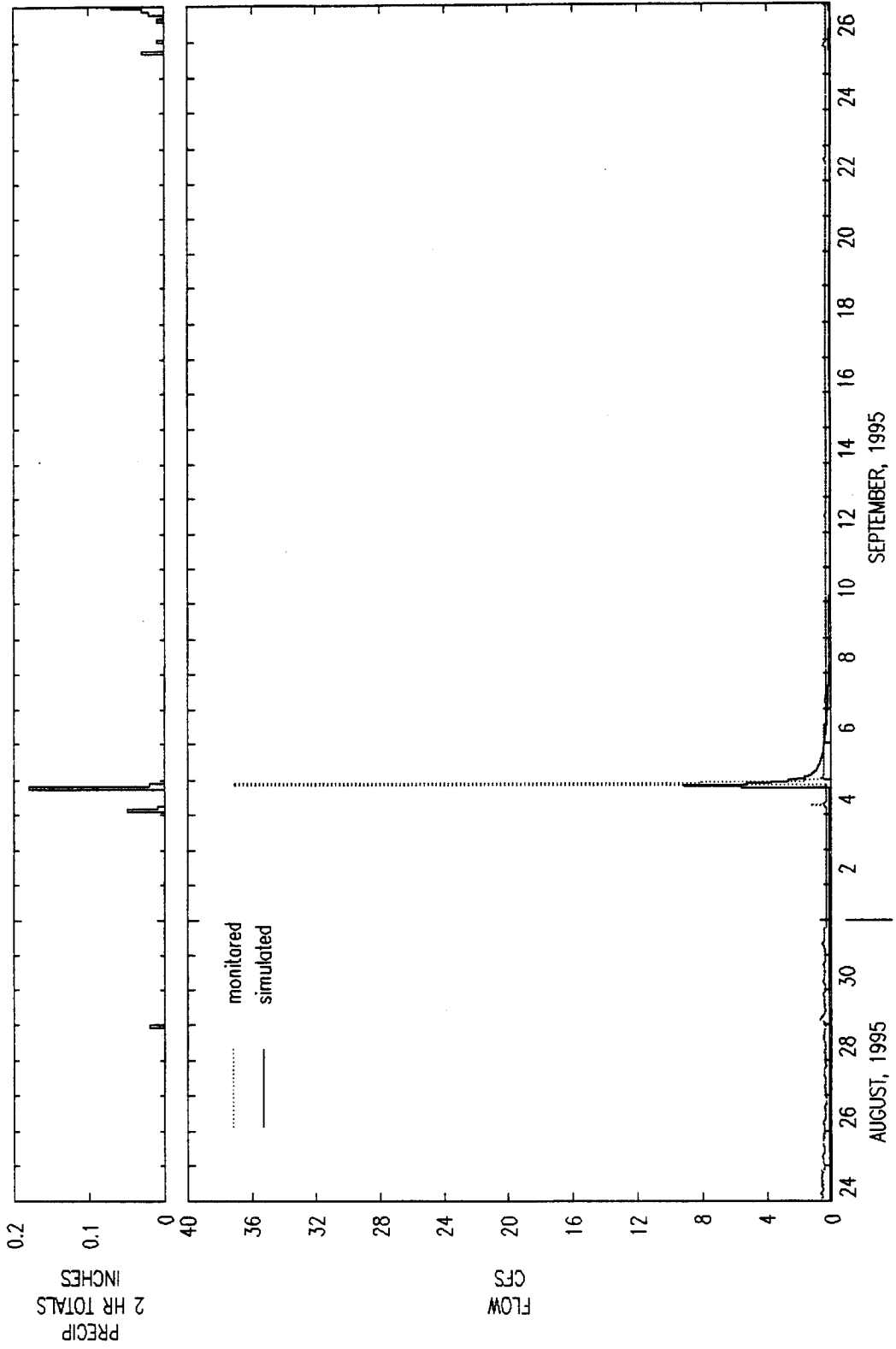




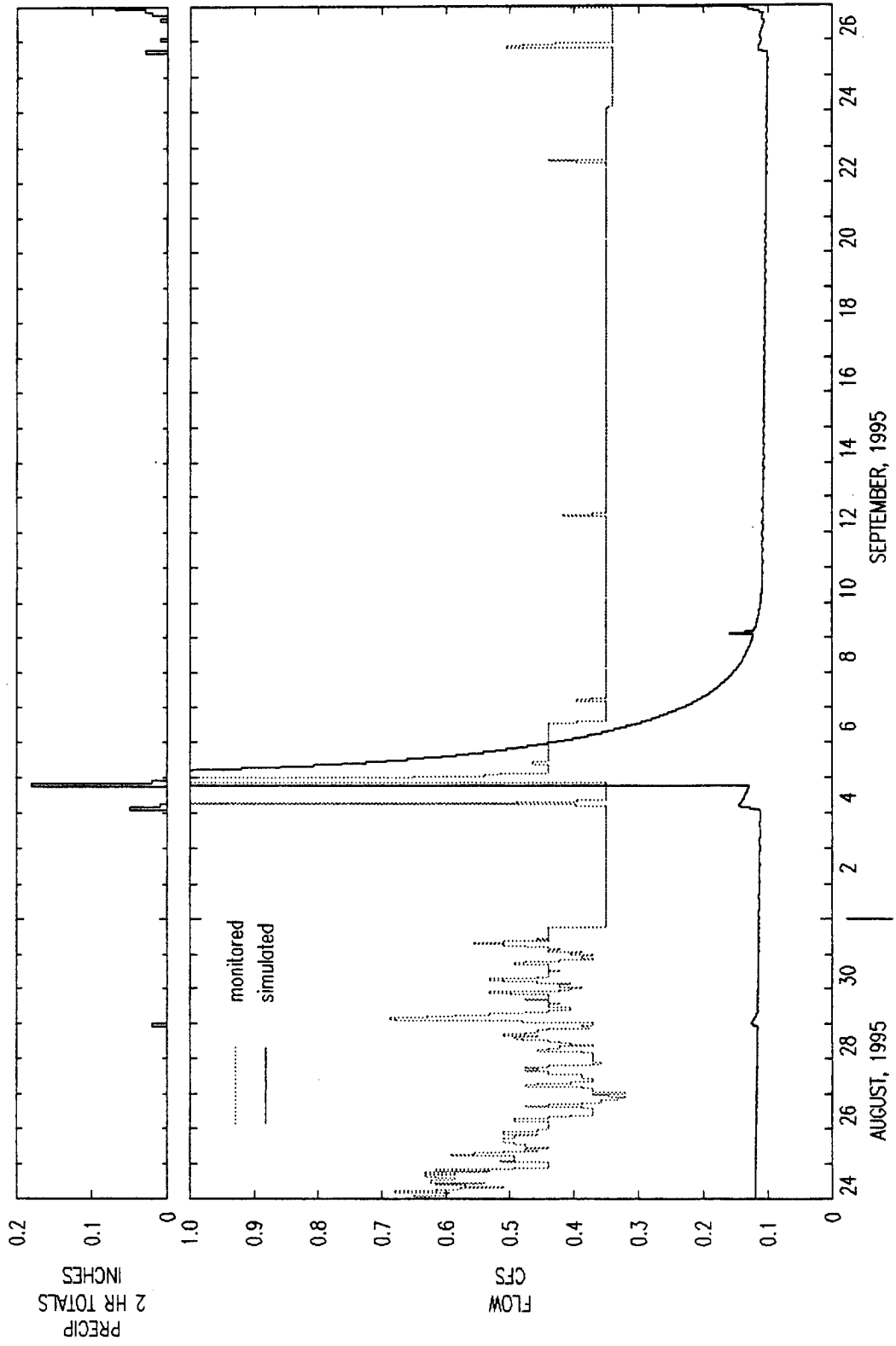
TIME, IN DAYS  
 DES MOINES CREEK  
 CALIBRATION PLOT  
 TYEE POND INFLOW



NOVEMBER, 1994  
 TIME, IN DAYS  
 DES MOINES CREEK  
 CALIBRATION PLOT  
 TYEE POND INFLOW



DES MOINES CREEK  
 CALIBRATION PLOT  
 TYEE POND INFLOW

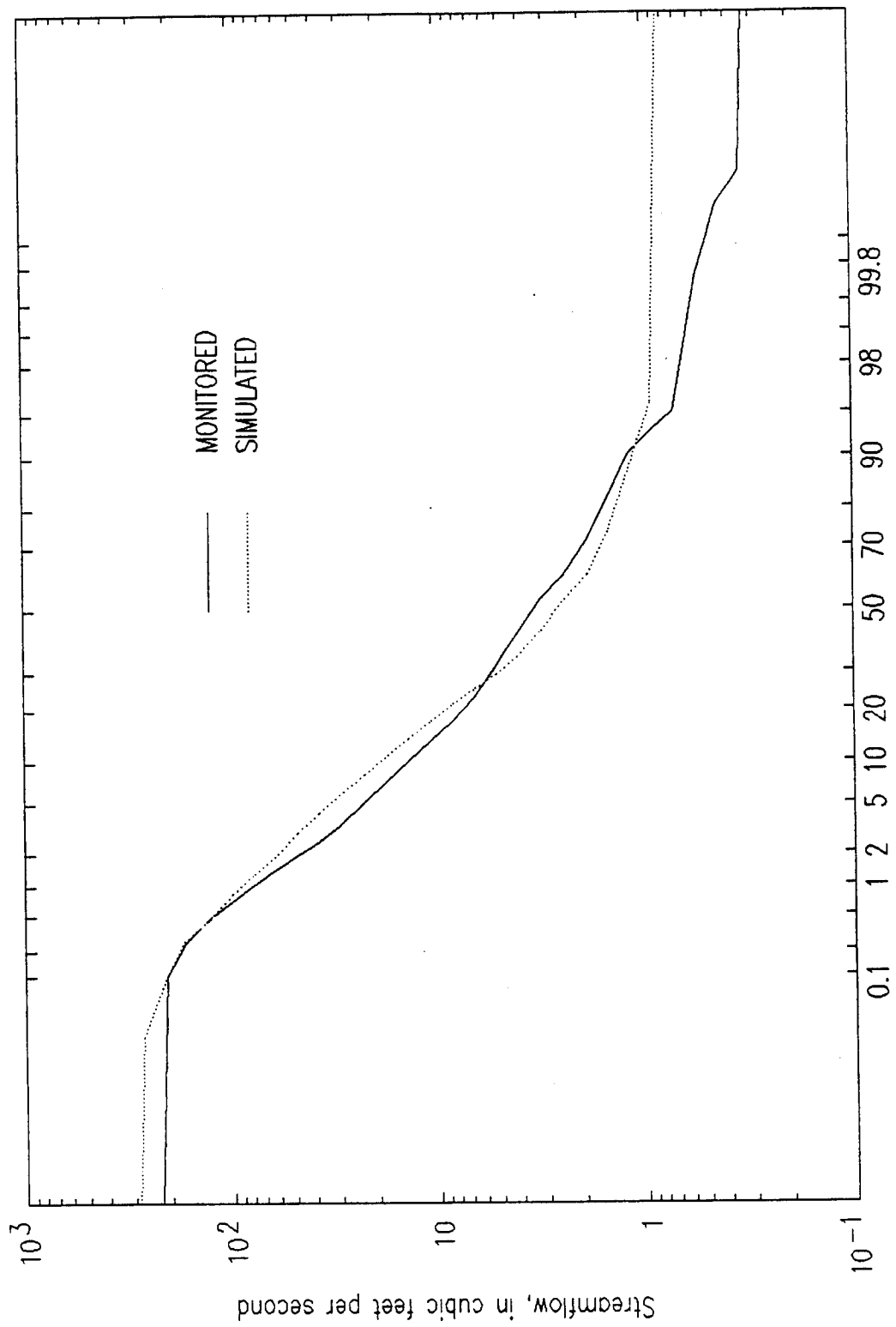


TIME, IN DAYS  
 DES MOINES CREEK  
 CALIBRATION PLOT  
 TYEE POND INFLOW

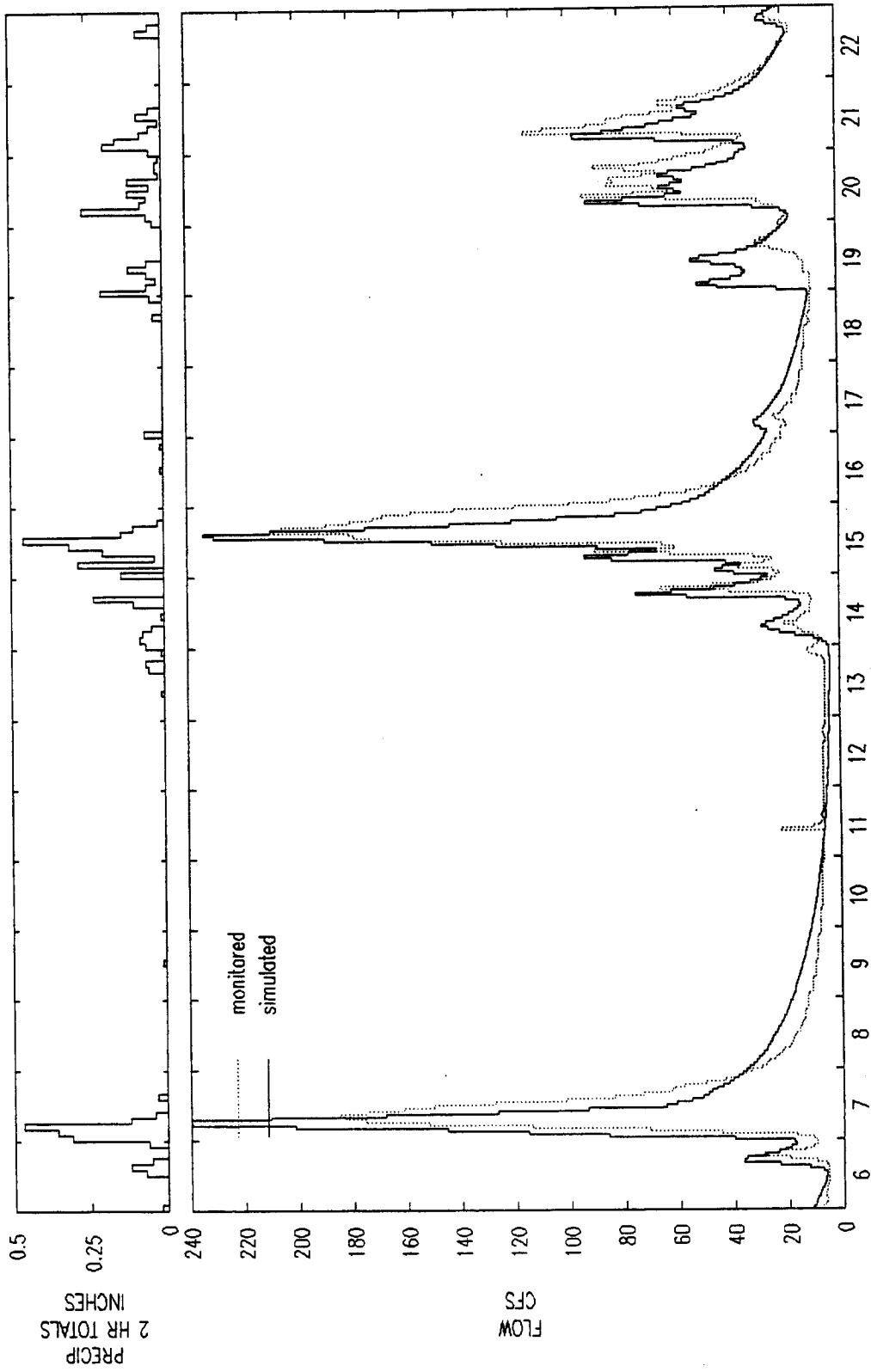
**U.S. GEOLOGICAL SURVEY  
PARAMETER VALUE HYDROGRAPHS**

**AR 010904**

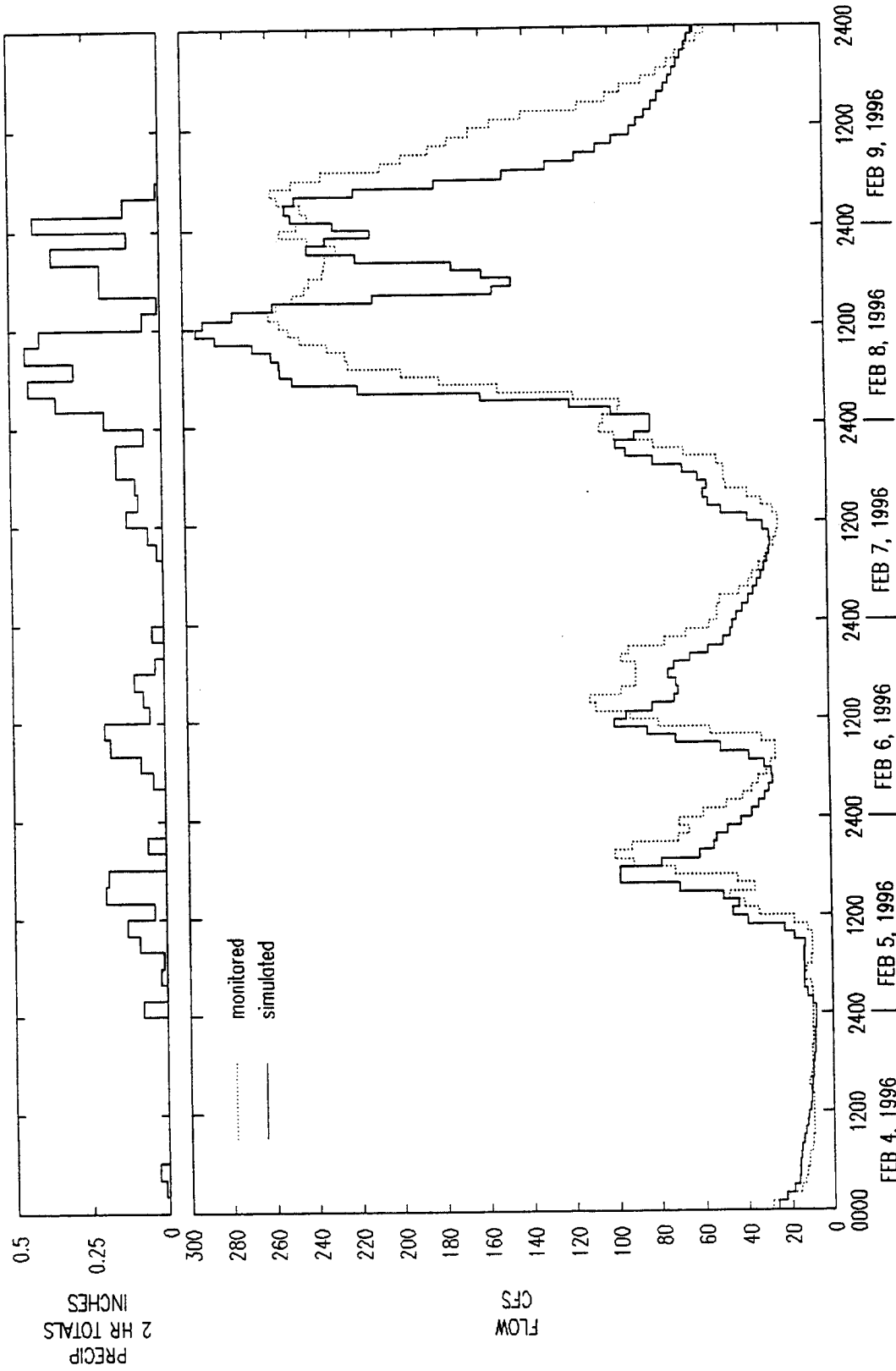




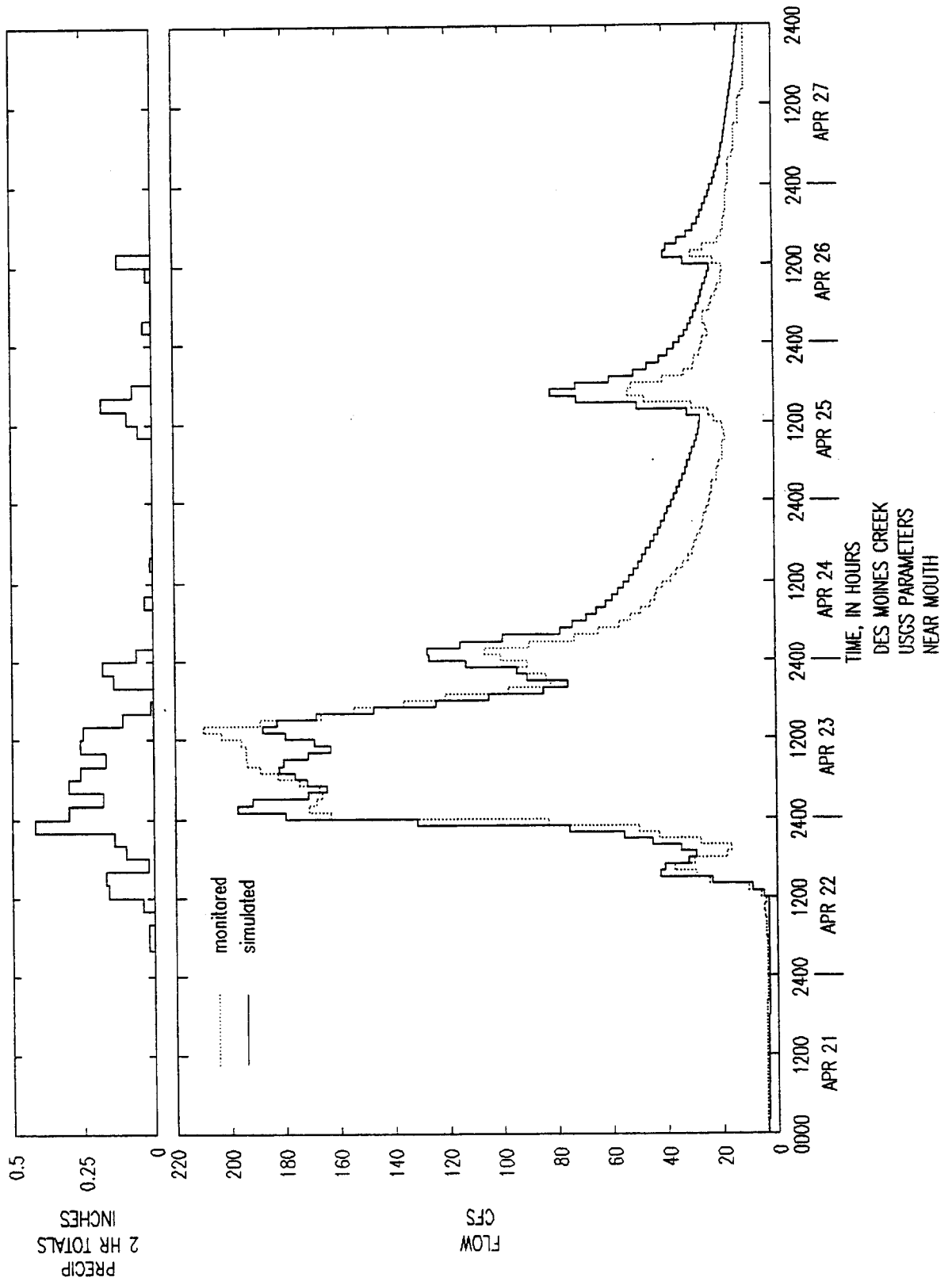
Percent chance flow exceeded  
 USGS VS MONITORED VALUES NEAR MOUTH

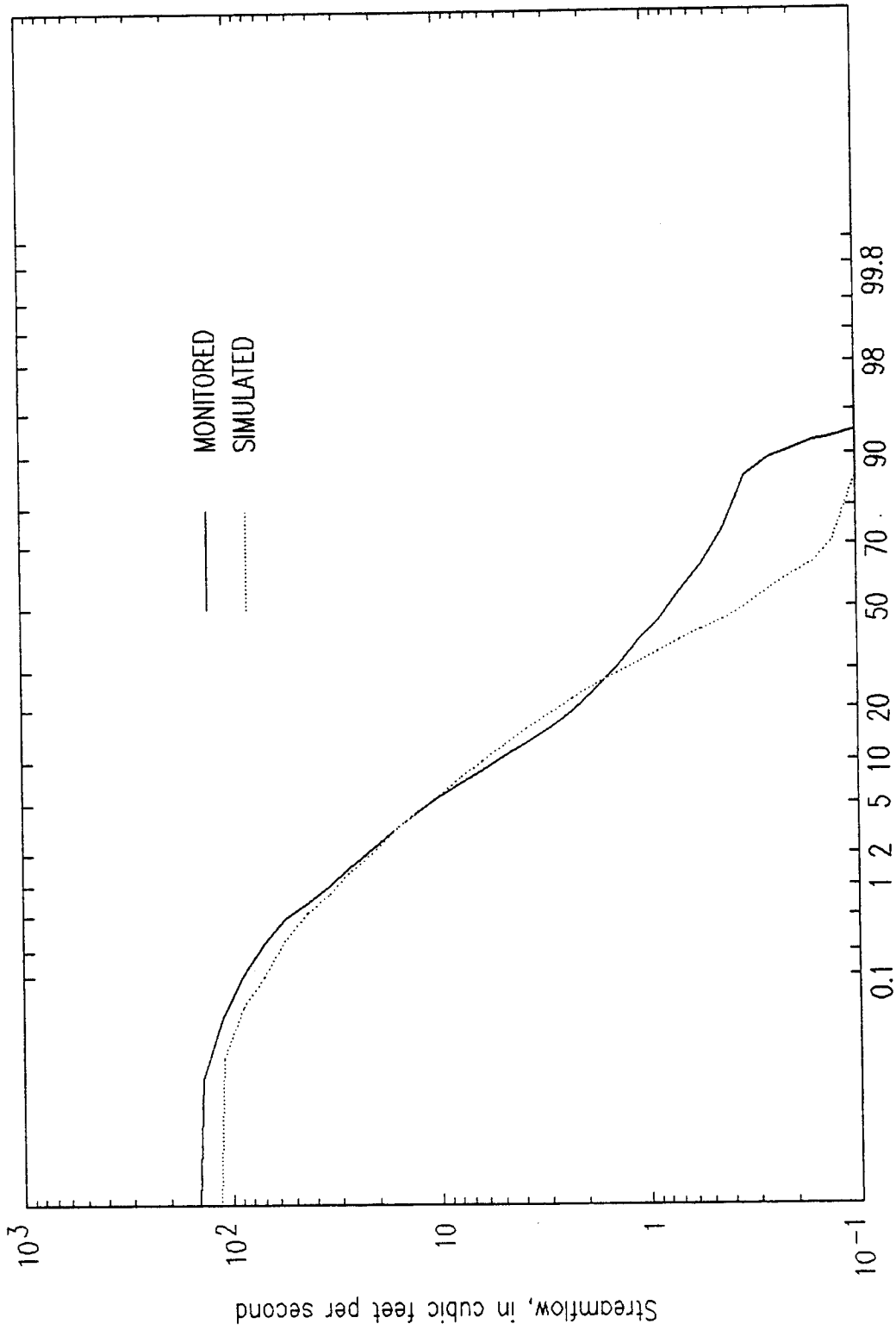


JANUARY, 1996  
 TIME, IN DAYS  
 DES MOINES CREEK  
 USGS PARAMETERS  
 NEAR MOUTH

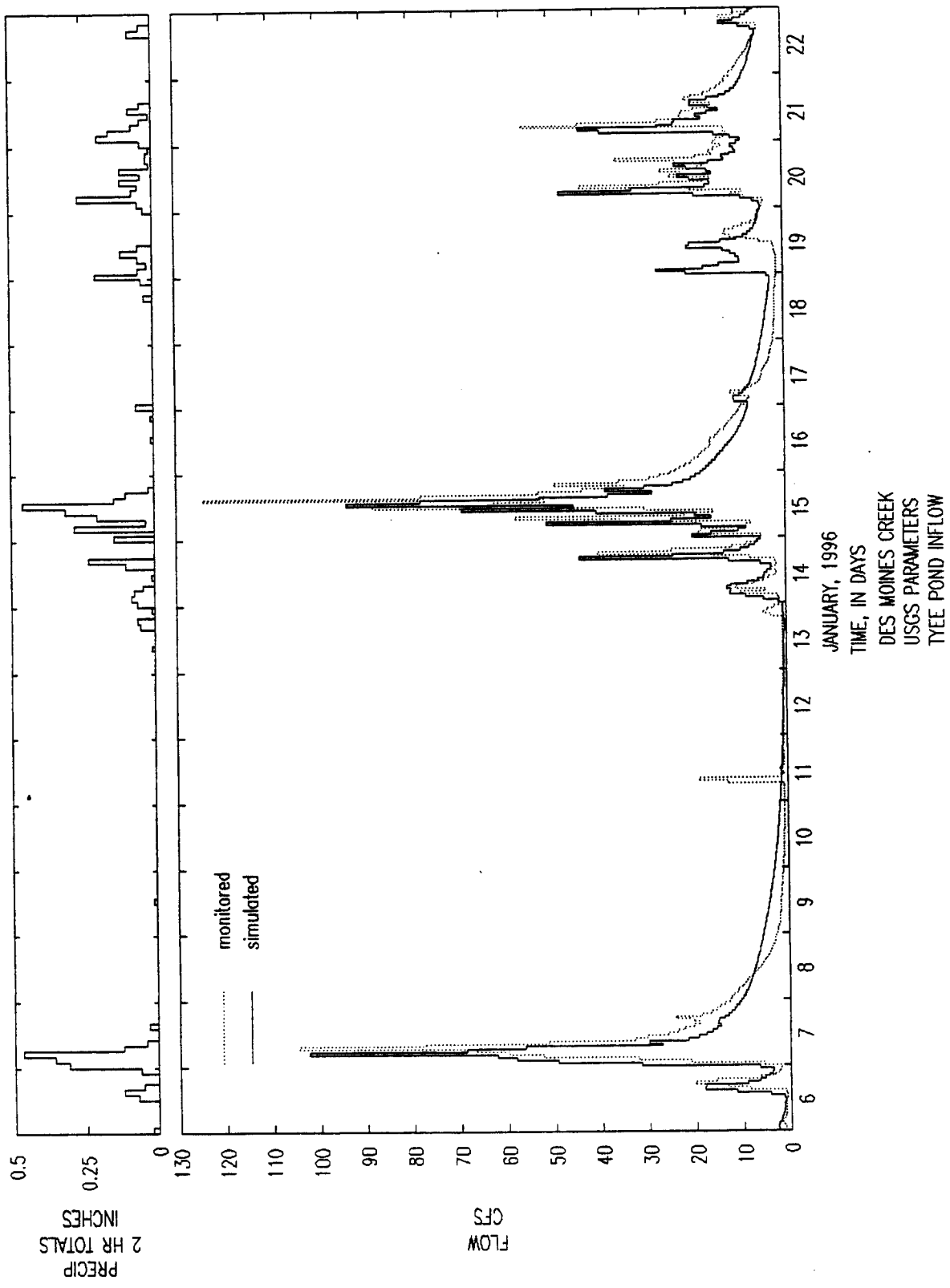


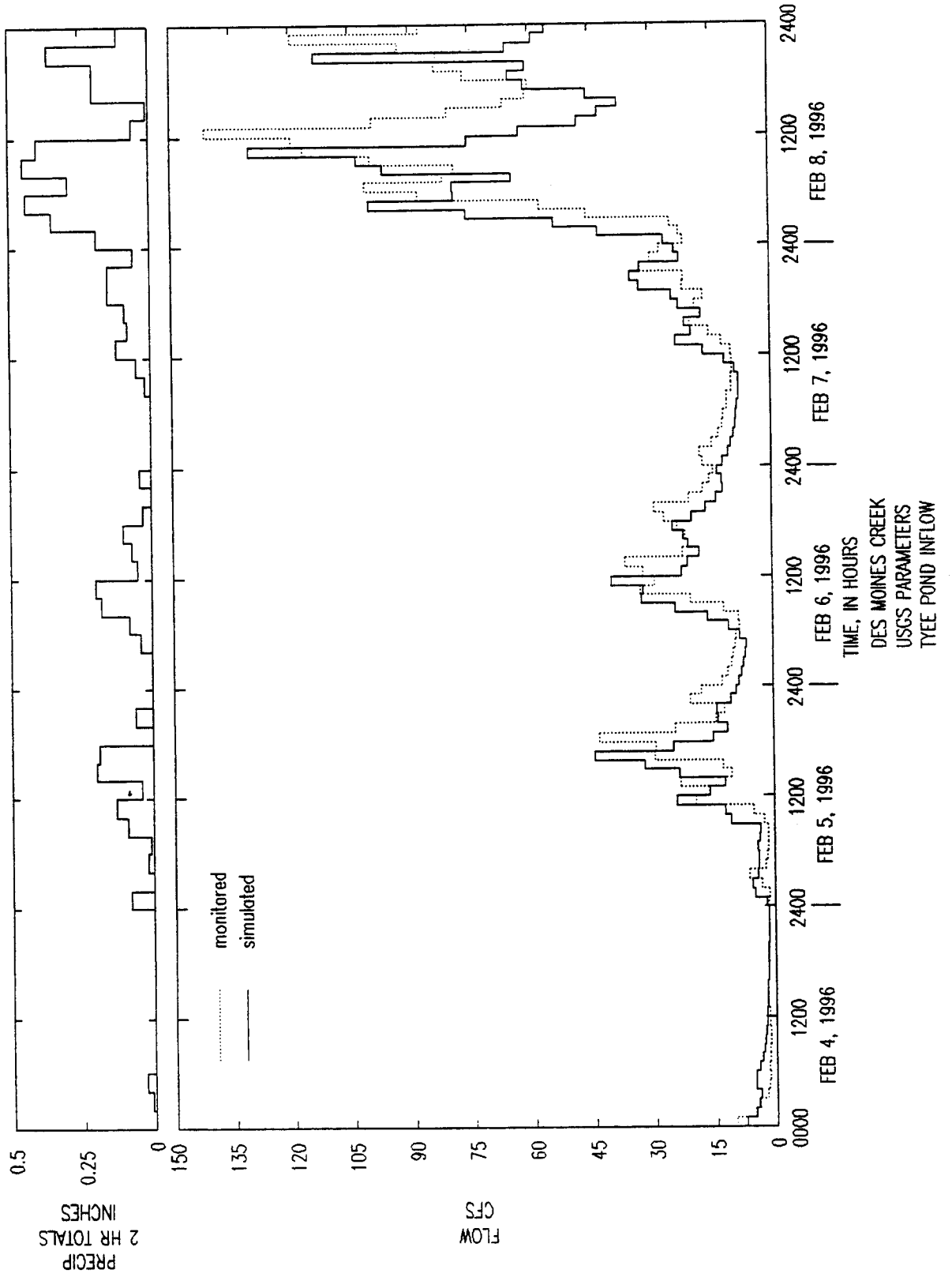
DES MOINES CREEK  
 USGS PARAMETERS  
 NEAR MOUTH

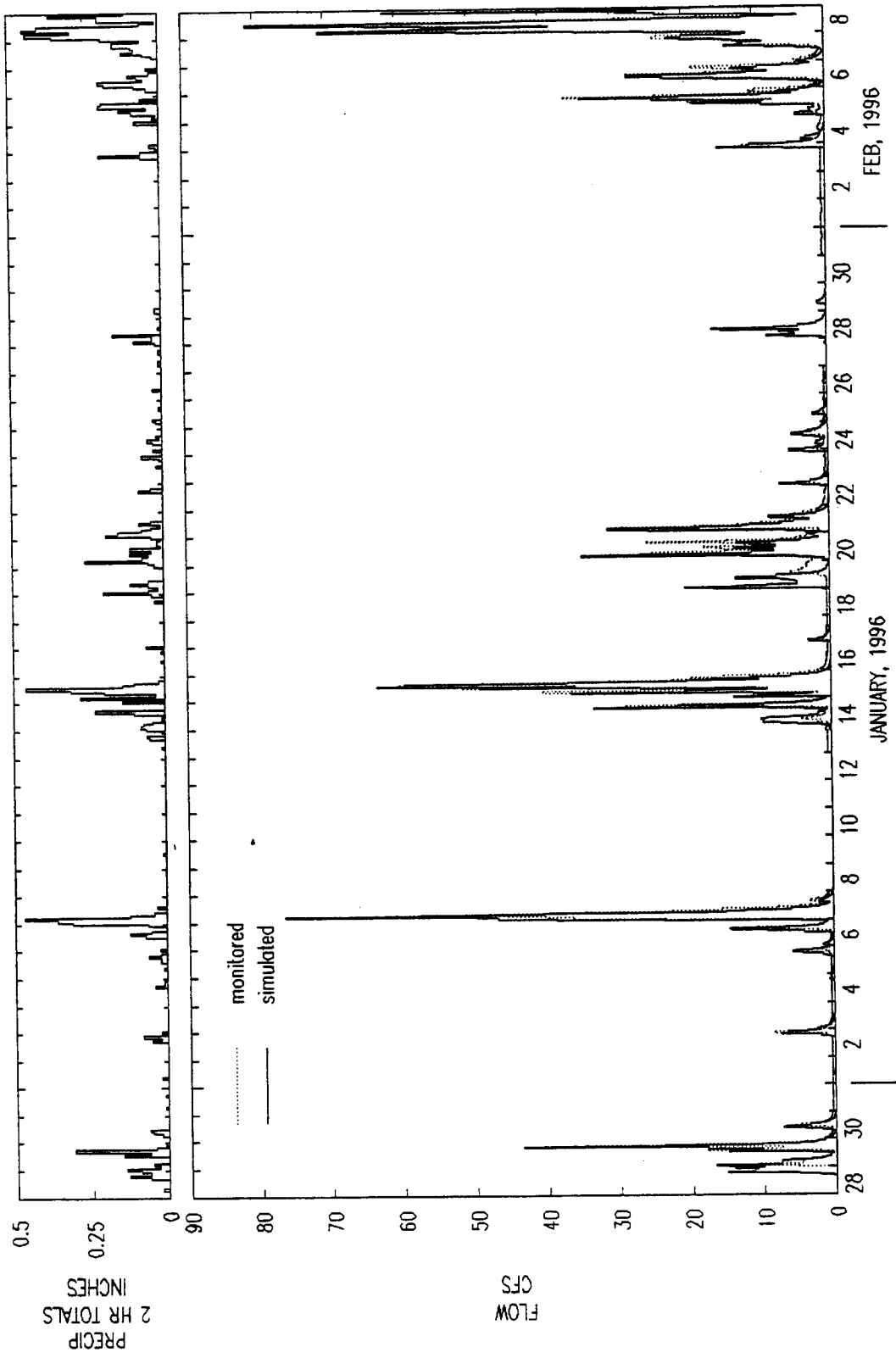




Percent chance flow exceeded  
 USGS VS MONITORED VALUES AT TYEE POND INFLOW

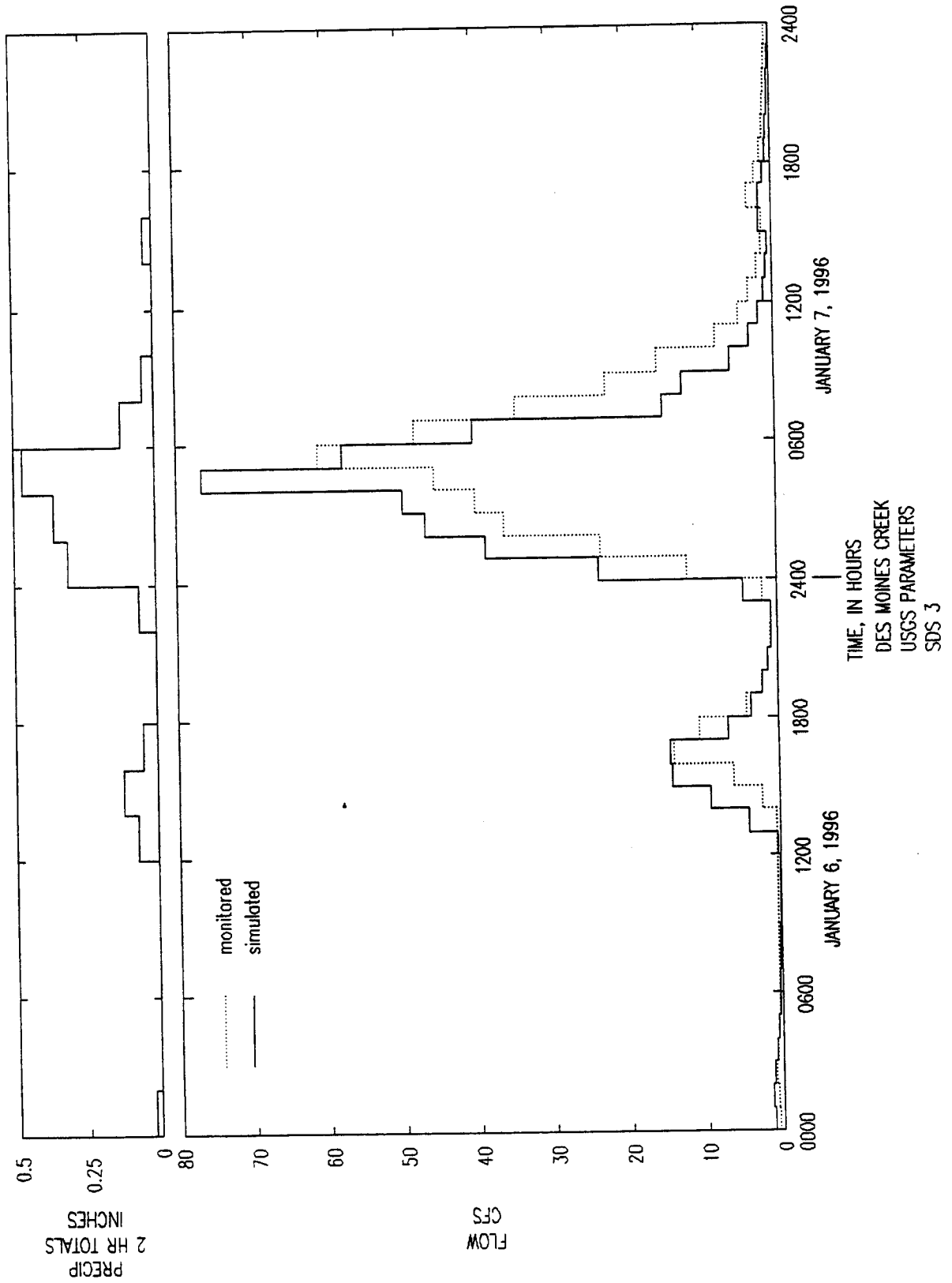


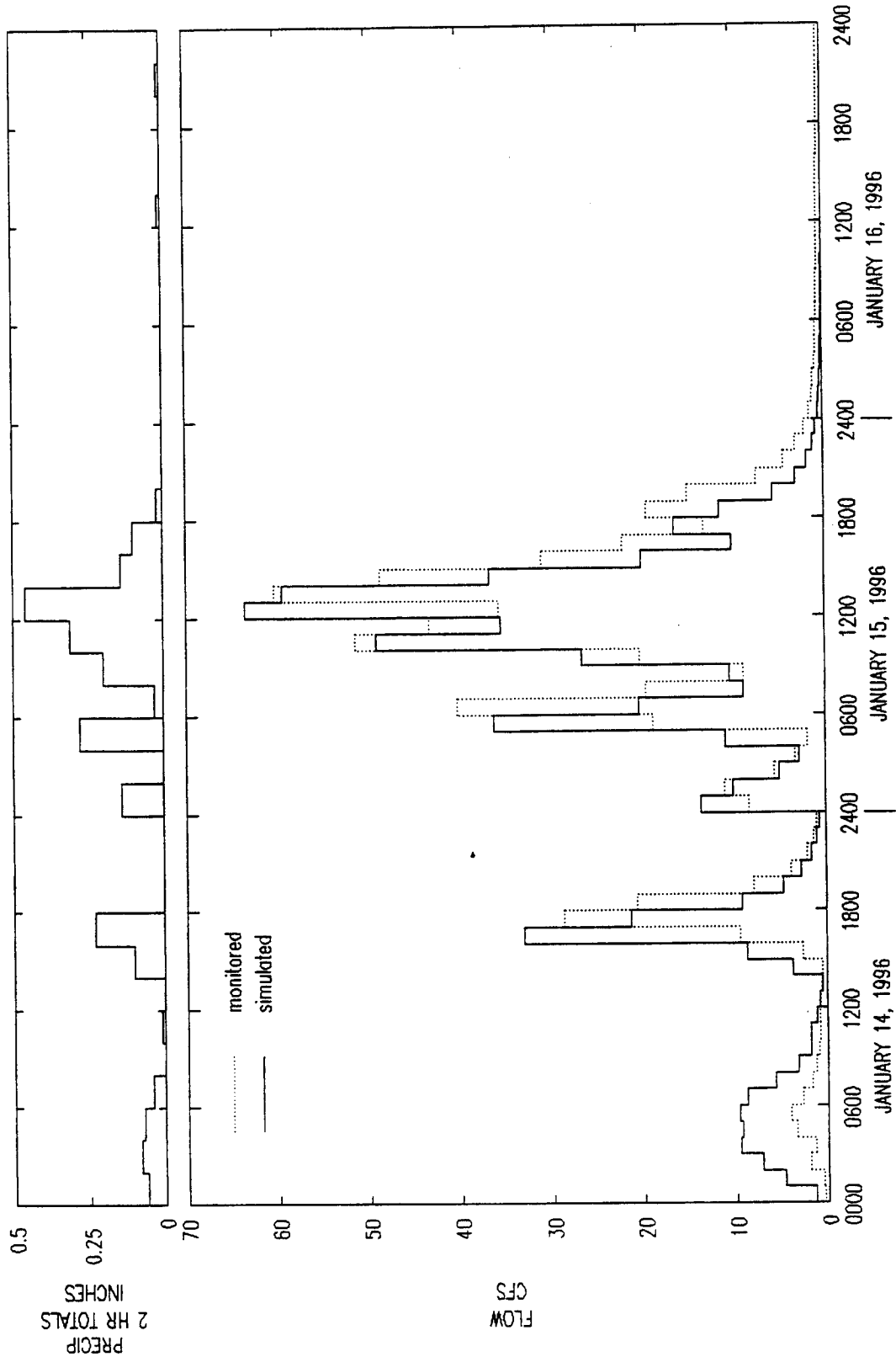




DES MOINES CREEK  
 USGS PARAMETERS  
 SDS 3

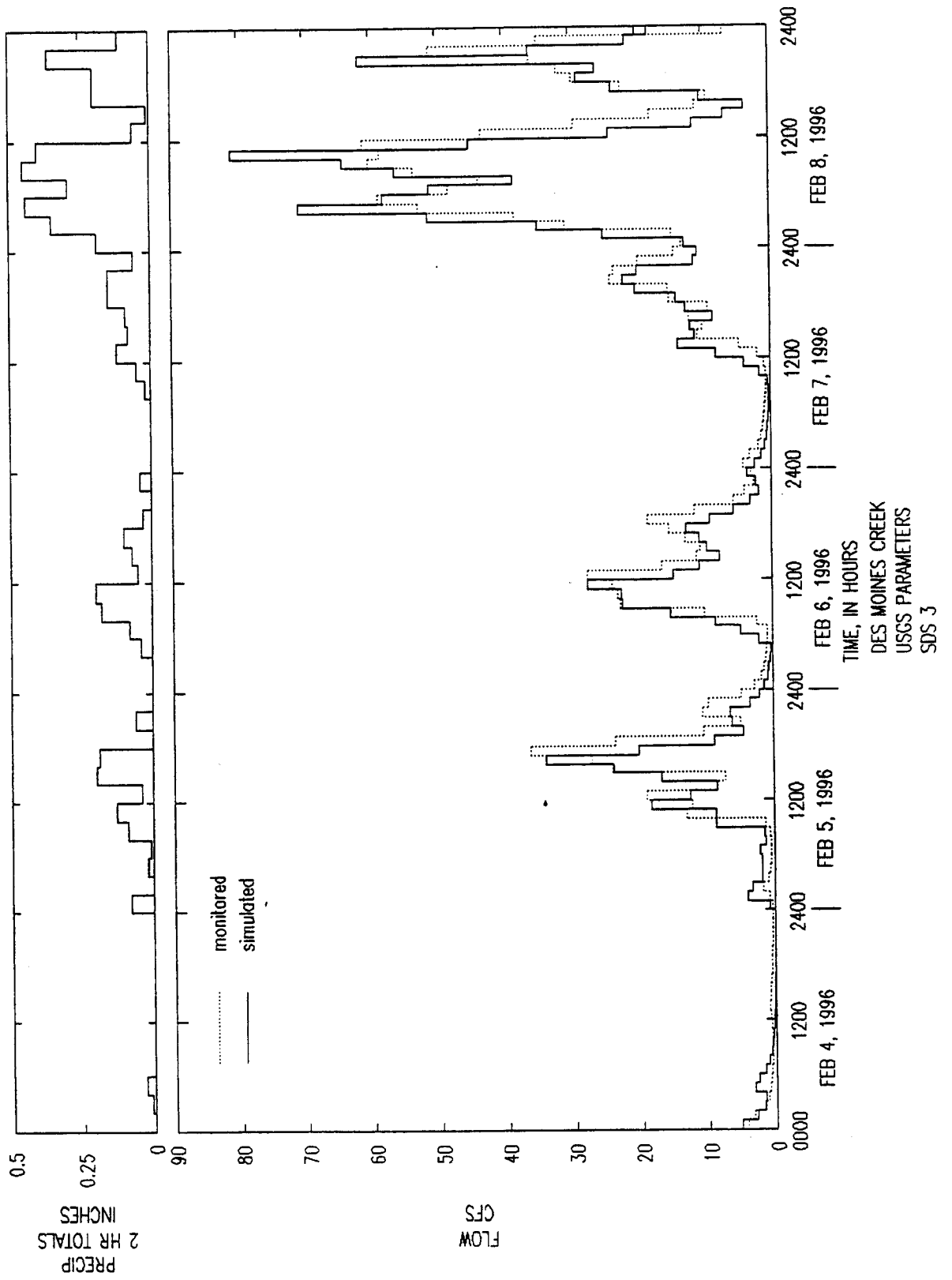






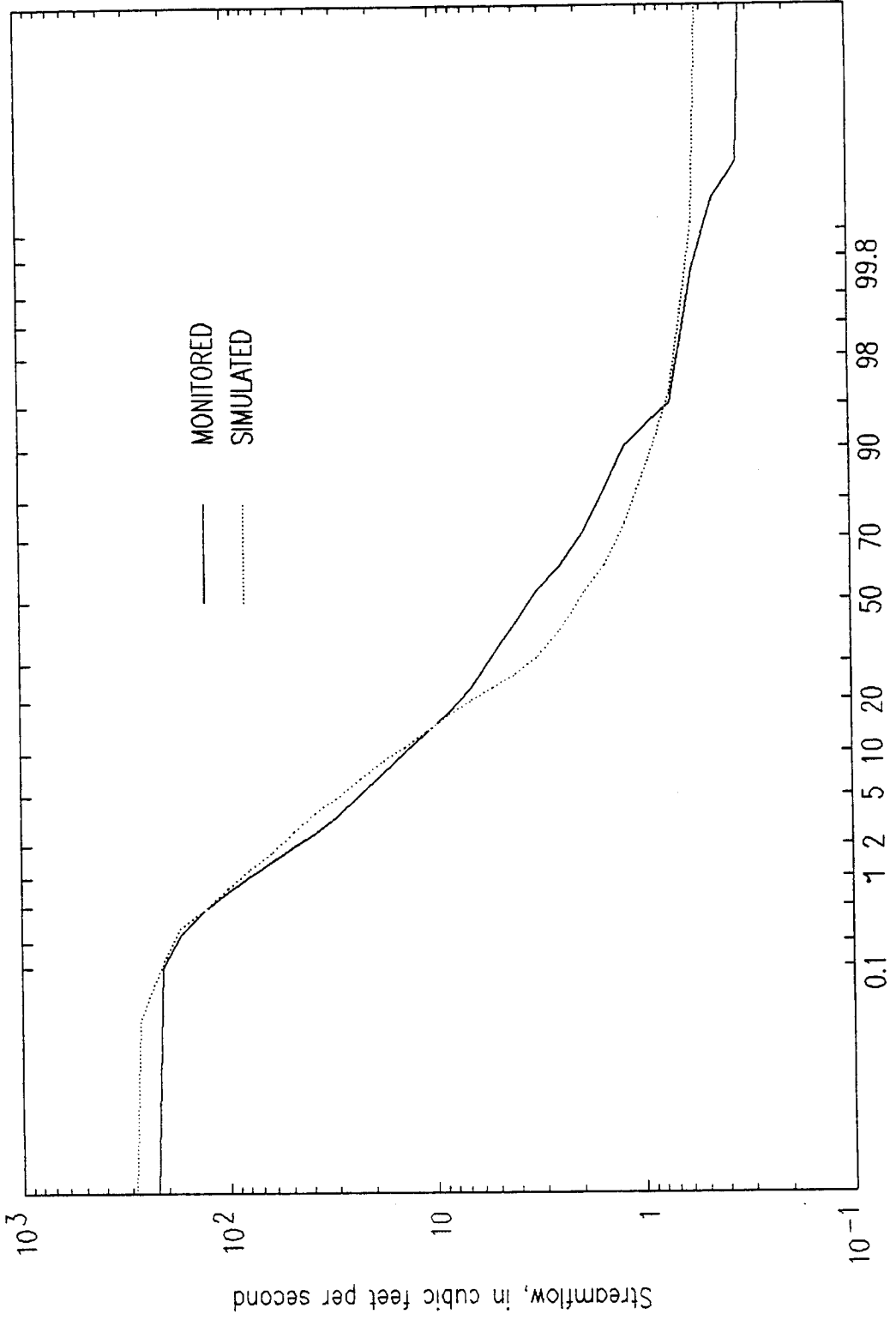
DES MOINES CREEK  
 USGS PARAMETERS  
 SDS 3

AR 010914

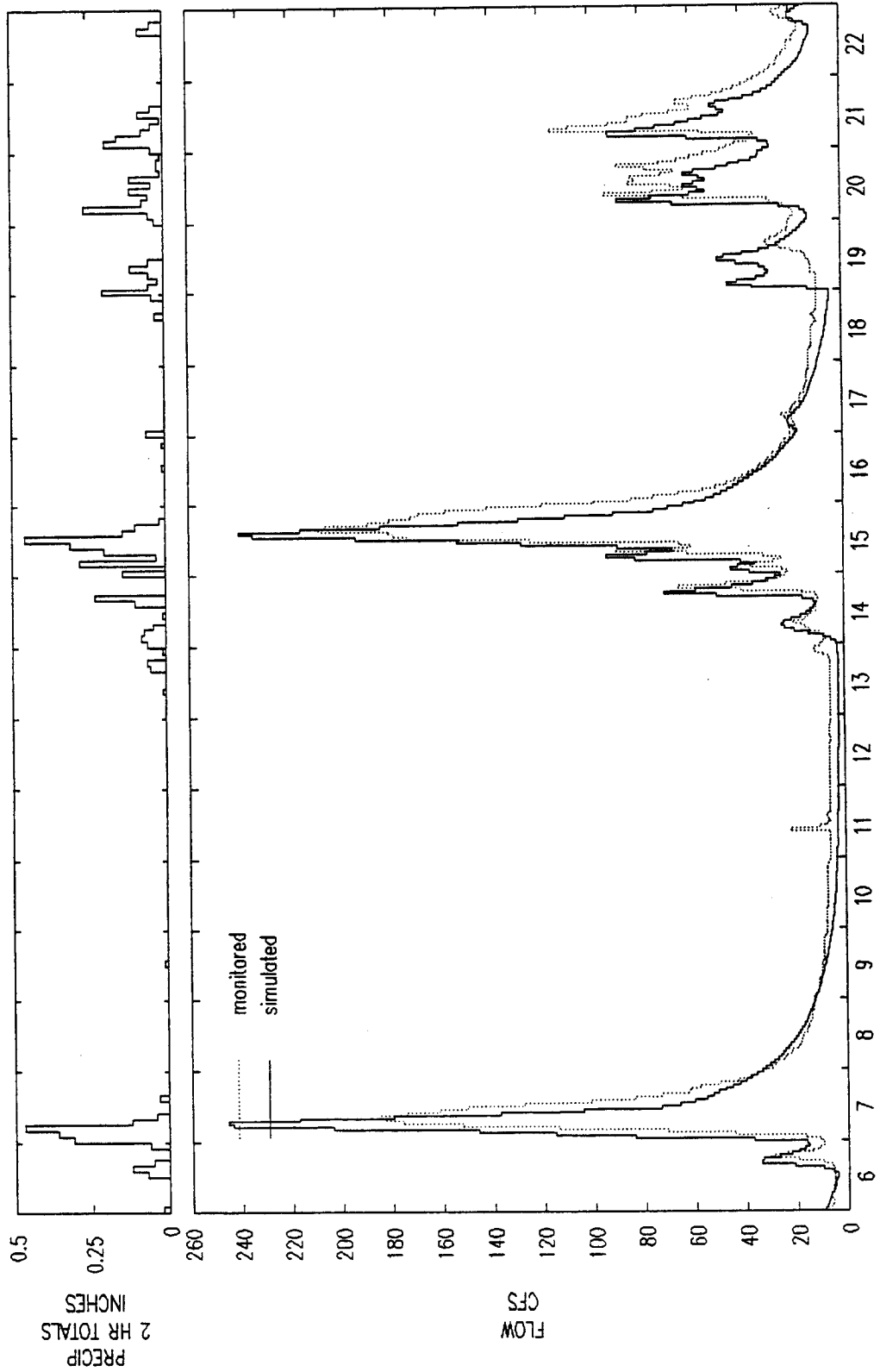


**DES MOINES CREEK BASIN PLAN  
PARAMETER VALUE HYDROGRAPHS**

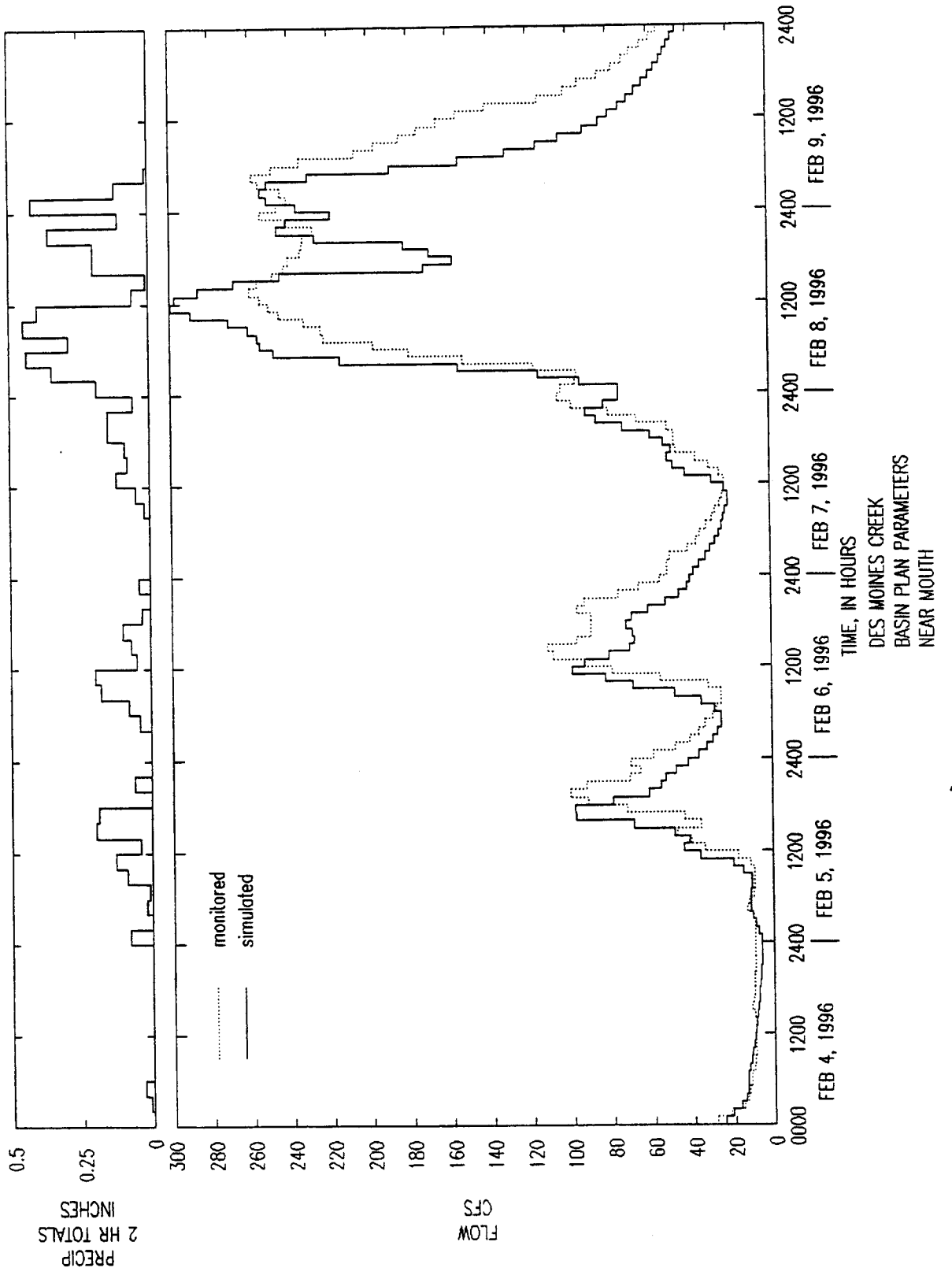
**AR 010916**

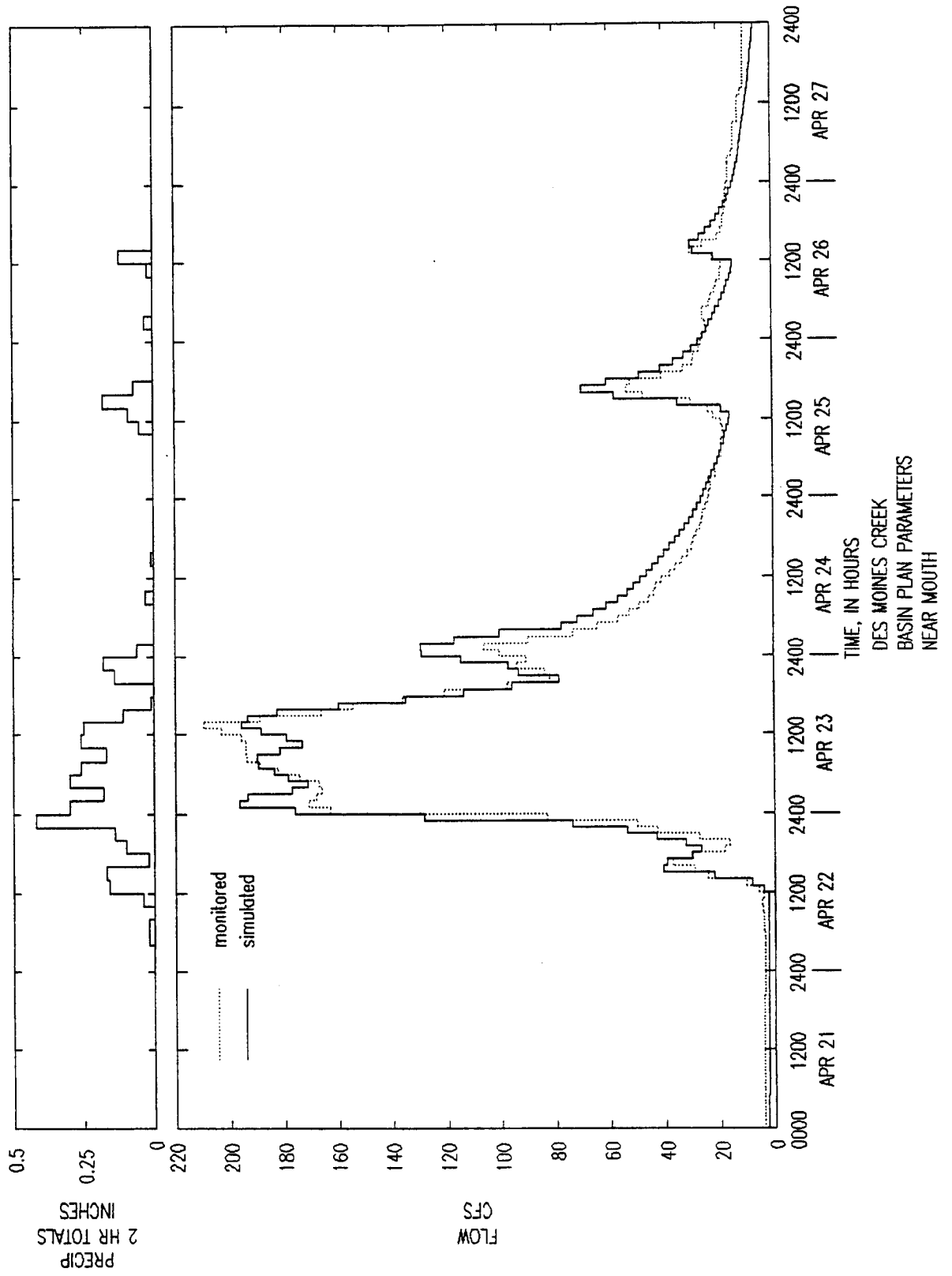


BASIN PLAN PARAMETERS VS MONITORED DATA NEAR MOUTH

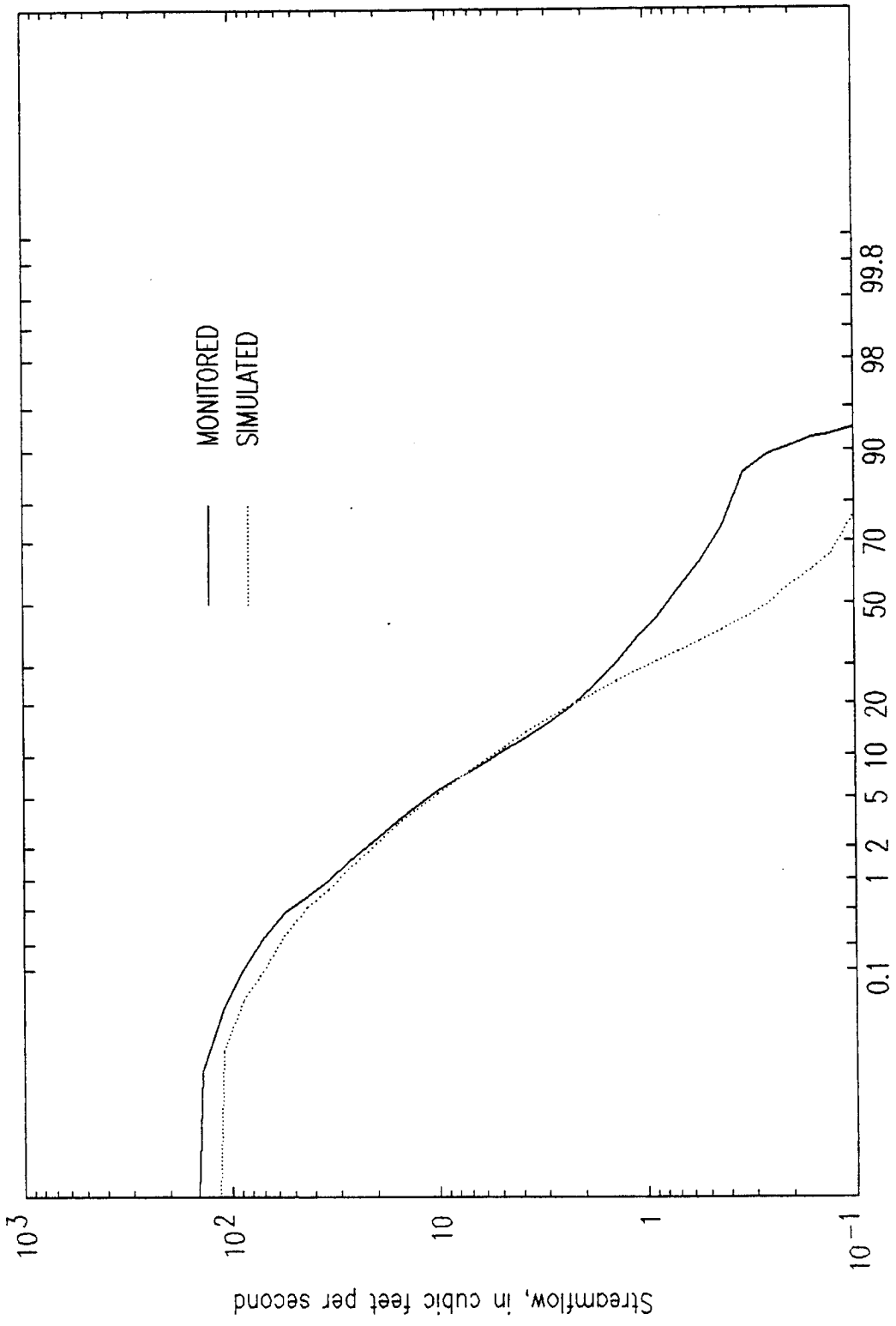


JANUARY, 1996  
 TIME, IN DAYS  
 DES MOINES CREEK  
 BASIN PLAN PARAMETERS  
 NEAR MOUTH

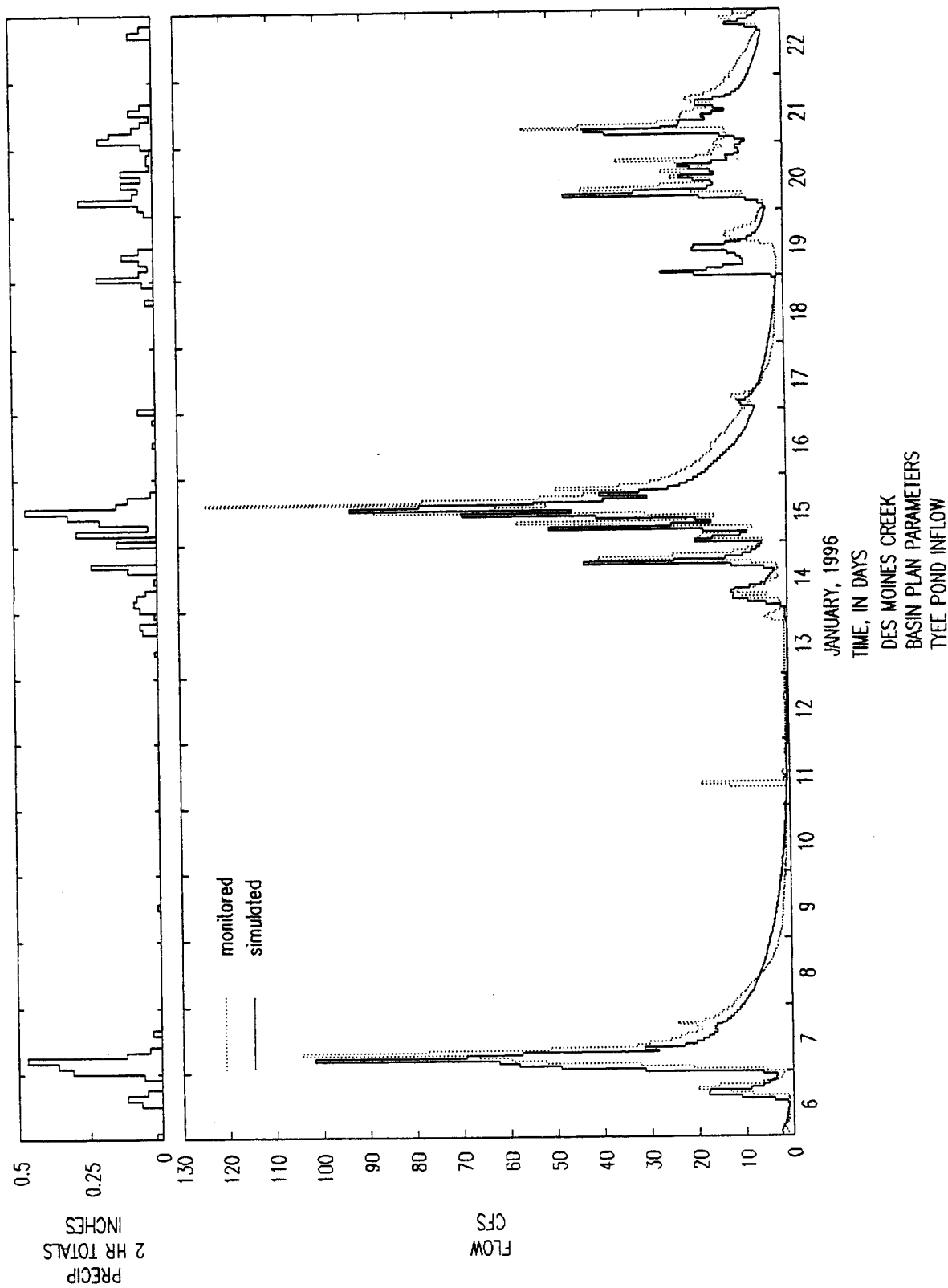


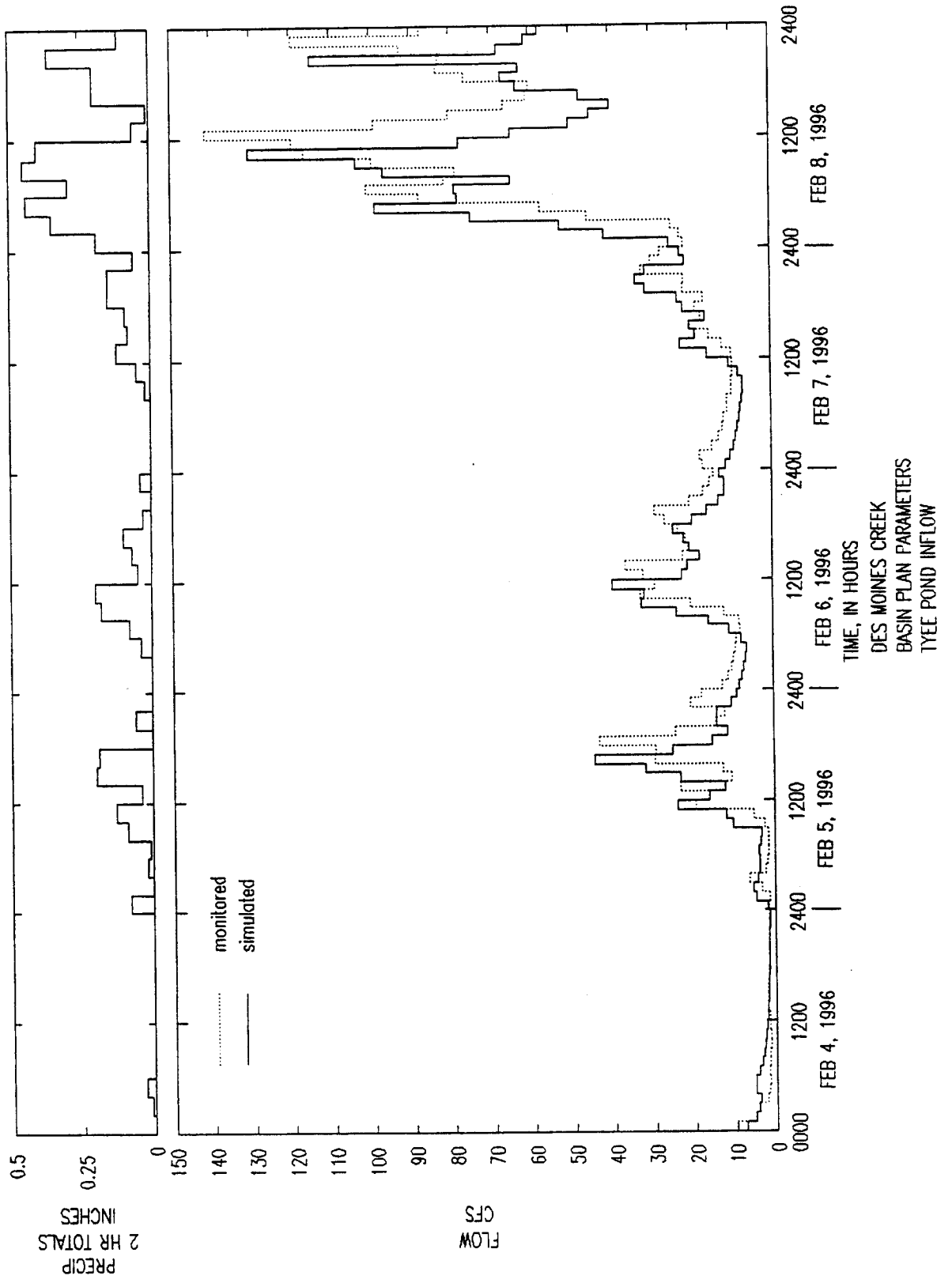


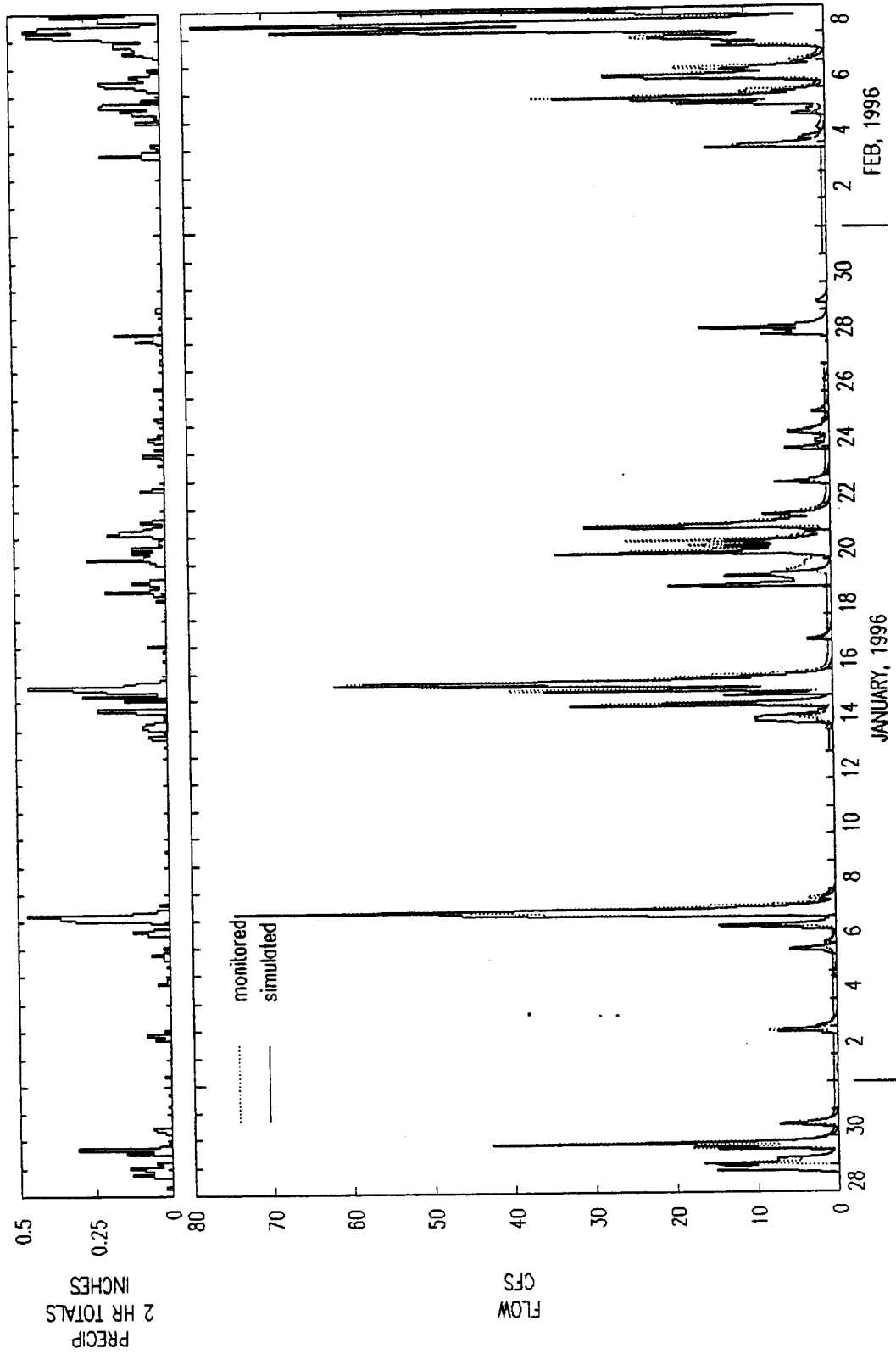




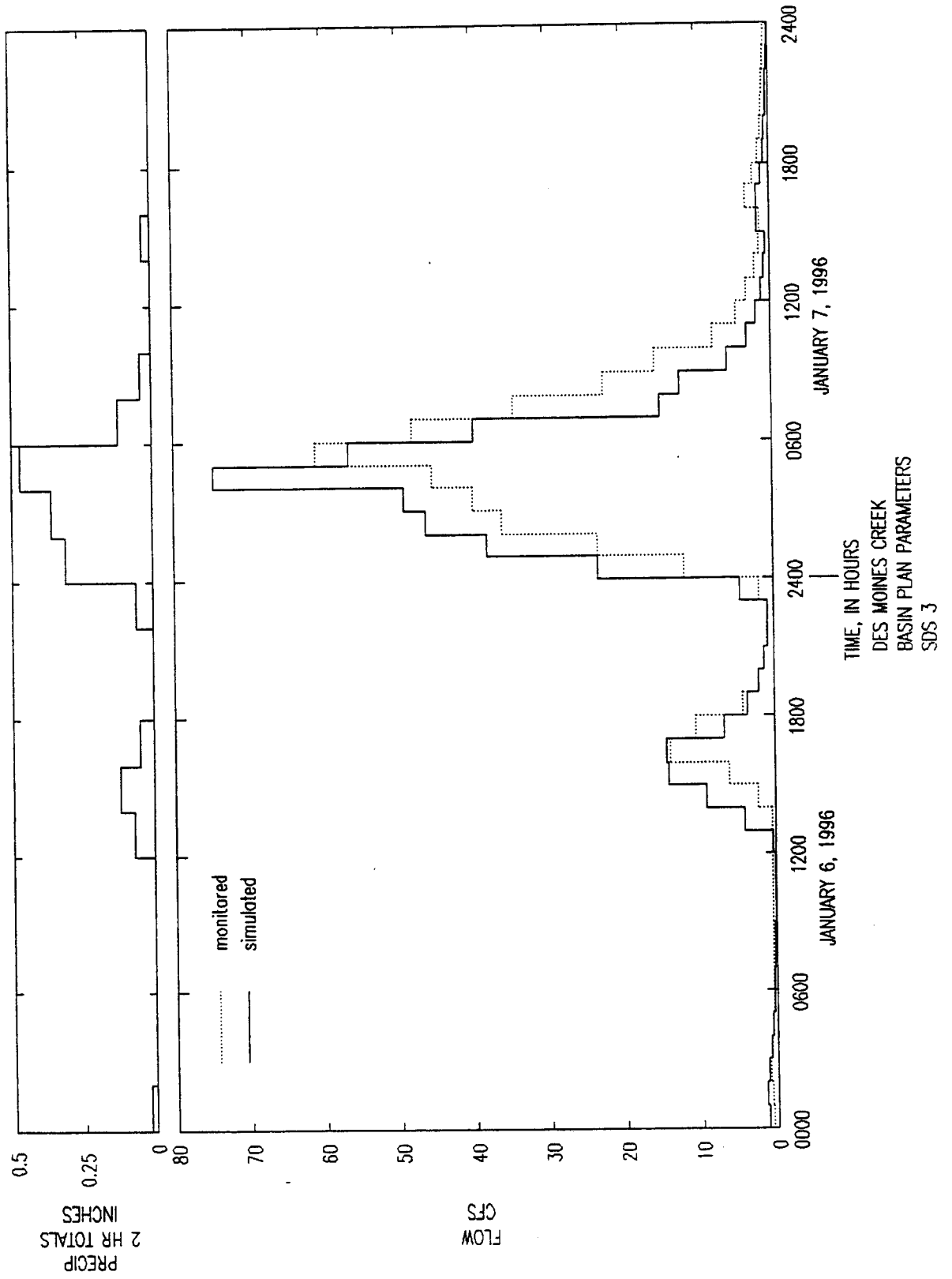
Percent chance flow exceeded  
 BASIN PLAN PARAMETERS VS MONITORED VALUES AT TYEE POND INFLOW

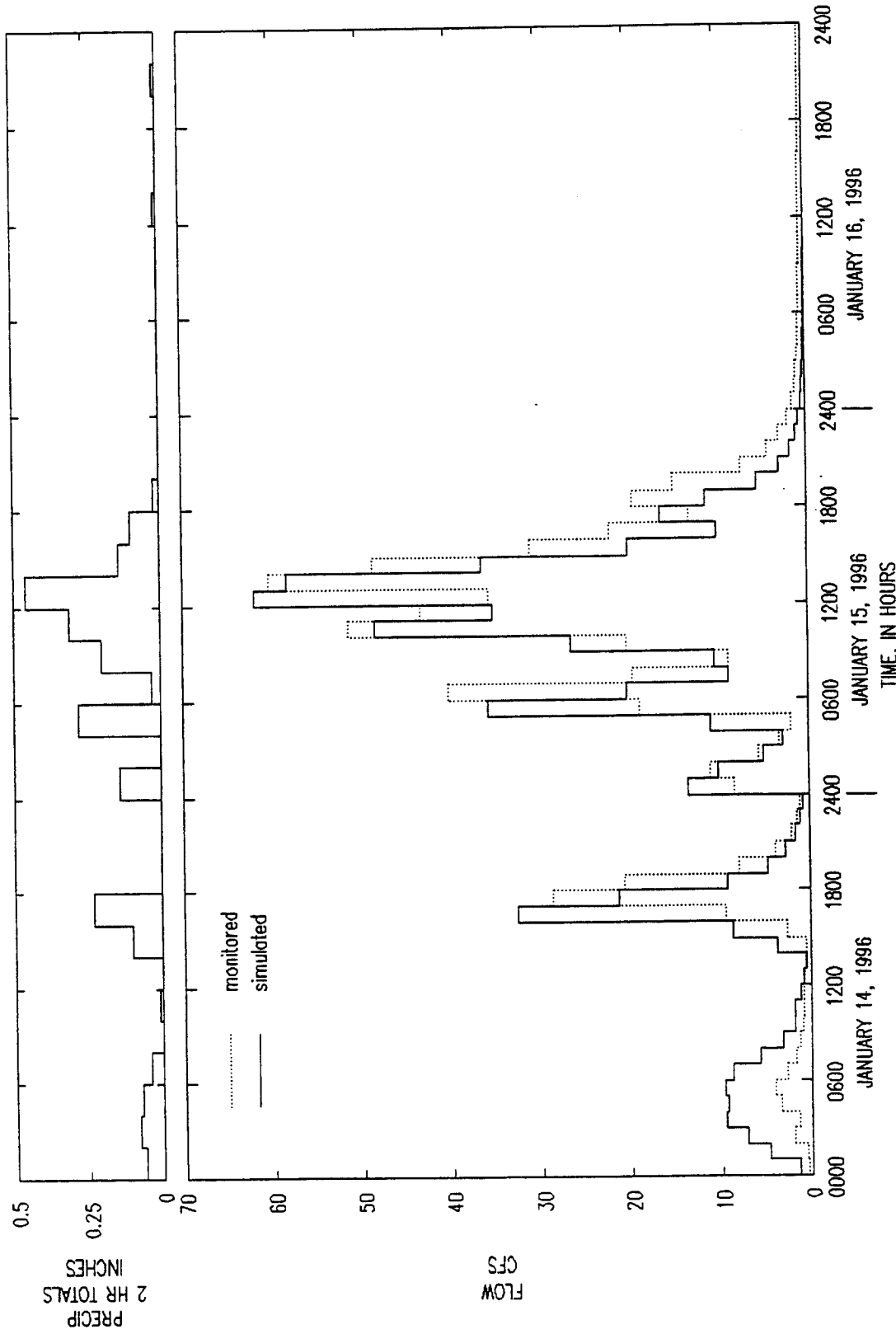




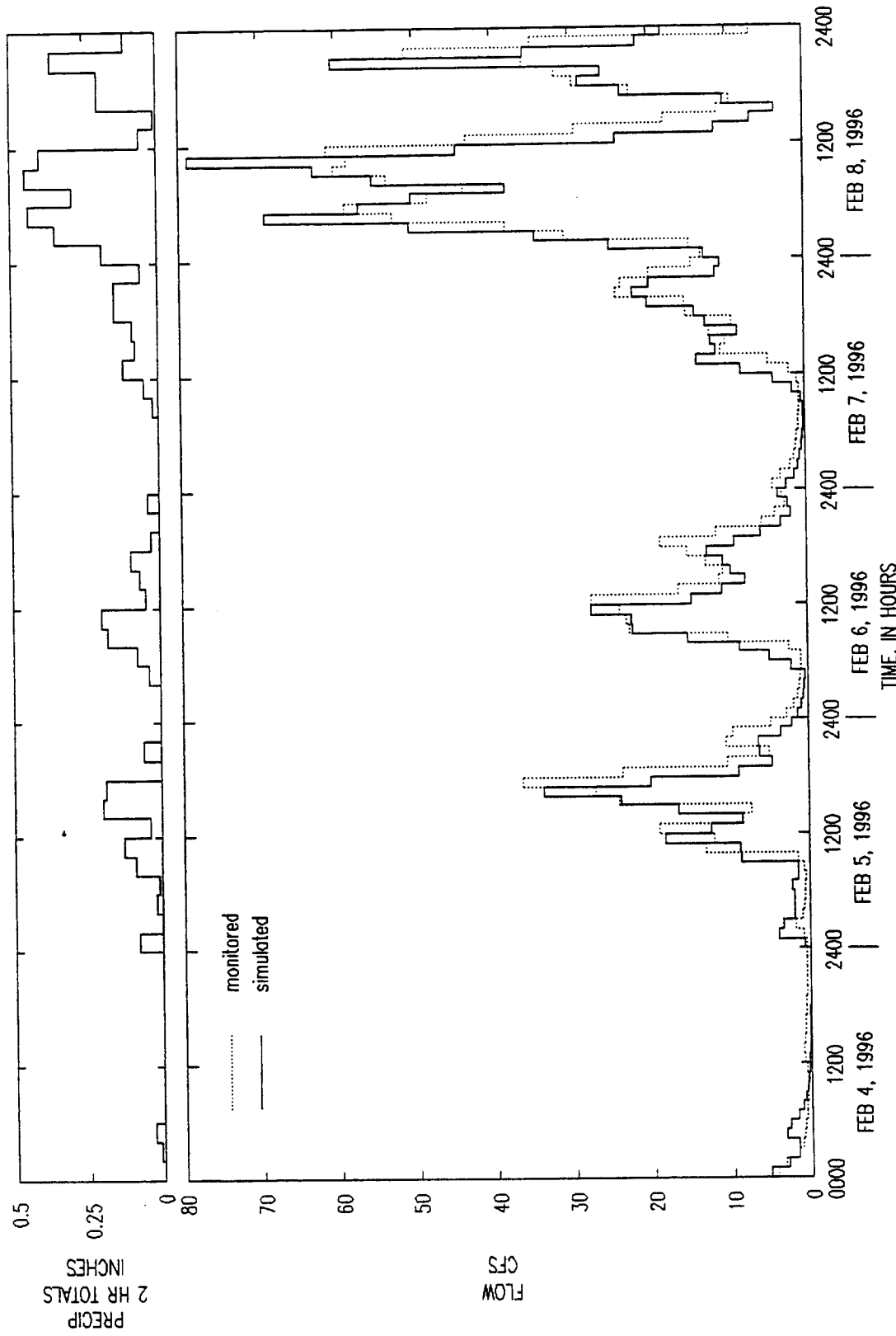


TIME, IN DAYS  
 DES MOINES CREEK  
 BASIN PLAN PARAMETERS  
 SDS 3





DES MOINES CREEK  
 BASIN PLAN PARAMETERS  
 SDS 3

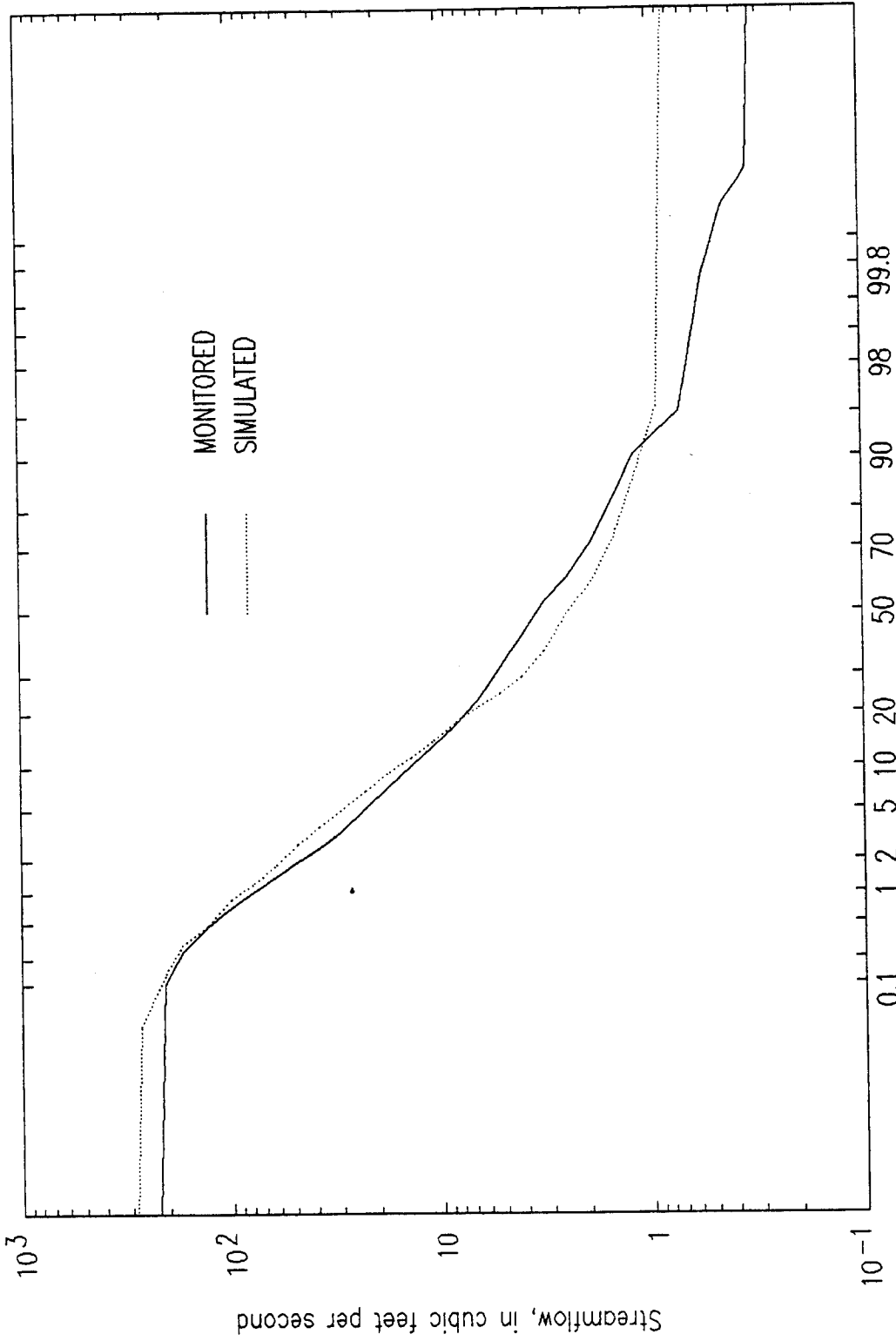


DES MOINES CREEK  
 BASIN PLAN PARAMETERS  
 SDS 3

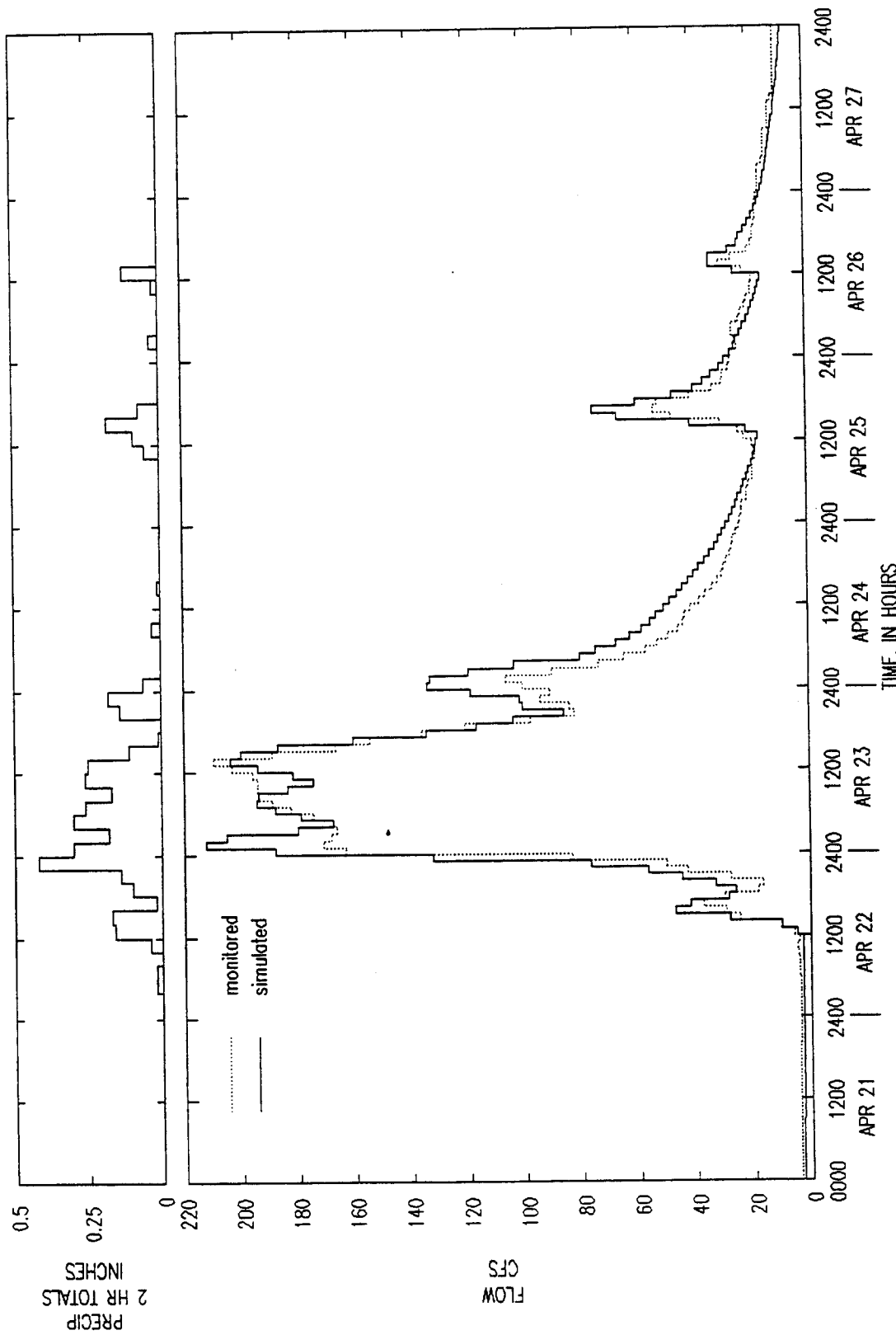
**NOVEMBER 1999 SMP  
DES MOINES CREEK MODEL HYDROGRAPHS**

**AR 010928**

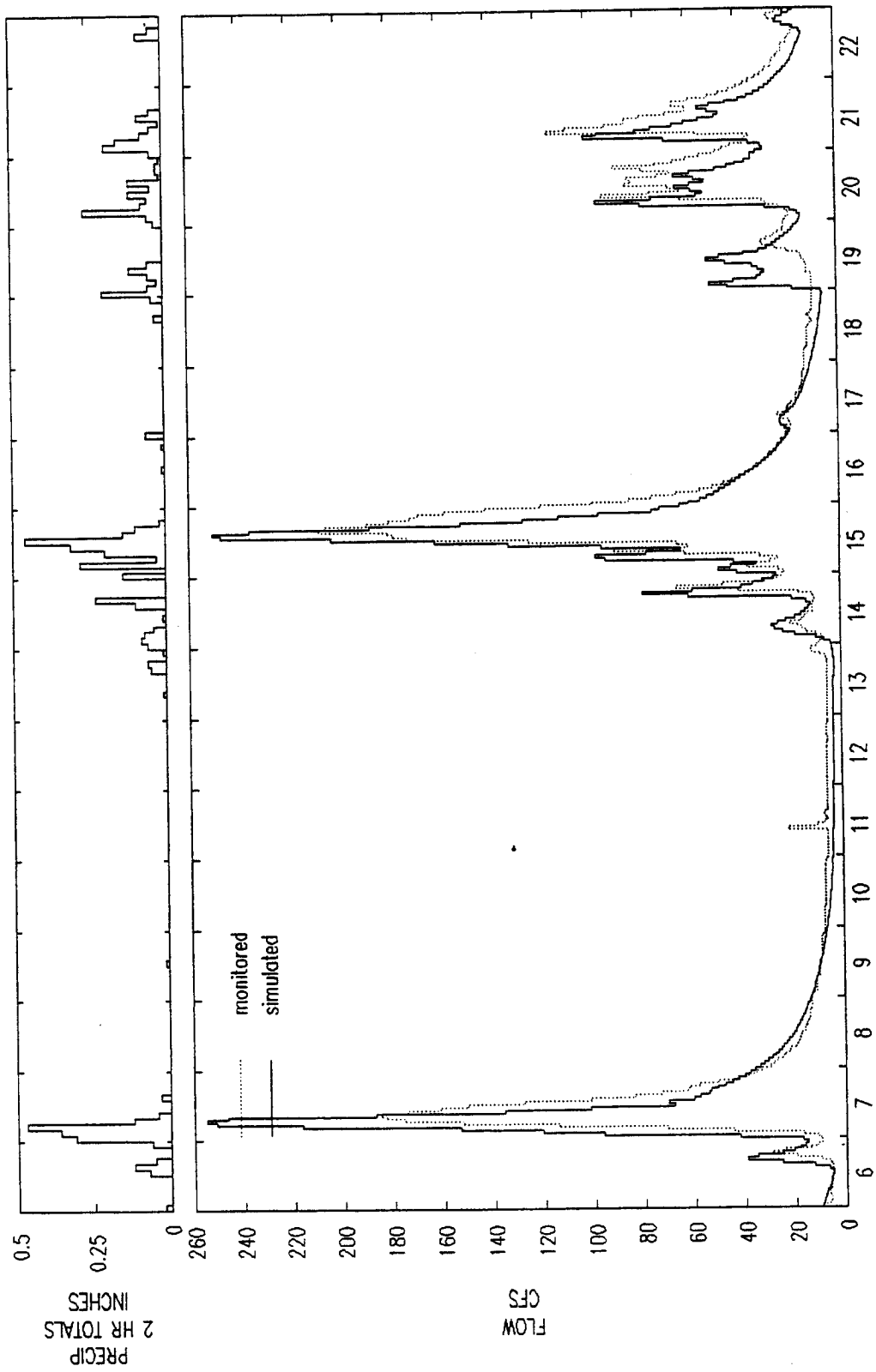




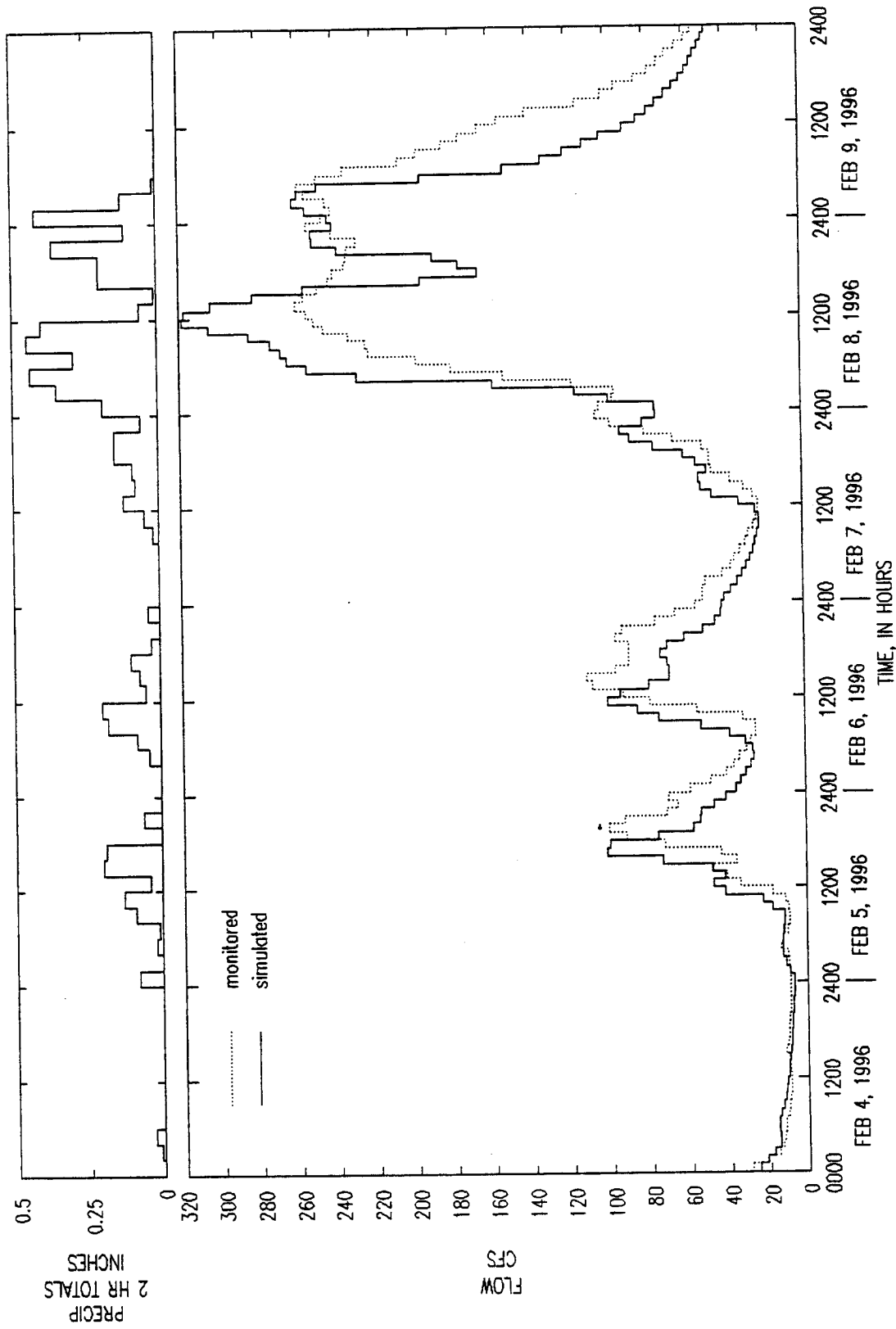
NOV. 1999 SMP VERSION OF 1994 CONDITIONS MODEL  
Near Mouth



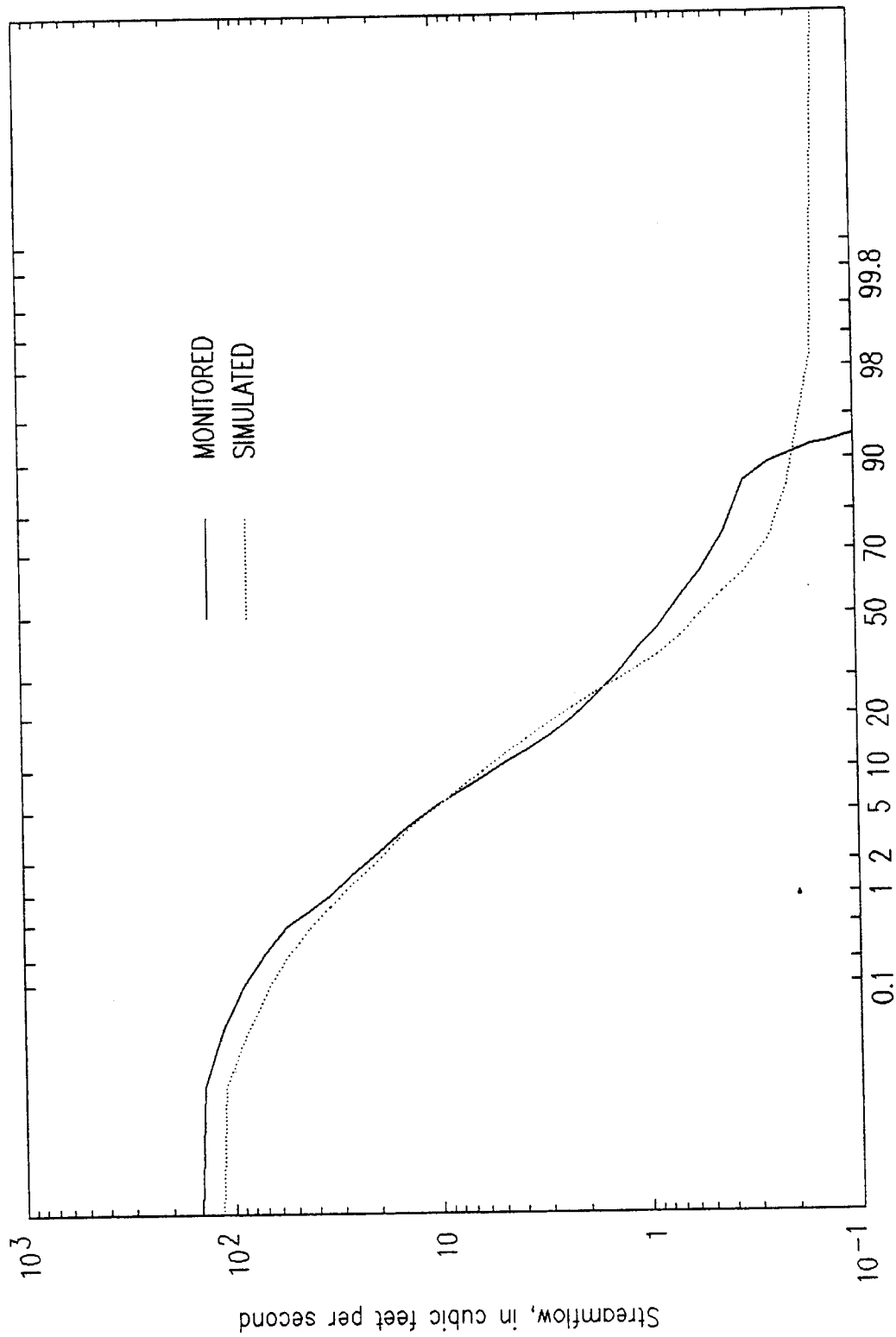
DES MOINES CREEK  
 NOV '99 SMP VER OF MODEL  
 NEAR MOUTH



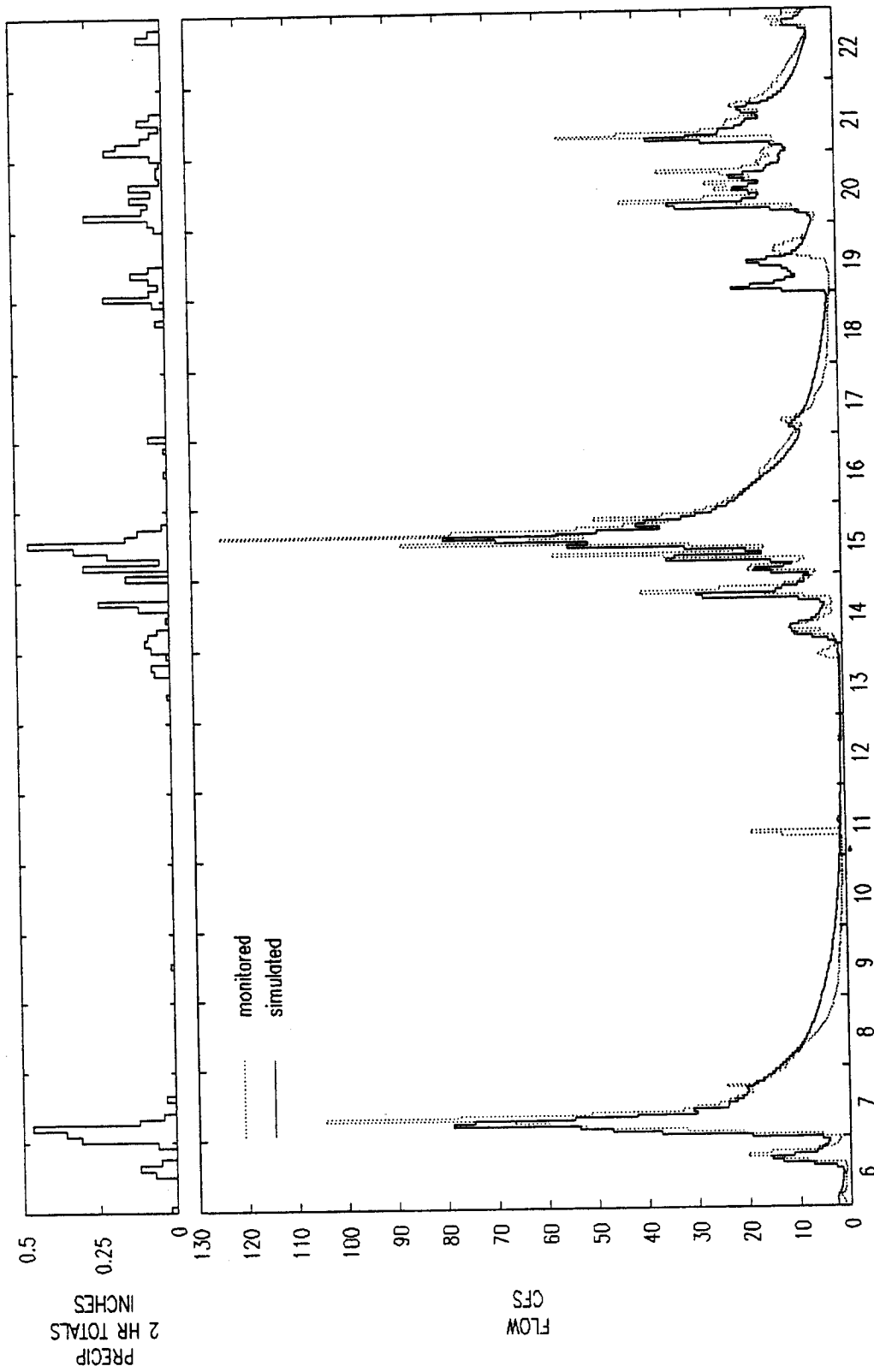
JANUARY, 1996  
 TIME, IN DAYS  
 DES MOINES CREEK  
 NOV '99 SMP VER OF MODEL  
 NEAR MOUTH



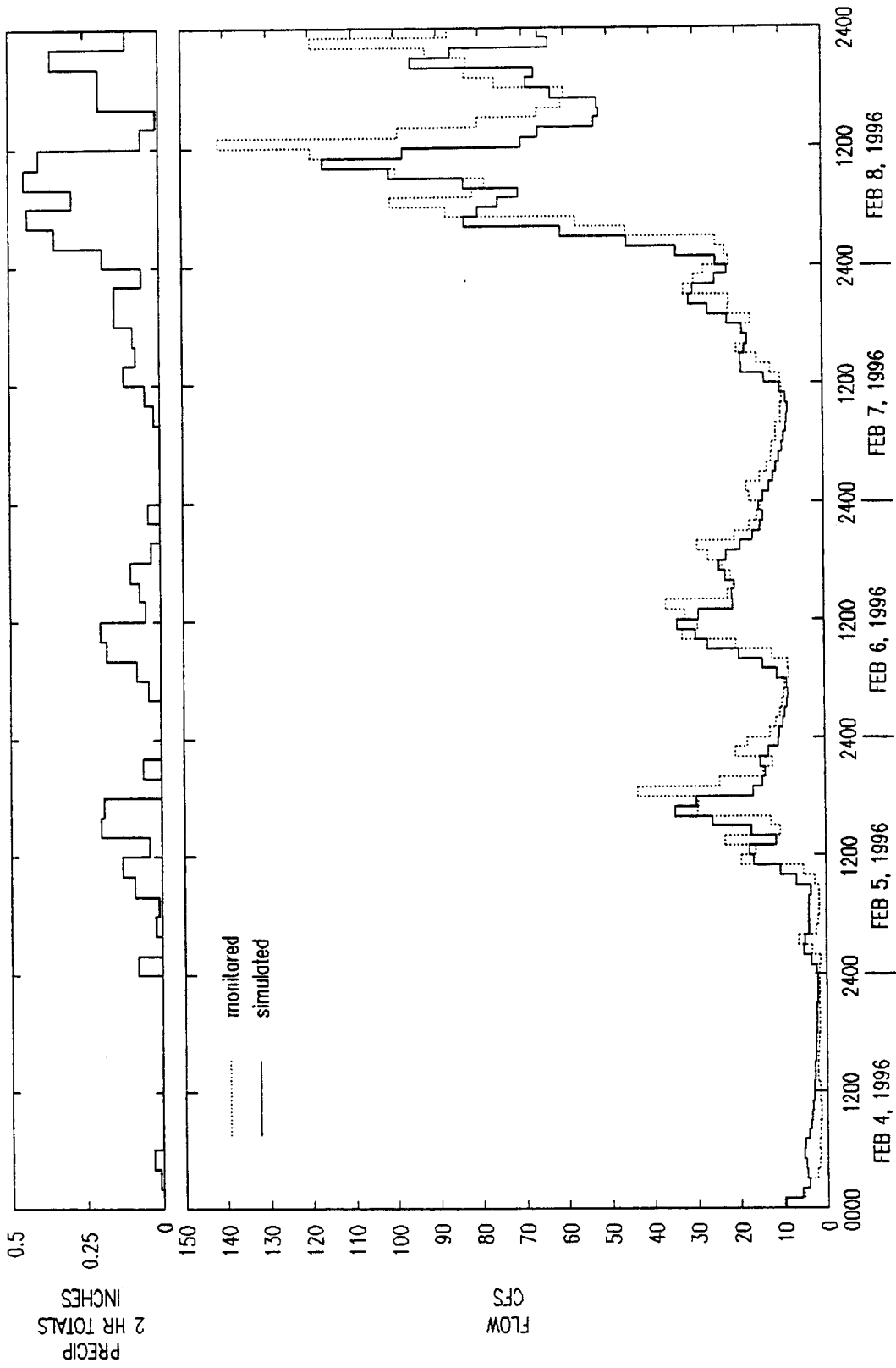
DES MOINES CREEK  
 NOV '99 SMP VER OF MODEL  
 NEAR MOUTH



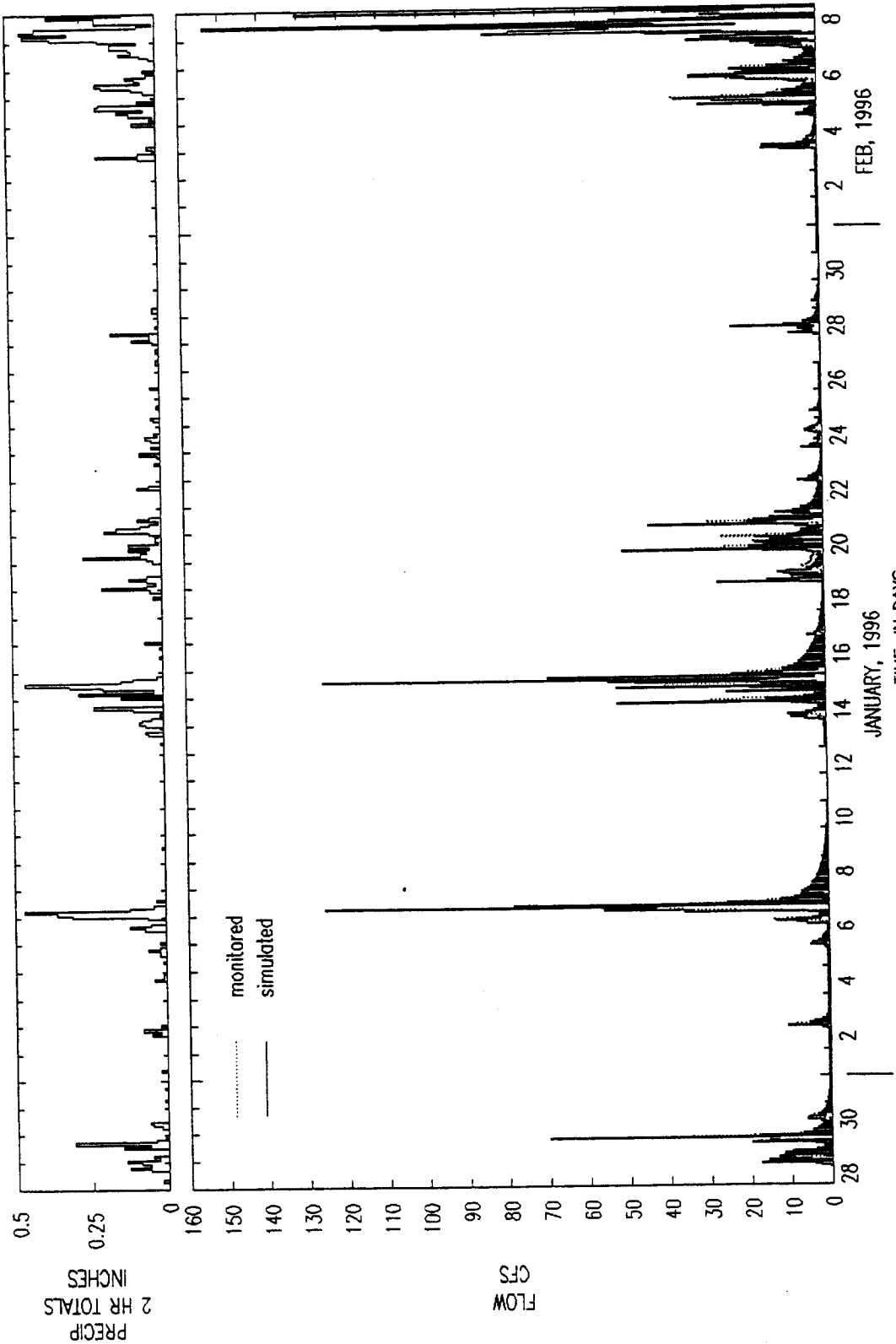
NOV. 1999 SMP VERSION OF 1994 CONDITIONS MODEL @ TYEE POND INFLOW



JANUARY, 1996  
 TIME, IN DAYS  
 DES MOINES CREEK  
 NOV '99 SMP VER OF MODEL  
 TYEE POND INFLOW

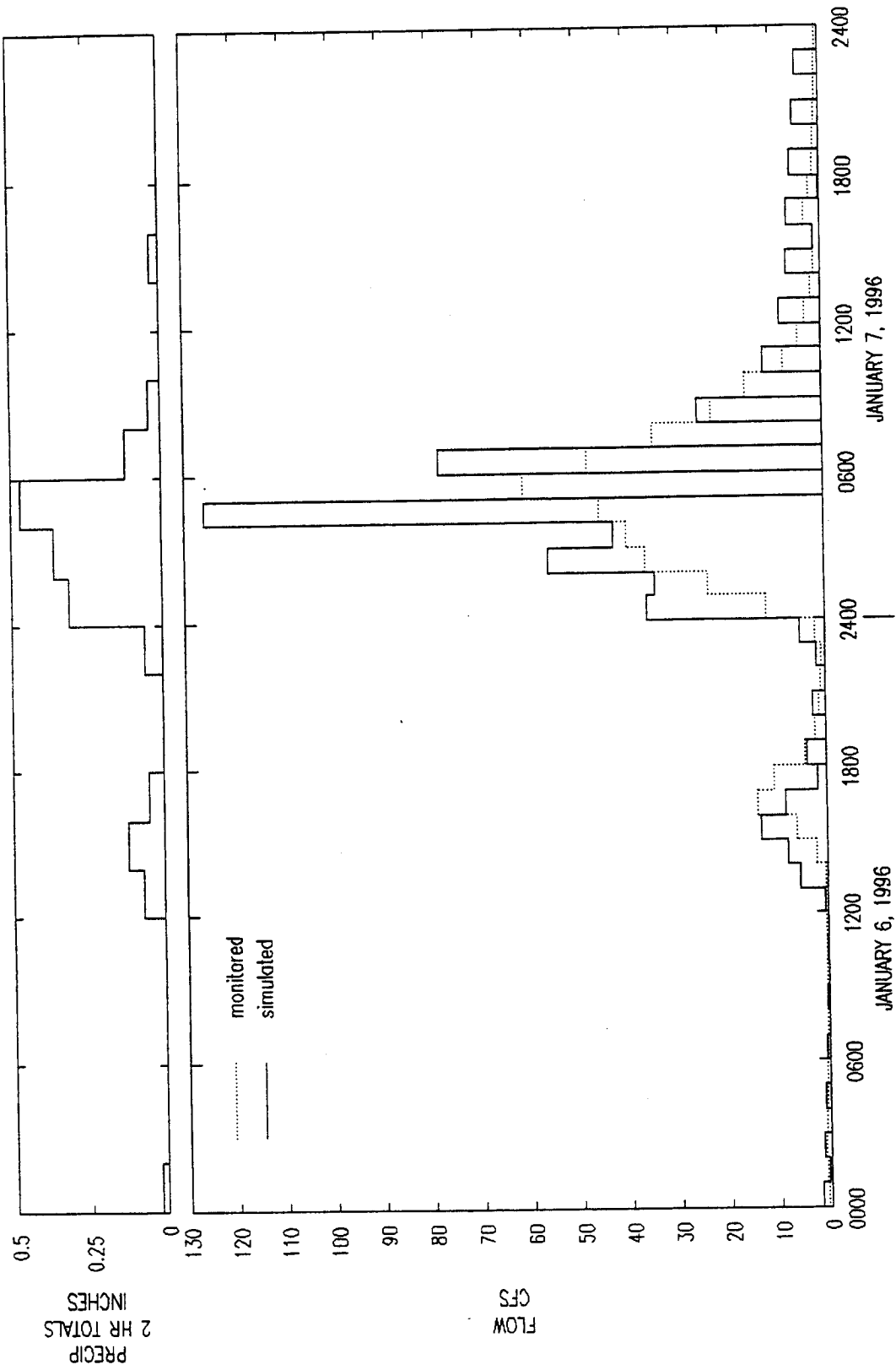


DES MOINES CREEK  
 NOV '99 SMP VER OF MODEL  
 TYEE POND INFLOW

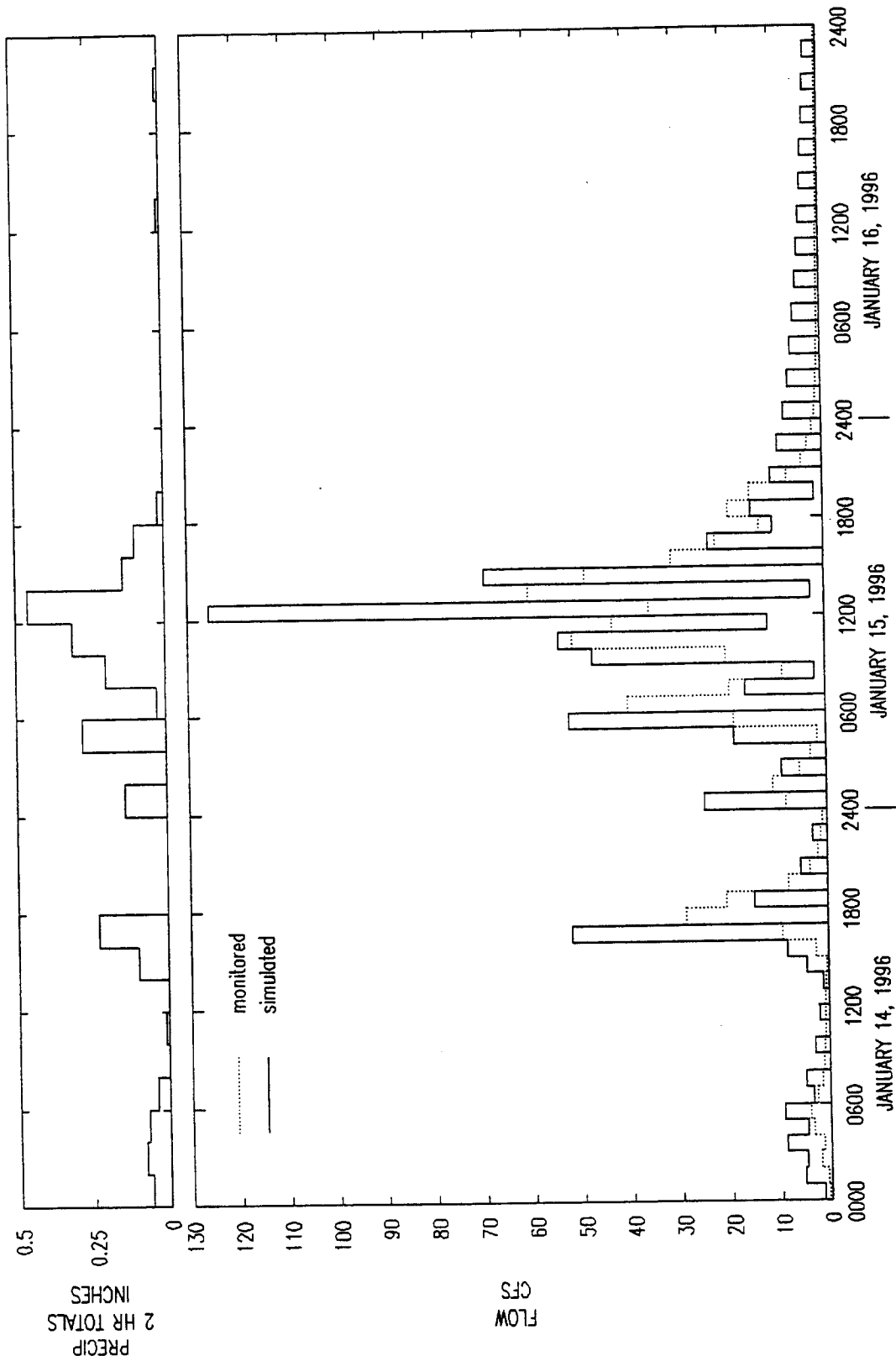


TIME, IN DAYS  
 DES MOINES CREEK  
 NOV '99 SMP VER OF MODEL  
 SDS 3

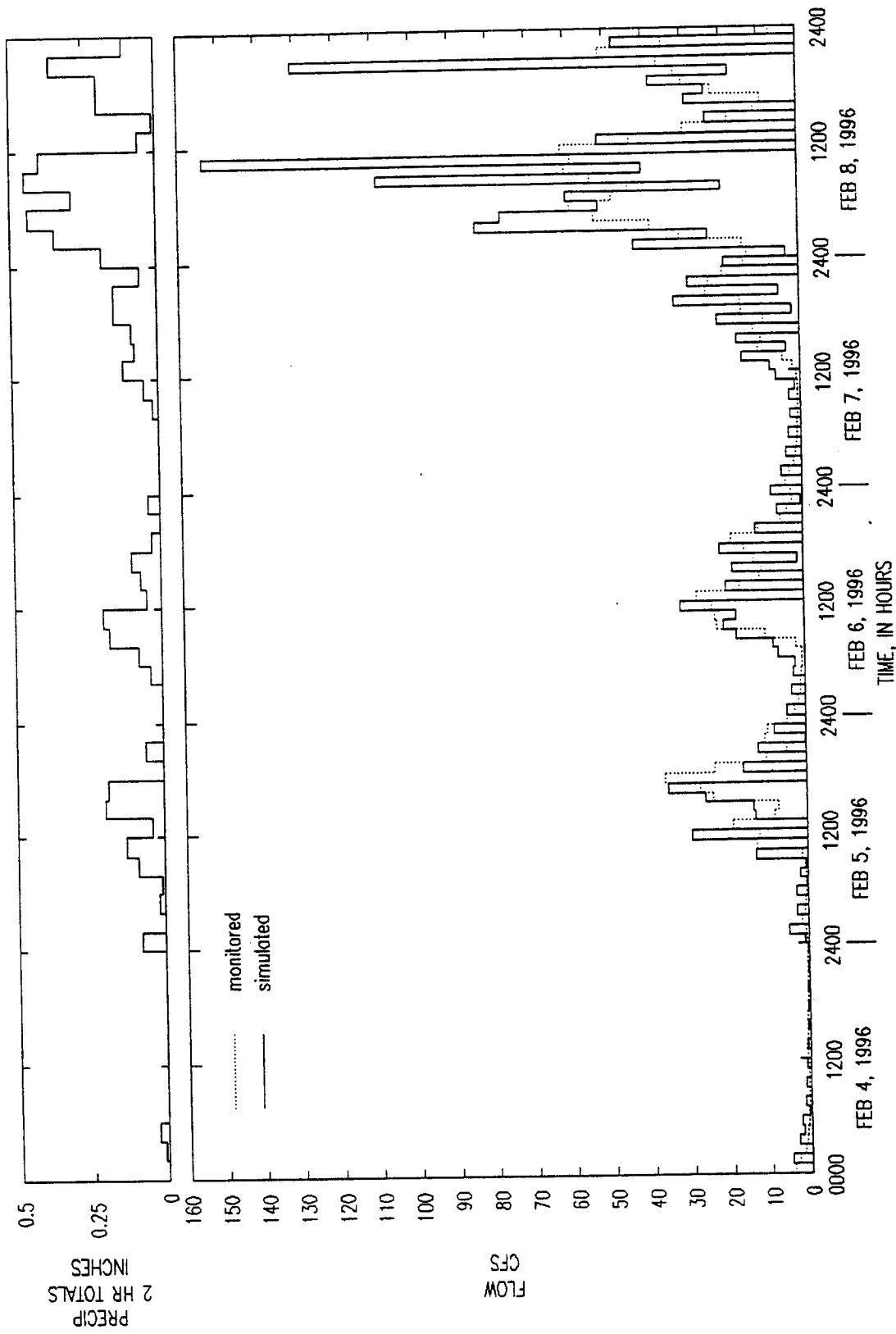




TIME, IN HOURS  
 DES MOINES CREEK  
 NOV '99 SMP VER OF MODEL  
 SDS 3



DES MOINES CREEK  
 NOV '99 SMP VER OF MODEL  
 SDS 3



DES MOINES CREEK  
 NOV '99 SMP VER OF MODEL  
 SDS 3



**Table B1-1. Des Moines Creek Subbasin Parameters - 1994 Base Condition**

Land Use	Sub-basin Areas (acres)																								
	Bow Lake		East Branch			DM-3 <sup>a</sup>			DM-4			DM-5			DM-6			DM-7			DM-8				
	DM-1 <sup>b</sup>	Effective	DM-2 <sup>b</sup>	Effective	DM-3 <sup>a</sup>	Effective	DM-4	Effective	DM-5	Effective	DM-6	Effective	DM-7	Effective	DM-8	Effective	DM-9	Effective	DM-10	Effective	DM-11	Effective			
% of total basin equal to EIA	171.29	40.0%	24.64	40.0%	123.72	60.2%	52.98	73.3%	44.32	31.3%	54.91	7.7%	170.22	16.9%	62.81	44.6%									
PERVIOUS IMPERVIOUS	12.19	-	-	-	0.33	0.03	0.44	0.02	0.05	1.31	4.05	0.03	0.11	6.61	1.94	0.03	0.00	0.11	0.11	0.11	0.11	0.11	0.11	0.11	
SA	12.19	-	-	-	0.33	0.03	0.44	0.02	0.05	1.31	4.05	0.03	0.11	6.61	1.94	0.03	0.00	0.11	0.11	0.11	0.11	0.11	0.11	0.11	
Lake	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
Other	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
<b>Basin total:</b>	<b>428.21</b>	<b>40.0%</b>	<b>24.64</b>	<b>40.0%</b>	<b>123.72</b>	<b>60.2%</b>	<b>52.98</b>	<b>73.3%</b>	<b>44.32</b>	<b>31.3%</b>	<b>54.91</b>	<b>7.7%</b>	<b>170.22</b>	<b>16.9%</b>	<b>62.81</b>	<b>44.6%</b>									
<b>PERLND areas:</b>																									
(14) TFF	10.32	-	-	-	-	-	2.67	4.22	1.57	14.15	9.72	9.72	4.21	2.42	-	-	-	-	-	-	-	-	-	-	
(16) TFM	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
(18) TFS	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
(24) TGF	-	-	-	-	29.47	3.44	3.44	3.41	2.23	5.67	2.33	2.33	27.25	8.62	-	-	-	-	-	-	-	-	-	-	
(26) TGM	132.94	-	14.78	-	11.80	3.41	3.41	3.41	4.46	8.13	8.13	8.13	-	-	-	-	-	-	-	-	-	-	-	-	
(28) TGS	-	-	-	-	7.66	-	-	-	-	0.04	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
LAC	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
(34) OF	7.19	-	-	-	-	-	-	-	1.36	-	-	-	30.13	8.51	-	-	-	-	-	-	-	-	-	-	
(44) OG	92.36	-	-	-	-	-	-	-	4.61	11.00	11.00	11.00	49.66	13.34	-	-	-	-	-	-	-	-	-	-	
(54) SA	14.11	-	-	-	0.33	0.44	0.44	0.44	2.05	4.05	4.05	4.05	6.61	1.94	-	-	-	-	-	-	-	-	-	-	
subtotal:	256.92	-	14.78	-	49.26	14.17	14.17	14.17	30.44	50.66	50.66	50.66	141.46	34.83	-	-	-	-	-	-	-	-	-	-	
<b>IMPLND areas:</b>																									
EIA	171.29	9.85	74.46	38.81	74.46	38.81	38.81	38.81	13.88	13.88	13.88	13.88	28.76	28.76	28.76	28.76	28.76	28.76	28.76	28.76	28.76	28.76	28.76	28.76	
<b>Basin total:</b>	<b>428.21</b>	<b>40.0%</b>	<b>24.64</b>	<b>40.0%</b>	<b>123.72</b>	<b>60.2%</b>	<b>52.98</b>	<b>73.3%</b>	<b>44.32</b>	<b>31.3%</b>	<b>54.91</b>	<b>7.7%</b>	<b>170.22</b>	<b>16.9%</b>	<b>62.81</b>	<b>44.6%</b>									
<b>Percent Impervious</b>																									
<b>NETWORK FACTORS</b>																									
PERLND 16 (TF)	0.860	-	-	-	-	-	0.574	0.571	1.310	1.814	1.814	1.814	2.189	0.202	-	-	-	-	-	-	-	-	-	-	
PERLND 26 (TG)	11.078	1.232	4.077	-	4.077	-	0.571	0.571	0.558	1.153	1.153	1.153	2.944	0.718	-	-	-	-	-	-	-	-	-	-	
PERLND 34 (OF)	0.599	-	-	-	-	-	-	-	0.114	-	-	-	1.966	0.709	-	-	-	-	-	-	-	-	-	-	
PERLND 44 (OG)	7.697	-	-	-	-	-	-	-	0.384	0.917	0.917	0.917	4.138	1.111	-	-	-	-	-	-	-	-	-	-	
PERLND 54 (SA)	1.176	-	-	-	0.028	0.037	0.037	0.037	0.171	0.337	0.337	0.337	0.551	0.162	-	-	-	-	-	-	-	-	-	-	
IMPLND 14 (EIA)	14.274	0.821	6.205	3.234	6.205	3.234	3.234	3.234	1.156	2.332	2.332	2.332	2.397	2.332	2.332	2.332	2.332	2.332	2.332	2.332	2.332	2.332	2.332	2.332	
SUM	35.684	2.053	10.310	4.415	10.310	4.415	4.415	4.415	3.694	4.576	4.576	4.576	14.185	5.234	-	-	-	-	-	-	-	-	-	-	

<sup>a</sup> - land use is extrapolated

<sup>b</sup> - areas prorated to new subbasin total areas from previous version of model. Shaded subbasins included in the STIA subbasin characteristics evaluation area.

Table B1-1. Des Moines Creek Subbasin Parameters - 1994 Base Condition

Land Use	% of total basin equal to Use Type EIA	Sub-basin Areas (acres)																				
		West Branch (NW Ponds)				Upper Main				Lower Main Stem												
		DM-9	DM-10	DM-11	DM-12	DM-13 <sup>a</sup>	DM-14 <sup>b</sup>	DM-16 <sup>b</sup>	DM-17 <sup>b</sup>	Effective	Effective	Effective	Effective									
AP	TGF 0%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	TGM 0%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	LAC 0%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	OGF 0%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	OGM 0%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
C	TGF 85%	-	-	1.36	-	7.71	-	-	-	-	-	-	-	3.61	20.48	-	-	-	-	-	-	-
	TGM 85%	-	-	-	-	-	-	-	-	-	-	-	-	0.37	2.11	-	-	-	-	-	-	-
	TGS 85%	-	-	-	-	-	-	-	-	-	-	-	-	0.16	0.89	-	-	-	-	-	-	-
	LAC 85%	-	-	1.12	-	6.35	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	OGF 85%	-	-	2.89	-	16.38	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	OGM 85%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
TR	TGF 30%	-	-	2.27	-	0.97	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	OGF 30%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	OGM 30%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
MF	TGF 47%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	TGM 47%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	OGF 47%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	OGM 47%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	OGS 47%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
HD	TGF 30%	-	-	-	-	-	-	-	-	-	-	-	-	0.06	0.02	-	-	-	-	-	-	-
	TGM 30%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	LAC 30%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
LD	TGF 13%	10.26	1.53	-	6.33	-	0.95	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	TGM 13%	2.89	0.43	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	TGS 13%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	LAC 13%	1.25	0.19	-	2.50	-	0.37	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	OGF 13%	8.69	1.30	-	0.67	-	0.10	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	OGM 13%	26.74	4.00	-	5.39	-	0.81	-	-	-	-	-	-	1.47	0.22	-	-	-	-	-	-	-
	OGS 13%	1.04	0.16	-	-	-	-	-	-	-	-	-	-	0.04	0.01	-	-	-	-	-	-	-
G	TGF 0%	-	-	1.41	-	-	-	-	-	-	-	-	-	9.54	-	-	-	-	-	-	-	-
	TGM 0%	-	-	-	-	-	-	-	-	-	-	-	-	8.89	-	-	-	-	-	-	-	-
	LAC 0%	-	-	-	0.02	-	-	-	0.01	-	-	-	-	-	-	-	-	-	-	-	-	-
	OGF 0%	-	-	-	0.06	-	-	-	23.44	-	-	-	-	-	-	-	-	-	-	-	-	-
	OGM 0%	-	-	-	0.00	-	-	-	1.94	-	-	-	-	-	-	-	-	-	-	-	-	-

**Table B1-1. Des Moines Creek Subbasin Parameters - 1994 Base Condition**

Land Use	Sub-basin Areas (acres)															
	West Branch (NW Ponds)				Upper Main				Lower Main Stem							
	DM-9		DM-10		DM-11		DM-12		DM-13 <sup>a</sup>		DM-14 <sup>b</sup>		DM-16 <sup>b</sup>		DM-17 <sup>b</sup>	
Land Use Type	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious
F	0%	0.18	3.13	-	-	5.95	-	-	-	-	-	-	-	-	-	-
TFM	0%	-	-	-	-	20.98	-	-	-	-	-	-	-	-	-	-
TFS	0%	-	-	-	-	5.28	-	-	-	-	-	-	-	-	-	-
LAC	0%	11.14	0.73	-	6.13	0.00	-	-	-	-	-	-	-	-	-	-
OFF	0%	8.78	12.33	-	0.02	0.02	-	-	-	-	-	-	-	-	-	-
OFM	0%	14.43	-	-	4.49	14.43	-	-	-	-	-	-	-	-	-	-
SA	0%	8.52	12.43	-	6.51	0.82	-	-	-	-	-	-	-	-	-	-
Lake	-	0.96	5.33	-	0.63	0.85	-	-	-	-	-	-	-	-	-	-
Other	0.02	0.06	0.02	-	-	0.06	-	-	-	-	-	-	-	-	-	0.44
<b>Basin total:</b>		<b>51.20</b>	<b>7.60</b>	<b>2.23</b>	<b>40.37</b>	<b>31.42</b>	<b>42.53</b>	<b>92.25</b>	<b>23.73</b>	<b>51.02</b>	<b>4.08</b>	<b>231.75</b>	<b>50.99</b>	<b>159.31</b>	<b>20.0%</b>	<b>31.86</b>
<b>PERLND areas:</b>																
(14) TFF	-	0.18	3.13	-	-	5.95	-	-	-	-	-	-	-	-	-	-
(16) TFM	-	-	-	-	-	20.98	-	-	-	-	-	-	-	-	-	-
(18) TFS	-	-	-	-	-	5.28	-	-	-	-	-	-	-	-	-	-
(24) TGF	10.26	6.33	5.04	-	-	13.21	-	-	-	-	-	-	-	-	-	-
(26) TGM	2.89	-	-	-	-	9.26	-	-	-	-	-	-	-	-	-	-
(28) TGS	-	-	-	-	-	0.16	-	-	-	-	-	-	-	-	-	-
LAC	1.27	13.66	1.85	-	6.14	0.00	-	-	-	-	-	-	-	-	-	-
(34) OF	0.19	23.21	12.33	-	4.51	14.45	-	-	-	-	-	-	-	-	-	-
(44) OG	36.47	6.12	5.59	-	25.37	22.14	-	-	-	-	-	-	-	-	-	-
(54) SA	0.12	8.52	12.43	-	6.51	0.82	-	-	-	-	-	-	-	-	-	-
subtotal:	51.20	58.02	40.37	-	42.53	92.25	-	-	-	-	-	-	-	-	-	-
<b>IMPLND areas:</b>																
EIA	7.60	2.23	31.42	-	-	23.73	-	-	-	-	-	-	-	-	-	-
<b>Basin total:</b>	<b>58.80</b>	<b>60.24</b>	<b>71.79</b>	<b>43.8%</b>	<b>42.53</b>	<b>115.98</b>	<b>20.5%</b>	<b>51.02</b>	<b>8.0%</b>	<b>231.75</b>	<b>22.0%</b>	<b>159.31</b>	<b>20.0%</b>	<b>31.86</b>		
<b>Percent Impervious</b>	<b>12.9%</b>	<b>3.7%</b>	<b>43.8%</b>		<b>0.0%</b>	<b>20.5%</b>		<b>8.0%</b>		<b>22.0%</b>		<b>20.0%</b>				
<b>NETWORK FACTORS</b>																
PERLND 16 (TF)	0.002	0.943	0.321	0.511	0.511	2.684	0.481	0.481	0.647	0.481	2.078	0.647	2.078			
PERLND 26 (TG)	1.200	0.738	0.514	0.001	0.001	1.886	0.295	0.295	5.573	0.295	2.261	5.573	2.261			
PERLND 34 (OF)	0.016	1.934	1.028	0.376	0.376	1.204	1.940	1.940	0.639	1.940	3.003	0.639	3.003			
PERLND 44 (OG)	3.039	0.510	0.466	2.114	2.114	1.845	1.195	1.195	8.023	1.195	3.280	8.023	3.280			
PERLND 54 (SA)	0.010	0.710	1.036	0.543	0.543	0.068	-	-	0.183	-	-	0.183	-			
IMPLND 14 (EIA)	0.633	0.185	2.618	-	-	1.978	0.340	0.340	4.249	0.340	2.655	4.249	2.655			
<b>SUM</b>	<b>4.900</b>	<b>5.020</b>	<b>5.983</b>	<b>3.544</b>	<b>3.544</b>	<b>9.665</b>	<b>4.252</b>	<b>4.252</b>	<b>19.313</b>	<b>4.252</b>	<b>13.276</b>	<b>19.313</b>	<b>13.276</b>			

<sup>a</sup> - land use is extrapolated  
<sup>b</sup> - areas prorated to new subbasin total areas from previous version of model.  
 shaded subbasins included in the STIA subbasin characteristics evaluation area.





**Table B1-1. Des Moines Creek Subbasin Parameters - 1994 Base Condition**

Land Use	Sub-basin Areas (acres)														
	Lower Main Stem					SDS Basins									
	DM-18 <sup>b</sup> Effective Pervious Impervious	DM-19 <sup>b</sup> Effective Pervious Impervious	DM-20 <sup>b</sup> Effective Pervious Impervious	DM-21 <sup>b</sup> Effective Pervious Impervious	DM-22 <sup>b</sup> Effective Pervious Impervious	SDE-4 Effective Pervious Impervious	SDE-5 Effective Pervious Impervious	SDE-6 Effective Pervious Impervious	SDE-7 Effective Pervious Impervious	SDE-8 Effective Pervious Impervious					
% of total basin equal to															
Land Soil Use Type															
F TFF	0%														
F TFM	0%														
F TFS	0%														
F LAC	0%														
F OFF	0%														
F OFM	0%														
- SA	0%														
- Lake															
- Other															
<b>Basin total:</b>															
<b>PERLND areas:</b>															
(14) TFF	9.47	2.19	48.08	25.72	4.58										
(16) TFM	-	-	-	-	-										
(18) TFS	-	-	-	-	-										
(24) TGF	3.32	72.22	79.49	75.68	55.85										
(26) TGM	-	-	-	-	-										
(28) TGS	-	-	-	-	-										
(34) OF	37.81	2.00	33.40	17.14	2.62										
(44) OG	13.27	66.63	55.22	50.46	31.44										
(54) SA	3.60	-	1.39	-	0.19										
subtotal:	67.48	143.04	217.58	168.99	94.67										
<b>IMPLND areas:</b>															
EIA	3.55	31.40	44.56	206.09	23.67										
<b>Basin total:</b>	71.03	174.44	262.14	206.09	118.34										
<b>Percent Impervious</b>	5.0%	18.0%	17.0%	18.0%	20.0%										
<b>NETWORK FACTORS</b>															
PERLND 16 (TF)	0.789	0.182	4.007	2.143	0.381										
PERLND 26 (TG)	0.277	6.019	6.624	6.306	4.654										
PERLND 34 (OF)	3.151	0.167	2.784	1.429	0.218										
PERLND 44 (OG)	1.106	5.552	4.602	4.205	2.620										
PERLND 54 (SA)	0.300	-	0.116	-	0.016										
IMPLND 14 (EIA)	0.296	2.617	3.714	3.091	1.972										
<b>SUM</b>	5.919	14.537	21.845	17.174	9.862										

<sup>a</sup> - land use is extrapolated

<sup>b</sup> - areas prorated to new subbasin total areas from previous version of model.  
shaded subbasins included in the STIA subbasin characteristics evaluation area.

**Table B1-1. Des Moines Creek Subbasin Parameters - 1994 Base Condition**

Land Use	Sub-basin Areas (acres)					
	SDS Basins			IWS Basins		
	SDS-1 Effective Previous Impervious	SDS-2/3/4 Effective Previous Impervious	SDS-5 Effective Previous Impervious	PRIMARY Effective Previous Impervious	SDS-6 Effective Previous Impervious	SDS-7 Effective Previous Impervious
AP	0%	0%	0%	0%	0%	0%
TGF	0%	0%	0%	0%	0%	0%
TGM	0%	0%	0%	0%	0%	0%
LAC	0%	0%	0%	0%	0%	0%
OGF	0%	0%	0%	0%	0%	0%
OGM	0%	0%	0%	0%	0%	0%
C	85%	85%	85%	85%	85%	85%
TGF	85%	85%	85%	85%	85%	85%
TGM	85%	85%	85%	85%	85%	85%
TGS	85%	85%	85%	85%	85%	85%
LAC	85%	85%	85%	85%	85%	85%
OGF	85%	85%	85%	85%	85%	85%
OGM	85%	85%	85%	85%	85%	85%
TR	30%	30%	30%	30%	30%	30%
TGF	30%	30%	30%	30%	30%	30%
OGF	30%	30%	30%	30%	30%	30%
OGM	30%	30%	30%	30%	30%	30%
MF	47%	47%	47%	47%	47%	47%
TGF	47%	47%	47%	47%	47%	47%
TGM	47%	47%	47%	47%	47%	47%
OGF	47%	47%	47%	47%	47%	47%
OGM	47%	47%	47%	47%	47%	47%
OGS	47%	47%	47%	47%	47%	47%
HD	30%	30%	30%	30%	30%	30%
TGF	30%	30%	30%	30%	30%	30%
TGM	30%	30%	30%	30%	30%	30%
LAC	30%	30%	30%	30%	30%	30%
LD	13%	13%	13%	13%	13%	13%
TGF	13%	13%	13%	13%	13%	13%
TGM	13%	13%	13%	13%	13%	13%
TGS	13%	13%	13%	13%	13%	13%
LAC	13%	13%	13%	13%	13%	13%
OGF	13%	13%	13%	13%	13%	13%
OGM	13%	13%	13%	13%	13%	13%
OGS	13%	13%	13%	13%	13%	13%
G	0%	0%	0%	0%	0%	0%
TGF	0%	0%	0%	0%	0%	0%
TGM	0%	0%	0%	0%	0%	0%
LAC	0%	0%	0%	0%	0%	0%
OGF	0%	0%	0%	0%	0%	0%
OGM	0%	0%	0%	0%	0%	0%

Table B1-1. Des Moines Creek Subbasin Parameters - 1994 Base Condition

Land Use	Sub-basin Areas (acres)						Total	Percent
	SDS Basins		IWS Basins		SDS10F			
% of total basin equal to EIA	SDS-4 Effective Pervious	SDS-2.5/6.7 Effective Pervious	PRIMARY Effective Pervious	SDS10F Effective Pervious	SDS-4 Impedimentous	SDS-2.5/6.7 Impedimentous	PRIMARY Impedimentous	SDS10F Impedimentous
TF	0.22	8.12	1.89	0.00	6.65	0.02	0.00	0.00
TFM	0.00	0.52	0.00	0.00	6.65	0.02	0.00	0.00
TFS	0.04	0.12	0.00	0.00	0.02	0.00	0.00	0.00
LAC	38.15	107.91	11.84	239.59	11.84	13.15	239.59	32.85
OFF	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
OFM	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
SA	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Lake	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Other	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
<b>Basin total:</b>	<b>38.15</b>	<b>107.91</b>	<b>11.84</b>	<b>239.59</b>	<b>11.84</b>	<b>13.15</b>	<b>239.59</b>	<b>32.85</b>
<b>PERLND areas:</b>								
(14) TFF	2.73	2.73	2.66	0.00	0.00	0.00	0.00	0.00
(16) TFM	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
(18) TFS	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
(24) TGF	9.05	96.03	6.04	0.28	1.10	0.15	0.28	0.15
(26) TGM	0.22	8.12	1.89	0.00	6.65	0.02	0.00	0.00
(28) TGS	28.87	0.51	0.11	0.00	0.00	0.00	0.00	0.00
(34) OF	0.00	0.52	0.00	0.00	0.00	0.00	0.00	0.00
(44) OG	38.15	107.91	11.84	0.43	0.00	0.00	0.43	0.00
(54) SA	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
subtotal:	19.37	13.15	251.43	33.28	239.59	33.28	33.28	33.28
<b>IMPLND areas:</b>								
EIA	157.52	121.06	251.43	33.28	121.06	10.9%	33.28	33.28
<b>Basin total:</b>	<b>157.52</b>	<b>121.06</b>	<b>251.43</b>	<b>33.28</b>	<b>121.06</b>	<b>10.9%</b>	<b>33.28</b>	<b>33.28</b>
<b>Percent Impervious</b>	<b>33.7%</b>	<b>33.7%</b>	<b>33.7%</b>	<b>33.7%</b>	<b>33.7%</b>	<b>33.7%</b>	<b>33.7%</b>	<b>33.7%</b>
<b>NETWORK FACTORS</b>								
PERLND 16 (TF)	0.755	0.228	0.222	0.000	0.598	0.036	0.036	0.036
PERLND 26 (TG)	0.018	0.002	0.158	0.000	0.158	0.000	0.000	0.000
PERLND 34 (OF)	2.495	0.042	0.009	0.000	0.000	0.000	0.000	0.000
PERLND 44 (OG)	0.000	0.043	0.000	0.000	0.000	0.000	0.000	0.000
PERLND 54 (SA)	1.974	1.096	19.966	2.738	19.966	2.738	2.738	2.738
IMPLND 14 (EIA)	4.793	10.088	20.953	2.774	20.953	2.774	2.774	2.774
<b>SUM</b>	<b>10.000</b>	<b>10.000</b>	<b>10.000</b>	<b>10.000</b>	<b>10.000</b>	<b>10.000</b>	<b>10.000</b>	<b>10.000</b>

<sup>a</sup> - land use is extrapolated

<sup>b</sup> - areas prorated to new subbasin total areas from previous version of model.  
shaded subbasins included in the STIA subbasin characteristics evaluation area.

**MILLER/WALKER CREEK HSPF MODEL CALIBRATION REPORT**

**VOLUME 3—APPENDIX B2**

Prepared for

**PARAMETRIX, INC.**

5808 Lake Washington Blvd. N.E., Suite 200  
Kirkland, Washington 98033-7350

Prepared by

Joseph T. Brascher  
AQUA TERRA Consultants

December 2000  
556-2912-001 (28)

**AR 010948**

**CERTIFICATE OF ENGINEER**

The technical material and data contained in this document were prepared under the supervision and direction of the undersigned.

---

Douglas Beyerlein, P.E., Senior Engineer

(affix seal here)

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## 1. INTRODUCTION

As part of Parametrix's study of the drainage impacts of the proposed Seattle-Tacoma International Airport (STIA) Master Plan Update (MPU) on both Walker and Miller Creeks, the Environmental Protection Agency's Hydrological Simulation Program - FORTRAN (HSPF) model was used to simulate the hydrology of the study area. Information resulting from this computer simulation effort will be used to evaluate the hydrologic impacts of urbanization in the watershed.

This report discusses the calibration effort for both the Walker and Miller Creek HSPF models. The calibration team consisted of Joe Brascher (Aqua Terra) and Dave Harms (formerly of Parametrix). Every aspect of each model was reviewed by the team, and the best possible data were used.

The calibration period consisted of water-years 1993 through 1996. Simulated streamflow volumes over the calibration period were within 10 percent of recorded volumes. Some difficulty was encountered in matching the magnitude of simulated streamflow peaks with recorded streamflow peaks. Sixty-five calibration runs were performed for Miller Creek and 21 calibration runs were performed for Walker Creek in an effort to match the magnitude of the recorded streamflow peaks. The best possible match for the given data has been achieved. Investigation of the recorded streamflow data at both gage sites for Miller Creek and both gage sites for Walker Creek demonstrates possible errors as a result of gage malfunctions and shifting control situations. The team concluded that these errors may have contributed to the inability to match observed streamflows with simulated streamflows.

## 2. CALIBRATION

### 2.1 HYDROMETEORLOGIC DATA

Two streamflow gages located in the Miller Creek watershed and two streamflow gages located in the Walker Creek watershed were used in the calibration effort (Figure B2-1). Data for the streamflow gages were collected by the King County Department of Natural Resources. The calibration period covered water-years 1993 through 1996. The team selected this time period because it is believed to contain the most reliable data. However, the observed data from these water-years still contained several gaps. The dates of the missing data are listed in Table B2-1.

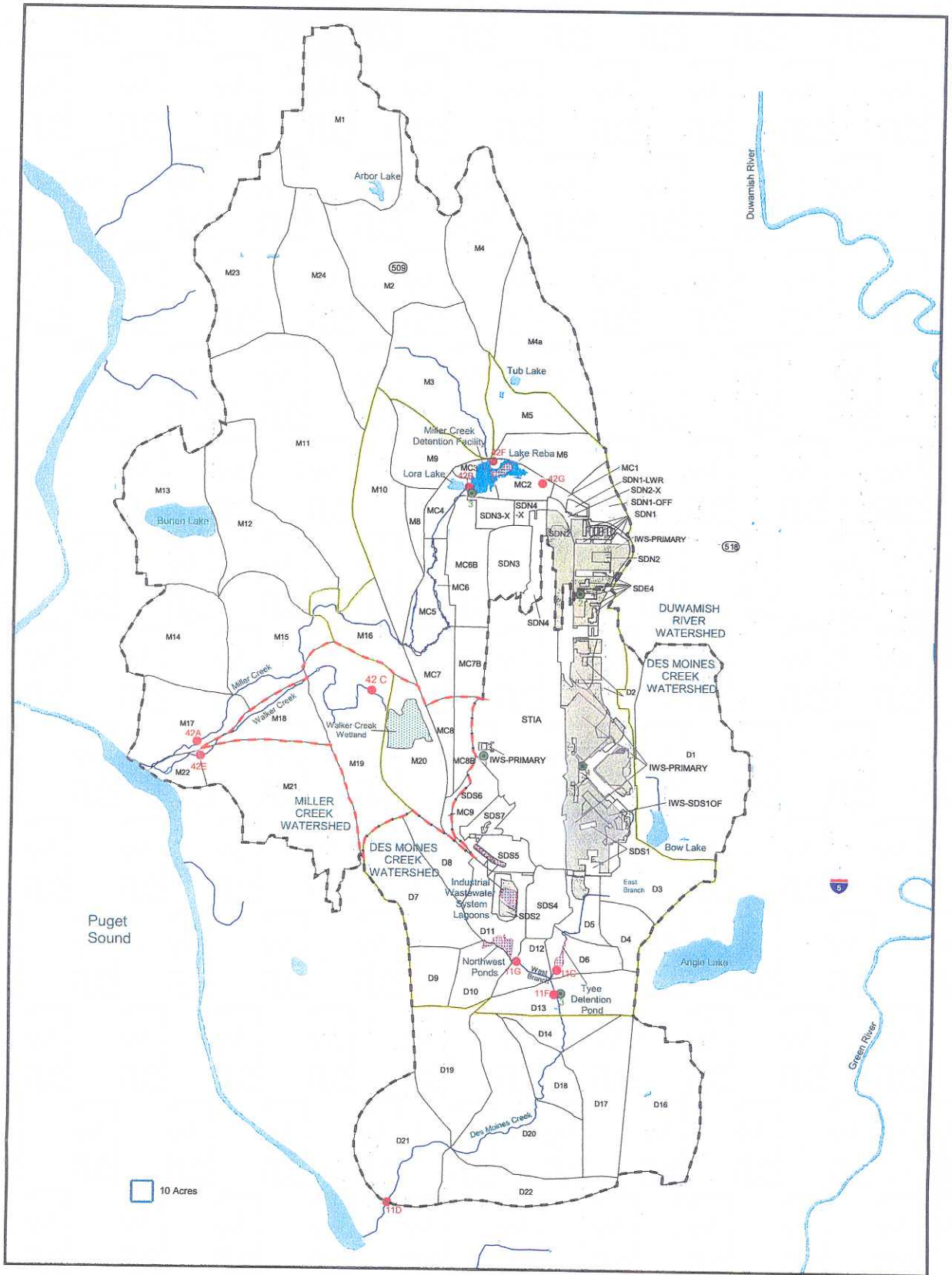
**Table B2-1. Dates of missing records at Miller and Walker Creek gages.**

	# obs
<b>RDF</b>	
10/1/92 1:00 – 10/15/92 17:00	352
11/23/93 16:00 – 12/29/93 13:00	861
9/17/94 3:00 – 10/14/94 1:00	646
12/5/94 14:00 – 1/3/95 14:00	696
<b>SR518</b>	
10/1/92 1:00 – 11/24/92 17:00	1312
11/23/93 17:00 – 12/16/93 16:00	551
11/28/94 16:00 – 1/3/95 16:00	864
<b>Walker Creek</b>	
2/2/93 16:00 – 3/18/93 14:00	1054
5/23/94 9:00 – 6/27/94 11:00	842
<b>Mouth of Miller Creek</b>	
10/1/92 1:00 – 10/8/92 15:00	182
12/2/93 12:00 – 12/29/93 15:00	651
11/29/94 13:00 – 12/30/94 10:00	741
6/21/95 17:00 – 10/1/95 0:00	2431

### 2.2 PRECIPITATION RECORD

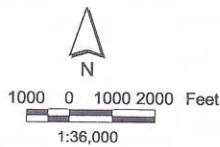
Precipitation data for the calibration period of 10/1/92 - 8/30/96 were hourly records taken at STIA. Due to the proximity to the Miller Creek watershed, these data can be used without any modifications.

The 46-month period of rainfall record was selected from the 49 years of record at STIA for the calibration period since this represented a wide range of precipitation conditions. The average annual rainfall for the 49 years of record was 38.3 inches. For the period of record, 1993 had the third lowest annual rainfall total (28.8 inches) and 1996 had the second highest annual rainfall total (50.7 inches). By comparison, the 1994 annual rainfall total was 3.5 inches below average, and the 1995 annual rainfall total was 4.3 inches above average. Therefore, the time interval between 1993 and 1996 provided a range of precipitation conditions for calibration at a time when the land use could be accurately estimated.



Parametrix, Inc. Sea-Tac Airport Stormwater Management Plans/558-2912-001(28) 8/00 File: K:\GIS\2912\Drawings\seatac-apdxs\_may2001.apr  
 Source: Roads based on King County data. Water bodies derived from USGS hydrography data. Detention boundaries are approximate.  
 Note: Subbasin boundaries shown outside of STIA area are for illustration and reference only.  
 STIA subbasins assume existing (1994) conditions.

July 2001  
 556-2912-001 (28)



AR 010955

- |   |                                    |       |                                 |   |   |
|---|------------------------------------|-------|---------------------------------|---|---|
| — | Roads                              | - - - | Subwatershed Boundary           | ● | Precipitation Gaging Stations:<br>Type 1 - National Weather Service (Gage relocated 1996)<br>Type 2 - POS Rainfall Monitoring<br>Type 3 - King County Rainfall Monitoring   |
| — | Existing (1994) Drainage Subbasins | - - - | Watershed Boundary              | ● | Streamflow King County Gaging Stations:<br>42A - Miller Ck @ SW 175th Pl & 12th Ave SW<br>42B - Miller Ck @ Lake Reba/RDF Outlet<br>42C - Walker Ck @ 171st Pl<br>42E - Walker Ck @ 12th Ave SW<br>42F - Miller Ck @ SR518<br>42G - Miller Ck @ East Branch |
| — | STIA Area (see note)               | —     | Rivers                          | ● | 11D - Des Moines Ck near mouth<br>11F - Des Moines Ck @ Golf Course<br>11C - Des Moines Ck @ Tyee Pond<br>11G - Des Moines Ck @ NW Ponds  |
| ■ | Constructed Water Features         | ■     | Water Bodies                    |   |   |
| ■ | IWS Drainage Area                  | ■     | Detention Facilities (existing) |   |   |

**Figure B2-1**  
**Map of Basins with**  
**Gage Locations**

## 2.3 EVAPORATION DATA

Daily evaporation data recorded at the Washington State Research and Extension Center gage at Puyallup have been used for this modeling effort. These data have been appended starting in water-year 1992 by King County monthly average evaporation data.

## 2.4 SOILS

Both the Miller and Walker Creek watersheds contain a mixture of glacial till, outwash, and wetland soils. Consideration was given to separating out an additional soil/geology type to represent non-typical glacial till soils that are present in both the Miller and Walker Creek watersheds. These glacial till soils are characterized by unusually deep layers of permeable material overlaying the till layer. U.S. Geological Survey (USGS) regional parameters for till soils are based on the assumption that the impermeable layer of till is underneath a few feet of highly permeable soil. Investigation of available soils data conducted by Parametrix staff indicated that there are areas within the Miller Creek basin where the impermeable till layer is beneath a much deeper layer of highly permeable soil – over 20 feet of depth. Since the till layer presumably is at approximately the same elevation throughout the basin (considering how it was formed), overall runoff volume should not be affected by the depth of permeable material over it. The depth of permeable material would have an impact on flow attenuation. Since the areas of deep till (greater than 20 feet deep) are spread throughout the basin and not located within one specific area, the team believes that the resulting calibration parameter values for till soils are average values representative of the basin as a whole and not biased toward a certain portion of the basin.

## 2.5 MODEL PARAMETERS

The HSPF regional parameters were used as a starting point for the calibration of both the Miller and Walker Creek watershed models. Based upon our professional judgment, changes were made to the PERLND parameters to better reflect the hydrology of the watershed. A complete listing of the HSPF model parameters for Miller and Walker Creeks is provided in Tables B2-2 and B2-3.

## 2.6 LAND USE DATA

The land use data for this model were taken from the Environmental Impact Statement prepared several years earlier. The source of these data is believed to be Montgomery Water Group. Since there is uncertainty about the original source of the land use data, and spot verification of land use indicated sizable discrepancies from observable data such as aerial photography, it was decided to use the latest version of King County Department of Natural Resources' land use for the calibration period. The King County land use is based on 1994 aerial photos. Soils data were obtained from King County for similar reasons. When King County's land use data were input to the model, a consistent drop in impervious area for the subbasins was observed (Tables B2-4 through B2-7). Model calibration using this data also did not distinguish between separate slope categories.

Table B2-2. Miller Creek PERLND parameter values: July 2000 calibration.

PERLND #	PERLND Type	LZSN	INFILT	LSUR	SLSUR	KVARY	AGWRC	INFEXP	INFILD	DEEPPR	BASETP
16	TFM	9.0	0.32	400	0.100	0.5	0.996	2	2	0.33	0
26	TGM	9.0	0.12	400	0.100	0.5	0.996	2	2	0.33	0
34	OF	10.0	2.00	400	0.050	0.3	0.996	2	2	0.33	0
44	OG	10.0	0.80	400	0.050	0.3	0.996	2	2	0.33	0
54	SAT	8.0	2.00	100	0.001	0.5	0.996	10	2	0.33	0

PERLND #	PERLND Type	AGWETP	CEPSC	UZSN	NSUR	INTFW	IRC	LZETP
16	TFM	0.0	0.2	0.75	0.35	9.0	0.7	0.70
26	TGM	0.0	0.1	0.38	0.25	9.0	0.7	0.25
34	OF	0.0	0.2	0.75	0.35	0.0	0.7	0.70
44	OG	0.0	0.1	0.75	0.25	0.0	0.7	0.25
54	SAT	0.7	0.1	2.25	0.50	1.0	0.7	0.80

Table B2-3. Walker Creek PERLND parameter values: July 2000 calibration.

PERLND #	PERLND Type	LZSN	INFILT	LSUR	SLSUR	KVARY	AGWRC	INFEXP	INFILD	DEEFPF	BASETP
16	TFM	4.5	0.08	400	0.100	0.5	0.996	2	2	0	0
26	TGM	4.5	0.03	400	0.100	0.5	0.996	2	2	0	0
34	OF	5.0	2.00	400	0.050	0.3	0.996	2	2	0	0
44	OG	5.0	0.80	400	0.050	0.3	0.996	2	2	0	0
54	SAT	4.0	2.00	100	0.001	0.5	0.996	10	2	0	0
64	Des Moines TGF	4.5	0.12	400	0.100	0.5	0.999	2	2	0	0

PERLND #	PERLND Type	AGWETP	CEPSC	UZSN	NSUR	INTFW	IRC	LZETP
16	TFM	0.0	0.2	0.50	0.35	2.0	0.15	0.70
26	TGM	0.0	0.1	0.25	0.25	2.0	0.15	0.25
34	OF	0.0	0.2	0.50	0.35	0.0	0.50	0.70
44	OG	0.0	0.1	0.50	0.25	0.0	0.50	0.25
54	SAT	0.7	0.1	3.00	0.50	1.0	0.70	0.80
64	Des Moines TGF	0.0	0.1	0.25	0.25	3.0	0.50	0.25

Table B2-4. Miller Creek subbasin characteristics - 1994 base condition (STIA basins).

Land Use	Sub-basin Areas (acres)													
	M-5		M-6		M-8		M-9		M-10		M-16		MC-1 to MC-7	
	Effective Pervious	Impervious	Effective Pervious	Impervious	Effective Pervious	Impervious	Effective Pervious	Impervious	Effective Pervious	Impervious	Effective Pervious	Impervious	Effective Pervious	Impervious
STIA	% of total basin equal to EIA M 0% Qvr 0% Qvt 0% Qw 0% TIA 100%													
COMM	0.53	3.01	-	-	-	-	-	-	5.35	30.34	0.02	0.12	-	-
	-	-	-	-	-	-	-	-	-	-	0.46	2.60	-	-
	2.18	12.34	0.23	1.31	-	-	-	-	-	-	-	-	-	-
TRANS	M 0% Qvr 0% Qvt 0% Qw 0% TIA 100%													
	-	-	0.03	-	-	-	-	-	-	-	7.22	-	0.10	-
	-	-	3.44	-	-	-	3.83	-	11.34	-	3.48	-	3.73	-
	-	-	-	-	-	-	0.36	-	4.32	-	-	-	-	-
	-	-	0.01	-	-	-	0.01	-	-	-	-	-	-	-
	-	-	-	4.79	-	-	-	7.21	-	16.75	-	5.32	-	0.09
MF	0.62	0.55	-	-	6.68	5.92	5.13	4.55	19.97	17.71	1.19	1.06	-	-
	-	-	-	-	-	-	-	-	-	-	0.20	0.17	-	-
HD	-	-	-	-	0.48	0.08	46.38	8.18	6.98	1.23	15.22	2.69	-	-
	-	-	-	-	-	-	-	-	-	-	16.63	2.94	-	-
	-	-	5.92	1.05	-	-	13.96	2.46	19.60	3.46	-	-	-	-
	-	-	-	-	-	-	-	-	-	-	-	-	-	-
LD	-	-	-	-	-	-	-	-	-	-	0.16	0.01	-	-
	-	-	-	-	-	-	-	-	-	-	0.01	0.00	-	-
	9.67	0.40	-	-	14.21	0.59	1.26	0.05	51.59	2.15	5.78	0.24	-	-
	-	-	-	-	-	-	-	-	-	-	9.37	0.39	-	-
	-	-	-	-	-	-	-	-	8.02	0.33	-	-	-	-
	-	-	-	-	-	-	-	-	-	-	-	-	-	-
OG	-	-	0.18	-	-	-	-	-	-	-	-	-	0.07	-
	39.22	-	9.95	-	0.84	-	0.10	-	-	-	1.98	-	4.48	-
	-	-	-	-	-	-	-	-	-	-	0.00	-	-	-
	8.11	-	34.71	-	-	-	-	-	-	-	-	-	-	-
	0.47	-	-	-	-	-	-	-	-	-	-	-	-	-
F	-	-	0.06	-	-	-	-	-	-	-	1.17	-	-	-

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Table B2-4. Miller Creek subbasin characteristics - 1994 base condition (STIA basins).

Land Use	Sub-basin Areas (acres)													
	M-5		M-6		M-8		M-9		M-10		M-16		MC-1 to MC-7	
	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious
% of total basin equal to EIA														
Qu	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Qvr	-	21.75	-	-	-	-	0.05	-	-	-	-	19.26	-	-
Qvrl	-	-	-	-	-	-	-	-	-	-	-	9.76	-	-
Qvt	-	10.60	-	-	-	-	4.94	-	4.15	-	-	-	-	-
Qw	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Wetland	10.27	0.80	-	-	-	-	-	-	-	-	-	-	-	0.27
Lake	0.44	-	-	-	-	-	-	-	-	-	-	-	-	-
Mcrk-df	-	0.07	-	-	-	-	-	-	-	-	-	-	-	-
Other	-	0.01	-	-	-	-	-	-	0.00	-	-	-	-	-
<b>Basin total:</b>	71.07	16.31	7.14	22.21	6.60	22.46	76.02	71.97	131.32	71.97	92.69	15.54	8.65	0.09
<b>PERLND areas:</b>														
(16) Till-Forest	-	10.66	-	-	-	-	4.94	-	4.15	-	10.93	-	-	-
(26) Till-Grass	10.29	41.08	-	-	-	-	14.32	-	31.94	-	30.30	-	0.17	-
(34) Outwash-Forest	-	21.75	-	-	-	-	0.05	-	-	-	20.03	-	-	-
(44) Outwash-Grass	50.04	13.39	22.21	-	-	-	56.70	-	95.23	-	31.42	-	8.21	-
(54) Wetland	10.74	0.82	-	-	-	-	0.01	-	-	-	-	-	0.27	-
subtotal:	71.07	87.70	22.21	22.21	6.60	22.46	76.02	71.97	131.32	71.97	92.69	15.54	8.65	0.09
<b>IMPLND areas:</b>														
EIA	16.31	7.14	6.60	22.46	71.97	131.32	71.97	131.32	71.97	131.32	71.97	15.54	8.65	0.09
<b>Basin total:</b>	87.38	94.85	28.80	98.48	203.30	108.23	87.38	203.30	108.23	87.38	203.30	108.23	87.38	1.1%
<b>Percent impervious:</b>	18.7%	7.5%	22.9%	22.8%	35.4%	14.4%	18.7%	35.4%	14.4%	18.7%	35.4%	14.4%	18.7%	1.1%





**Table B2-4. Miller Creek subbasin characteristics - 1994 base condition (STIA basins).**

Land Use	Sub-basin Areas (acres)													
	MC-2		MC-3		MC-4		MC-5		MC-6		MC-7		SDS Basins	
Land Use	Soil Type	% of total basin equal to EIA	Effective		Effective		Effective		Effective		Effective		Effective	
			Pervious	Impervious	Pervious	Impervious	Pervious	Impervious	Pervious	Impervious	Pervious	Impervious	Pervious	Impervious
Qu	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Qvr	6.72	-	5.44	-	-	-	-	-	-	-	-	-	-	-
Qvri	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Qvt	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Qw	4.17	-	1.28	-	-	-	-	-	-	-	-	-	-	-
Wetland	10.30	0%	0.52	5.77	5.95	4.16	6.54	4.16	0.24	-	-	-	-	-
Lake	2.98	-	-	3.35	0.64	-	0.40	-	-	-	-	-	-	-
Mcrk-df	7.00	-	13.17	-	-	-	-	-	-	-	-	-	-	-
Other	-	-	-	-	-	-	-	-	-	-	-	-	-	-
<b>Basin total:</b>	<b>33.11</b>	<b>0.27</b>	<b>12.75</b>	<b>31.73</b>	<b>50.50</b>	<b>50.34</b>	<b>24.29</b>	<b>2.50</b>	<b>0.95</b>	<b>3.88</b>	<b>5.54</b>	<b>8.29</b>	<b>5.54</b>	<b>8.29</b>
<b>PERLND areas:</b>														
(16) Till-Forest	0.08	-	-	-	-	-	-	-	-	-	-	-	-	-
(26) Till-Grass	0.64	-	-	-	13.49	-	-	-	-	-	-	-	12.66	3.23
(34) Outwash-Forest	6.72	5.44	-	-	-	-	-	-	-	-	-	-	-	-
(44) Outwash-Grass	10.43	5.03	5.03	17.32	31.06	33.53	17.75	31.06	33.53	33.53	2.11	2.11	2.11	2.11
(54) Wetland	15.25	2.28	2.28	14.41	5.95	4.16	6.54	5.95	4.16	4.16	0.20	0.20	0.20	0.20
subtotal:	33.11	12.75	12.75	31.73	50.50	50.34	24.29	50.50	50.34	50.34	5.54	5.54	5.54	5.54
<b>IMPLND areas:</b>														
EIA	0.27	0.11	0.11	1.77	2.50	3.88	0.95	2.50	3.88	3.88	8.29	8.29	8.29	8.29
<b>Basin total:</b>	<b>33.38</b>	<b>0.27</b>	<b>12.87</b>	<b>33.50</b>	<b>53.00</b>	<b>54.23</b>	<b>25.24</b>	<b>4.7%</b>	<b>3.8%</b>	<b>7.2%</b>	<b>13.84</b>	<b>13.84</b>	<b>13.84</b>	<b>59.9%</b>
<b>Percent Impervious:</b>		<b>0.8%</b>	<b>0.9%</b>	<b>5.3%</b>	<b>4.7%</b>	<b>7.2%</b>	<b>3.8%</b>	<b>4.7%</b>	<b>3.8%</b>	<b>7.2%</b>	<b>13.84</b>	<b>13.84</b>	<b>13.84</b>	<b>59.9%</b>

Table B2-4. Miller Creek subbasin characteristics - 1994 base condition (STIA basins).

Land Use		Sub-basin Areas (acres)													
		SDN1-LWR		SDN1-OFF		SDN-2		SDN-2X		SDN-3		SDN-3X		SDN-4	
Land Use	Soil Type	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious
STIA	M	0%	0%	8.43	-	-	-	-	-	7.17	-	-	-	19.66	-
	Qvr	0%	0%	1.02	-	-	-	-	-	0.00	-	-	-	0.02	-
	Qvt	0%	0%	-	-	-	-	-	-	18.87	-	-	-	0.06	-
	Qw	0%	0%	-	-	-	-	-	-	-	-	-	-	-	-
	TIA	100%	100%	-	33.22	-	-	-	-	-	15.82	-	-	-	2.61
COMM	Qvr	85%	-	-	-	-	-	-	-	-	-	-	-	-	-
	Qvrl	85%	-	-	-	-	-	-	-	-	-	-	-	-	-
	Qvt	85%	-	-	-	-	-	-	-	-	-	-	-	-	-
TRANS	M	0%	0.68	-	-	-	-	-	-	-	-	-	-	-	-
	Qvr	0%	3.28	-	-	-	0.10	-	-	-	-	-	-	-	-
	Qvrl	0%	-	-	-	-	-	-	-	-	-	-	-	-	-
	Qvt	0%	1.76	-	-	-	-	-	-	-	-	-	-	-	-
	Qw	0%	1.63	-	-	-	-	-	-	-	-	-	-	-	-
	TIA	100%	0.38	7.88	-	-	0.28	-	-	-	-	-	-	-	-
MF	Qvr	47%	-	-	-	-	-	-	-	-	-	-	-	-	-
	Qvrl	47%	-	-	-	-	-	-	-	-	-	-	-	-	-
HD	Qvr	15%	-	-	-	-	-	-	-	-	-	-	-	-	-
	Qvrl	15%	-	-	-	-	-	-	-	-	-	-	-	-	-
	Qvt	15%	20.51	3.62	-	-	-	-	-	-	-	-	-	-	-
	Qw	15%	-	-	-	-	-	-	-	-	-	-	-	-	-
LD	M	4%	-	-	-	-	-	-	-	-	-	-	-	-	-
	Qu	4%	-	-	-	-	-	-	-	-	-	-	-	-	-
	Qvr	4%	-	-	-	-	-	-	-	-	-	-	-	-	-
	Qvrl	4%	-	-	-	-	-	-	-	-	-	-	-	-	-
	Qvt	4%	-	-	-	-	-	-	-	-	-	-	-	-	-
	Qw	4%	-	-	-	-	-	-	-	-	-	-	-	-	-
OG	M	0%	0.00	1.98	-	-	1.37	-	-	0.21	-	-	-	4.70	-
	Qvr	0%	4.87	2.02	-	-	5.74	-	-	0.00	-	-	-	3.16	-
	Qvrl	0%	-	-	-	-	-	-	-	-	-	-	-	-	-
	Qvt	0%	-	-	-	-	-	-	-	23.53	-	-	-	-	-
	Qw	0%	6.17	-	-	-	-	-	-	-	-	-	-	-	-
F	M	0%	-	-	-	-	-	-	-	-	-	-	-	-	-

Table B2-4. Miller Creek subbasin characteristics - 1994 base condition (STIA basins).

Land Use	Sub-basin Areas (acres)												
	SDS Basins												
	SDN1-LWR		SDN1-OFF		SDN-2		SDN-2X		SDN-3		SDN-3X		SDN-4
Land Use	Soil Type	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious
% of total basin equal to EIA													
Qu	0%	-	-	-	-	-	-	-	-	-	-	-	-
Qvr	0%	-	-	-	-	-	-	-	-	-	-	-	-
Qvrl	0%	-	-	-	-	-	-	-	-	-	-	-	-
Qvt	0%	-	-	-	-	-	-	-	-	-	-	-	-
Qw	0%	-	-	-	-	-	-	-	-	-	-	-	-
Wetland	0%	0.07	0.04	-	-	-	-	-	-	-	-	-	-
Lake		-	-	-	-	-	-	-	-	-	-	-	-
Mcrk-df		-	-	-	-	-	-	-	-	-	-	-	-
Other		-	0.04	-	-	-	-	-	-	-	-	-	-
<b>Basin total:</b>		5.04	34.41	11.50	13.45	33.22	7.21	0.28	49.79	15.82	25.38	27.62	2.61
<b>PERLND areas:</b>													
(16) Till-Forest		-	-	-	-	-	-	-	-	-	-	-	-
(26) Till-Grass		-	29.12	-	10.41	-	1.37	-	49.79	-	0.65	-	24.43
(34) Outwash-Forest		-	-	-	-	-	-	-	-	-	5.17	-	-
(44) Outwash-Grass		4.97	3.62	3.04	3.04	0.00	5.84	-	0.00	13.64	5.34	3.19	-
(54) Wetland		0.07	1.67	-	-	-	-	-	-	-	0.57	-	-
subtotal:		5.04	34.41	13.45	13.45	49.79	7.21	0.28	49.79	15.82	25.38	27.62	2.61
<b>IMPLND areas:</b>													
EIA		0.38	11.50	33.22	0.28	15.82	0.28	0.28	65.62	25.38	30.23	30.23	8.6%
<b>Basin total:</b>		5.42	45.90	46.67	7.49	71.2%	7.49	3.7%	71.2%	24.1%	0.0%	30.23	8.6%
<b>Percent impervious:</b>		7.0%	25.0%	71.2%	3.7%	24.1%	0.0%	3.7%	24.1%	0.0%	0.0%	30.23	8.6%

Table B2-4. Miller Creek subbasin characteristics - 1994 base condition (STIA basins).

Land Use	Land Use	Soil Type	% of total basin equal to EIA	Sub-basin Areas (acres)													
				SDN-4X				MC-6B				MC-7B				Total	
				Pervious	Impervious	Effective	% of total	Pervious	Impervious	Effective	% of total	Pervious	Impervious	Effective	% of total	Pervious	Impervious
STIA	M	0%	0%	1.29	-	0.82	-	4.92	-	42.29	-	-	-	3.39%	0.00%	-	-
	Qvr	0%	0%	0.65	-	-	-	-	-	2.17	-	-	-	0.17%	0.00%	-	-
	Qvt	0%	0%	-	-	-	0.46	-	21.13	-	-	-	1.69%	0.00%	-	-	-
	Qw	0%	0%	-	-	-	-	-	0.17	-	-	-	0.01%	0.00%	-	-	-
	TIA	100%	100%	-	-	-	-	0.66	-	-	-	-	0.00%	4.76%	59.40	-	-
COMM	Qvr	85%	85%	-	-	-	-	-	5.91	33.48	-	-	0.47%	2.68%	-	-	-
	Qvrl	85%	85%	-	-	-	-	-	0.46	2.60	-	-	0.04%	0.21%	-	-	-
	Qvt	85%	85%	-	-	-	-	-	2.41	13.65	-	-	0.19%	1.09%	-	-	-
TRANS	M	0%	0%	-	-	-	-	-	0.83	-	-	-	0.07%	0.00%	-	-	-
	Qvr	0%	0%	-	-	-	-	-	37.71	-	-	-	3.02%	0.00%	-	-	-
	Qvrl	0%	0%	-	-	-	-	-	3.48	-	-	-	0.28%	0.00%	-	-	-
	Qvt	0%	0%	-	-	-	-	-	6.43	-	-	-	0.52%	0.00%	-	-	-
	Qw	0%	0%	-	-	-	-	-	2.33	-	-	-	0.19%	0.00%	-	-	-
	TIA	100%	100%	-	-	-	-	-	-	44.19	-	-	0.00%	3.54%	-	-	-
MF	Qvr	47%	47%	-	-	-	-	-	34.37	30.48	-	-	2.76%	2.44%	-	-	-
	Qvrl	47%	47%	-	-	-	-	-	0.20	0.17	-	-	0.02%	0.01%	-	-	-
HD	Qvr	15%	15%	-	12.21	2.15	0.89	0.16	95.41	16.84	-	-	7.65%	1.35%	-	-	-
	Qvrl	15%	15%	-	-	-	-	-	16.63	2.94	-	-	1.33%	0.24%	-	-	-
	Qvt	15%	15%	-	-	-	6.81	1.20	79.29	13.99	-	-	6.36%	1.12%	-	-	-
	Qw	15%	15%	-	-	-	-	-	0.90	0.16	-	-	0.07%	0.01%	-	-	-
LD	M	4%	4%	-	-	-	-	-	0.16	0.01	-	-	0.01%	0.00%	-	-	-
	Qu	4%	4%	-	-	-	-	-	0.01	0.00	-	-	0.00%	0.00%	-	-	-
	Qvr	4%	4%	-	23.70	0.99	2.44	0.10	186.09	7.75	-	-	14.92%	0.62%	-	-	-
	Qvrl	4%	4%	-	-	-	-	-	9.45	0.39	-	-	0.76%	0.03%	-	-	-
	Qvt	4%	4%	-	-	-	-	-	21.56	0.90	-	-	1.73%	0.07%	-	-	-
	Qw	4%	4%	-	-	-	-	-	0.33	0.01	-	-	0.03%	0.00%	-	-	-
OG	M	0%	0%	0.02	1.66	-	0.83	-	11.66	-	-	-	0.93%	0.00%	-	-	-
	Qvr	0%	0%	9.35	17.01	-	5.14	-	130.14	-	-	-	10.43%	0.00%	-	-	-
	Qvrl	0%	0%	-	-	-	-	-	0.00	-	-	-	0.00%	0.00%	-	-	-
	Qvt	0%	0%	0.25	32.46	-	23.14	-	135.09	-	-	-	10.83%	0.00%	-	-	-
	Qw	0%	0%	-	0.06	-	-	-	8.56	-	-	-	0.69%	0.00%	-	-	-
F	M	0%	0%	-	-	-	-	-	1.31	-	-	-	0.11%	0.00%	-	-	-

**Table B2-4. Miller Creek subbasin characteristics - 1994 base condition (STIA basins).**

Land Use	Sub-basin Areas (acres)										
	% of total basin equal to EIA	SDS Basins				MC-7B				Total	
		SDN-4X	MC-6B		MC-7B		Total		Percent of Total		
Land Use	Soil Type	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious	Effective Pervious	Effective Impervious	Pervious	Impervious
Qu	0%	-	-	-	-	-	-	0.77	-	0.06%	0.00%
Qvr	0%	1.16	-	7.81	-	-	-	75.83	-	6.08%	0.00%
Qvrl	0%	-	-	-	-	-	-	9.76	-	0.78%	0.00%
Qvt	0%	-	-	-	-	-	-	20.34	-	1.63%	0.00%
Qw	0%	-	-	-	-	-	-	5.45	-	0.44%	0.00%
Wetland	0%	-	-	4.55	-	1.92	-	51.73	-	4.15%	0.00%
Lake		-	-	-	-	-	-	8.06	-	0.65%	0.00%
Mcrk-df		-	-	-	-	-	-	20.24	-	1.62%	0.00%
Other		-	-	-	-	-	-	0.04	-	0.00%	0.00%
<b>Basin total:</b>		12.73	0.00	100.27	3.14	46.54	2.12	1020.36	226.97	81.80%	18.20%
<b>PERLND areas:</b>											
(16) Till-Forest		-	-	-	-	-	-	31.42	-	2.52%	-
(26) Till-Grass		1.57	34.94	7.81	-	36.16	-	351.08	-	28.15%	-
(34) Outwash-Forest		1.16	7.81	-	-	-	-	76.60	-	6.14%	-
(44) Outwash-Grass		10.01	52.91	4.61	8.46	1.92	-	491.81	-	39.43%	-
(54) Wetland		-	4.61	100.27	1.92	46.54	-	69.46	-	5.57%	-
subtotal:		12.73	100.27	100.27	3.14	46.54	2.12	1020.36	226.97	81.80%	18.20%
<b>IMPLND areas:</b>											
EIA		0.00	0.00	3.14	3.14	48.66	4.4%	1247.33	226.97	100.00%	18.20%
<b>Basin total:</b>		12.73	103.41	103.41	3.0%	48.66	4.4%	1247.33	226.97	100.00%	18.20%
<b>Percent impervious:</b>		0.0%	0.0%	3.0%	3.0%	4.4%	4.4%	1247.33	226.97	100.00%	18.20%

Table B2-5. Miller Creek subbasin characteristics - 1994 base condition (non-STIA basins).

Land Use	% of total basin equal to Soil Type EIA	Sub-basin Areas (acres)															
		Non-STIA Basins of M-1 to M-24															
		M-1		M-2		M-3		M-4		M-4A		M-11		M-12		M-13	
		Effective	Effective	Effective	Effective	Effective	Effective	Effective	Effective	Effective	Effective	Effective	Effective	Effective	Effective	Effective	
		Pervious	Impervious	Pervious	Impervious	Pervious	Impervious	Pervious	Impervious	Pervious	Impervious	Pervious	Impervious	Pervious	Impervious	Pervious	Impervious
Bare	85%	-	-	-	-	-	-	0.00	0.01	-	-	-	-	0.24	1.33	-	-
Ground	85%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Asphalt	85%	0.76	4.31	0.85	4.82	1.58	8.96	0.56	3.18	0.99	5.61	11.64	65.96	1.21	6.86	1.21	6.86
	85%	3.62	20.53	1.99	11.26	0.10	0.56	0.12	0.71	0.54	3.03	9.34	52.91	5.32	30.15	0.25	1.42
	85%	-	-	-	-	-	-	-	-	0.07	0.37	-	-	-	-	-	-
	85%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Bare	85%	-	-	-	-	-	-	-	-	-	-	0.01	0.04	-	-	-	-
Rock	85%	0.05	0.26	-	-	0.02	0.13	-	-	-	-	0.59	3.36	0.11	0.62	-	-
Concrete	85%	0.07	0.39	-	-	-	-	-	-	-	-	0.26	1.48	0.14	0.80	-	-
	85%	-	-	-	-	-	-	-	-	-	-	0.11	0.61	-	-	-	-
Develop	85%	-	-	0.22	1.27	-	-	-	-	0.08	0.47	-	-	-	-	-	-
High	85%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Intensity	85%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	85%	0.43	2.46	0.06	0.34	0.59	3.33	0.23	1.31	0.10	0.57	7.59	42.96	0.50	2.81	-	-
	85%	0.87	4.95	0.47	2.66	0.24	1.38	0.08	0.45	1.70	9.64	0.12	0.70	0.18	1.03	-	-
	85%	0.00	0.02	-	-	-	-	-	-	0.00	0.02	5.96	33.78	2.64	14.97	0.87	4.95
	85%	-	-	-	-	-	-	-	-	-	-	-	-	0.27	1.52	-	-
Develop	10%	-	-	0.43	0.05	-	-	0.87	0.10	1.89	0.21	0.67	0.07	-	-	-	-
Medium	10%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Intensity	10%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	10%	25.02	2.78	23.96	2.66	30.84	3.43	43.41	4.82	11.93	1.33	34.17	3.80	29.76	3.31	-	-
	10%	153.51	17.06	113.34	12.59	51.26	5.70	37.01	4.11	19.59	2.18	161.00	17.89	65.20	7.24	122.70	13.63
	10%	0.42	0.05	-	-	0.04	0.00	-	-	0.35	0.04	-	-	0.05	0.01	-	-
	10%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Develop.	4%	-	-	-	-	-	-	1.01	0.04	1.81	0.08	0.45	0.02	-	-	-	-
Low	4%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Intensity	4%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	4%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	4%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	4%	10.82	0.45	13.18	0.55	42.19	1.76	44.72	1.86	49.05	2.04	10.49	0.44	21.25	0.89	-	-
	4%	-	-	-	-	-	-	-	-	-	-	0.15	0.01	3.02	0.13	-	-
	4%	66.93	2.79	77.05	3.21	49.44	2.06	43.74	1.82	27.82	1.16	36.85	1.54	18.11	0.75	69.05	2.88
	4%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	4%	1.98	0.08	-	-	-	-	-	-	0.82	0.03	-	-	0.07	0.00	-	-
	4%	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

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Table B2-6. Walker Creek subbasin characteristics - 1994 base condition (STIA basins).

Land Use	Sub-basin Areas (acres)																
	M-20				MC-8				MC-9				MC-8B				
	Pervious	Impervious	Effective	Total	Pervious	Impervious	Effective	Total	Pervious	Impervious	Effective	Total	Pervious	Impervious	Effective	Total	
Land Use	% of total basin equal to EIA																
Wetland	33.43	-	-	-	2.91	-	-	-	0.08	-	-	-	0.61	-	-	-	15.85%
Lake	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.00%
Mcrk-of	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.00%
Other	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.00%
<b>Basin total:</b>	<b>99.40</b>	<b>52.83</b>	<b>29.02</b>	<b>1.79</b>	<b>13.34</b>	<b>0.32</b>	<b>1.71</b>	<b>177.03</b>	<b>35.27</b>	<b>1.71</b>	<b>56.65</b>	<b>177.03</b>	<b>75.76%</b>	<b>24.24%</b>			
<b>PERLND areas:</b>																	
(16) Till-Forest	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.00%
(26) Till-Grass	12.53	-	5.19	-	9.33	-	19.21	46.25	19.21	-	46.25	46.25	19.79%	0.00%			
(34) Outwash-Forest	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.00%
(44) Outwash-Grass	53.43	-	20.92	-	3.93	-	15.45	93.74	15.45	-	93.74	93.74	40.11%	0.00%			
(54) Wetland	33.43	-	2.91	-	0.08	-	0.61	37.04	0.61	-	37.04	37.04	15.85%	0.00%			
subtotal:	<b>99.40</b>	<b>52.83</b>	<b>29.02</b>	<b>1.79</b>	<b>13.34</b>	<b>0.32</b>	<b>177.03</b>	<b>177.03</b>	<b>35.27</b>	<b>1.71</b>	<b>56.65</b>	<b>177.03</b>	<b>75.76%</b>	<b>24.24%</b>			
<b>IMPLND areas:</b>																	
EIA	52.83	-	1.79	-	0.32	-	1.71	233.68	1.71	-	56.65	233.68	100.00%				
<b>Basin total:</b>	<b>152.23</b>	<b>34.7%</b>	<b>30.81</b>	<b>5.8%</b>	<b>13.66</b>	<b>2.3%</b>	<b>233.68</b>	<b>233.68</b>	<b>36.98</b>	<b>4.6%</b>	<b>56.65</b>	<b>233.68</b>	<b>100.00%</b>				
<b>Percent impervious:</b>																	





### 3. MILLER CREEK

#### 3.1 WATERSHED BOUNDARIES

Separate field investigations by Dave Harms and Ron Simmons of Parametrix identified areas where subbasin boundaries required modification. The boundary for subbasin M11 was moved to the north to include a portion of M24 and additional area previously considered to be out of the Miller Creek basin. A new subbasin (M23) was created to include area previously outside of the drainage basin to the west. City of Burien drainage basin mapping supports creation of subbasin M23 for this model. Subbasin M23 discharges through the Hermes depression. Another new subbasin (M24) was created, primarily as a result of subdividing M2, to separate two conveyance systems. M4 was subdivided into subbasins M4 and M4A. M4A consists primarily of airport buyout areas, where the team estimated the percent effective impervious area from aerial photography to be approximately 7 percent.

#### 3.2 GROUNDWATER ROUTING

Groundwater plays a key role in properly calibrating the Miller Creek watershed. Using the standard assumption that all groundwater generated within the Miller Creek watershed surface water boundaries drains to Miller Creek upstream of the gage site, simulated streamflows at the mouth of Miller Creek are nearly double that of observed streamflows. This result indicated that the Miller Creek watershed contributes to recharging a deeper underlying aquifer. The exact relationship between this deep aquifer and Miller Creek is unknown. However, a groundwater contour map (Figure B2-2) was prepared as part of a groundwater study performed by S.S. Papadopulos and Associates, Inc. using nearly 700 well logs. This map was used by the team as the basis for the groundwater routing in the calibration of the Miller Creek watershed. Figures B2-3a and B2-3b show detailed routing schematics, which include the routing of all surface water, interflow, and groundwater.

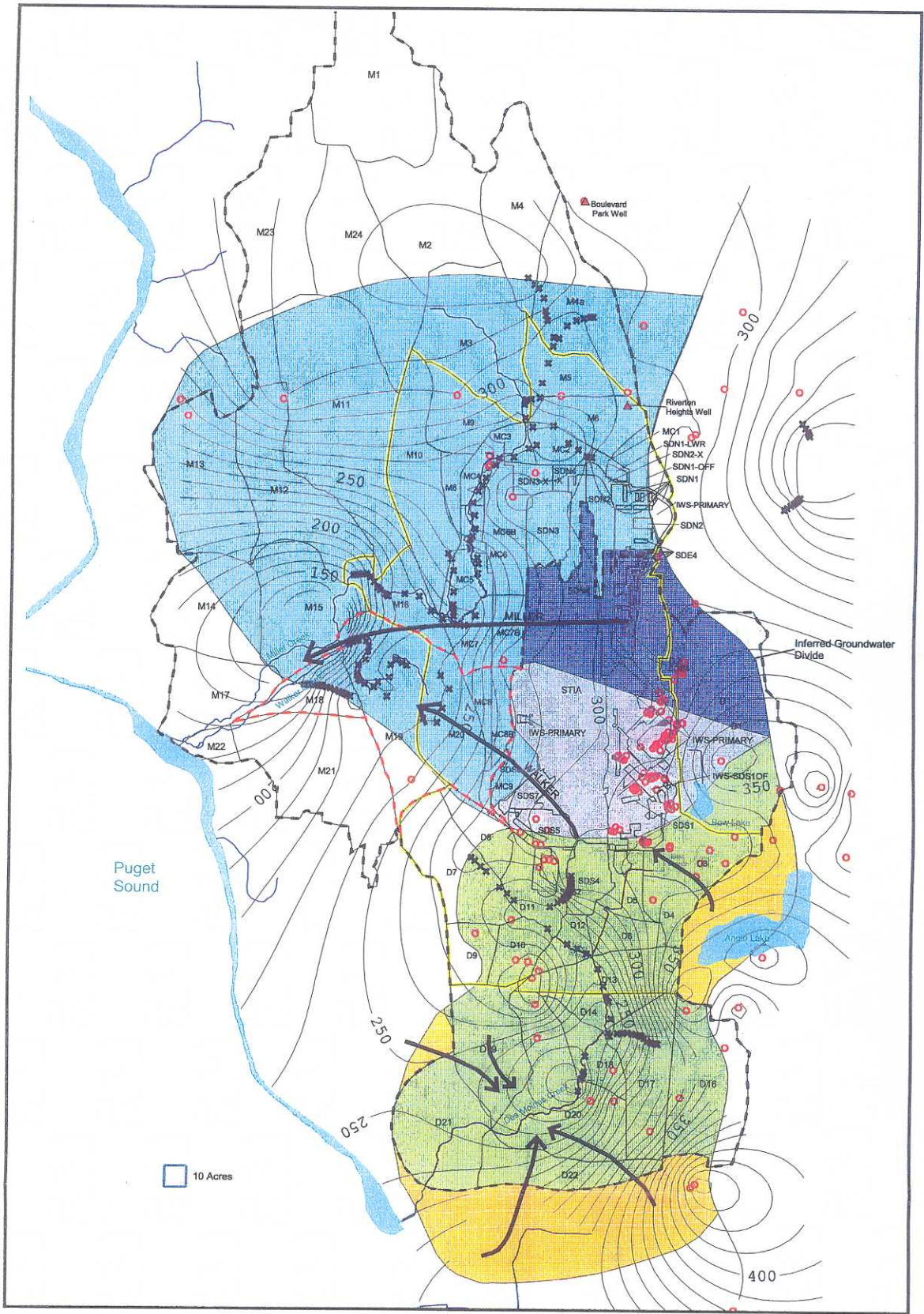
#### 3.3 FTABLES

All the stage-storage-discharge relationships from the previous model were reviewed. As a result, FTABLEs 1, 4, 10, 11, 12, 13, 14, 15, 16, 17, 34, 35, 50, 53, and 54 were modified.

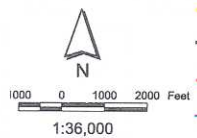
In addition to the above changes, four new FTABLES were added to the model:

- 1 Subbasin M3 detention facility. RCHRES number 33.
- 2 Vacca Farm. RCHRES number 135.
- 3 Hermes depression. RCHRES number 23.
- 4 Downstream of Hermes depression. RCHRES number 24.





Parametrix, Inc. See-Tac Airport Stormwater Management Plan/556-2912-001(28) 7/00 File: K:\GIS\2912\ArcView\wrs\stia-epdu\_may2001  
 Source: Water bodies derived from USGS hypsography data. Ground water contours from Seattle-Tacoma International Airport Ground Water Study. Associated Earth Sciences, Inc., and S. S. Papadopoulos & Assoc., 1999.  
 Note: STIA Subbasin GIS coverage obtained where conditions may change between 1994 and other conditions; subbasin boundaries shown outside of STIA area are for illustration and reference only.



- Existing (1994) Drainage Subbasins
- STIA Area (see note)
- Watershed Boundary
- Subwatershed Boundaries
- Rivers
- Roads
- Water Bodies
- Noncontiguous Miller Creek groundwater area
- Noncontiguous Walker Creek groundwater area
- Contiguous Miller/Walker groundwater area
- Noncontiguous Des Moines Creek groundwater area
- Contiguous Des Moines Creek groundwater area
- Ground water elevation contour (10 ft. interval) for shallow (C1) aquifer
- Data Point (e.g. monitoring well)
- Wells
- Locations where ground water elevation intersects stream channel
- General ground water flow direction

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 556-2912-001 (28)

**Figure B2-2  
 Groundwater  
 Flow  
 Direction and  
 Boundaries**



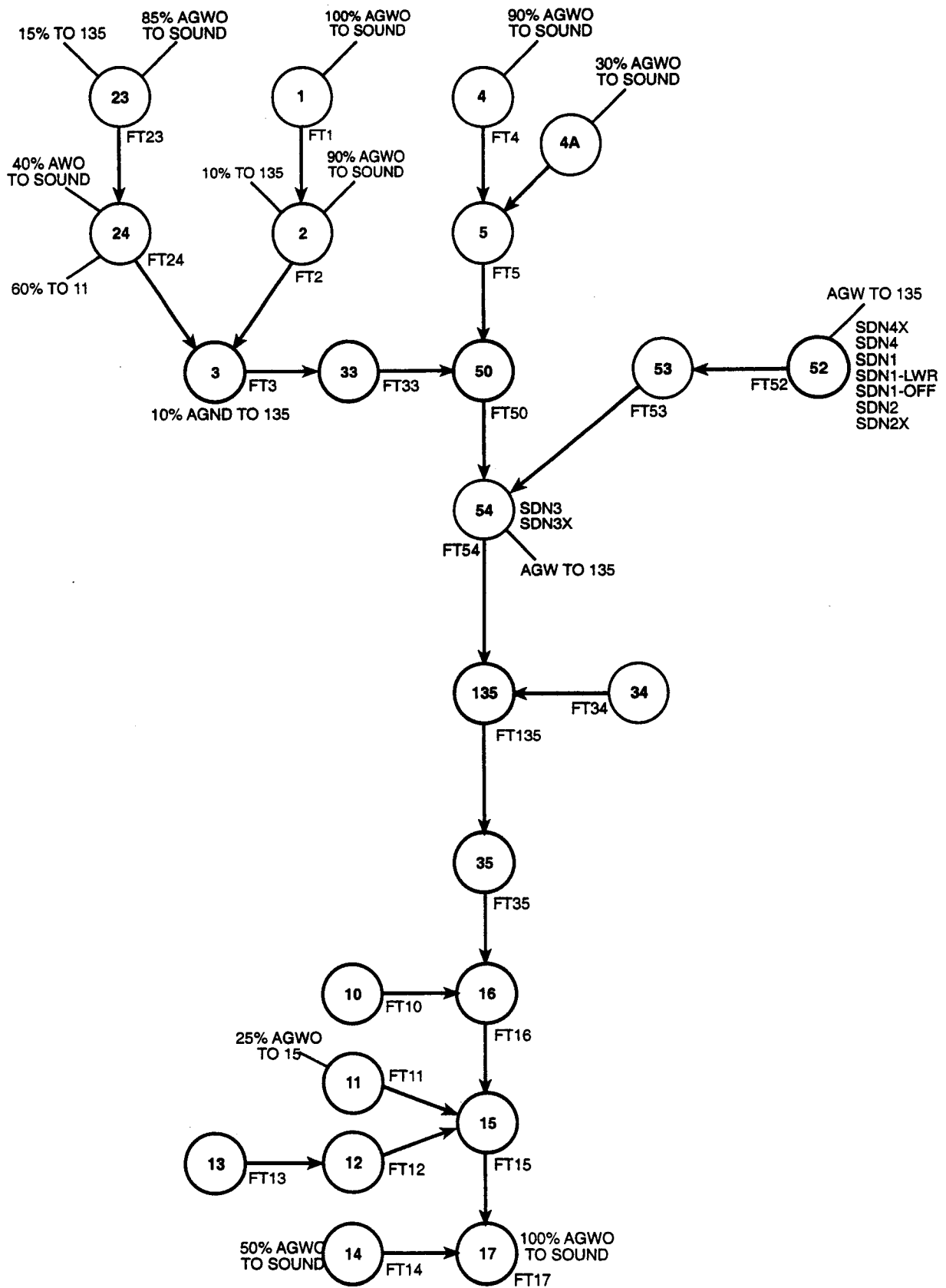
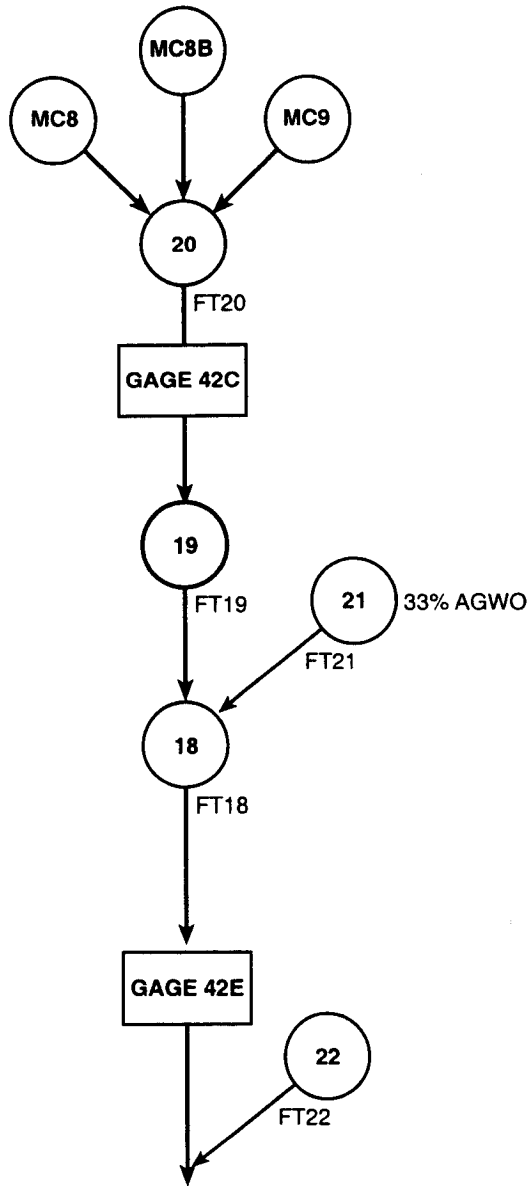


Figure B2-3a  
HSPF Model Schematic  
Miller Creek



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**Figure B2-3b**  
**HSPF Model Schematic**  
**Walker Creek**

### 3.4 ROUTING

The following changes were made to the Miller Creek model to reflect updated information about the watershed drainage characteristics:

- MC9 was routed to Lora Lake.
- MC4 & MC6 were routed downstream to Reach 35.
- MC6B was routed to Reach 35.
- MC7B was routed to Reach 35.
- M8 was routed to Reach 35.
- M6 was routed directly to Lake Reba.
- M9 was routed directly to Lora Lake.
- Reaches 51 and 52 were combined to form Reach 52, with FTABLE 52.
- MC3 was routed to Reach 54.
- M3 detention storage was modified based on field visit.
- Reach 50 was routed directly to reach 54 (MCDF).
- A new reach was added upstream of Reach 50.

### 3.5 LAKES

Arbor, Tub, and Lora Lakes are all underlain by outwash soils. Field inspection indicated that these lakes can drop several feet below the outlets during the summer dry period. This cannot be accomplished by evaporation alone. As a result, a small amount of seepage (0.11 cfs) from each of the lakes was modeled.

### 3.6 HERMES POND

Hermes Pond is a natural depression that has been modified to serve as a stormwater detention facility. All of subbasin 23 drains to this depression and there is no natural outlet. As a result, two pumps have been installed in an attempt to keep the depression from flooding. The maximum capacity of the outlet pipe is 0.5 cfs. Using this set of assumptions, the simulated elevation of the depression is much higher than the observed maximum flood stage. As with the lakes, Hermes depression is underlain by outwash soils. Using seepage in the range of 0.05 to 0.4 cfs resulted in a corresponding maximum flood stage near that which has been observed.

### 3.7 CALIBRATION PERLND PARAMETERS

Seven PERLND parameters were adjusted in the following manner:

- The LZSN value for all soils was doubled from 4.5 to 9.0 for till soils and from 5.0 to 10.0 for outwash soils. This was done to increase the ability of the soil to store moisture. The basis for this change is related to the thick layers of permeable soil overlaying the till layer for much of the till soils.
- INFILT was multiplied by four for all till soils in an effort to infiltrate more runoff.
- INFEXP was set to 2.0 for all soils.
- DEEPPFR was set to 0.33 for all soil types. This represents the loss of groundwater from the Miller Creek watershed to a deeper aquifer.
- UZSN was doubled for all soils. This increases evaporation from the soil surface and decreases summer and fall peaks. This adjustment improved the overall simulation performance during these time frames.
- INTFW was set to nine for till soils. This reduces the amount of surface runoff, again reducing peak flows.
- IRC has been set to 0.7 for all till soils.

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## 4. MILLER CREEK CALIBRATION RESULTS

### 4.1 MILLER CREEK STREAMFLOW AT THE MOUTH

Calibration was performed for water-years 1993 through 1996. Total simulated runoff volume for the calibration period was within 0.75 inch of the total observed volume (Table B2-8). Simulated runoff volumes for water-years 1994 and 1996 are within 0.60 inch of observed runoff volumes. Simulated runoff volumes for water-years 1993 and 1995 are within 2.45 inches of observed runoff volumes. However, the streamflow gage was down for 4 months during water-year 1995. Examination of the recorded data after a gage malfunction in December and January of water-year 1995 indicates possible disruption in the reliability of the recorded data for the months of January through May. The gage again malfunctioned starting in June of 1995 and lasting until the middle of October 1995. These chronic problems during water-year 1995 call into question the overall reliability of the gaged streamflow data for this water-year. Figures B2-4 through B2-7 show the annual simulated and observed streamflow for water-years 1993 through 1996 at the mouth of Miller Creek.

### 4.2 MILLER CREEK STREAMFLOW DOWNSTREAM OF THE REGIONAL DETENTION FACILITY.

Calibration was performed for water-years 1993 through 1996. Total simulated runoff volume for the calibration period is within 0.9 inch of the total observed volume. Simulated runoff volumes for three of the four water-years are higher than observed runoff volumes. The largest over-simulation error is 0.56 inch, which occurs in water-year 1993. All other water-years are within 0.44 inch of observed volumes. Figures B2-8 through B2-11 show the annual simulated and observed streamflow for water-years 1993 through 1996 downstream of the Miller Creek Regional Detention Facility (RDF).

### 4.3 PEAK FLOW EVENTS

Five peak flow events have been examined during the calibration process and are listed below:

- January 20, 1993 through January 31, 1993
- March 16, 1993 through March 31, 1993
- February 16, 1994 through March 15, 1994
- November 21, 1995 through December 21, 1995
- January 1, 1996 through January 31, 1996

Calibration plots from these events are listed in Figures B2-12 through B2-21.

No events from water-year 1995 were used as a result of the documented malfunctions of the stream gage during this time period. Although the February 1996 flood event is the largest of the calibration period, it was not used because of the manipulation of the outlet gate from the RDF during this event.

Simulated streamflows at both the mouth of Miller Creek and downstream of the RDF generally match the timing, shape, and magnitude of the observed streamflows. Some events such as the January 1993 event match very well, while others such as the January 1996 event do not. However, close examination of the January 1996 event shows some potential errors in the observed data. Figure B2-22 shows the observed streamflow for Miller Creek at both the downstream end of the RDF and at the mouth. This figure clearly indicates that observed streamflows at the RDF are higher than at the mouth. This is obviously not possible, so as a result, the team concluded that some error in the observed data is likely. A comparison of observed flows at the RDF and at the mouth reveals that peak flows at the mouth are generally three or four times higher than peak flows at the RDF. If the ratio drops to below two, then the observed streamflow can be called into question.

**Table B2-8. Miller Creek runoff volumes: calibration period water-years 1993 through 1996.**

Water-Year	Observed Streamflow Volumes		Simulated Streamflow Volumes		Simulated Minus Observed	
	Regional Detention Facility (inches)	Mouth (inches)	Regional Detention Facility (inches)	Mouth (inches)	Regional Detention Facility (inches)	Mouth (inches)
1993	5.41	8.53	5.97	10.53	0.56	2.00
1994	3.66	7.79	3.43	7.22	-0.23	-0.57
1995	6.60	12.02	6.85	9.68	0.25	-2.34
1996	13.64	20.94	14.25	21.65	0.61	0.71
Total	29.31	49.28	30.50	49.08	1.19	-0.20

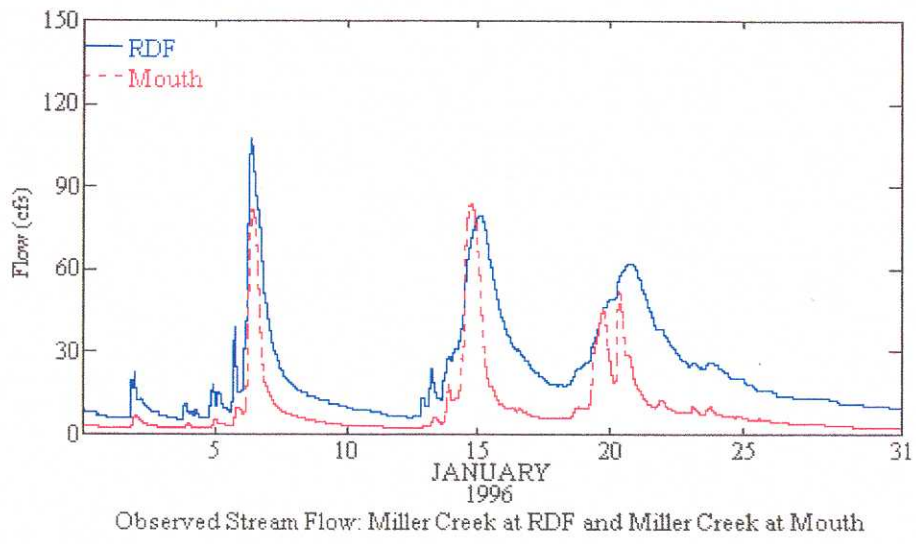


Figure B2-22



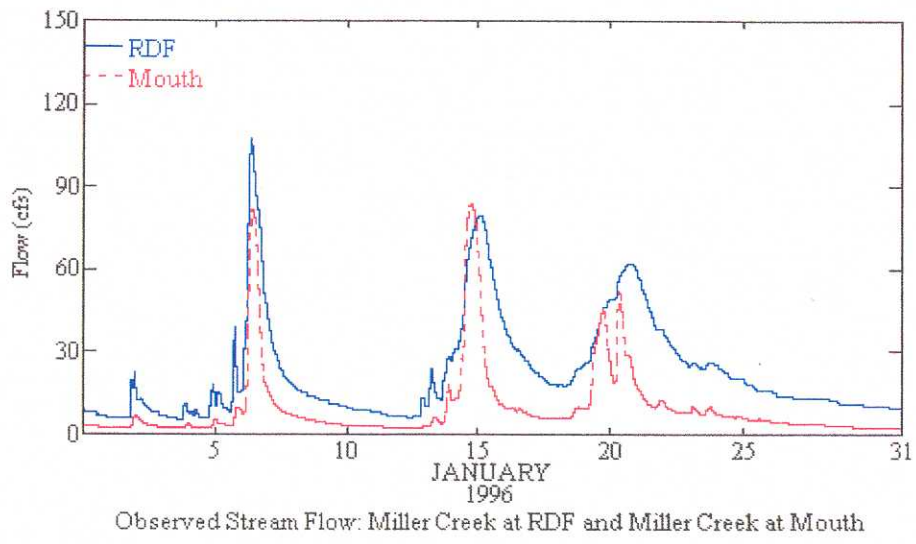


Figure B2-22

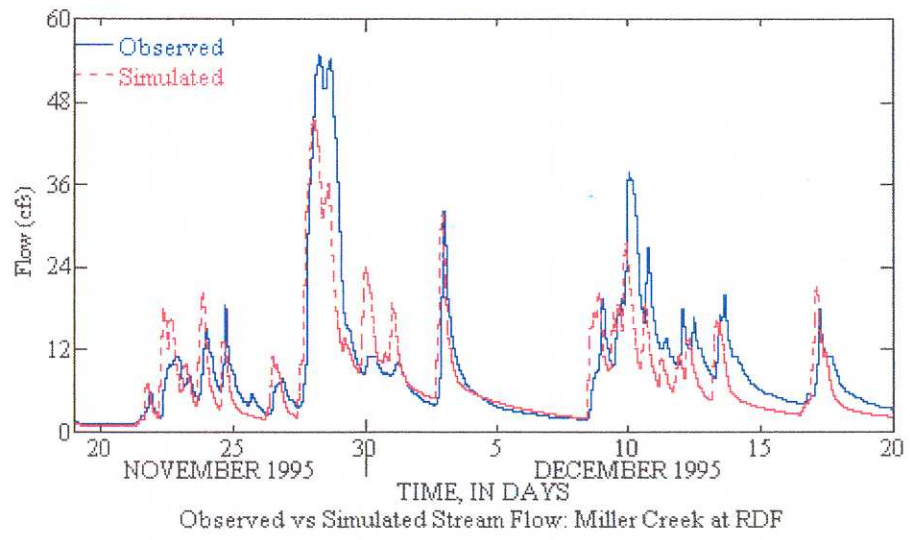


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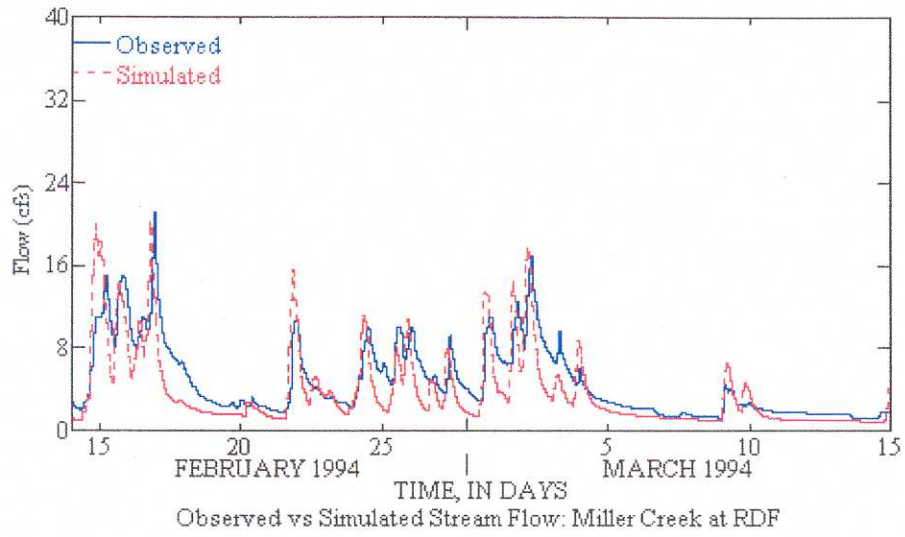


Figure B2-19

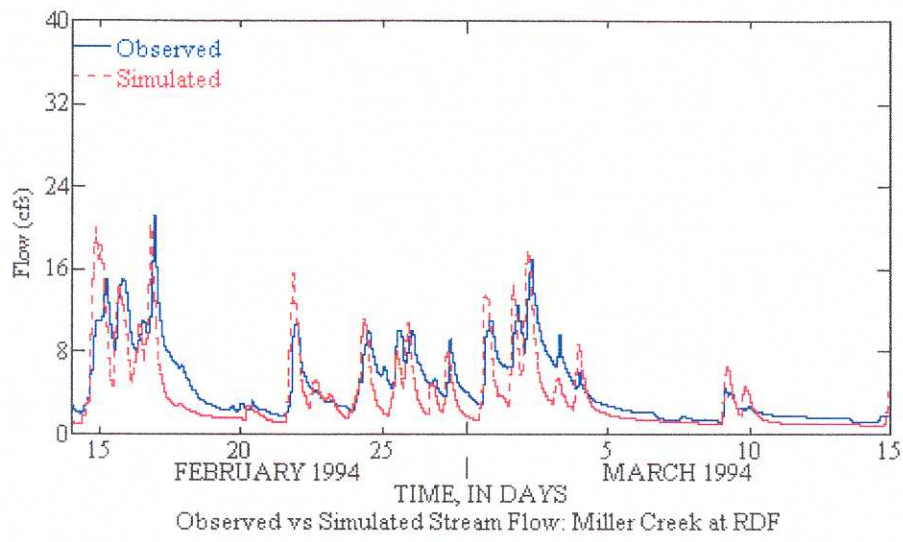


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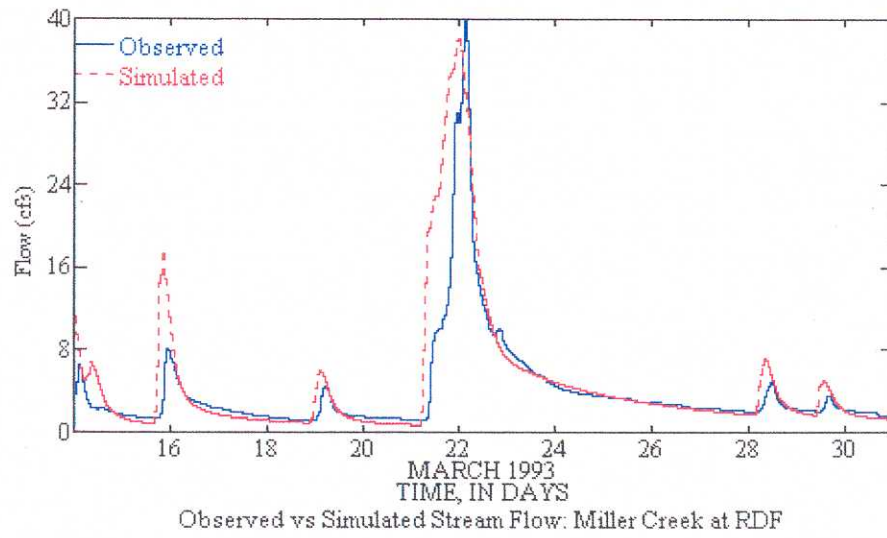


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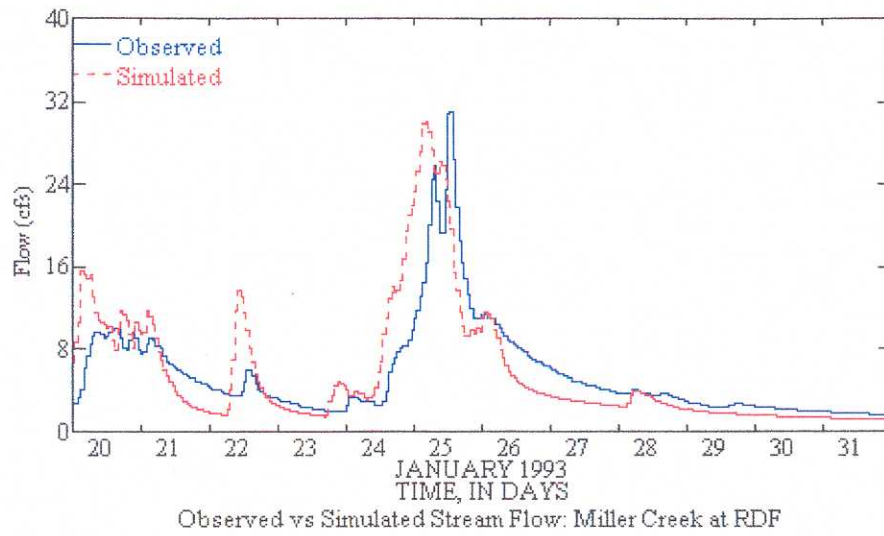


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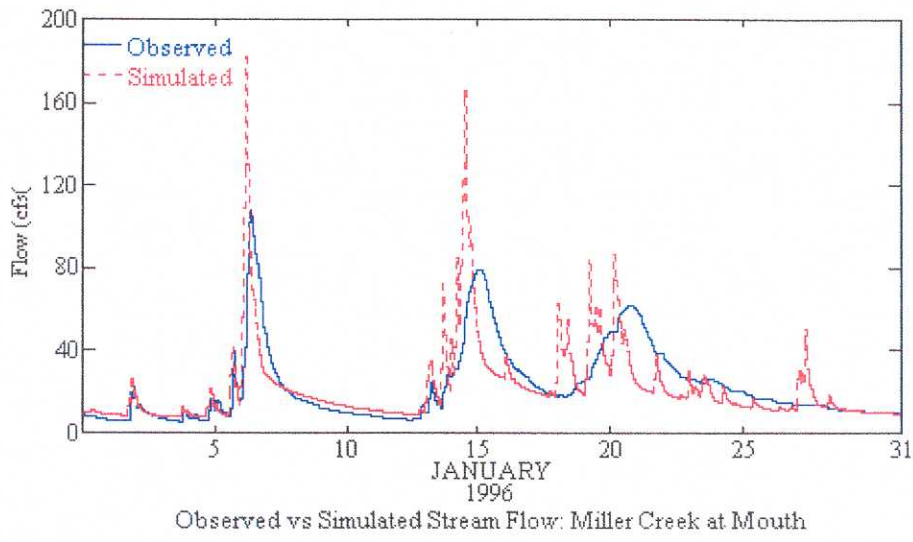


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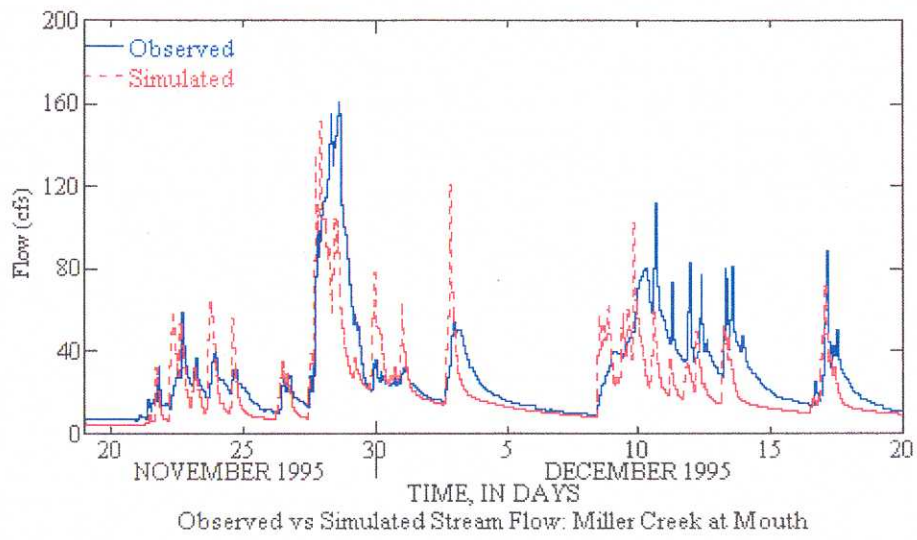


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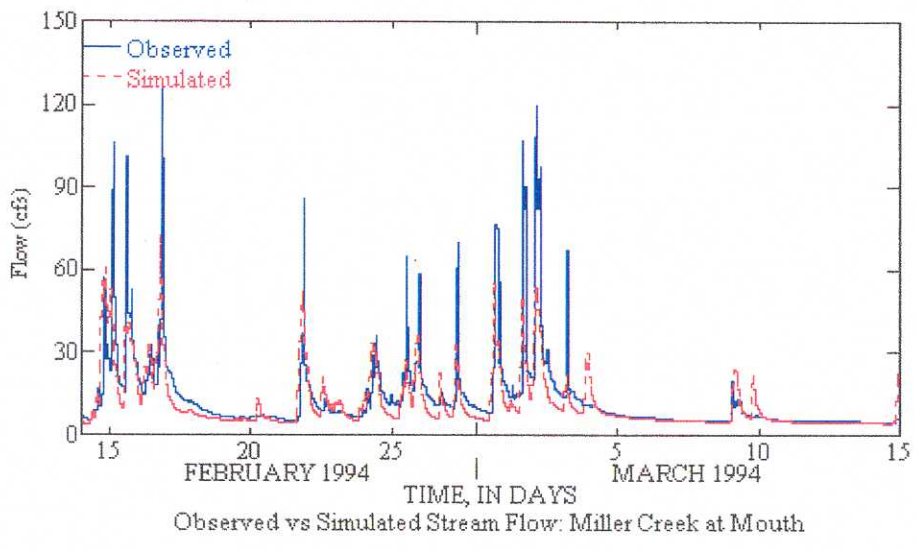


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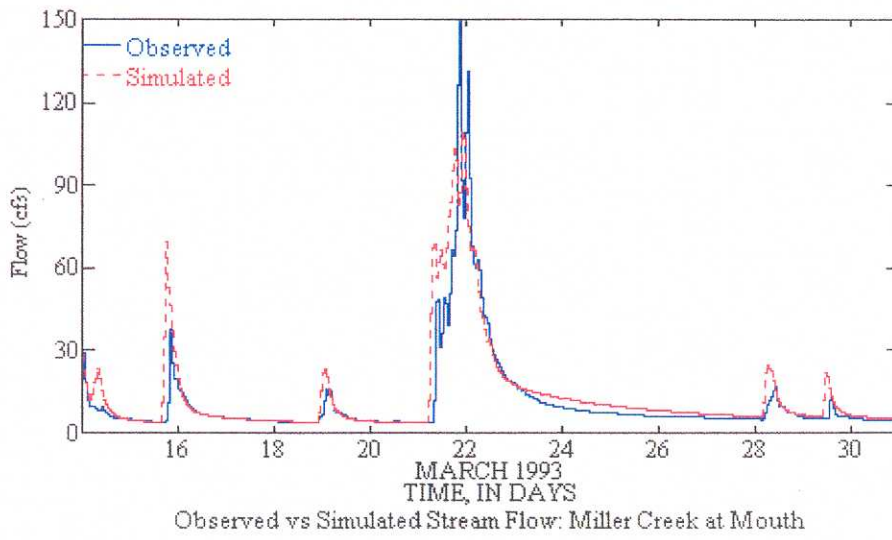


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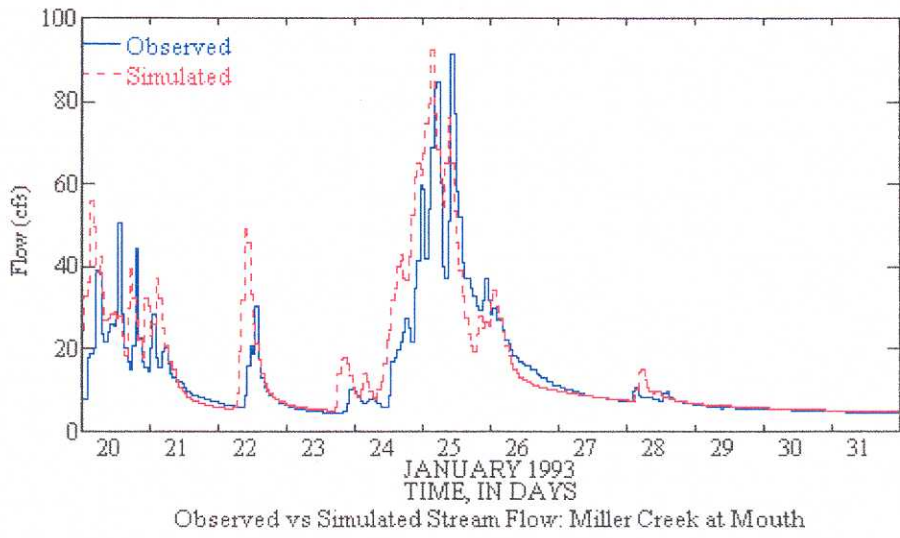


Figure B2-12

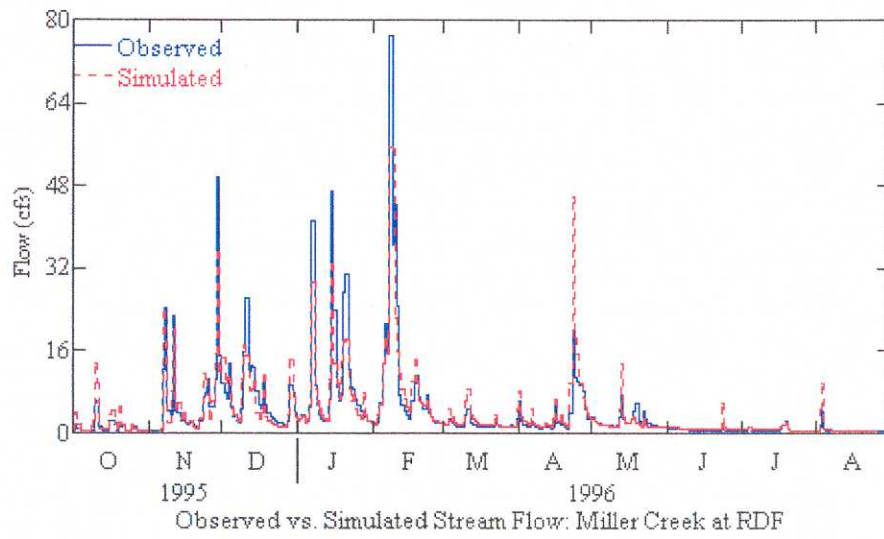


Figure B2-11

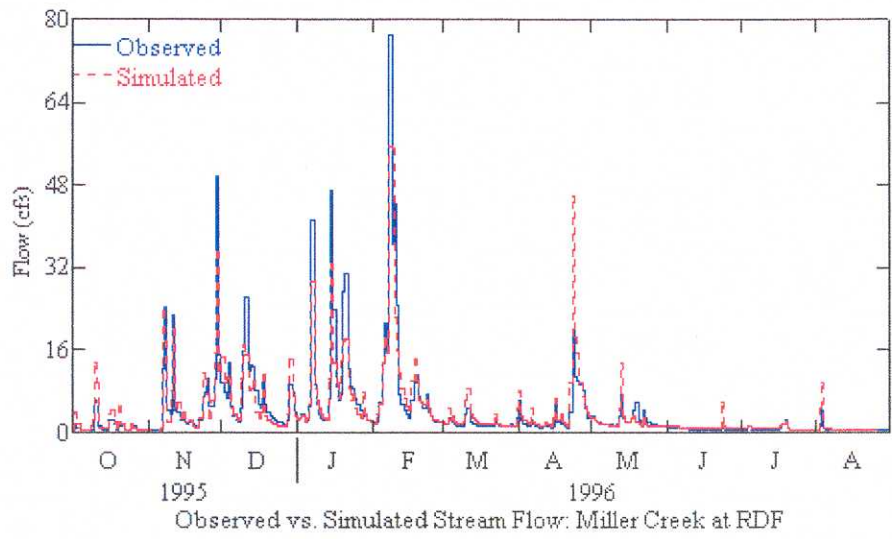


Figure B2-11

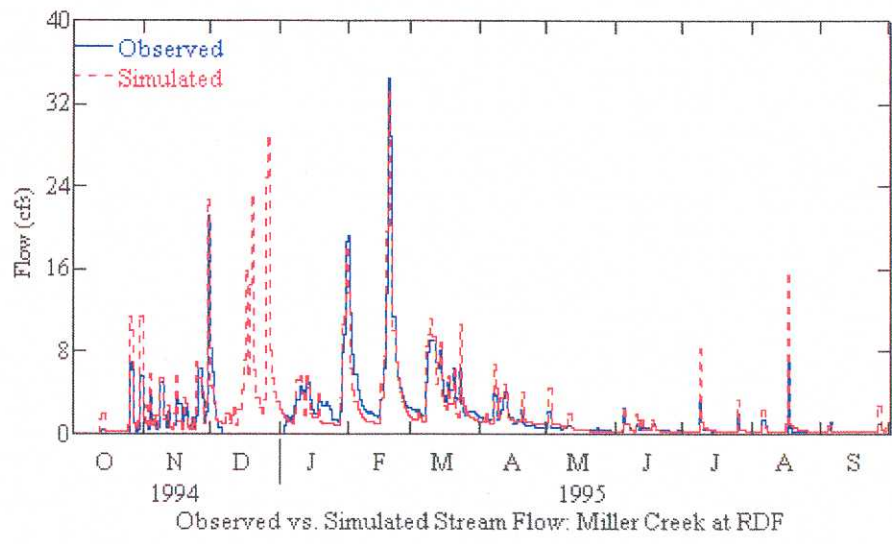


Figure B2-10

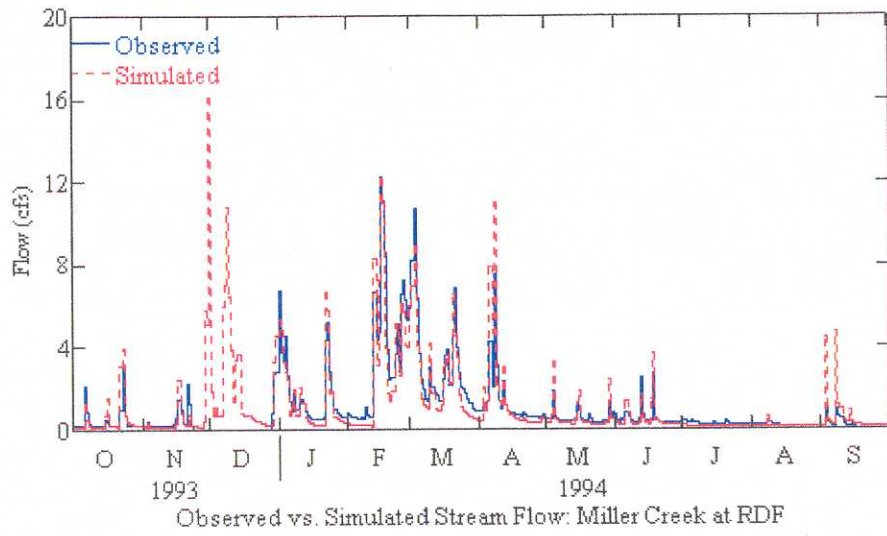


Figure B2-9

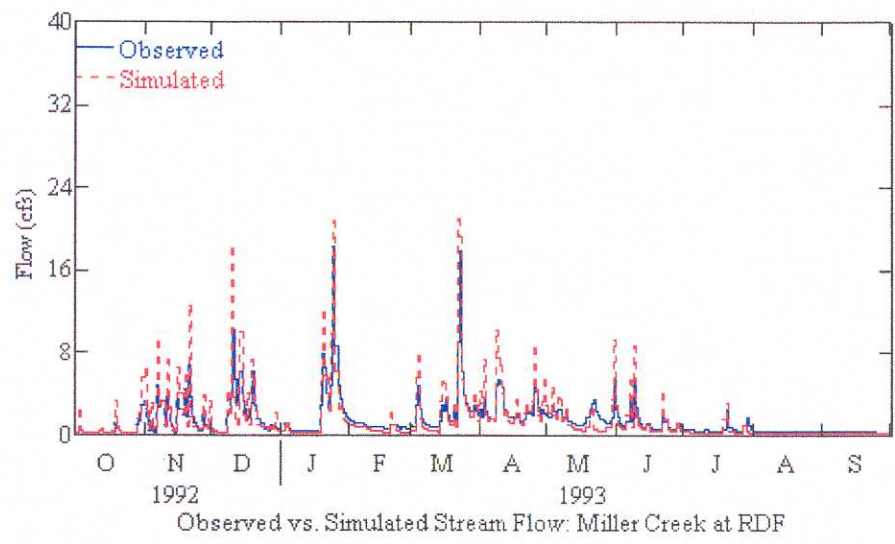


Figure B2-8



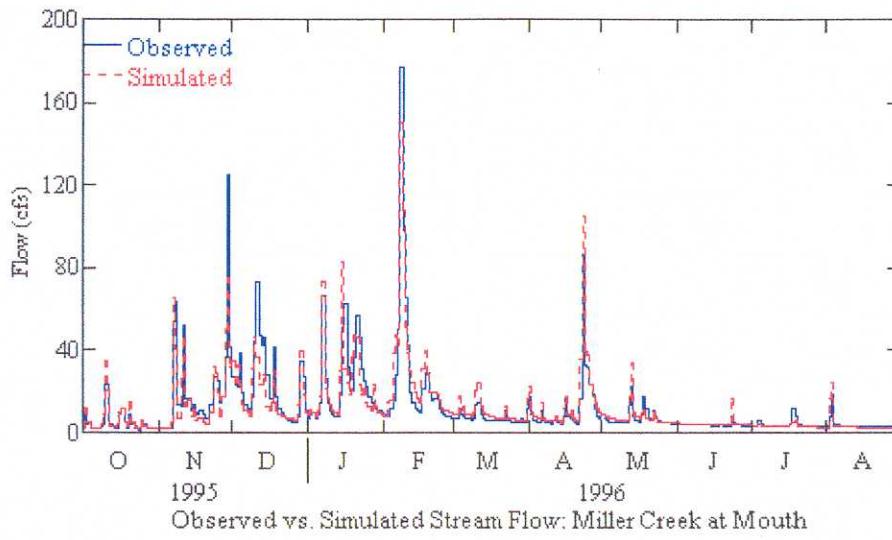


Figure B2-7

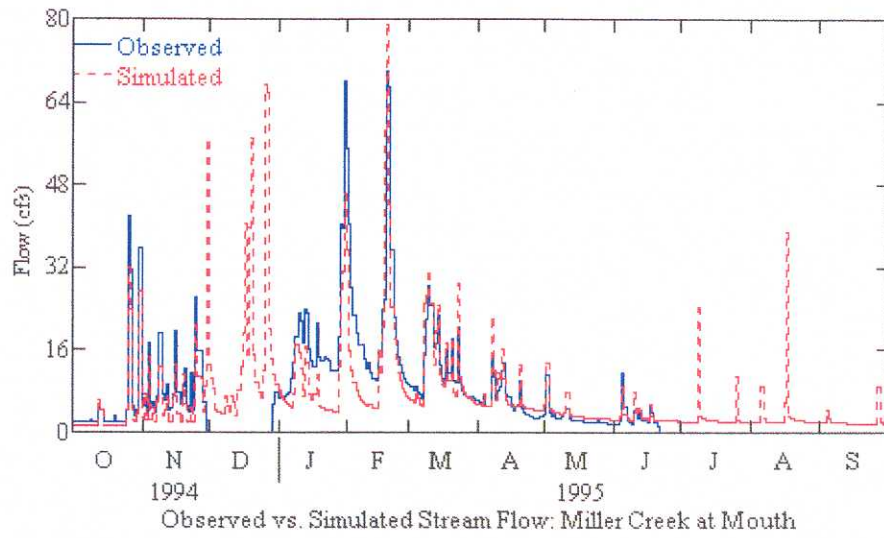


Figure B2-6

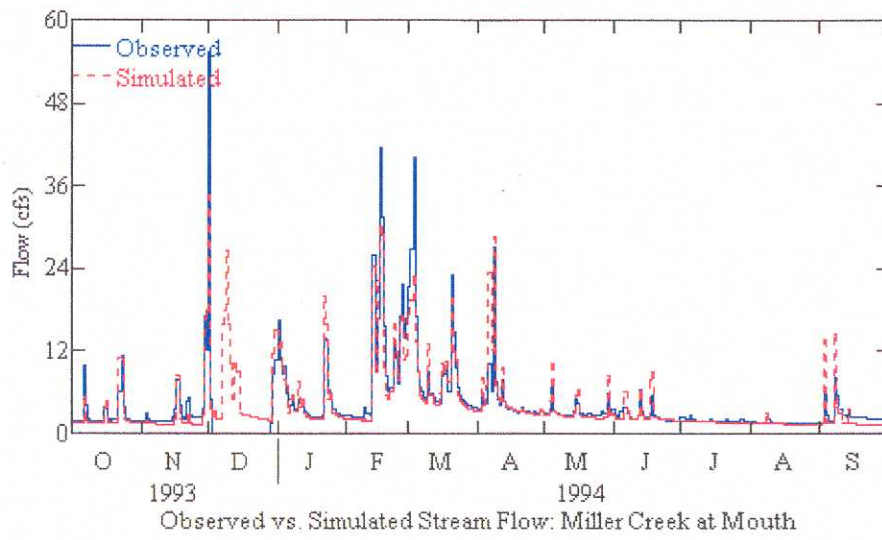


Figure B2-5

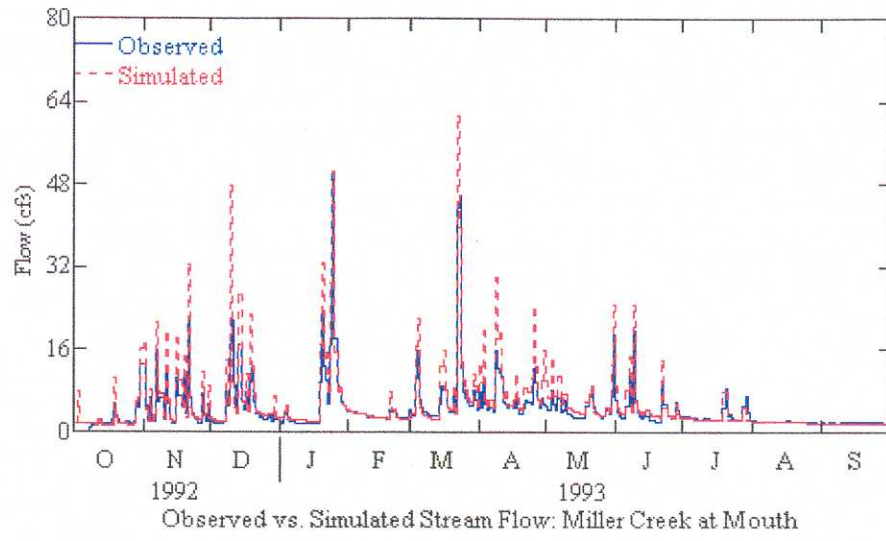


Figure B2-4

## 5. WALKER CREEK

### 5.1 WATERSHED BOUNDARIES

The Walker Creek watershed boundary has been verified and will be used without any alterations. However, based on topographical mapping, one-third of the groundwater from subbasin M21 has been routed upstream of the Walker Creek gage (Figure B2-23).

### 5.2 GROUNDWATER ROUTING

As with the Miller Creek watershed, groundwater plays a key role in properly calibrating the Walker Creek watershed. Using the standard assumption that all groundwater generated within the Walker Creek surface water boundaries drains to Walker Creek upstream of the gage site, simulated streamflows at the mouth of Walker Creek are approximately only half that of observed streamflows. This clearly indicates that Walker Creek is receiving groundwater from outside the surface water boundaries. Groundwater mapping (see Figure B2-2) indicates that approximately 630 acres of the Des Moines Creek watershed contribute groundwater to the Walker Creek watershed. As a result, 630 acres of till-grass soil has been added to the Walker Creek model. This PERLND uses an INFILT of 0.12, while the rest of the parameters correspond to regional values. Only the groundwater from this PERLND contributes to the Walker Creek watershed.

### 5.3 WALKER CREEK WETLAND

The Walker Creek wetland controls all of the runoff from subbasins MC-8, MC-8B, MC-9, and 20. There is a semi-blocked culvert that carries flows under Des Moines Way. Exact flow capacity of the culvert is unknown; however, using streamflow gage 42c, a rough approximation of the capacity is that of a 36-inch culvert.

### 5.4 FTABLES

All the stage-storage-discharge relationships from the previous model were reviewed. As a result, FTABLEs 18, 19, and 20 were modified.

FTABLE 20 represents a large wetland at the headwaters of Walker Creek. The stage-storage-discharge relationship for this wetland is based on previous modeling efforts and is believed to be accurate.

### 5.5 CALIBRATION PERLND PARAMETERS

Four PERLND parameters were adjusted in an effort to obtain an acceptable calibration:

- INFEXP was set to 2.0 for all soils.
- AGWRC was set to 0.996.

**AR 011005**

- INTFW was set to two for till soils. This moves most of the surface runoff away from interflow.
- IRC was set to 0.15 for all soils. This allows the interflow to respond rapidly, causing higher peak flows.

## 5.6 WALKER CREEK CALIBRATION RESULTS

Calibration was performed for water-years 1993 through 1996. Total simulated runoff volume at the mouth of Walker Creek for the calibration period is within 3.28 inches of the total observed volume (Table B2-9). Simulated runoff volumes for water-years 1994, 1995, and 1996 are within 0.7 inch of observed runoff volumes. The simulated runoff volume for water-year 1993 is below the observed runoff volume by 2.86 inches, which represents most of the volume deficiency. However, the streamflow gage installed in September 1992 malfunctioned at different times during water-years 1993 and 1994. This led the team to question the accuracy of the observed data for water-year 1993. Figures B2-24 through B2-27 show the annual simulated and observed streamflow for water-years 1993 through 1996 at the mouth of Walker Creek. Total simulated runoff volume at the upper gage of Walker Creek for the calibration period is within 1.36 inches of the total observed volume (Table B2-9). Figures B2-28 through B2-31 show the annual simulated and observed streamflow for water-years 1993 through 1996 at the upper Walker Creek gage.

## 5.7 PEAK FLOW EVENTS

Five peak flow events have been examined during the calibration process and are listed below:

- February 5, 1996 through February 10, 1996
- February 13, 1995 through February 28, 1995
- December 13, 1994 through December 31, 1994
- April 2, 1996 through April 30, 1996
- November 2, 1995 through November 30, 1995

Calibration plots from these events are listed in Figures B2-32 through B2-41.

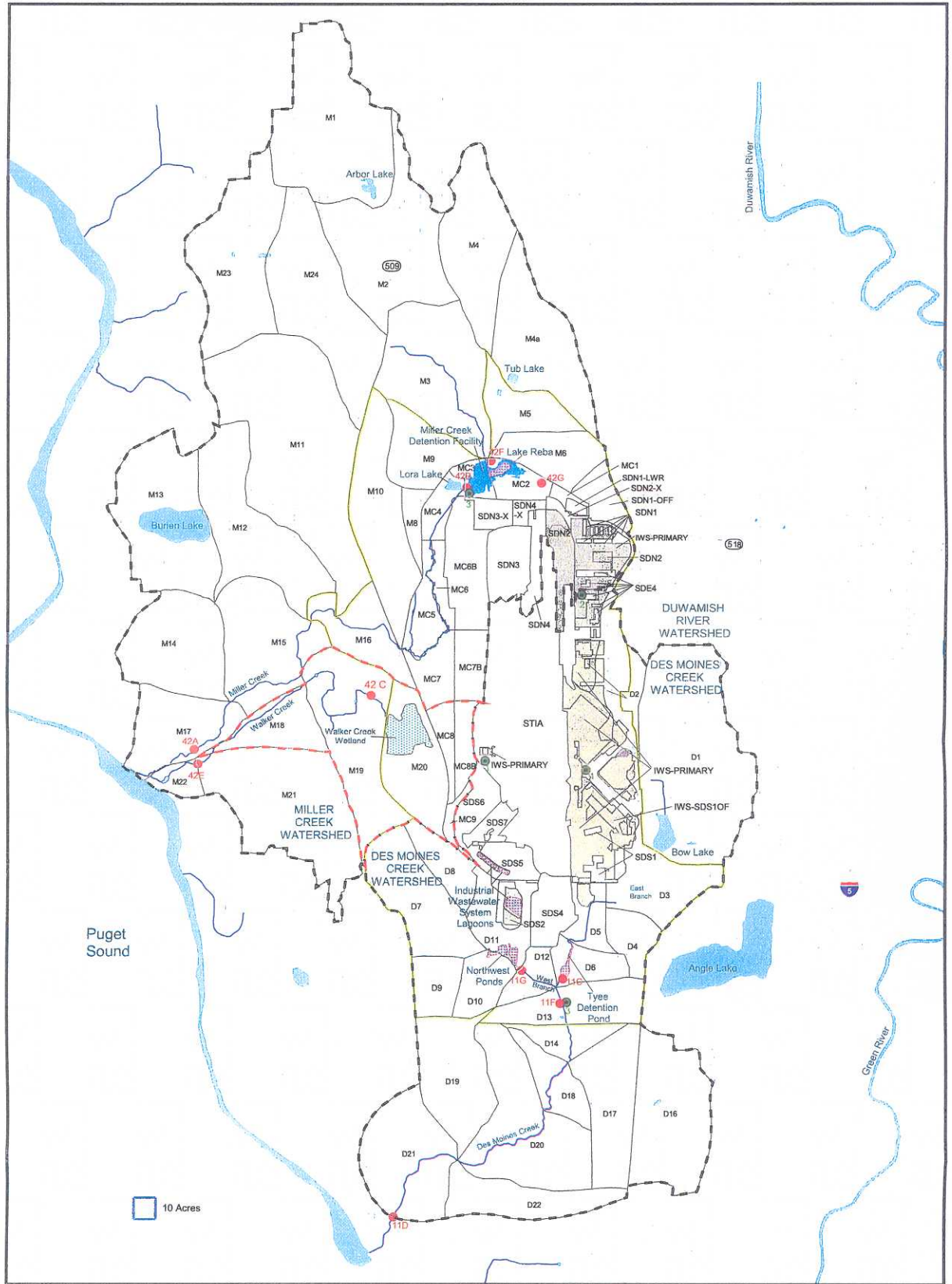
No events from water-year 1993 were used as a result of the documented malfunctions of the stream gage during this time period.

Simulated streamflows at the mouth of Walker Creek generally match the timing and shape of the observed streamflows. However, the peak flows for water-years 1993 and 1994 are generally too low at both the upper gage and at the mouth. Peak streamflows for water-years 1995 and 1996 are much closer to observed flows for both gage locations.

In addition to the recorded streamflow data at the upper Walker Creek gage 42c, hand-measured data points were recorded for this same location. These data points were compared to the simulated streamflow data and the recorded streamflow data for each of the days that data exists (Table B2-10). Since hand-recorded streamflow data can occur at any time during the day, it should not be expected that these data points should match exactly. A comparison of the daily average flow and the daily maximum flow against the hand-measured streamflow shows that for summer low flow

conditions the numbers are nearly identical, but for winter peak flow events the numbers vary greatly. Overall, the hand-measured streamflows, the recorded or (gaged) streamflows, and the simulated streamflows exhibit a fairly good match, indicating that the model has replicated general streamflow characteristics.

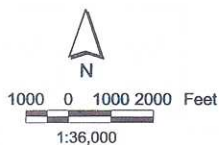
**AR 011007**



Parametrix, Inc. Sea-Tac Airport Stormwater Management Plan/556-2912-001(28) 6/00 File: K:\GIS\2912\Arcview\rsasetac-apdx.mxd/2001.apr  
 Source: Roads based on King County data. Water bodies derived from USGS hypsography data. Detention boundaries are approximate.  
 Note: Subbasin boundaries shown outside of STIA area are for illustration and reference only.  
 STIA subbasins assume existing (1994) conditions.

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- |  |  |   |
|--|--|---|
| <ul style="list-style-type: none"> <li>— Roads</li> <li>— Existing (1994) Drainage Subbasins</li> <li>— STIA Area (see note)</li> <li>▨ Constructed Water Features</li> <li>▨ IWS Drainage Area</li> </ul> | <ul style="list-style-type: none"> <li>- - - Subwatershed Boundary</li> <li>- - - Watershed Boundary</li> <li>— Rivers</li> <li>▨ Water Bodies</li> <li>▨ Detention Facilities (existing)</li> </ul> | <ul style="list-style-type: none"> <li>● Precipitation Gaging Stations:           <ul style="list-style-type: none"> <li>Type 1 - National Weather Service (Gage relocated 1996)</li> <li>Type 2 - POS Rainfall Monitoring</li> <li>Type 3 - King County Rainfall Monitoring</li> </ul> </li> <li>● Streamflow King County Gaging Stations:           <ul style="list-style-type: none"> <li>42A - Miller Ck @ SW 175th Pl &amp; 12th Ave SW</li> <li>42B - Miller Ck @ Lake Reba/RDF Outlet</li> <li>42C - Walker Ck @ 171st Pl</li> <li>42E - Walker Ck @ 12th Ave SW</li> <li>42F - Miller Ck @ SR516</li> <li>42G - Miller Ck @ East Branch</li> <li>11D - Des Moines Ck near mouth</li> <li>11F - Des Moines Ck @ Golf Course</li> <li>11G - Des Moines Ck @ Tyeck Pond</li> <li>11G - Des Moines Ck @ NW Ponds</li> </ul> </li> </ul> |
|--|--|---|

**Figure B2-23**  
**Walker Creek**  
**Subbasin Boundary**



**Table B2-9. Walker Creek runoff volumes: calibration period water-years 1993 through 1996.**

Walker Creek at mouth runoff volumes: calibration period water-years 1993 through 1996.

<b>Water-Year</b>	<b>Observed Mouth (inches)</b>	<b>Simulated Mouth (inches)</b>	<b>Simulated minus Observed Mouth (inches)</b>
1993	15.64	12.78	-2.86
1994	11.41	10.7	-0.71
1995	17.54	17.59	0.05
1996	24.04	24.28	0.24
<b>Total</b>	<b>68.63</b>	<b>65.35</b>	<b>-3.28</b>

Walker Creek upper gage runoff volumes: calibration period water-years 1993 through 1996.

<b>Water-Year</b>	<b>Observed Mouth (inches)</b>	<b>Simulated Mouth (inches)</b>	<b>Simulated minus Observed Mouth (inches)</b>
1993	9.64	8.16	-1.48
1994	8.38	6.75	-1.63
1995	9.01	9.43	0.42
1996	11.09	12.42	1.33
<b>Total</b>	<b>38.12</b>	<b>36.76</b>	<b>-1.36</b>

**AR 011009**

**Table B2-10. Spot field-measured flows for Walker Creek gage 42c (upper Walker Creek gage).**

Field Measured Data		Observed Gage 42C		Simulated Data	
Date	Flow (cfs)	Daily Average (cfs)	Hourly Max For Day (cfs)	Daily Average (cfs)	Hourly Max For Day (cfs)
09/06/90	0.93	N/A	N/A	N/A	N/A
10/07/90	0.94	1.04	1.14	0.70	0.71
11/06/90	1.23	1.27	1.30	0.86	0.86
11/09/90	19.15	11.94	23.00	9.32	17.76
02/05/91	2.69	3.93	10.80	2.99	5.15
03/26/91	1.47	1.49	1.60	2.21	2.25
04/04/92	24.71	22.35	26.00	16.26	21.21
05/17/91	1.36	1.30	1.40	1.55	1.62
09/05/91	1.32	1.34	1.40	0.95	0.96
10/14/91	1.37	1.40	1.40	0.77	0.77
11/14/91	1.57	1.55	1.60	0.8	0.81
11/19/91	6.74	3.78	7.10	2.61	3.83
12/05/91	11.51	5.89	11.00	4.71	7.15
01/29/92	10.76	6.77	14.00	4.51	8.61
01/30/92	4.25	5.71	9.50	4.93	7.23
03/19/92	1.46	1.55	1.60	1.76	1.79
04/16/92	2.39	2.09	4.90	3.37	10.65
05/22/92	1.38	1.20	1.20	1.2	1.21
08/05/92	1.14	1.30	1.30	0.81	0.81
09/10/92	1.31	1.24	1.40	0.73	0.73
11/17/92	1.04	1.92	3.00	1.44	3.28
02/10/93	1.21	1.29	1.30	1.44	1.47
03/16/93	1.25	1.97	5.10	2.37	6.88
04/16/93	1.25	1.62	1.70	1.88	2.49
09/16/93	0.76	0.90	0.90	0.74	0.74
10/26/93	0.97	1.20	1.20	0.7	0.70
11/30/93	1.52	1.96	2.92	1.84	7.48
01/19/94	0.98	1.30	1.30	1.06	1.07
02/14/94	1.82	3.11	4.48	1.59	2.39
04/21/94	1.35	0.93	0.93	1.5	1.56
07/18/94	0.68	1.00	1.00	0.79	0.79
08/25/94	0.88	0.82	0.82	0.67	0.68
09/22/94	0.84	0.87	0.89	0.65	0.66
10/20/94	0.94	0.88	0.89	0.63	0.66

**AR 011010**

**Table B2-10. Spot field-measured flows for Walker Creek gage 42c (upper Walker Creek gage) (continued).**

<b>Field Measured Data</b>		<b>Observed Gage 42C</b>		<b>Simulated Data</b>	
<b>Date</b>	<b>Flow (cfs)</b>	<b>Daily Average (cfs)</b>	<b>Hourly Max For Day (cfs)</b>	<b>Daily Average (cfs)</b>	<b>Hourly Max For Day (cfs)</b>
01/11/95	2.36	2.41	3.00	2.61	3.40
02/09/95	1.34	1.43	1.60	2	2.03
05/25/95	1.32	1.30	1.30	1.16	1.17
07/10/95	1.43	1.36	2.40	1.04	1.19
08/08/95	1.16	1.16	1.25	0.86	0.86
09/13/95	0.81	0.72	0.72	0.79	0.79
10/20/95	3.17	1.50	3.03	1.77	4.21
01/05/96	1.21	1.22	1.30	2.68	3.58
04/10/96	1.21	1.23	1.30	1.86	1.88

**AR 011011**

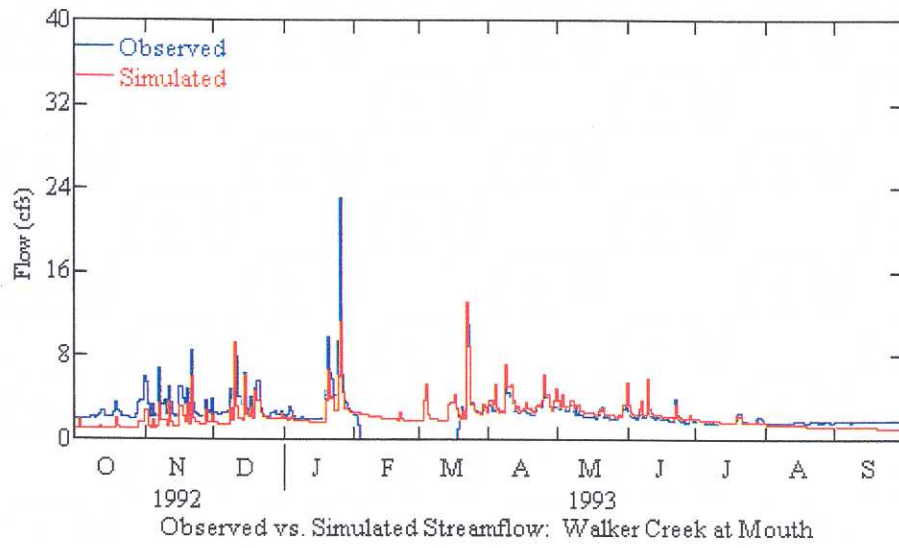


Figure B2-24

**AR 011012**

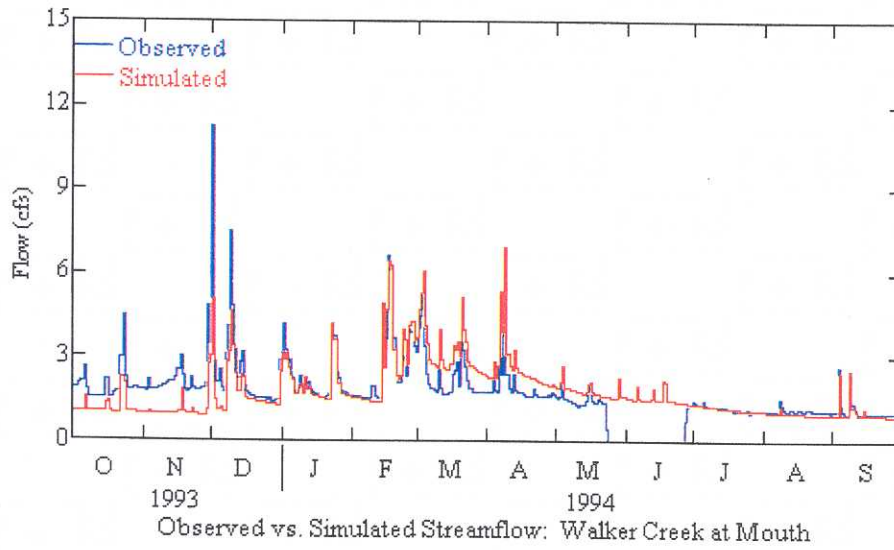


Figure B2-25

**AR 011013**

July 2001  
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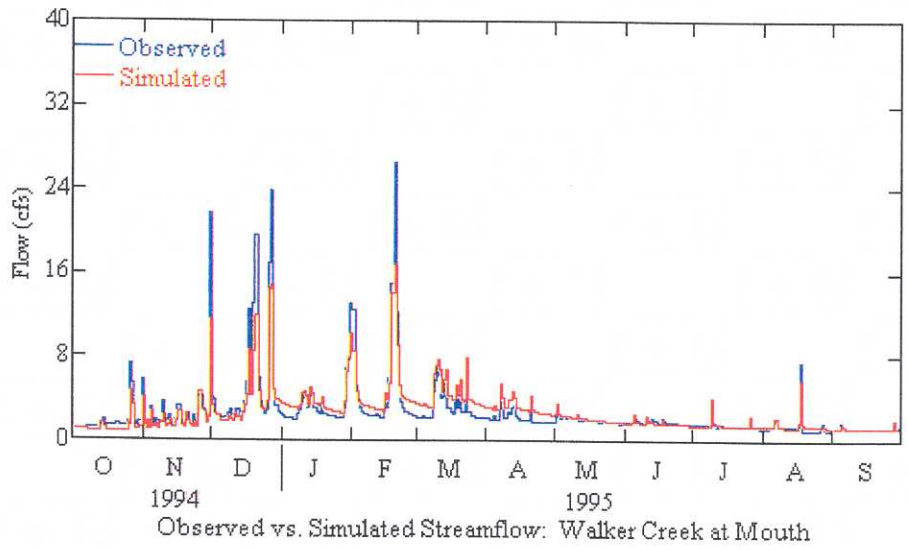


Figure B2-26

**AR 011014**

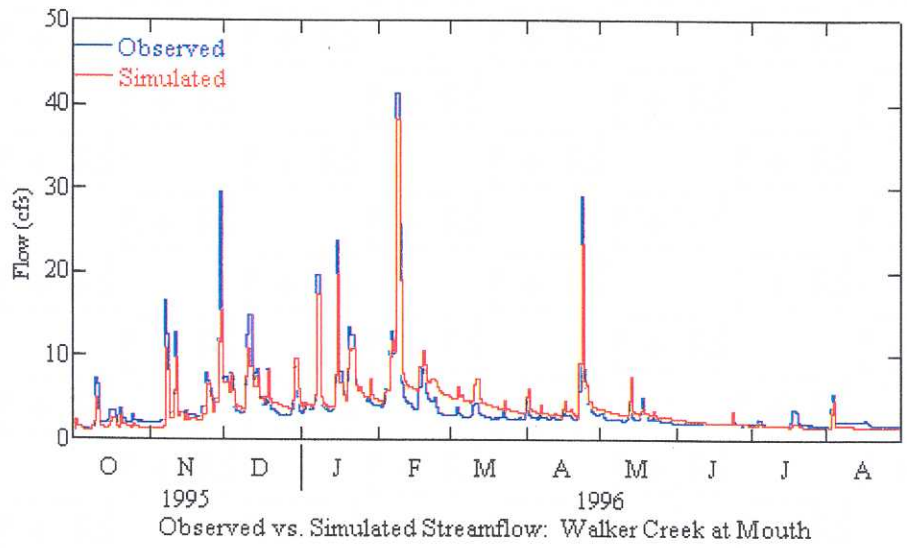


Figure B2-27

**AR 011015**

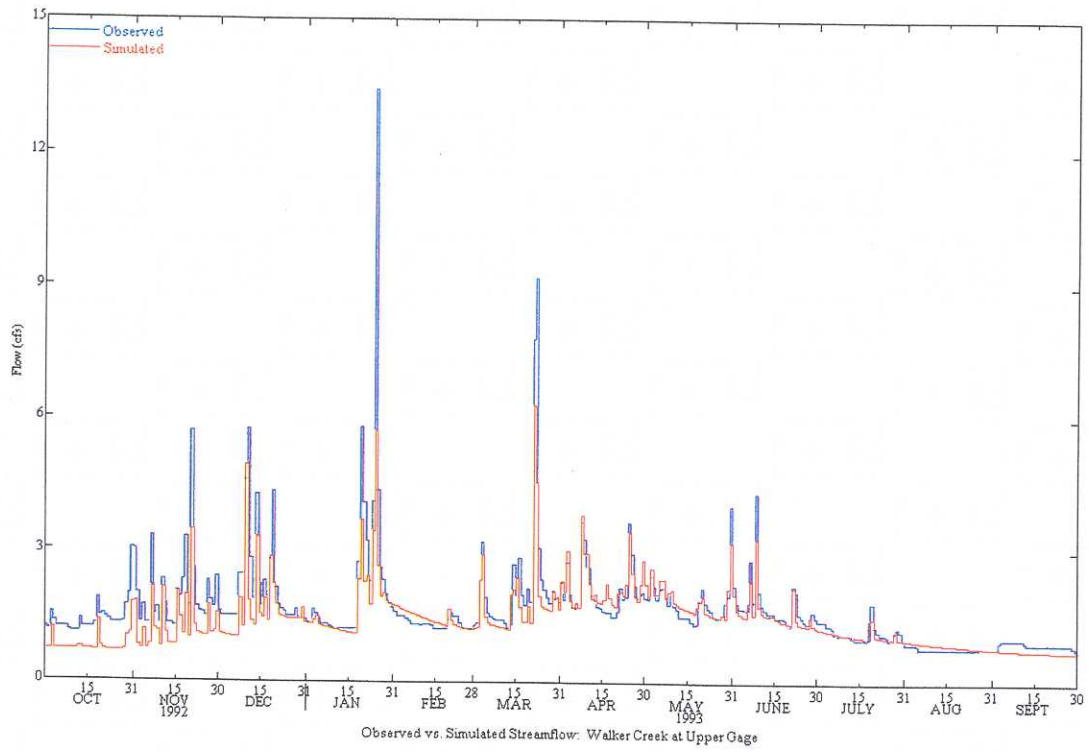


Figure B2-28

AR 011016

July 2001  
556-2912-001 (28)



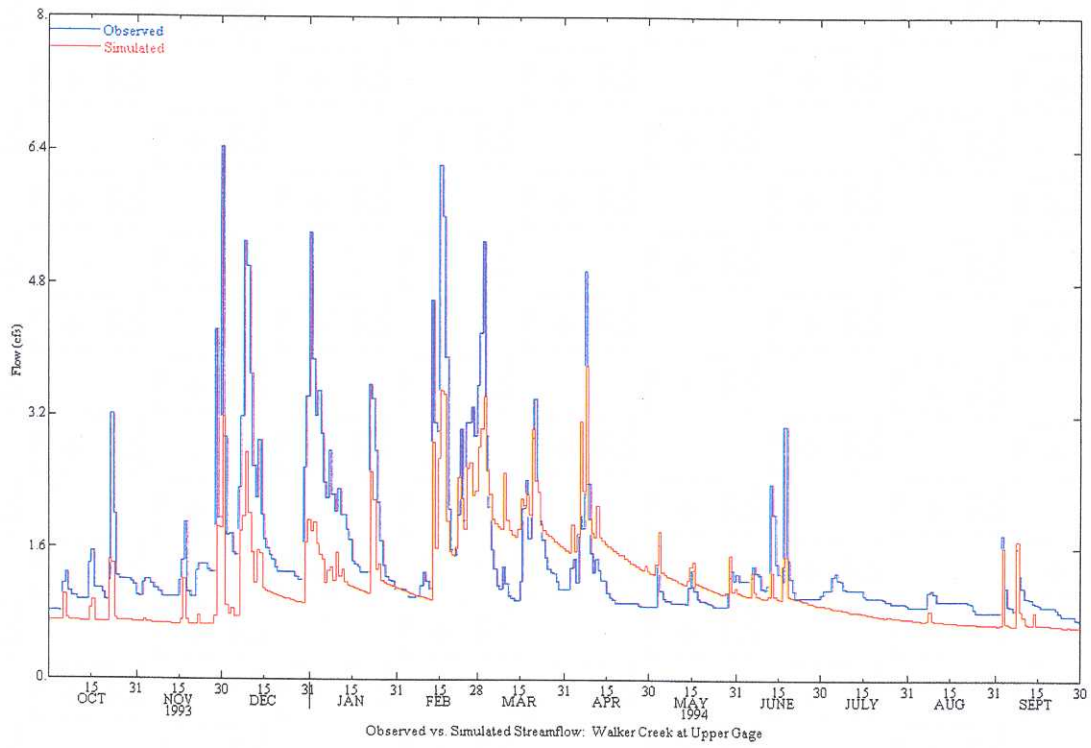


Figure B2-29

AR 011017

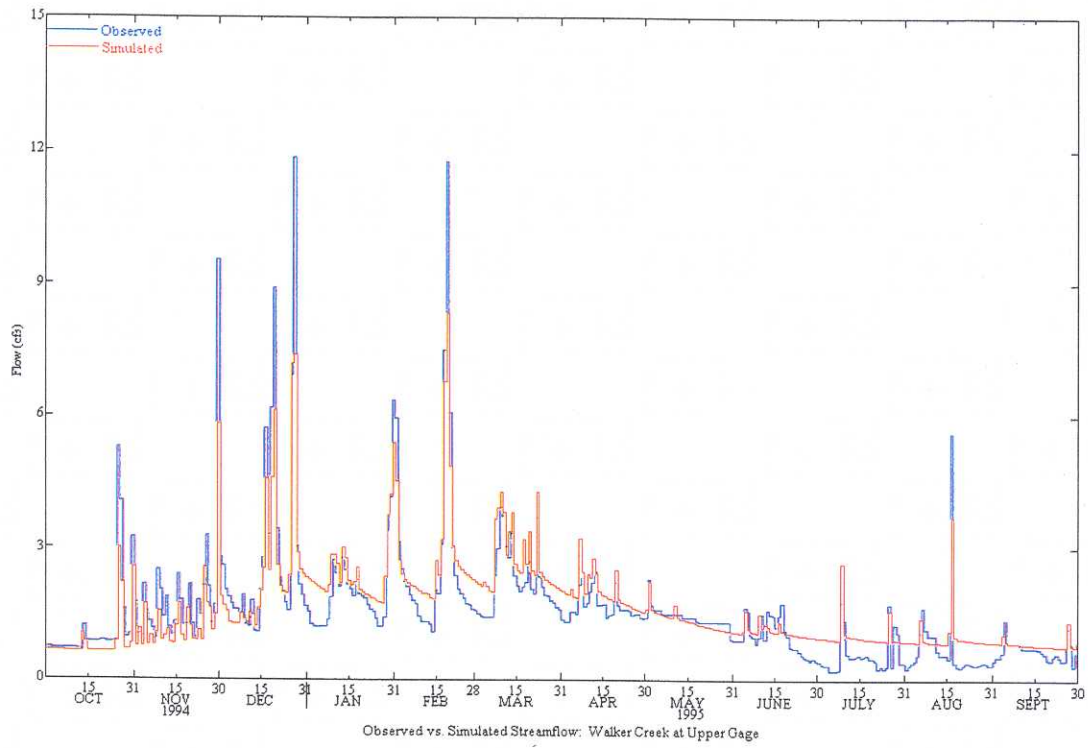


Figure B2-30

AR 011018

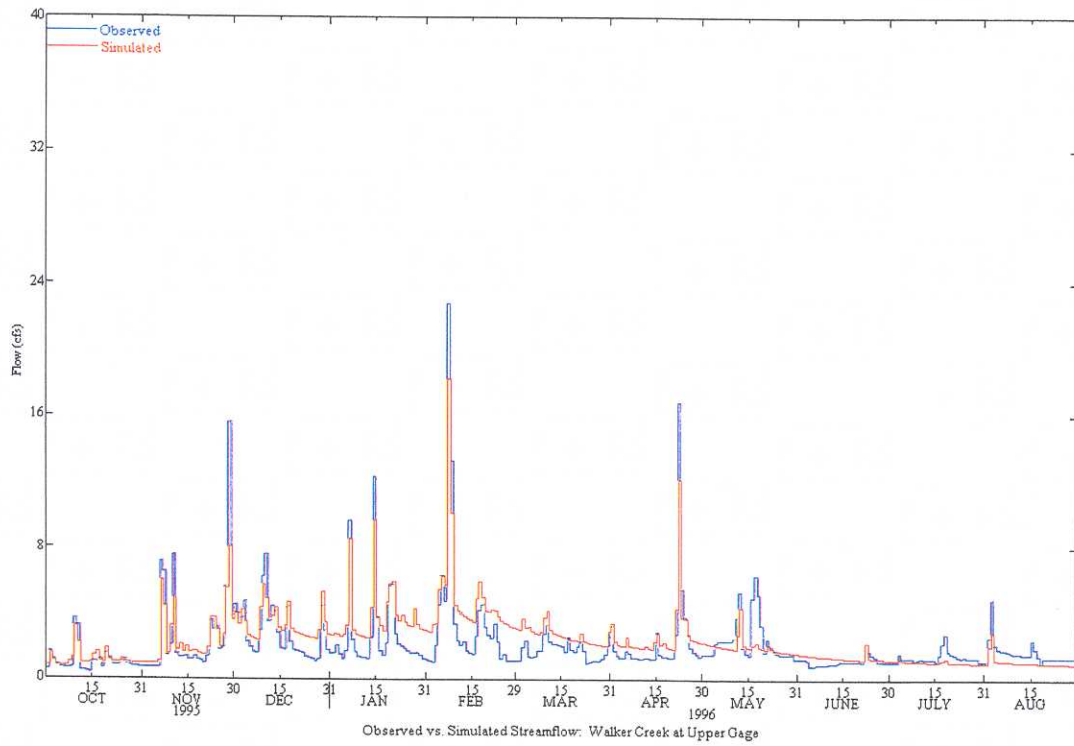


Figure B2-31

AR 011019

July 2001  
556-2912-001 (28)

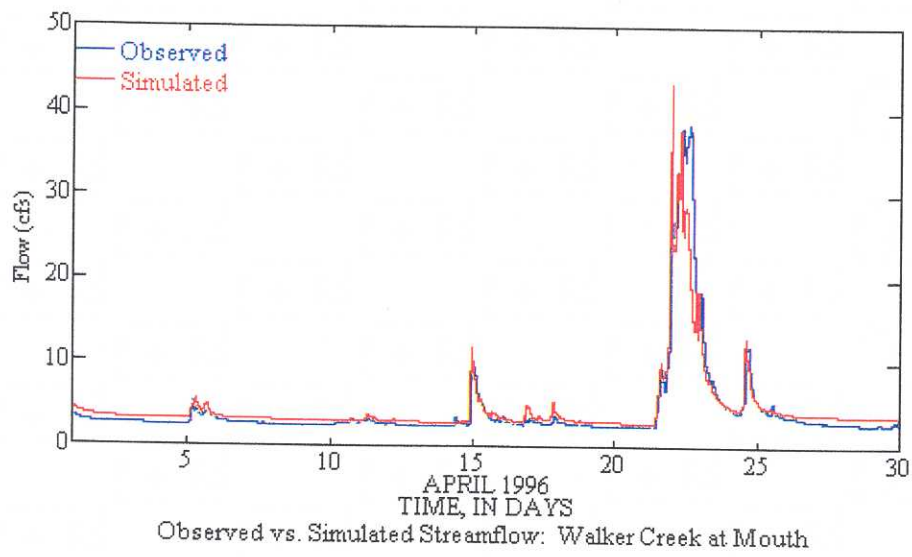


Figure B2-32

AR 011020

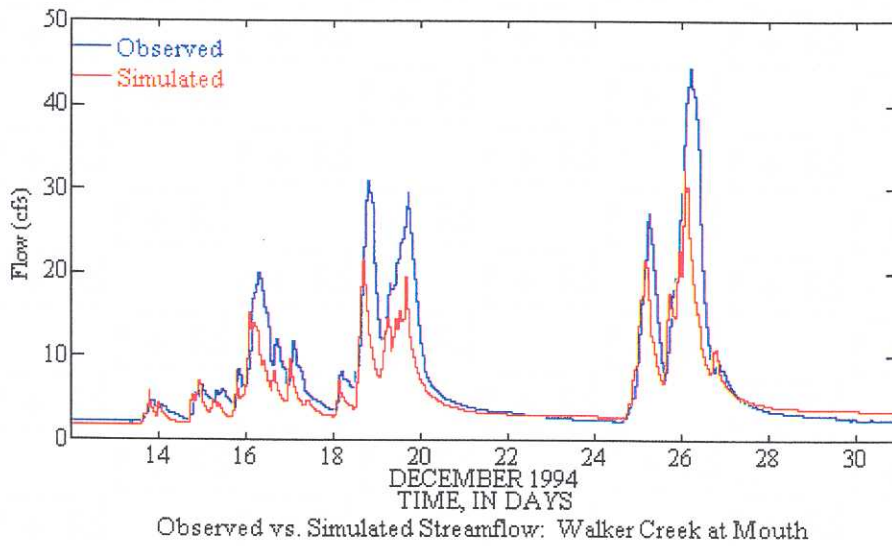


Figure B2-33

AR 011021

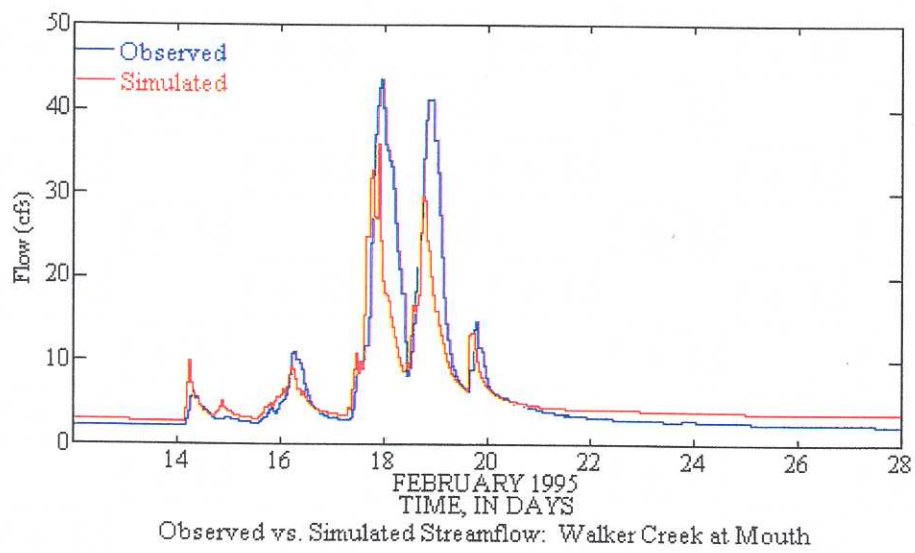
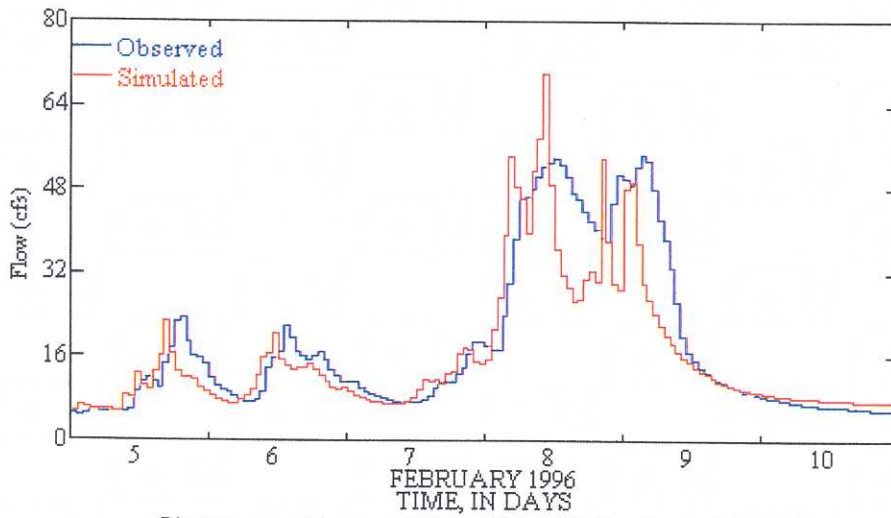


Figure B2-34

AR 011022



Observed vs. Simulated Streamflow: Walker Creek at Mouth

Figure B2-35

**AR 011023**

July 2001  
556-2912-001 (28)

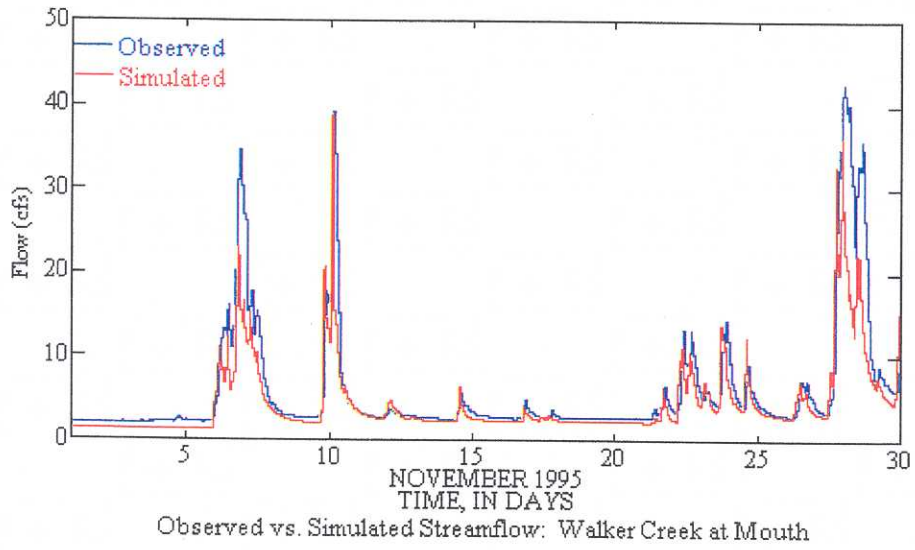


Figure B2-36

AR 011024



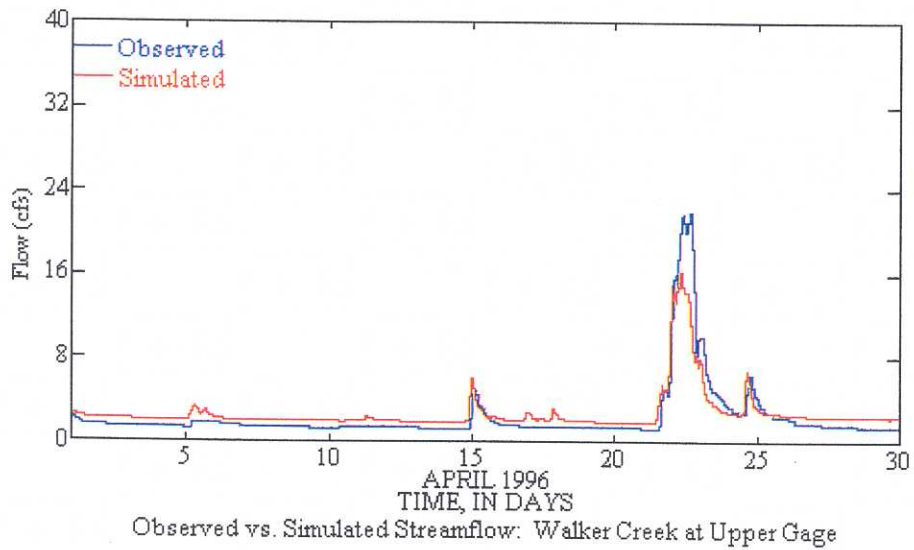


Figure B2-37

AR 011025

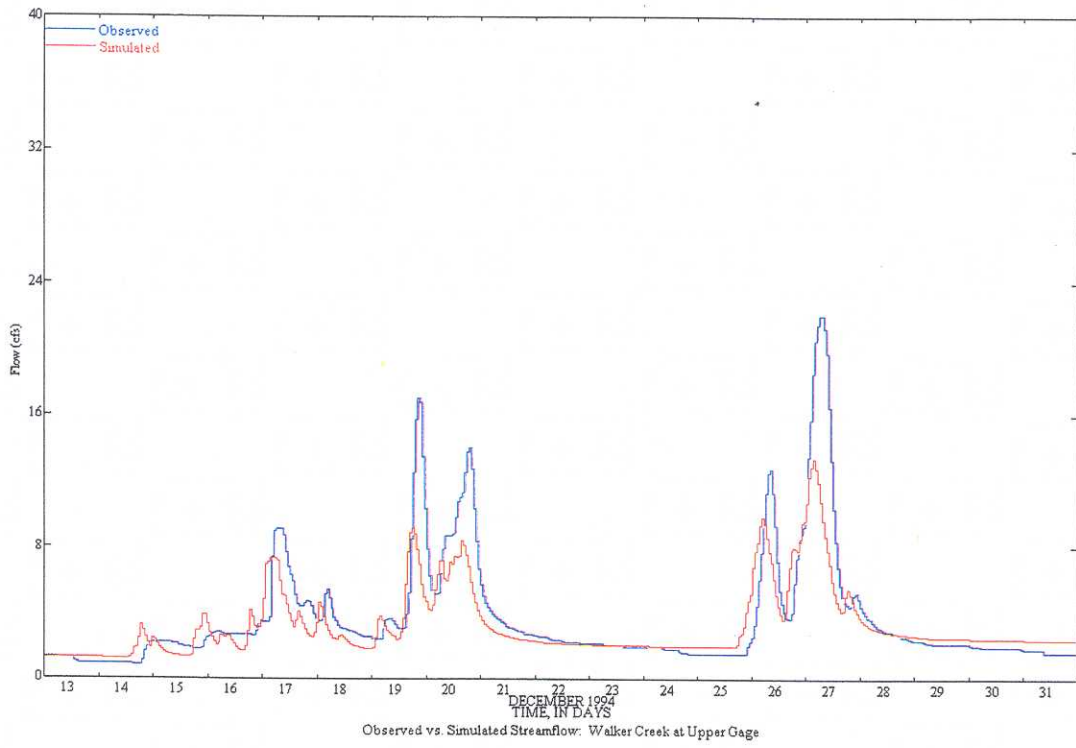


Figure B2-38

AR 011026

July 2001  
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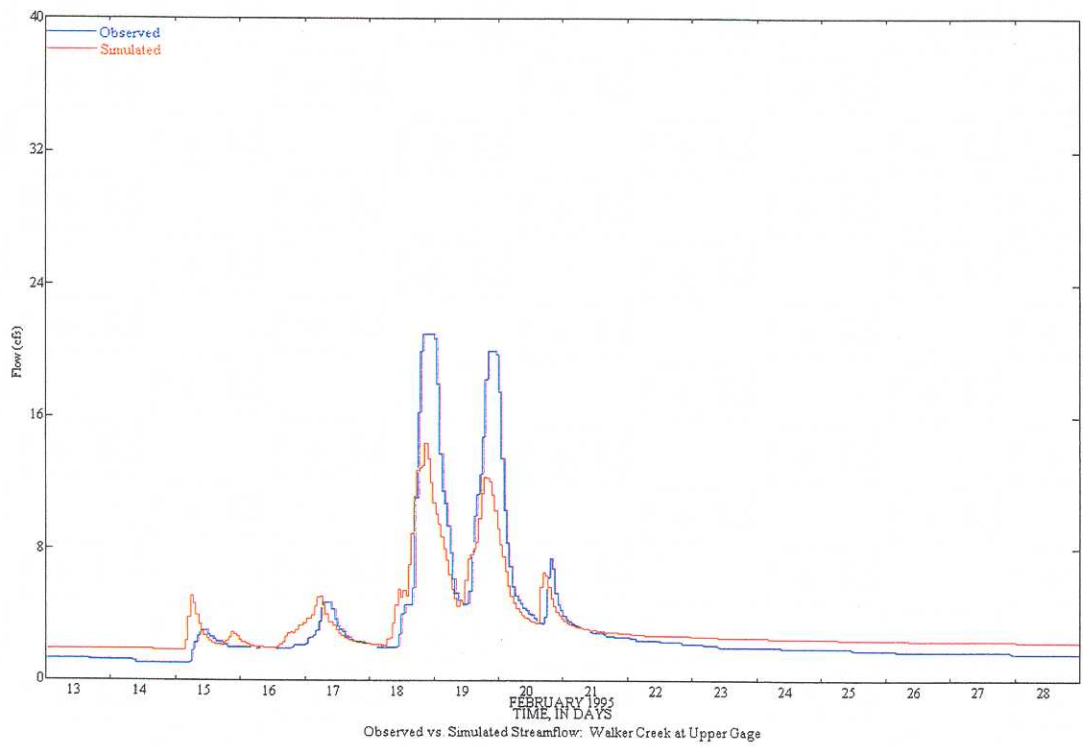
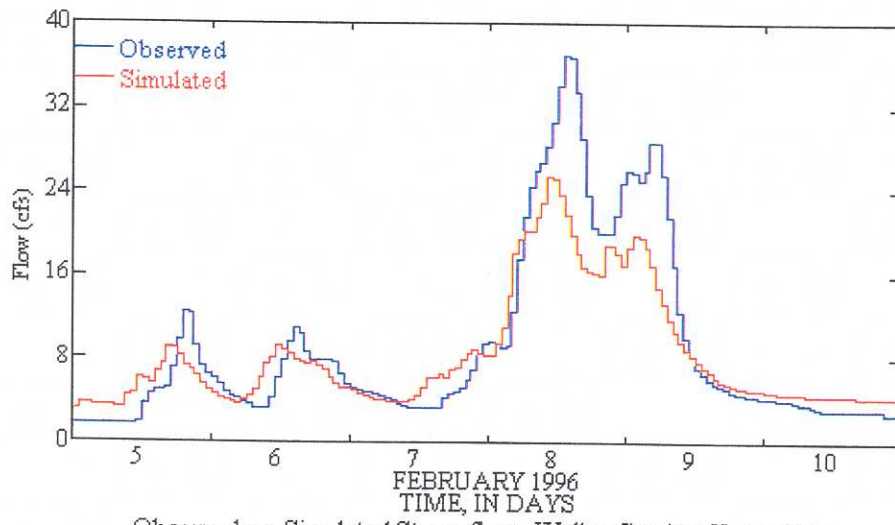


Figure B2-39

AR 011027



Observed vs. Simulated Streamflow: Walker Creek at Upper Gage

Figure B2-40

AR 011028

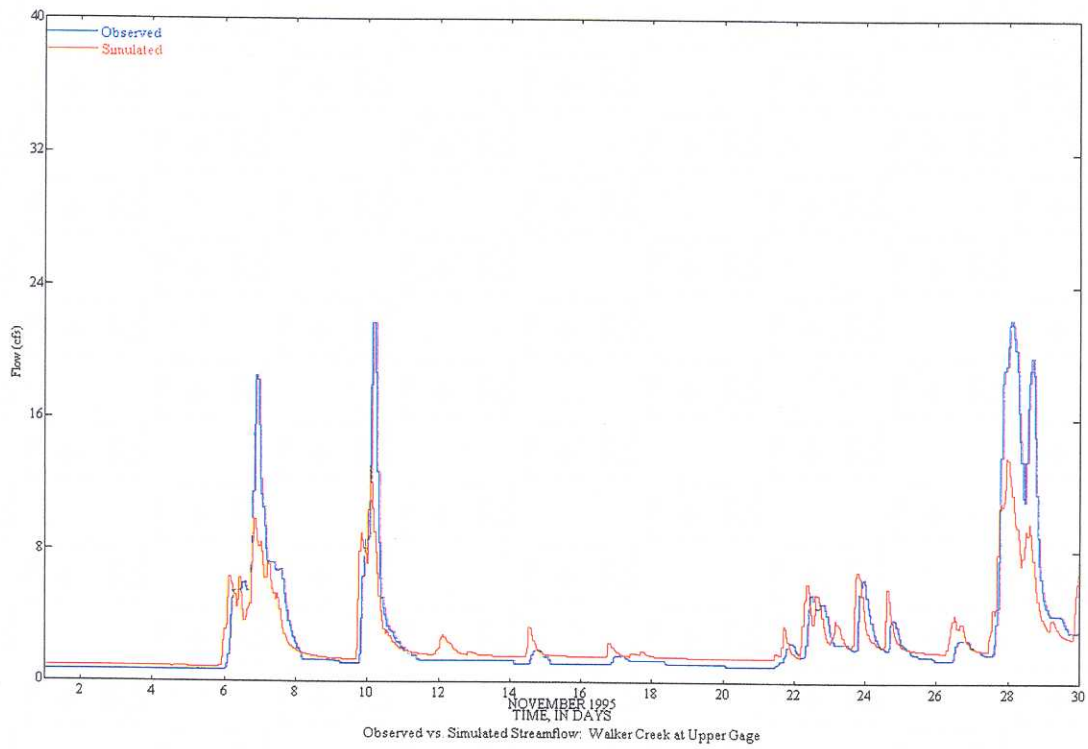


Figure B2-41

**AR 011029**

## 6. SUMMARY

This computer calibration effort resulted in calibrated models for both the Miller and Walker Creek watersheds. The final calibrations include all of the best available data. Each stage-storage-discharge relationship was derived from field data or topographical mapping. The land use data were updated based on the King County land use coverage. The watershed boundaries were altered to include an additional 300 or so acres to the northwest as a result of field investigations.

Simulated streamflow volumes over the calibration period are within 10 percent of recorded volumes. There was some difficulty matching the magnitude of recorded streamflow peaks. A total of 65 calibration runs were performed for Miller Creek and 21 calibration runs were performed for Walker Creek in an effort to match the magnitude of the recorded streamflow peaks. The best possible match for the given data has been achieved. Investigation of the recorded streamflow data at both gage sites for Miller Creek and both gage sites at Walker Creek demonstrates some possible errors as a result of gage malfunctions and shifting control situations. The team believes these errors may contribute to the inability for simulated streamflows to match observed streamflows.

Most importantly, both calibration HSPF models behave in a consistent manner, which will enable them to be used to study the impacts of differing land use conditions that are proposed for both these watersheds.

**AR 011030**

**MILLER CREEK HSPF CALIBRATION  
MODEL INPUT FILE**

**AR 011031**

RUN

GLOBAL

\*\*\* FILE: mill65.inp REVISED Aug 2000 Joe Brascher(atc)  
\*\*\* for parameterix  
\*\*\* SEATAC AIRPORT HSPF BASIN MODEL OF MILLER CREEK  
\*\*\* - POST-MILLER CK DETENTION FACIITY 10/92-6/93  
\*\*\* m23 AND M24 new area west of m2. Flows to rdf  
MILLER CREEK BASIN HSPF MODEL  
START 1989 01 1 0 0 END 1996 8 30 24 0  
RUN INTERP OUTPUT LEVEL 3  
RESUME 0 RUN 1

END GLOBAL

FILES

<type> <fun>\*\*\*<-----fname----->  
MESSU 24 D:\PARA\SEATAC\MILLER\MILL.MES  
WDM 25 D:\PARA\SEATAC\MILLER\MILL.WDM  
61 D:\PARA\SEATAC\MILLER\PER.L61  
62 D:\PARA\SEATAC\MILLER\RCH.L62

END FILES

OPN SEQUENCE

INGRP INDELT 01:00  
PERLND 14  
PERLND 16  
PERLND 18  
PERLND 24  
PERLND 26  
PERLND 28  
PERLND 34  
PERLND 44  
PERLND 54  
IMPLND 14  
RCHRES 1  
RCHRES 23  
RCHRES 24  
RCHRES 2  
RCHRES 3  
RCHRES 33  
RCHRES 4  
RCHRES 5  
RCHRES 50  
RCHRES 52  
RCHRES 53  
RCHRES 54  
RCHRES 34  
RCHRES 135  
RCHRES 35  
RCHRES 10  
RCHRES 16  
RCHRES 11  
RCHRES 13  
RCHRES 12  
RCHRES 15  
RCHRES 14  
RCHRES 17

END INGRP

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END OPN SEQUENCE

\*\*\*

COPY

TIMESERIES

Copy-opn

# - # NPT NMN  
1 5 1

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\*\*\*

END TIMESERIES

END COPY

PERLND

GEN-INFO

<PLS >

# - #

Name

NBLKS

Unit-systems

Printer

User t-series Engl Metr  
in out

\*\*\*  
\*\*\*  
\*\*\*

14	TFF- TILL FOR FLT	1	1	1	1	61	0
16	TFM- TILL FOR MOD	1	1	1	1	61	0
18	TFS- TILL FOR STP	1	1	1	1	61	0
24	TGF- TILL GR FLT	1	1	1	1	61	0
26	TGM- TILL GR MOD	1	1	1	1	61	0
28	TGS- TILL GR STP	1	1	1	1	61	0
34	OF - OUTWASH FOR	1	1	1	1	61	0
44	OG - OUTWASH GR	1	1	1	1	61	0

\*\*\*PERLND FOR NEW AIRPORT FILL; NONE IN CALIBRATION

45	AIRPORT FILL	1	1	1	1	61	0
54	SA - WETLANDS	1	1	1	1	61	0
64	RES- GROUNDWATER	1	1	1	1	61	0

END GEN-INFO

ACTIVITY

<PLS > \*\*\*\*\* Active Sections \*\*\*\*\*

#	-	#	ATMP	SNOW	PWAT	SED	PST	PWG	PQAL	MSTL	PEST	NITR	PHOS	TRAC
14	200	0	0	1	0	0	0	0	0	0	0	0	0	0

\*\*\*

END ACTIVITY

PRINT-INFO

<PLS > \*\*\*\*\* Print-flags \*\*\*\*\* PIVL PYR

#	-	#	ATMP	SNOW	PWAT	SED	PST	PWG	PQAL	MSTL	PEST	NITR	PHOS	TRAC	PIV	PYR
14	200	0	0	5	0	0	0	0	0	0	0	0	0	0	1	9

END PRINT-INFO

PWAT-PARM1

<PLS > \*\*\*\*\* Flags \*\*\*\*\*

#	-	#	CSNO	RTOP	UZFG	VCS	VUZ	VNN	VIFW	VIRC	VLE
14		0	0	0	0	0	0	0	0	0	0
16		0	0	0	0	0	0	0	0	0	0
18		0	0	0	0	0	0	0	0	0	0
24		0	0	0	0	0	0	0	0	0	0
26		0	0	0	0	0	0	0	0	0	0
28		0	0	0	0	0	0	0	0	0	0
34		0	0	0	0	0	0	0	0	0	0
44		0	0	0	0	0	0	0	0	0	0
45		0	0	0	0	0	0	0	0	0	0
54		0	0	0	0	0	0	0	0	0	0
64		0	0	0	0	0	0	0	0	0	0

\*\*\*

END PWAT-PARM1

PWAT-PARM2

<PLS > \*\*\*

#	-	#	***FOREST	LZSN	INFILT	LSUR	SLSUR	KVARY	AGWRC
14				9.0000	0.3200	400.00	0.0500	0.5000	0.9960

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16	9.0000	0.3200	400.00	0.1000	0.5000	0.9960
18	9.0000	0.3200	200.00	0.2000	0.5000	0.9960
24	9.0000	0.1200	400.00	0.0500	0.5000	0.9960
26	9.0000	0.1200	400.00	0.1000	0.5000	0.9960
28	9.0000	0.1200	200.00	0.2000	0.5000	0.9960
34	10.0000	2.0000	400.00	0.0500	0.3000	0.9960
44	10.0000	0.8000	400.00	0.0500	0.3000	0.9960
45	7.5000	0.0200	300.00	0.0700	0.0000	0.9000
54	8.0000	2.0000	100.00	0.0010	0.5000	0.9960

END PWAT-PARM2

PWAT-PARM3

<PLS >\*\*\*

# - #***	PETMAX	PETMIN	INFEXP	INFILD	DEEPPFR	BASETP	AGWETP
14			2.0000	2.0000	0.33	0.00	0.0
16			2.0000	2.0000	0.33	0.00	0.0
18			2.0000	2.0000	0.33	0.00	0.0
24			2.0000	2.0000	0.33	0.00	0.
26			2.0000	2.0000	0.33	0.	0.
28			2.0000	2.0000	0.33	0.	0.
34			2.0000	2.0000	0.33	0.00	0.0
44			2.0000	2.0000	0.33	0.	0.
54			10.0000	2.0000	0.33	0.	0.7

END PWAT-PARM3

PWAT-PARM4

<PLS >

# - #	CEPSC	UZSN	NSUR	INTFW	IRC	LZETP***
14	0.2000	1.5000	0.3500	9.000	0.7000	0.7000
16	0.2000	0.7500	0.3500	9.000	0.7000	0.7000
18	0.2000	0.4500	0.3500	9.000	0.3000	0.7000
24	0.1000	0.7500	0.2500	9.000	0.7000	0.2500
26	0.1000	0.3750	0.2500	9.000	0.7000	0.2500
28	0.1000	0.2250	0.2500	9.000	0.3000	0.2500
34	0.2000	0.7500	0.3500	0.000	0.7000	0.7000
44	0.1000	0.7500	0.2500	0.000	0.7000	0.2500
54	0.1000	2.2500	0.5000	1.000	0.7000	0.8000

END PWAT-PARM4

PWAT-STATE1

<PLS > PWATER state variables\*\*\*

# - #***	CEPS	SURS	UZS	IFWS	LZS	AGWS	GWVS
14	0.078	0.	0.2500	0.10	2.000	2.000	0.000
16	0.078	0.	0.2500	0.10	2.000	2.000	0.000
18	0.078	0.	0.2500	0.10	2.000	2.000	0.000
24	0.051	0.	0.2500	0.10	2.000	2.000	0.000
26	0.051	0.	0.2500	0.10	2.000	2.000	0.000
28	0.051	0.	0.2500	0.10	2.000	2.000	0.000
34	0.078	0.	0.2500	0.10	2.000	2.000	0.000
44	0.051	0.	0.2500	0.10	2.000	2.000	0.000
45	0.051	0.	0.2500	0.10	2.000	2.000	0.000
54	0.051	0.	0.2500	0.10	2.000	2.000	0.000
64	0.051	0.	0.2500	0.10	2.000	2.000	0.000

END PWAT-STATE1

END PERLND

IMPLND

GEN-INFO

<ILS >

# - #

Unit-systems Printer  
User t-series Engl Metr  
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```

14      IMPERVIOUS          1    1    1    60    0
END GEN-INFO
ACTIVITY
<ILS > ***** Active Sections ****
# - # ATMP SNOW IWAT  SLD  IWG IQAL  ***
14      0    0    1    0    0    0
END ACTIVITY
PRINT-INFO
<ILS > ***** Print-flags ***** PIVL  PYR
# - # ATMP SNOW IWAT  SLD  IWG IQAL  *****
14      0    0    6    0    0    0    1    9
END PRINT-INFO
IWAT-PARM1
<ILS >          Flags          ***
# - # CSNO RTOP  VRS  VNN RTLI  ***
14      0    0    0    0    0
END IWAT-PARM1
IWAT-PARM2
<ILS >          ***
# - #          LSUR          SLSUR          NSUR          RETSC
14      100.00    0.0100    0.1000    0.1000
END IWAT-PARM2
IWAT-PARM3
<ILS >          ***
# - #          PETMAX    PETMIN
14
END IWAT-PARM3
IWAT-STATE1
<ILS >  IWATER state variables
# - #          RETS          SURS
14      1.0000E-3  1.0000E-3
END IWAT-STATE1
END IMPLND
EXT SOURCES
*** NOTE: The only RCHRES that precip and PET are applied to are lakes.
<-Volume-> <Member> SsysSgap<--Mult-->Tran <-Target vols> <-Grp> <-Member->
<Name> # <Name> # tem strg<-factor->strg <Name> # # <Name> # #
*** PRECIP/EVAP TO PERVIOUS/IMPERV SURFACES
WDM      2  PREC      ENGLZERO  1.00          PERLND  14  200  EXTNL  PREC
WDM      2  PREC      ENGLZERO  1.00          IMPLND  14          EXTNL  PREC
WDM     1010  EVAP      ENGLZERO  0.8          PERLND  14  18  EXTNL  PETINP
WDM     1010  EVAP      ENGLZERO  0.8          PERLND  24  28  EXTNL  PETINP
WDM     1010  EVAP      ENGLZERO  0.8          PERLND  34  54  EXTNL  PETINP
WDM     1010  EVAP      ENGLZERO  0.8          PERLND  64          EXTNL  PETINP
WDM     1010  EVAP      ENGLZERO  0.8          IMPLND  14          EXTNL  PETINP
*** PRECIP/EVAP TO LAKES
WDM      2  PREC      ENGLZERO          RCHRES  1          EXTNL  PREC
WDM     1010  EVAP      ENGLZERO  0.8          RCHRES  1          EXTNL  POTEV
WDM      2  PREC      ENGLZERO          RCHRES  4          EXTNL  PREC
WDM     1010  EVAP      ENGLZERO  0.8          RCHRES  4          EXTNL  POTEV
WDM     1010  EVAP      ENGLZERO  0.8          RCHRES  1          EXTNL  POTEV
WDM      2  PREC      ENGLZERO          RCHRES  1          EXTNL  PREC
WDM     1010  EVAP      ENGLZERO  0.8          RCHRES  4          EXTNL  POTEV
WDM      2  PREC      ENGLZERO          RCHRES  4          EXTNL  PREC

```

WDM	2	PREC	ENGLZERO		RCHRES	11	EXTNL	PREC
WDM	1010	EVAP	ENGLZERO	0.8	RCHRES	11	EXTNL	POTEV
WDM	2	PREC	ENGLZERO		RCHRES	13	EXTNL	PREC
WDM	1010	EVAP	ENGLZERO	0.8	RCHRES	13	EXTNL	POTEV
WDM	2	PREC	ENGLZERO		RCHRES	23	EXTNL	PREC
WDM	1010	EVAP	ENGLZERO	0.8	RCHRES	23	EXTNL	POTEV
WDM	2	PREC	ENGLZERO		RCHRES	34	EXTNL	PREC
WDM	1010	EVAP	ENGLZERO	0.8	RCHRES	34	EXTNL	POTEV
WDM	2	PREC	ENGLZERO		RCHRES	53	EXTNL	PREC
WDM	1010	EVAP	ENGLZERO	0.8	RCHRES	53	EXTNL	POTEV
WDM	2	PREC	ENGLZERO		RCHRES	54	EXTNL	PREC
WDM	1010	EVAP	ENGLZERO	0.8	RCHRES	54	EXTNL	POTEV

END EXT SOURCES

EXT TARGETS

```

<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Volume-> <Member> Tsys Tgap Amd ***
<Name> # <Name> # #<-factor->strg <Name> # <Name> tem strg strg***
*** UPPER MILLER CREEK GROUNDWATER PUMPING
COPY *** 1 OUTPUT MEAN 1 12.1 WDM 18 FLOW ENGL REPL
*** GAUGE POINTS (17=MOUTH, 54=MILLER RDF, 50=SR 518, 18=WALKER CK)
RCHRES 17 HYDR RO WDM 33 FLOW ENGL REPL
RCHRES 54 HYDR RO WDM 34 FLOW ENGL REPL
RCHRES 50 HYDR RO *** WDM 35 FLOW ENGL REPL
RCHRES 18 HYDR RO *** WDM 36 FLOW ENGL REPL
*** MISC (20=WALKER WETLAND, 55=SR509, 56=1ST AVE, 1=ARBOR LAKE)
RCHRES 23 HYDR STAGE *** WDM 91 STAG ENGL REPL
RCHRES 20 HYDR RO *** WDM 37 FLOW ENGL REPL
RCHRES 55 HYDR RO *** WDM 38 FLOW ENGL REPL
RCHRES 62 HYDR RO *** WDM 39 FLOW ENGL REPL
RCHRES 1 HYDR RO *** WDM 80 FLOW ENGL REPL
<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Volume-> <Member> Tsys Tgap Amd ***
<Name> # <Name> # #<-factor->strg <Name> # <Name> tem strg strg***
***MOUTH
RCHRES 54 HYDR RO 1 1 0.000419 WDM 60 SIMQ ENGL REPL
RCHRES 17 HYDR RO 1 1 0.000213 WDM 70 SIMQ ENGL REPL
END EXT TARGETS

```

\*\*\*

SCHEMATIC

```

<-Source-> <--Area--> <-Target-> MBLK ***
<Name> # <-factor-> <Name> # Tbl# ***
*** SUB-CATCHMENT 1 all agwo goes to sound
PERLND 16 3.41 RCHRES 1 6
PERLND 26 232.36 RCHRES 1 6
PERLND 34 3.07 RCHRES 1 6
PERLND 44 38.03 RCHRES 1 6
PERLND 54 3.87 RCHRES 1 6
IMPLND 14 56.14 RCHRES 1 2
*** SUB-CATCHMENT 2 10% of area GW goes to vaca 90% goes to sound
PERLND 16 5.56 RCHRES 2 6
PERLND 26 200.05 RCHRES 2 6
PERLND 34 0.46 RCHRES 2 6
PERLND 44 38.71 RCHRES 2 6
PERLND 16 0.56 RCHRES 135 7
PERLND 26 20.01 RCHRES 135 7
PERLND 34 0.05 RCHRES 135 7
PERLND 44 3.87 RCHRES 135 7

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IMPLND	14	42.22	RCHRES	2	2
*** SUB-CATCHMENT 23 New subbasin 15 % OF GW GOES TO VACCA 85% TO SOUND					
PERLND	16	3.09	RCHRES	23	6
PERLND	26	156.15	RCHRES	23	6
PERLND	34	2.25	RCHRES	23	6
PERLND	44	45.84	RCHRES	23	6
PERLND	16	0.46	RCHRES	135	7
PERLND	26	23.42	RCHRES	135	7
PERLND	34	0.34	RCHRES	135	7
PERLND	44	6.88	RCHRES	135	7
IMPLND	14	58.44	RCHRES	23	2
*** SUB-CATCHMENT 24 New subbasin 60 % OF GW GOES TO 11 40% TO SOUND					
PERLND	26	135.43	RCHRES	24	6
PERLND	34	2.02	RCHRES	24	6
PERLND	44	69.29	RCHRES	24	6
PERLND	26	81.26	RCHRES	11	7
PERLND	34	1.21	RCHRES	11	7
PERLND	44	41.57	RCHRES	11	7
IMPLND	14	79.98	RCHRES	24	2
*** SUB-CATCHMENT 3 agwo goes to vaca(135)					
PERLND	16	8.26	RCHRES	3	6
PERLND	26	108.38	RCHRES	3	6
PERLND	34	16.02	RCHRES	3	6
PERLND	44	102.89	RCHRES	3	6
PERLND	54	0.04	RCHRES	3	6
PERLND	16	8.26	RCHRES	135	7
PERLND	26	108.38	RCHRES	135	7
PERLND	34	16.02	RCHRES	135	7
PERLND	44	102.89	RCHRES	135	7
PERLND	54	0.04	RCHRES	135	7
IMPLND	14	27.30	RCHRES	3	2
*** SUB-CATCHMENT 4 10% of agwo goes to rchres 90% goes to sound					
PERLND	16	2.95	RCHRES	4	6
PERLND	26	85.95	RCHRES	4	6
PERLND	34	3.75	RCHRES	4	6
PERLND	44	92.06	RCHRES	4	6
PERLND	16	0.30	RCHRES	4	7
PERLND	26	8.60	RCHRES	4	7
PERLND	34	0.38	RCHRES	4	7
PERLND	44	9.21	RCHRES	4	7
IMPLND	14	18.43	RCHRES	4	2
*** SUB-CATCHMENT 4a 70% of agwo goes to rchres 30% goes to sound					
PERLND	16	8.66	RCHRES	4	6
PERLND	26	61.64	RCHRES	4	6
PERLND	34	22.06	RCHRES	4	6
PERLND	44	78.09	RCHRES	4	6
PERLND	54	12.50	RCHRES	4	6
PERLND	16	6.06	RCHRES	4	7
PERLND	26	43.15	RCHRES	4	7
PERLND	34	15.44	RCHRES	4	7
PERLND	44	54.66	RCHRES	4	7
PERLND	54	8.75	RCHRES	4	7
IMPLND	14	29.14	RCHRES	4	2
*** SUB-CATCHMENT 5					
PERLND	26	10.29	RCHRES	5	1
PERLND	44	50.04	RCHRES	5	1
PERLND	54	10.74	RCHRES	5	1

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IMPLND	14	16.31	RCHRES	5	2
*** SUB-CATCHMENT 6					
PERLND	16	10.66	RCHRES	53	1
PERLND	26	41.08	RCHRES	53	1
PERLND	34	21.75	RCHRES	53	1
PERLND	44	13.39	RCHRES	53	1
PERLND	54	0.82	RCHRES	53	1
IMPLND	14	7.14	RCHRES	53	2
*** SUB-CATCHMENT 8					
PERLND	44	22.21	RCHRES	35	1
IMPLND	14	6.60	RCHRES	35	2
*** SUB-CATCHMENT 9					
PERLND	16	4.94	RCHRES	34	1
PERLND	26	14.32	RCHRES	34	1
PERLND	34	0.05	RCHRES	34	1
PERLND	44	56.70	RCHRES	34	1
PERLND	54	0.01	RCHRES	34	1
IMPLND	14	22.46	RCHRES	34	2
*** SUB-CATCHMENT 10					
PERLND	16	4.15	RCHRES	10	1
PERLND	26	31.94	RCHRES	10	1
PERLND	44	95.23	RCHRES	10	1
IMPLND	14	71.97	RCHRES	10	2
*** SUB-CATCHMENT 11 25% OF AGWO GOES TO 15					
PERLND	16	0.89	RCHRES	11	6
PERLND	26	217.92	RCHRES	11	6
PERLND	34	1.32	RCHRES	11	6
PERLND	44	65.65	RCHRES	11	6
PERLND	16	0.67	RCHRES	11	7
PERLND	26	163.44	RCHRES	11	7
PERLND	34	0.99	RCHRES	11	7
PERLND	44	49.24	RCHRES	11	7
PERLND	16	0.22	RCHRES	15	7
PERLND	26	54.48	RCHRES	15	7
PERLND	34	0.33	RCHRES	15	7
PERLND	44	16.41	RCHRES	15	7
IMPLND	14	230.80	RCHRES	11	2
*** SUB-CATCHMENT 12					
PERLND	16	0.39	RCHRES	12	1
PERLND	26	101.18	RCHRES	12	1
PERLND	34	5.64	RCHRES	12	1
PERLND	44	54.98	RCHRES	12	1
PERLND	54	0.64	RCHRES	12	1
IMPLND	14	79.83	RCHRES	12	2
*** SUB-CATCHMENT 13					
PERLND	16	0.79	RCHRES	13	1
PERLND	26	197.68	RCHRES	13	1
IMPLND	14	27.66	RCHRES	13	2
*** SUB-CATCHMENT 14 50% OF AGWO GOES TO SOUND					
PERLND	16	0.24	RCHRES	14	6
PERLND	26	118.67	RCHRES	14	6
PERLND	34	13.46	RCHRES	14	6
PERLND	44	41.91	RCHRES	14	6
PERLND	16	0.12	RCHRES	14	7
PERLND	26	59.34	RCHRES	14	7
PERLND	34	6.73	RCHRES	14	7
PERLND	44	20.95	RCHRES	14	7

IMPLND	14	20.66	RCHRES	14	2
*** SUB-CATCHMENT 15					
PERLND	16	6.59	RCHRES	15	1
PERLND	26	49.55	RCHRES	15	1
PERLND	34	50.09	RCHRES	15	1
PERLND	44	86.52	RCHRES	15	1
IMPLND	14	19.47	RCHRES	15	2
*** SUB-CATCHMENT 16					
PERLND	16	10.93	RCHRES	16	1
PERLND	26	30.30	RCHRES	16	1
PERLND	34	20.03	RCHRES	16	1
PERLND	44	31.42	RCHRES	16	1
IMPLND	14	15.54	RCHRES	16	2
*** SUB-CATCHMENT 17 AGWO GOES TO SOUND					
PERLND	16	0.90	RCHRES	17	6
PERLND	26	16.31	RCHRES	17	6
PERLND	34	34.82	RCHRES	17	6
PERLND	44	82.11	RCHRES	17	6
PERLND	54	2.19	RCHRES	17	6
IMPLND	14	10.49	RCHRES	17	2
*** SUB-CATCHMENT MC-1					
PERLND	26	0.17	RCHRES	52	1
PERLND	44	8.21	RCHRES	52	1
PERLND	54	0.27	RCHRES	52	1
IMPLND	14	0.09	RCHRES	52	2
*** SUB-CATCHMENT MC-2					
PERLND	16	0.08	RCHRES	53	1
PERLND	26	0.64	RCHRES	53	1
PERLND	34	6.72	RCHRES	53	1
PERLND	44	10.43	RCHRES	53	1
PERLND	54	15.25	RCHRES	53	1
IMPLND	14	0.27	RCHRES	53	2
*** SUB-CATCHMENT MC-3					
PERLND	34	5.44	RCHRES	54	1
PERLND	44	5.03	RCHRES	54	1
PERLND	54	2.28	RCHRES	54	1
IMPLND	14	0.11	RCHRES	54	2
*** SUB-CATCHMENT MC-4					
PERLND	44	17.32	RCHRES	135	1
PERLND	54	14.41	RCHRES	135	1
IMPLND	14	1.77	RCHRES	135	2
*** SUB-CATCHMENT MC-5					
PERLND	26	13.49	RCHRES	35	1
PERLND	44	31.06	RCHRES	35	1
PERLND	54	5.95	RCHRES	35	1
IMPLND	14	2.50	RCHRES	35	2
*** SUB-CATCHMENT MC-6					
PERLND	44	17.75	RCHRES	35	1
PERLND	54	6.54	RCHRES	35	1
IMPLND	14	0.95	RCHRES	35	2
*** SUB-CATCHMENT MC-6B					
PERLND	26	34.94	RCHRES	35	1
PERLND	34	7.81	RCHRES	35	1
PERLND	44	52.91	RCHRES	35	1
PERLND	54	4.61	RCHRES	35	1
IMPLND	14	3.14	RCHRES	35	2
*** SUB-CATCHMENT MC-7					

PERLND	26	12.66	RCHRES	16	1
PERLND	44	33.53	RCHRES	16	1
PERLND	54	4.16	RCHRES	16	1
IMPLND	14	3.88	RCHRES	16	2
*** SUB-CATCHMENT MC-7B					
PERLND	26	36.16	RCHRES	16	1
PERLND	44	8.46	RCHRES	16	1
PERLND	54	1.92	RCHRES	16	1
IMPLND	14	2.12	RCHRES	16	2
***all sdn basin agwo goes to 35					
*** SUB-CATCHMENT SDN-1					
PERLND	26	3.23	RCHRES	52	6
PERLND	44	2.11	RCHRES	52	6
PERLND	54	0.20	RCHRES	52	6
PERLND	26	3.23	RCHRES	135	7
PERLND	44	2.11	RCHRES	135	7
PERLND	54	0.20	RCHRES	135	7
IMPLND	14	8.29	RCHRES	52	2
*** SUB-CATCHMENT SDN-1-LWR					
PERLND	44	4.97	RCHRES	52	6
PERLND	54	0.07	RCHRES	52	6
PERLND	44	4.97	RCHRES	135	7
PERLND	54	0.07	RCHRES	135	7
IMPLND	14	0.38	RCHRES	52	2
*** SUB-CATCHMENT SDN-1-OFF					
PERLND	26	29.12	RCHRES	52	6
PERLND	44	3.62	RCHRES	52	6
PERLND	54	1.67	RCHRES	52	6
PERLND	26	29.12	RCHRES	135	7
PERLND	44	3.62	RCHRES	135	7
PERLND	54	1.67	RCHRES	135	7
IMPLND	14	11.50	RCHRES	52	2
*** SUB-CATCHMENT SDN-2					
PERLND	26	10.41	RCHRES	52	6
PERLND	44	3.04	RCHRES	52	6
PERLND	26	10.41	RCHRES	135	7
PERLND	44	3.04	RCHRES	135	7
IMPLND	14	33.22	RCHRES	52	2
*** SUB-CATCHMENT SDN-2X					
PERLND	26	1.37	RCHRES	52	6
PERLND	44	5.84	RCHRES	52	6
PERLND	26	1.37	RCHRES	135	7
PERLND	44	5.84	RCHRES	135	7
IMPLND	14	0.28	RCHRES	52	2
*** SUB-CATCHMENT SDN-3					
PERLND	26	49.79	RCHRES	54	6
PERLND	26	49.79	RCHRES	135	7
IMPLND	14	15.82	RCHRES	54	2
*** SUB-CATCHMENT SDN-3X					
PERLND	16	0.65	RCHRES	54	6
PERLND	26	5.17	RCHRES	54	6

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PERLND	34	13.64	RCHRES	54	6
PERLND	44	5.34	RCHRES	54	6
PERLND	54	0.57	RCHRES	54	6
PERLND	16	0.65	RCHRES	135	7
PERLND	26	5.17	RCHRES	135	7
PERLND	34	13.64	RCHRES	135	7
PERLND	44	5.34	RCHRES	135	7
PERLND	54	0.57	RCHRES	135	7

\*\*\* SUB-CATCHMENT SDN-4

PERLND	26	24.43	RCHRES	52	6
PERLND	44	3.19	RCHRES	52	6
PERLND	26	24.43	RCHRES	135	7
PERLND	44	3.19	RCHRES	135	7
IMPLND	14	2.61	RCHRES	52	2

\*\*\* SUB-CATCHMENT SDN-4X

PERLND	26	1.57	RCHRES	52	6
PERLND	34	1.16	RCHRES	52	6
PERLND	44	10.01	RCHRES	52	6
PERLND	26	1.57	RCHRES	135	7
PERLND	34	1.16	RCHRES	135	7
PERLND	44	10.01	RCHRES	135	7

\*\*\*ROUTING FOR MILLER CREEK

\*\*\* M1 TO M2 TO M3 TO STORAGE 50. M4 TO M5 TO STORAGE 50

RCHRES	1		RCHRES	2	4
RCHRES	23		RCHRES	24	4
RCHRES	24		RCHRES	3	3
RCHRES	2		RCHRES	3	3
RCHRES	3		RCHRES	33	3
RCHRES	33		RCHRES	50	3
RCHRES	4		RCHRES	5	4
RCHRES	5		RCHRES	50	3

\*\*\* NEW STREAM REACH 52 TO LAKE REBA 53 TO RDF 54

RCHRES	52		RCHRES	53	3
RCHRES	53		RCHRES	54	3
RCHRES	50		RCHRES	54	3

\*\*\* RDF 54 TO 35

RCHRES	54		RCHRES	135	3
RCHRES	34		RCHRES	135	4
RCHRES	34		RCHRES	135	5
RCHRES	135		RCHRES	35	3
RCHRES	10		RCHRES	16	3
RCHRES	35		RCHRES	16	3
RCHRES	11		RCHRES	15	3
RCHRES	13		RCHRES	12	4
RCHRES	13		RCHRES	12	5
RCHRES	12		RCHRES	15	3
RCHRES	16		RCHRES	15	3
RCHRES	14		RCHRES	17	3
RCHRES	15		RCHRES	17	3

END SCHEMATIC

NETWORK

\*\*\* <MEMBER> SSSYGAP<--MULT-->TRAN <-TARGET VOLS>

\*\*\* <-MEMBER->

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<NAME> # <NAME> TEM STRG<-FACTOR->STRG <NAME> # # <-GRP> <NAME> # # \*\*\*  
 \*\*\*

END NETWORK

RCHRES

GEN-INFO

RCHRES	Name	Nexits	Unit	Systems	Printer					
# - #	<-----><----->	User	T-series	Engl	Metr	LKFG				
			in	out						
1	Arbor Lake M 1	2	1	1	1	62	0	0		
2	Arbor Ck -03710 M 2	1	1	1	1	62	0	0		
3	Arbor Ck M 3	1	1	1	1	62	0	0		
4	Tub Lake M 4	2	1	1	1	62	0	0		
5	Miller Ck SR518 M5	1	1	1	1	62	0	0		
10	Trib (0371G) M 10	1	1	1	1	62	0	0		
11	M11 Ambaum Detention	1	1	1	1	62	0	0		
12	Trib(0354) M 12	1	1	1	1	62	0	0		
13	Burien Lake M 13	2	1	1	1	62	0	0		
14	Trib (0353) M 14	1	1	1	1	62	0	0		
15	M/S U/S OF 17	1	1	1	1	62	0	0		
16	U/S OF 15 M/S	1	1	1	1	62	0	0		
17	GAGE	1	1	1	1	62	0	0		
23	BASIN M23	2	1	1	1	62	0	0		
24	BASIN M24	1	1	1	1	62	0	0		
33	detention m3	1	1	1	1	62	0	0		
34	LORA LAKE	2	1	1	1	62	0	0		
35	D/S OF VACA FARM	1	1	1	1	62	0	0		
38	MC basins	1	1	1	1	62	0	0		
50	sr 518	1	1	1	1	62	0	0		
52	U/S OF LAKE REBA	1	1	1	1	62	0	0		
53	Reba outflow	1	1	1	1	62	0	0		
54	Miller RDF outflow	1	1	1	1	62	0	0		
135	VACA FARMS	1	1	1	1	62	0	0		

END GEN-INFO

ACTIVITY

RCHRES	***** Active Sections *****										
# - #	HYFG	ADFG	CNFG	HTFG	SDFG	GQFG	OXFG	NUFG	PKFG	PHFG	
1 999	1	0	0	0	0	0	0	0	0	0	0

END ACTIVITY

PRINT-INFO

RCHRES	***** Printout Flags *****											PIVL	PYR
# - #	HYDR	ADCA	CONS	HEAT	SED	GQL	OXRX	NUTR	PLNK	PHCB	*****		
1 999	5	0	0	0	0	0	0	0	0	0	0	1	9

END PRINT-INFO

HYDR-PARM1

RCHRES	Flags for each HYDR Section														*****					
# - #	VC	A1	A2	A3	ODFVFG for each					ODGTFG for each				FUNCT for each						
	FG	FG	FG	FG	possible exit					possible exit				possible exit						
	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	
1	0	1	0	0	4	5	0	0	0	0	0	0	0	0	0	2	2	2	2	2
2	0	0	0	0	4	0	0	0	0	0	0	0	0	0	0	2	2	2	2	2
3	0	0	0	0	4	0	0	0	0	0	0	0	0	0	0	2	2	2	2	2
4	0	1	0	0	4	5	0	0	0	0	0	0	0	0	0	2	2	2	2	2
5	12	0	0	0	4	0	0	0	0	0	0	0	0	0	0	2	2	2	2	2
13		0	1	0	4	5	0	0	0	0	0	0	0	0	0	2	2	2	2	2
14	22	0	0	0	4	0	0	0	0	0	0	0	0	0	0	2	2	2	2	2
23		0	1	0	4	5	0	0	0	0	0	0	0	0	0	2	2	2	2	2

24	33	0	0	0	0	4	0	0	0	0	0	0	0	0	2	2	2	2	2
34		0	1	0	0	4	5	0	0	0	0	0	0	0	2	2	2	2	2
35	999	0	0	0	0	4	0	0	0	0	0	0	0	0	2	2	2	2	2

END HYDR-PARM1

HYDR-PARM2

RCHRES		FTABNO	LEN	DELTH	STCOR	KS	DB50	***
#	- #							***
1		1	0.010			0.3		***
2		2	0.776			0.3		***
3		3	0.980			0.3		***
4		4	0.010			0.3		***
5		5	0.380			0.3		***
10		10	0.380			0.3		***
11		11	0.010			0.3		***
12		12	1.000			0.3		***
13		13	0.015			0.3		***
14		14	0.450			0.3		***
15		15	0.735			0.3		***
16		16	0.587			0.3		***
17		17	0.379			0.3		***
23		23	0.379		300.0	0.3		***
24		24	0.379			0.3		***
33		33	0.200			0.3		***
34		34	0.852			0.3		***
35		35	0.663			0.3		***
38		38	0.010			0.3		***
50		50	0.010			0.3		***
52		52	0.010			0.3		***
53		53	0.010			0.3		***
54		54	0.010			0.3		***
135		135	0.350			0.3		***

END HYDR-PARM2

HYDR-INIT

RCHRES		Initial conditions for each HYDR section	***
#	- #	VOL	Initial value of COLIND
		*** ac-ft	for each possible exit
			Initial value of OUTDGT
			for each possible exit
1		2.0	4.0 5.0
2		0.0	4.0
3		0.0	4.0 5.0
4		2.0	4.0
5		0.0	4.0
10		0.0	4.0
11		0.0	4.0
12		0.0	4.0
13		10.0	4.0 5.0
14		0.0	4.0
15		0.0	4.0
16		0.0	4.0
17		0.0	4.0
23		6.0	4.0 5.0
24		0.0	4.0
33		0.0	4.0
34		9.0	4.0 5.0
35		0.1	4.0
38		0.1	4.0

50	0.0	4.0
52	0.0	4.0
53	0.1	4.0
54	2.25	4.0
135	0.00	4.0

END HYDR-INIT  
END RCHRES

FTABLES  
\*\*\*UPPER BASIN  
\*\*\*=====

FTABLE 1  
ROWS COLS \*\*\*  
11 5

DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2***
0.00	3.00	0.00	0.00	0.00
2.50	3.00	7.50	0.00	0.11
3.00	3.00	9.00	1.80	0.11
3.50	3.30	10.58	5.00	0.11
4.00	3.60	12.30	10.90	0.11
4.50	3.90	14.18	17.50	0.11
5.00	4.10	16.18	26.20	0.11
5.50	4.30	18.28	32.50	0.11
6.00	4.50	20.48	35.90	0.11
7.00	5.00	25.23	38.10	0.11
8.00	5.50	30.48	46.40	0.11

END FTABLE 1

FTABLE 2  
ROWS COLS \*\*\*  
9 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.2571	0.0129	0.16	
0.500	0.3873	0.1417	6.53	
1.000	0.5501	0.3761	25.95	
1.500	0.7128	0.6918	59.86	
2.000	0.8756	1.0889	110.67	
3.000	1.2011	2.1273	272.24	
3.500	1.3639	2.7685	387.38	
4.000	1.5266	3.4912	528.19	

END FTABLE 2

FTABLE 3  
ROWS COLS \*\*\*  
12 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.9669	0.0483	0.13	
0.500	1.0637	0.4545	4.92	
1.000	1.1846	1.0165	17.12	
1.500	1.3055	1.6390	34.92	
2.000	1.4264	2.3220	57.95	
2.500	1.5473	3.0654	86.14	
3.000	1.6682	3.8693	119.53	
3.500	1.7891	4.7336	158.24	

4.000	1.9100	5.6584	202.41
4.500	2.0294	6.6310	251.52
5.000	2.1488	7.6624	306.28

END FTABLE 3

FTABLE 4  
ROWS COLS \*\*\*  
7 5

DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2***
0.00	3.00	0.00	0.00	0.00
2.50	4.50	9.38	0.00	0.11
3.00	6.00	12.00	6.00	0.11
4.00	10.00	20.00	13.00	0.11
5.00	15.00	32.50	20.00	0.11
6.00	20.00	50.00	26.00	0.11
7.00	25.00	72.50	168.00	0.11

END FTABLE 4

FTABLE 5  
ROWS COLS \*\*\*  
10 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.1010	0.0051	0.03	
0.500	0.1754	0.0603	1.46	
1.000	0.2684	0.1713	6.16	
1.500	0.3614	0.3288	14.89	
2.000	0.4544	0.5327	28.48	
2.500	0.5474	0.7832	47.70	
3.000	0.6404	1.0801	73.29	
3.500	0.7334	1.4236	105.94	
4.000	0.8264	1.8136	146.33	

END FTABLE 5

FTABLE 10  
ROWS COLS \*\*\*  
9 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.1010	0.0051	0.06	
0.500	0.1660	0.0585	2.27	
1.000	0.2472	0.1618	9.32	
1.500	0.3285	0.3057	22.08	
2.000	0.4097	0.4902	41.66	
2.500	0.4909	0.7154	69.09	
3.000	0.5722	0.9811	105.37	
4.000	0.6887	1.6116	209.70	

END FTABLE 10

POST AMBAUM DETENTION \*\*\*  
FTABLE 11  
ROWS COLS \*\*\*  
11 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
1.000	0.1000	0.2300	3.90	
2.000	0.2000	0.6000	6.30	

3.000	0.3000	0.9700	8.10
4.000	0.4000	1.3400	11.10
5.000	0.5000	1.8200	16.00
6.000	0.6000	2.2700	19.10
7.000	0.7000	2.8300	21.60
8.000	0.8000	3.3700	30.80
9.000	0.9000	4.0000	38.10
10.000	1.0000	4.6500	74.10
10.500	1.1000	5.2000	133.00
11.000	1.1500	5.3000	500.00

END FTABLE 11

FTABLE 12  
ROWS COLS \*\*\*  
6 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.6327	0.0316	0.15	
0.500	0.7960	0.3174	5.87	
1.000	1.0002	0.7664	21.53	
1.500	1.2043	1.3176	46.43	
2.000	1.4085	1.9708	81.20	
3.000	1.8168	3.5834	183.79	
4.000	2.2251	5.6044	336.22	
5.000	2.6335	8.0337	545.30	
6.000	3.0418	10.8713	817.51	

END FTABLE 12

FTABLE 13  
ROWS COLS \*\*\*  
7 5

DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2***
0.000	40.000	0.0000	0.00	0.00
1.000	41.400	40.000	0.00	0.11
1.500	42.000	60.000	10.00	0.11
2.000	42.700	80.000	16.00	0.11
2.500	43.300	100.00	20.00	0.11
3.000	44.000	120.00	28.00	0.11
5.000	45.000	210.00	45.00	0.11

END FTABLE 13

FTABLE 14  
ROWS COLS \*\*\*  
6 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.3361	0.0168	0.24	
0.500	0.3809	0.1602	9.04	
1.000	0.4370	0.3647	31.61	
1.500	0.4930	0.5972	65.00	
2.000	0.5491	0.8577	108.85	
2.500	0.6051	1.1462	163.33	
3.000	0.6612	1.4628	228.78	

END FTABLE 14

FTABLE 15  
ROWS COLS \*\*\*

```

4      4
DEPTH  AREA  VOLUME  OUTFLOW  ***
0.00   0.10   0.00    0.00
1.00   1.00   0.55    91.00
2.00   1.10   1.60   268.00
3.00   1.20   2.75   493.00
END FTABLE 15

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FTABLE      16
ROWS COLS  ***
4      4
DEPTH  AREA  VOLUME  OUTFLOW  ***
0.00   0.10   0.00    0.00
1.00   1.00   0.55    74.00
2.00   1.10   1.60   219.00
3.00   1.20   2.75   403.00
END FTABLE 16

```

```

FTABLE      17
ROWS COLS  ***
5      4
DEPTH  AREA  VOLUME  OUTFLOW  ***
0.00   0.10   0.00    0.00
1.00   1.00   0.55    59.00
2.00   1.10   1.60   173.00
3.00   1.20   2.75   318.00
4.00   1.30   4.00   484.00
END FTABLE 17

```

```

FTABLE      23
ROWS COLS  ***  HERMES
9      5
DEPTH  AREA  VOLUME  OUTFLOW  OUTFLOW  ***
0.00   0.00   0.00    0.00    0.00    0.00
5.00   0.50   1.91    0.00    0.00   305.00
11.00  0.79   5.79    0.00    0.00   311.00
15.00  1.13   9.64    0.50    0.01   315.00
19.00  1.72  15.34    0.50    0.05   319.00
29.00  2.86  38.25    0.50    0.10   329.00
39.00  4.40  74.55    0.50    0.20   339.00
50.00  6.22 132.98    0.50    0.30   350.00
60.00 10.00 212.98    0.50    0.40   360.00
END FTABLE 23

```

```

FTABLE      24
ROWS COLS  ***
9      4
DEPTH  AREA  VOLUME  OUTFLOW  ***
0.0000 0.0000 0.0000 0.00
0.100  0.2571 0.0129 0.16
0.500  0.3873 0.1417 6.53
1.000  0.5501 0.3761 25.95
1.500  0.7128 0.6918 59.86
2.000  0.8756 1.0889 110.67
3.000  1.2011 2.1273 272.24
3.500  1.3639 2.7685 387.38
4.000  1.5266 3.4912 528.19

```

END FTABLE 24

FTABLE 33  
ROWS COLS \*\*\*

11	4				***
DEPTH	AREA	VOLUME	OUTFLOW		
0.00	1.00	0.00	0.00		
0.50	1.20	0.55	2.00		
1.00	1.40	1.20	6.00		
1.50	1.60	1.95	9.00		
2.00	1.80	2.80	13.00		
2.50	2.00	3.75	16.50		
3.00	2.20	4.80	20.00		
3.50	2.40	5.95	23.00		
4.00	2.60	7.20	26.00		
5.00	2.80	9.90	104.00		
6.00	3.00	12.80	246.00		

END FTABLE 33

FTABLE 34  
ROWS COLS \*\*\* REVISED 11/19/97 BASED ON HEC-RAS MODEL

6	5					***
DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2		
0.00	3.00	0.00	0.00	0.00		
3.00	3.05	9.08	0.00	0.11		
4.00	3.10	12.15	0.00	0.11		
5.00	3.15	15.28	0.00	0.11		
6.00	3.20	18.45	72.0	0.11		
7.00	3.25	21.68	225.0	0.11		

END FTABLE 34

FTABLE 35  
ROWS COLS \*\*\* REVISED 11/19/97 BASED ON HECRAS MODEL

5	4				***
DEPTH	AREA	VOLUME	OUTFLOW		
0.00	0.10	0.00	0.00		
1.00	1.10	0.60	38.00		
2.00	1.20	1.75	108.00		
3.00	1.30	3.00	194.00		
4.00	1.40	4.35	290.00		

END FTABLE 35

FTABLE 38  
ROWS COLS \*\*\*

7	4				***
DEPTH	AREA	VOLUME	OUTFLOW		
0.000	0.0000	0.0000	0.00		
1.000	0.4000	0.4000	2.00		
1.500	0.5000	1.0000	4.00		
2.000	0.9000	1.3000	11.00		
2.500	1.3000	1.6000	15.00		
3.000	1.6000	2.0000	18.00		
3.500	1.9000	2.5000	20.80		

END FTABLE 38

FTABLE 45  
ROWS COLS \*\*\*



NORTH EMPLOYEE PARKING LOT VAULT (AS-BUILT)\*\*\*

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.2200	0.0000	0.00	
2.000	0.2200	0.4500	1.20	
4.000	0.2200	0.9000	1.70	
6.000	0.2200	1.3400	2.10	
8.000	0.2200	1.7900	2.40	
10.000	0.2200	2.2400	2.70	
12.240	0.2200	2.7400	3.00	
14.000	0.2200	3.1400	6.90	
15.440	0.2200	3.4600	8.30	
16.000	0.2200	3.5800	10.30	
18.000	0.2200	4.0300	13.60	
20.000	0.2200	4.4800	30.79	

END FTABLE 45

FTABLE 50  
ROWS COLS \*\*\*

DEPTH	AREA	VOLUME	OUTFLOW	***
0.00	1.00	0.00	0.00	
0.50	1.10	0.53	5.00	
1.00	1.20	1.10	15.00	
1.50	1.30	1.73	25.00	
2.00	1.40	2.40	35.00	
2.50	1.50	3.13	52.00	
3.00	1.60	3.90	70.00	
3.50	1.70	4.73	87.00	
4.00	1.80	5.60	105.00	
6.00	1.90	9.30	165.00	

END FTABLE 50

FTABLE 52  
ROWS COLS \*\*\*

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.3680	0.0184	0.25	
0.500	0.3717	0.1664	9.39	
1.000	0.3763	0.3534	31.06	
2.000	0.3819	0.7325	94.37	
3.000	0.3874	1.1171	174.33	

END FTABLE 52

FTABLE 53  
OLD LAKE REBA \*\*\*  
MAX DEPTH = 4.9 FEET \*\*\*  
30" CMP, 40 CFS DISCHARGE AT MAX DEPTH \*\*\*  
ROWS COLS \*\*\*

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	2.4000	0.0000	0.00	
1.000	2.5800	2.5000	18.00	
2.000	2.9400	5.3000	26.00	
3.000	3.4100	8.4000	31.00	
4.000	3.8800	12.100	36.00	

4.900	4.3000	15.800	40.00
6.000	4.3000	15.810	500.00

END FTABLE 53

FTABLE 54  
 EXISTING MILLER CREEK DETENTION FACILITY\*\*\* REVISED STORAGE/Q DATA  
 GATE SETTING: 2.0 FEET\*\*\*

ROWS COLS \*\*\*  
 12 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.00	0.00	0.00	
1.300	0.01	0.01	10.00	
2.000	0.01	0.02	20.00	
2.900	0.70	0.40	30.00	
4.000	1.50	1.50	40.00	
5.400	3.50	4.90	50.00	
7.000	8.60	13.30	60.00	
8.800	15.60	34.80	70.00	
10.000	19.90	57.30	76.00	
10.500	21.50	68.00	92.00	
11.000	23.10	78.80	179.00	
11.500	24.70	88.60	303.00	

END FTABLE 54

PRE AMBAUM DETENTION \*\*\*  
 FTABLE 111  
 ROWS COLS \*\*\*  
 12 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.500	0.2160	0.0750	5.30	
1.000	0.2730	0.1990	21.10	
1.500	0.2890	0.3410	43.90	
2.000	0.2900	0.4830	68.80	
2.500	0.2910	0.6070	89.10	
3.000	0.2950	0.6820	90.00	
3.500	0.3000	2.1000	100.00	
4.000	0.3050	2.5000	105.00	
4.500	0.3100	3.0000	110.00	
5.000	0.3200	3.5000	120.00	
5.500	0.3300	4.0000	130.00	

END FTABLE111

FTABLE 135  
 ROWS COLS \*\*\* VACA FARM  
 6 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.00	0.10	0.00	0.00	
1.00	0.10	0.10	4.00	
2.00	0.11	0.21	8.00	
2.50	1.00	0.48	13.00	
3.50	6.50	4.23	86.00	
4.50	13.00	13.98	235.00	

END FTABLE135

\*\*\*  
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END FTABLES

```
MASS-LINK
<Volume>  <-Grp> <-Member-><--Mult-->  <Target>  <-Grp> <-Member->***
<Name>    <Name> # #<-factor->  <Name>  <Name> # #***
  MASS-LINK      1
  conversion from acre-inches to acre-ft (1/12)      ***
PERLND  PWATER  PERO      0.0833333  RCHRES  INFLOW  IVOL
  END MASS-LINK      1

  MASS-LINK      2
IMPLND  IWATER  SURO      0.0833333  RCHRES  INFLOW  IVOL
  END MASS-LINK      2

  MASS-LINK      3
RCHRES  ROFLOW  RCHRES  INFLOW
  END MASS-LINK      3

  MASS-LINK      4
RCHRES  OFLOW  OVOL      1  RCHRES  INFLOW  IVOL
  END MASS-LINK      4

  MASS-LINK      5
RCHRES  OFLOW  OVOL      2  RCHRES  INFLOW  IVOL
  END MASS-LINK      5

  MASS-LINK      6
PERLND  PWATER  SURO      0.0833333  RCHRES  INFLOW  IVOL
PERLND  PWATER  IFWO      0.0833333  RCHRES  INFLOW  IVOL
  END MASS-LINK      6

  MASS-LINK      7
PERLND  PWATER  AGWO      0.0833333  RCHRES  INFLOW  IVOL
  END MASS-LINK      7

  MASS-LINK      8
PERLND  PWATER  PERO      0.0833333  COPY    INPUT  MEAN
  END MASS-LINK      8
  MASS-LINK      12
PERLND  PWATER  AGWO      0.0833333  COPY    INPUT  MEAN
  END MASS-LINK      12

  MASS-LINK      9
IMPLND  IWATER  SURO      0.0833333  COPY    INPUT  MEAN
  END MASS-LINK      9

  MASS-LINK      10
COPY    OUTPUT  MEAN      RCHRES  INFLOW  IVOL
  END MASS-LINK      10

END MASS-LINK
END RUN
```

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**WALKER CREEK HSPF CALIBRATION  
MODEL INPUT FILE**

This page included for guidance only.  
(Not to be included in document.)

**AR 011052**

RUN

GLOBAL

\*\*\* FILE: WALKER21.inp REVISED March 2001 Joe Brascher ATC  
 \*\*\* SEATAC AIRPORT HSPF BASIN MODEL OF WALKER CREEK (Calibration)  
 \*\*\* DEVELOPED BY PARAMETRIX (JRD) 2000 BASED ON 1999 SMP MODEL (KWR)  
 WALKER CREEK BASIN HSPF MODEL  
 START 1989 01 1 0 0 END 1996 8 30 24 0  
 RUN INTERP OUTPUT LEVEL 3  
 RESUME 0 RUN 1

END GLOBAL

FILES

<type> <fun>\*\*\*<-----fname----->  
 MESSU 24 D:\PARA\SEATAC\MILLER\WALKER.MES  
 WDM 25 D:\PARA\SEATAC\MILLER\MILL.WDM  
 61 D:\PARA\SEATAC\MILLER\WPER.L61  
 62 D:\PARA\SEATAC\MILLER\WRCH.L62

END FILES

OPN SEQUENCE

INGRP INDELT 01:00  
 PERLND 14  
 PERLND 16  
 PERLND 18  
 PERLND 24  
 PERLND 26  
 PERLND 28  
 PERLND 34  
 PERLND 44  
 PERLND 45  
 PERLND 54  
 PERLND 64  
 PERLND 65  
 IMPLND 14  
 RCHRES 20  
 RCHRES 19  
 RCHRES 18

END INGRP

END OPN SEQUENCE

\*\*\*

COPY

TIMESERIES

Copy-opn

# - # NPT NMN  
 1 5 1

END TIMESERIES

END COPY

\*\*\*

\*\*\*

PERLND

GEN-INFO

<PLS >

# - #

#	-	#	Name	NBLKS	Unit-systems		Printer	
					User	t-series	Engl	Metr
					in	out		
14			TFF- TILL FOR FLT	1	1	1	61	0
16			TFM- TILL FOR MOD	1	1	1	61	0
18			TFS- TILL FOR STP	1	1	1	61	0
24			TGF- TILL GR FLT	1	1	1	61	0

\*\*\*

\*\*\*

\*\*\*

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```

26      TGM- TILL GR MOD      1   1   1   1   61   0
28      TGS- TILL GR STP      1   1   1   1   61   0
34      OF - OUTWASH FOR      1   1   1   1   61   0
44      OG - OUTWASH GR       1   1   1   1   61   0
***PERLND FOR NEW AIRPORT FILL; NONE IN CALIBRATION
45      AIRPORT FILL          1   1   1   1   61   0
54      SA - WETLANDS         1   1   1   1   61   0
64      TGM DES MOINES        1   1   1   1   61   0
65      OG DES MOINES         1   1   1   1   61   0

```

END GEN-INFO

ACTIVITY

<PLS > \*\*\*\*\* Active Sections \*\*\*\*\*

```

# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC      ***
14 200  0  0  1  0  0  0  0  0  0  0  0  0  0

```

END ACTIVITY

PRINT-INFO

<PLS > \*\*\*\*\* Print-flags \*\*\*\*\* PIVL PYR

```

# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC *****
14 200  0  0  5  0  0  0  0  0  0  0  0  0  1  9

```

END PRINT-INFO

PWAT-PARM1

<PLS > \*\*\*\*\* Flags \*\*\*\*\*

```

# - # CSNO RTOP UZFG VCS VUZ VNN VIFW VIRC VLE      ***
14      0  0  0  0  0  0  0  0  0  0
16      0  0  0  0  0  0  0  0  0  0
18      0  0  0  0  0  0  0  0  0  0
24      0  0  0  0  0  0  0  0  0  0
26      0  0  0  0  0  0  0  0  0  0
28      0  0  0  0  0  0  0  0  0  0
34      0  0  0  0  0  0  0  0  0  0
44      0  0  0  0  0  0  0  0  0  0
45      0  0  0  0  0  0  0  0  0  0
54      0  0  0  0  0  0  0  0  0  0
64      0  0  0  0  0  0  0  0  0  0

```

END PWAT-PARM1

PWAT-PARM2

<PLS > \*\*\*

```

# - # ***FOREST      LZSN      INFILT      LSUR      SLSUR      KVARY      AGWRC
14      4.5000      0.0800      400.00      0.0500      0.5000      0.9960
16      4.5000      0.0800      400.00      0.1000      0.5000      0.9960
18      4.5000      0.0800      200.00      0.2000      0.5000      0.9960
24      4.5000      0.0300      400.00      0.0500      0.5000      0.9960
26      4.5000      0.0300      400.00      0.1000      0.5000      0.9960
28      4.5000      0.0300      200.00      0.2000      0.5000      0.9960
34      5.0000      2.0000      400.00      0.0500      0.3000      0.9960
44      5.0000      0.8000      400.00      0.0500      0.3000      0.9960
45      7.5000      0.0200      300.00      0.0700      0.0000      0.9960
54      4.0000      2.0000      100.00      0.0010      0.5000      0.9960
64      4.5000      0.1200      400.00      0.1000      0.5000      0.9990
65      5.0000      0.8000      400.00      0.0500      0.5000      0.9960

```

END PWAT-PARM2

PWAT-PARM3

<PLS >\*\*\*

```

# - #*** PETMAX      PETMIN      INFEXP      INFILD      DEEPFR      BASETP      AGWETP
14      2.0000      2.0000      0.00      0.00      0.00      0.0
16      2.0000      2.0000      0.00      0.00      0.00      0.0
18      2.0000      2.0000      0.00      0.00      0.00      0.0

```

24	2.0000	2.0000	0.00	0.00	0.
26	2.0000	2.0000	0.00	0.	0.
28	2.0000	2.0000	0.00	0.	0.
34	2.0000	2.0000	0.00	0.00	0.0
44	2.0000	2.0000	0.00	0.	0.
45	2.0000	2.0000	0.00	0.	0.
54	10.000	2.0000	0.00	0.	0.7
64	2.0000	2.0000	0.00	0.	0.0

END PWAT-PARM3

PWAT-PARM4

```
<PLS >                                     ***
# - #      CEPSC      UZSN      NSUR      INTFW      IRC      LZETP***
14          0.2000     1.0000     0.3500     2.000     0.1500     0.7000
16          0.2000     0.5000     0.3500     2.000     0.1500     0.7000
18          0.2000     0.3000     0.3500     2.000     0.1500     0.7000
24          0.1000     0.5000     0.2500     2.000     0.1500     0.2500
26          0.1000     0.2500     0.2500     2.000     0.1500     0.2500
28          0.1000     0.1500     0.2500     2.000     0.1500     0.2500
34          0.2000     0.5000     0.3500     0.000     0.5000     0.7000
44          0.1000     0.5000     0.2500     0.000     0.5000     0.2500
45          0.1000     0.2800     0.2500     0.000     0.5000     0.2500
54          0.1000     3.0000     0.5000     1.000     0.7000     0.8000
64          0.1000     0.2500     0.2500     3.000     0.5000     0.2500
65          0.1000     0.5000     0.2500     0.000     0.5000     0.2500
```

END PWAT-PARM4

PWAT-STATE1

```
<PLS > PWATER state variables***
# - #***    CEPS      SURS      UZS      IFWS      LZS      AGWS      GWVS
14          0.078     0.      0.2500     0.10     2.500     2.00     0.000
16          0.078     0.      0.2500     0.10     2.500     2.00     0.000
18          0.078     0.      0.2500     0.10     2.500     2.00     0.000
24          0.051     0.      0.2500     0.10     2.500     2.00     0.000
26          0.051     0.      0.2500     0.10     2.500     2.00     0.000
28          0.051     0.      0.2500     0.10     2.500     2.00     0.000
34          0.078     0.      0.2500     0.10     0.000     2.00     0.000
44          0.051     0.      0.2500     0.10     0.000     2.00     0.000
45          0.051     0.      0.2500     0.10     0.000     2.00     0.000
54          0.051     0.      0.2500     0.10     2.000     2.00     0.000
64          0.051     0.      0.2500     0.10     2.000     20.00     0.000
65          0.051     0.      0.2500     0.10     0.000     20.00     0.000
```

END PWAT-STATE1

END PERLND

IMPLND

GEN-INFO

```
<ILS >      Name      Unit-systems      Printer      ***
# - #      User t-series Engl Metr      ***
              in out      ***
14      IMPERVIOUS      1      1      1      60      0
```

END GEN-INFO

ACTIVITY

```
<ILS > ***** Active Sections ****
# - # ATMP SNOW IWAT SLD IWG IQAL ***
14      0      0      1      0      0      0
```

END ACTIVITY

PRINT-INFO

```
<ILS > ***** Print-flags ***** PIVL PYR
# - # ATMP SNOW IWAT SLD IWG IQAL *****
```

```

14      0      0      6      0      0      0      1      9
END PRINT-INFO
IWAT-PARM1
  <ILS >          Flags          ***
  # - # CSNO RTOP  VRS  VNN RTLI  ***
14      0      0      0      0      0
END IWAT-PARM1
IWAT-PARM2
  <ILS >          ***
  # - #      LSUR      SLSUR      NSUR      RETSC      ***
14      100.00      0.0100      0.1000      0.1000
END IWAT-PARM2
IWAT-PARM3
  <ILS >          ***
  # - #      PETMAX      PETMIN          ***
14
END IWAT-PARM3
IWAT-STATE1
  <ILS > IWATER state variables          ***
  # - #      RETS      SURS          ***
14      1.0000E-3 1.0000E-3
END IWAT-STATE1
END IMPLND
EXT SOURCES
*** NOTE: The only RCHRES that precip and PET are applied to are lakes.
***
<-Volume-> <Member> SsysSgap<--Mult-->Tran <-Target vols> <-Grp> <-Member-> ***
<Name> # <Name> # tem strg<-factor->strg <Name> # # <Name> # # ***
*** PRECIP/EVAP TO PERVIOUS/IMPERV SURFACES
WDM      2 PREC      ENGLZERO 1.00      PERLND 14 200 EXTNL PREC
WDM      2 PREC      ENGLZERO 1.00      IMPLND 14      EXTNL PREC
WDM      1010 EVAP      ENGLZERO 0.8      PERLND 14 65 EXTNL PETINP
WDM      1010 EVAP      ENGLZERO 0.8      IMPLND 14      EXTNL PETINP
*** PRECIP/EVAP TO LAKES
WDM      2 PREC      ENGLZERO          RCHRES 20      EXTNL PREC
WDM      1010 EVAP      ENGLZERO 0.8      RCHRES 20      EXTNL POTEV
END EXT SOURCES
EXT TARGETS
<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Volume-> <Member> Tsys Tgap Amd ***
<Name> # <Name> # #<-factor->strg <Name> # <Name> tem strg strg***
*** GAUGE POINTS (17=MOUTH, 54=MILLER RDF, 50=SR 518, 18=WALKER CK)
RCHRES 18 HYDR RO          WDM 36 FLOW ENGL REPL
*** MISC (20=WALKER WETLAND, 55=SR509, 56=1ST AVE)
RCHRES 20 HYDR RO          WDM 37 FLOW ENGL REPL
RCHRES 20 HYDR STAGE 1 1 *** WDM 1037 STAG ENGL REPL
<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Volume-> <Member> Tsys Tgap Amd ***
<Name> # <Name> # #<-factor->strg <Name> # <Name> tem strg strg***
***WALKER NR MTH
RCHRES 18 HYDR RO 1 1 0.000764 WDM 50 SIMQ ENGL REPL
RCHRES 20 HYDR RO 1 1 0.006520 WDM 1051 SIMQ ENGL REPL
END EXT TARGETS
SCHEMATIC

```

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<-Source-> <Name> #	<--Area--> <-factor-->	<-Target-> <Name> #	MBLK Tbl#	*** ***
***WALKER CREEK				
*** SUB-CATCHMENT MC 8				
PERLND 26	5.19	RCHRES 20	1	
PERLND 44	20.92	RCHRES 20	1	
PERLND 54	2.91	RCHRES 20	1	
IMPLND 14	1.79	RCHRES 20	2	
*** SUB-CATCHMENT MC 8B				
PERLND 26	19.21	RCHRES 20	1	
PERLND 44	15.45	RCHRES 20	1	
PERLND 54	0.61	RCHRES 20	1	
IMPLND 14	1.71	RCHRES 20	2	
*** SUB-CATCHMENT MC 9				
PERLND 26	9.33	RCHRES 20	1	
PERLND 44	3.93	RCHRES 20	1	
PERLND 54	0.08	RCHRES 20	1	
IMPLND 14	0.32	RCHRES 20	2	
*** SUB-CATCHMENT 18				
PERLND 16	0.76	RCHRES 18	1	
PERLND 26	16.08	RCHRES 18	1	
PERLND 34	20.95	RCHRES 18	1	
PERLND 44	49.22	RCHRES 18	1	
IMPLND 14	3.30	RCHRES 18	2	
*** SUB-CATCHMENT 19				
PERLND 16	12.72	RCHRES 19	1	
PERLND 26	92.07	RCHRES 19	1	
PERLND 34	8.39	RCHRES 19	1	
PERLND 44	95.55	RCHRES 19	1	
IMPLND 14	30.53	RCHRES 19	2	
*** SUB-CATCHMENT 20				
PERLND 26	12.53	RCHRES 20	1	
PERLND 44	53.43	RCHRES 20	1	
PERLND 54	33.43	RCHRES 20	1	
IMPLND 14	52.83	RCHRES 20	2	
*** DOWN STREAM OF WALKER CREEK GAGE				
*** SUB-CATCHMENT 21 33% OF GW GOES TO GAGE REST GOES TO SOUND				
PERLND 16	2.54	RCHRES 18	7	
PERLND 26	44.30	RCHRES 18	7	
PERLND 34	2.03	RCHRES 18	7	
PERLND 44	41.13	RCHRES 18	7	
*** SUB-CATCHMENT 22				
PERLND 34	4.30	RCHRES 22	1	
PERLND 44	19.49	RCHRES 22	1	
PERLND 54	3.21	RCHRES 22	1	
IMPLND 14	3.95	RCHRES 22	2	
***GROUNDWATER FROM OUTSIDE OF WALKER CREEK				
PERLND 64	630.00	RCHRES 20	7	
PERLND 65	*** 130.00	RCHRES 20	7	
***STREAM ROUTING				
RCHRES 20		RCHRES 19	3	
RCHRES 19		RCHRES 18	3	

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END SCHEMATIC

NETWORK

```

***          <MEMBER> SSYSSGAP<--MULT-->TRAN <-TARGET VOLS>          <-MEMBER->
<NAME>    # <NAME>    TEM STRG<-FACTOR->STRG <NAME>    #    # <-GRP> <NAME> # # ***
***

```

END NETWORK

RCHRES

GEN-INFO

```

RCHRES          Name          Nexits    Unit Systems    Printer          ***
# - #<-----><----> User T-series  Engl Metr LKFG          ***
              in  out          ***
18    Trib (0371A) M 18          1    1    1    1    62    0    0
19    Trib (0371A) M 19          1    1    1    1    62    0    0
20    Trib    M 20              1    1    1    1    62    0    1
21    Trib (0371H) M 21          1    1    1    1    62    0    0
22    Trib (0371A) M 22          1    1    1    1    62    0    0

```

END GEN-INFO

ACTIVITY

```

RCHRES ***** Active Sections *****
# - # HYFG ADFG CNFG HTFG SDFG GQFG OXFG NUFG PKFG PHFG          ***
1  63  1  0  0  0  0  0  0  0  0  0  0

```

END ACTIVITY

PRINT-INFO

```

RCHRES ***** Printout Flags ***** PIVL  PYR
# - # HYDR ADCA CONS HEAT  SED  GQL OXRX NUTR PLNK PHCB *****
1  63  5  0  0  0  0  0  0  0  0  0  0  1  9

```

END PRINT-INFO

HYDR-PARM1

```

RCHRES  Flags for each HYDR Section          ***
# - # VC A1 A2 A3  ODFVFG for each *** ODGTFG for each  FUNCT for each
      FG FG FG FG  possible exit *** possible exit  possible exit
      * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *
1  99  0  1  0  0    4  0  0  0  0    0  0  0  0  0    2  2  2  2  2

```

END HYDR-PARM1

HYDR-PARM2

```

RCHRES          ***
# - #          FTABNO          LEN          DELTH          STCOR          KS          DB50          ***
<-----><-----><-----><-----><-----><-----><----->
18          18    0.800          0.3
19          19    0.568          0.3
20          20    0.379          0.3
21          21    0.450          0.3
22          22    0.300          0.3

```

END HYDR-PARM2

HYDR-INIT

```

RCHRES  Initial conditions for each HYDR section          ***
# - # *** VOL          Initial value of COLIND          Initial value of OUTDGT
      *** ac-ft          for each possible exit          for each possible exit
<-----><----->          <-----><-----><-----><----->          *** <-----><-----><-----><----->
18          0.1          4.0
19          0.1          4.0

```

20	10.0	4.0
21	0.1	4.0
22	0.1	4.0

END HYDR-INIT  
END RCHRES

FTABLES

FTABLE 18  
ROWS COLS \*\*\*  
3 4  
DEPTH AREA VOLUME OUTFLOW \*\*\*  
0.00 1.30 0.00 0.00  
1.00 1.30 1.30 166.00  
2.00 1.40 2.65 490.00  
END FTABLE 18

FTABLE 19  
ROWS COLS \*\*\*  
3 4  
DEPTH AREA VOLUME OUTFLOW \*\*\*  
0.00 1.10 0.00 0.00  
1.00 1.10 1.10 65.00  
2.00 1.20 2.25 223.00  
END FTABLE 19

FTABLE 20  
\*\*\* WALKER CREEK WETLAND  
ROWS COLS \*\*\*  
10 4  
DEPTH AREA VOLUME OUTFLOW OUTFLOW \*\*\*  
0.00 0.00 0.00 0.00  
1.00 2.50 1.25 7.04  
2.00 5.00 5.00 17.84  
3.00 12.00 13.50 32.17  
4.00 19.00 29.00 45.13  
5.00 22.00 49.50 54.95  
6.00 23.00 72.00 61.62  
6.10 23.00 74.30 62.15  
7.00 23.50 95.25 67.00  
7.24 24.10 101.10 100.00  
END FTABLE 20

FTABLE 21  
ROWS COLS \*\*\*  
8 4  
DEPTH AREA VOLUME OUTFLOW \*\*\*  
0.000 0.0000 0.0000 0.00  
0.100 0.2259 0.0113 0.11  
0.500 0.2707 0.1106 4.27  
1.000 0.3268 0.2600 15.13  
1.500 0.3828 0.4374 31.67  
2.000 0.4389 0.6428 54.02  
2.500 0.4949 0.8763 82.52  
3.000 0.5510 1.1377 117.55  
END FTABLE 21

```

FTABLE      22
ROWS COLS  ***
  9      4
  DEPTH    AREA    VOLUME    OUTFLOW  ***
  0.000    0.0000  0.0000    0.00
  0.100    0.3680  0.0184    0.25
  0.500    0.3717  0.1664    9.39
  1.000    0.3763  0.3534   31.06
  2.000    0.3819  0.7325   94.37
  3.000    0.3874  1.1171  174.33
  4.000    0.3930  1.5073  265.38
  5.000    0.3985  1.9030  364.68
  6.000    0.4040  2.3043  470.60

```

END FTABLE 22

END FTABLES

MASS-LINK

```

<Volume>  <-Grp> <-Member-><--Mult-->  <Target>  <-Grp> <-Member->***
<Name>      <Name> # #<-factor->  <Name>      <Name> # #***

```

```

MASS-LINK      1
conversion from acre-inches to acre-ft (1/12)      ***
PERLND  PWATER PERO      0.0833333  RCHRES  INFLOW IVOL
END MASS-LINK  1

```

```

MASS-LINK      2
IMPLND  IWATER SURO      0.0833333  RCHRES  INFLOW IVOL
END MASS-LINK  2

```

```

MASS-LINK      3
RCHRES  ROFLOW      RCHRES  INFLOW
END MASS-LINK  3

```

```

MASS-LINK      4
RCHRES  OFLOW  OVOL  1  RCHRES  INFLOW IVOL
END MASS-LINK  4

```

```

MASS-LINK      5
RCHRES  OFLOW  OVOL  2  RCHRES  INFLOW IVOL
END MASS-LINK  5

```

```

MASS-LINK      6
PERLND  PWATER SURO      0.0833333  RCHRES  INFLOW IVOL
PERLND  PWATER IFWO      0.0833333  RCHRES  INFLOW IVOL
END MASS-LINK  6

```

```

MASS-LINK      7
PERLND  PWATER AGWO      0.0833333  RCHRES  INFLOW IVOL
END MASS-LINK  7

```

```

MASS-LINK      8
PERLND  PWATER PERO      0.0833333  COPY    INPUT  MEAN
END MASS-LINK  8

```

```

MASS-LINK      12
PERLND  PWATER AGWO      0.0833333  COPY    INPUT  MEAN
END MASS-LINK  12

```

```

MASS-LINK      9
IMPLND  IWATER SURO      0.0833333  COPY    INPUT  MEAN

```

END MASS-LINK 9  
MASS-LINK 10  
COPY OUTPUT MEAN RCHRES INFLOW IVOL  
END MASS-LINK 10  
END MASS-LINK  
END RUN

SMP Volume 4 Sections  
D, E, F, H, M, Q, W and Z only

## **PREFACE**

### **Comprehensive Stormwater Management Plan Seattle-Tacoma International Airport Master Plan Update Improvements**

**July 2001**

This document contains replacement pages developed in response to comments received from the Washington State Department of Ecology (Ecology) on Volumes 1 through 4 of the December 2000 Comprehensive Stormwater Management Plan (SMP) for the Seattle-Tacoma International Airport Master Plan Update Improvements. A facilitated process was used to document specific revisions required by Ecology to the December 2000 SMP. Each SMP volume contains an itemized list of replacement pages, and the replacement pages are identified by a July 2001 footer.

**SMP VOLUME 4**  
**JULY 2001 REPLACEMENT PAGES**

Preface (before pg. i)

List of Volume 4 Replacement Pages

APPENDIX D (*Entire Appendix replaced*):

List of Pond Plan and Profiles;

Exhibits C131; C132; C133; C133.1; C134; C134.1; C134.2; C134.3; C135;  
C135.1; C136; C136.1; C136.2; C137; C138; C139; C140; C141; C145; C146;  
C147; C148; C149; C150; C151; and D-2

October 27, 2000 Vault Structural Feasibility Memo (1 pg.)

April 23, 2001 Supplemental Vault Structural Feasibility Memo (3 pgs.)

April 21, 2001 Geotechnical Assessment of Constructing Vaults in Fill (2 pgs.)

April 26, 2001 Feasibility of Vault Maintenance Memo (3 pgs.)

APPENDIX F

Pg. 1 of December 14, 2000 Infiltration Memo

May 23, 2001 Infiltration Feasibility Memo for Pond F (10 pgs.)

APPENDIX G:

Section 3.2 (pg. G-5)

Table G-2 (pg. G-6)

APPENDIX H:

Table H-6 (pg. H-10)

APPENDIX Q:

Exhibits C113, C114; C116; C118; C127; C128; C129; and C130

APPENDIX R:

R-4 Pond B Drawing (Sheet C40)

APPENDIX Y:

*Entire Appendix replaced (17 pgs.)*

*Note: Listed page totals do not include "guidance" pages (the guidance pages do not need to be inserted as replacement pages).*



**COMPREHENSIVE STORMWATER MANAGEMENT PLAN**

**VOLUME 4**

**APPENDICES C THROUGH Z**

**SEATTLE-TACOMA INTERNATIONAL AIRPORT  
MASTER PLAN UPDATE IMPROVEMENTS**

**FOR AGENCY REVIEW**

Prepared for

**PORT OF SEATTLE**  
Seattle-Tacoma International Airport  
P.O. Box 69727  
Seattle, Washington 98168-0727

Prepared by  
**PARAMETRIX, INC.**  
5808 Lake Washington Blvd. N.E., Suite 200  
Kirkland, Washington 98033-7350

December 2000  
556-2912-001 (28)

**AR 011064**

## VOLUME 4

- C EXISTING DETENTION FACILITY DATA
- D PROPOSED DETENTION FACILITY FIGURES
- E BMP SIZING ESTIMATIONS PER UNIT PGIS AREA
- F FEASIBILITY OF STORMWATER INFILTRATION REPORT
- G WATER RIGHTS ON MILLER CREEK
- H WATER QUALITY TREATMENT FACILITY SIZING CALCULATIONS FOR SASA, STEP, AND THIRD RUNWAY
- I LANDSCAPE MANAGEMENT PLAN
- J MILLER CREEK FLOODPLAIN ANALYSIS SUMMARY
- K EMBANKMENT AQUIFER CONSTRUCTION MEMO
- L GEOTECHNICAL ENGINEERING REPORT FOR THE THIRD RUNWAY EMBANKMENT CONSTRUCTION
- M WATER QUALITY BMP COST ESTIMATES FOR AREAS DETERMINED TO BE NON-PRACTICABLE FOR RETROFITTING
- N INDUSTRIAL WASTEWATER SYSTEM MONTHLY DISCHARGE MONITORING REPORTS (JANUARY 1999 – OCTOBER 2000)
- O IWS OUTFALL HYDRAULIC CAPACITY MODELING RESULTS
- P DES MOINES CREEK AND MILLER CREEK DOWNSTREAM ANALYSES
- Q PROPOSED STORMWATER CONVEYANCE FIGURES
- R CONSTRUCTION EROSION CONTROL
- S HDR EVALUATION OF STORM CONVEYANCE SYSTEM
- T PHYSICAL PROPERTIES OF PROPOSED ROOFING MEMBRANES AND PRELIMINARY ROOF SURVEY FIGURES
- U RUNWAY/TAXIWAY GRASS INFIELD LANDSCAPE MANAGEMENT PLAN
- V SEATTLE TACOMA INTERNATIONAL AIRPORT SPILL PREVENTION CONTROL AND COUNTERMEASURES PLAN
- W ENERGY DISSIPATION DESIGN CRITERIA AND PARAMETERS
- X SUPPLEMENTAL NEPL SOIL INFORMATION
- Y STORMWATER ANALYSIS OF THE ASR SITE
- Z IWS LAGOON STORAGE CAPACITY MODELING

INTERNATIONAL BOULEVARD (HWY 99)

POTENTIAL STORAGE VOLUMES  
 STORAGE DEPTHS  
 16 AF 5 FEET  
 34 AF 10 FEET  
 55 AF 15 FEET  
 78 AF 20 FEET (SHOWN)  
 VOLUME REQUIRED = 33.4 ACRE FEET

N 11000

Delta Hangar

MIN. ELEV. = 300 FT.  
 MAX. ELEV. = 317-325 FT.

DETENTION FACILITY PROPOSED  
 EXISTING STORM DRAIN  
 OUTFALL LOCATION

SOUTH AVENUE SOUTH

SOUTH AVENUE SOUTH

ALVARADO EIGHT CENTER

CONTROL STRUCTURE

SOUTH ACCESS ROADWAY CORRIDOR

S 188TH STREET

RETAINING WALL

EXISTING 60-INCH STORM DRAIN

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SHEET NO  
D-2

FIGURE D-2  
 NEW STORMWATER DETENTION FACILITY  
 SAS DETENTION FACILITY

PROJECT NAME  
 MASTER PLAN PROJECTS  
 SEATTLE INTERNATIONAL AIRPORT  
 JOB NO. 55-2912-01-28 FILE NAME  
 5529120128-0109

Parametrix, Inc. Quality Service Through Employee Ownership  
 5508 Lake Washington  
 Kirkland, WA 98033  
 Ph: (425) 822-8800  
 Fax: (425) 888-8800  
 Web: www.parametrix.com  
 WASHINGTON  
 SUMMIT  
 KIRKLAND  
 OREGON  
 PORTLAND

July 2001  
 556-2912-001 (28)

**DRAFT**

0 1" = 100'  
 TWO INCHES FULL SCALE  
 IF NOT SCALE ACCORDINGLY  
 SCALE 1" = 100'  
 DATE DECEMBER 2000

NO.	REVISIONS	DATE	BY	DESIGNED
			K. RITLAND	DRAIN
			S. NEWELL	CHECKED
				APPROVED

DATE: 05/04/01  
 INCHES: 29120128-01-01  
 DATE: 05/04/01  
 INCHES: 29120128-01-01

AR 011066

To Michael Cheyne, Program Manager Date 10/27/00  
From Tom Cossette, P.E.  
Subject Port of Seattle – Stormwater Master Plan (SMP)  
Stormwater Detention Vaults – Structural Feasibility

The storage of storm water runoff as part of a comprehensive drainage system is critical to the viability of the proposed Master Plan Improvements at Sea-Tac. As structural engineers who have had experience with large water storage tanks and buried structures, HNTB structural engineering group was asked to assess the feasibility of constructing a number of large buried stormwater detention vaults necessary to meet the requirements of these improvements. The capacity of the vaults currently proposed range from 4.3 to 98.1 acre-feet.

For the purposes of feasibility analysis, geotechnical and soil data was assumed to be similar to that investigated elsewhere on the airport site for buried vault facilities. Based on currently available information and professional knowledge of structural considerations for above and below grade facilities, HNTB offers the following comments:

There are contractors in this country that have built numerous buried water storage facilities under a variety of site-specific conditions. One such contractor, Preload Inc., Garden City, N.Y, has constructed thousands of prestressed concrete water tanks in the one to ten million gallon range, and advertises sizes to 40 million gallons (approx. 123 acre-feet) and beyond. As evidenced by numerous above and below grade potable water storage tanks and industrial chemical storage tanks around the country, steel containment vessels have also been designed and constructed to meet similar applications. It was concluded that the sizes of the proposed vaults are apparently well within what contractors can build with proven construction means and methods.

There are existing stormwater detention vaults on site, between S. 188<sup>th</sup> Way and Runway 34L, with a total capacity of 6.2 acre-feet. They are below grade cast-in-place reinforced concrete box structures, approximately 40 ft. wide x 20 ft. tall x 300 ft. long, with a single interior wall that divides the vault longitudinally into two 20 ft. wide cells. For purposes of evaluating structural feasibility, a simple analysis of replicating the existing structures to obtain the necessary volumes was evaluated. The analysis concluded that similar construction could be used for the proposed 98.1 acre-foot capacity vault. The resulting configuration could be a box structure approximately 260 ft. wide x 26 ft. tall x 680 ft. long, consisting of thirteen 20 ft. wide cells. Furthermore, while the ultimate size of the vault might be constrained by land use features or topography, it is not constrained (horizontally) by structural limitations.

The invert depths and heights of soil cover on the proposed vaults are not excessive and the proposed locations of vaults do not require the capability to support large aircraft loads. No significant structural issues have been identified due to soil cover and surface loadings on the proposed vaults.

Based on structural considerations, stormwater detention vaults of the magnitude shown in the SMP are feasible and well within the engineering and constructability standards and practices used in industry for similar facilities.



The HNTB Companies

600 108th Ave., Suite 400, Bellevue, WA 98004  
(425) 455-3555 • Fax (425) 453-9179 • Architectural Fax (425) 450-2597

## Supplemental Memorandum

To Michael Cheyne, Program Manager Date April 23, 2001

From Tom Cossette/Alan Black

Subject Port of Seattle – Stormwater Master Plan (SMP)  
Stormwater Detention Vaults – Structural Feasibility

HNTB structural engineering group was asked to provide additional assessment for the feasibility of two non-standard vault design conditions that are shown in the Stormwater Master Plan Document: above ground detention/retention vaults and vaults within embankment areas.

### Above Ground Vaults

There are three above ground vaults currently proposed in the Stormwater Master Plan Document.

Vault	Volume	Description of Condition
SDS3	12.9 Ac.Ft.	220' diameter x 18' deep portion above grade varies from 0' on one side to 10' on the opposite side
SDS7	21.4 Ac.Ft.	300' x 135' x 22' deep almost entirely above grade
SDN3	25.6 Ac.Ft.	300' x 187' x 25' deep. Mostly buried but the top 15 feet of one corner would be above grade

Based on currently available information and professional knowledge of structural considerations for these above grade facilities, HNTB offers the following comments:

SDS3 - Circular concrete tanks (vaults) are one of the most common types of water storage structures – whether above ground, partially buried, or fully buried. Preload Inc., Natgun Corp., and Crom Corp. are three examples of contractors who construct these tanks in various diameters and heights and in storage volume capacities that range from 0.6 to 120 Ac.Ft. Design and construction of circular concrete tanks is addressed by publications from the American Concrete Institute (ACI) and the Portland Cement Association (PCA).

SDS7 – This vault could be constructed as rectangular, but further investigation found that two circular vaults would be more cost effective to provide the required storage volume at this location. The two vaults can be hydraulically connected via large diameter pipe(s), either through the walls or through the floor. Penetrations through such structural elements can be accommodated by localized addition of steel reinforcing bars and/or member thickening. See SDS3 for discussion regarding circular tanks.

SDN3 – SeaTac has a an existing buried reinforced concrete stormwater detention vault on site, between S. 188<sup>th</sup> Way and Runway 34L, that is smaller but similar to this proposed vault. Design and construction of partially buried and buried rectangular concrete water storage vaults has a history and a future. A few comparable facilities elsewhere in the United States are described below.

HNTB is completing conceptual design for Lansing’s Art and Education Center that is adjacent to a 30.6 Ac.Ft. partially buried water storage vault that was built in 1938. The water company owner is expecting that the vault will serve their needs for another 50 to 60 years.

A 20.7 Ac.Ft. buried reinforced concrete stormwater storage vault was constructed ten years ago as part of a new hold apron at Chicago’s Midway Airport.

The Norumbega Reservoir, a 350 Ac.Ft. buried water storage vault, is currently being constructed by the Massachusetts Water Resources Authority.

One corner of this vault would be above grade. The differential earth and hydrostatic pressures generated by the sloping fill can be accommodated in the vault design. This generally takes the form of localized additions of steel reinforcing bars and/or member thickening.

### **Vaults within Embankment Areas**

There are two vaults currently proposed to be within the airfield embankment.

Vault	Volume	Description of Condition
SDW1A	13.8 Ac.Ft.	200' x 100' x 30' deep below grade – set as close to edge of embankment as possible to minimize area for aircraft loading design
SDN3A	10.2 Ac.Ft.	Two 120' x 120' x 23' deep below grade vaults connected by a 48-in diameter equalizer pipe– set near the edge of embankment (or retaining wall) as possible to minimize area for aircraft loading design.

Based on currently available information and professional knowledge of structural considerations for these above grade facilities, HNTB offers the following comments:

SDW1A – Structural investigations of a particular conceptual layout for a rectangular vault was made for two contrasting siting conditions. The investigations included 2-dimensional frame analyses to arrive at preliminary foundation, wall, and top slab sizes. The assumed values for the geotechnical soil parameters (soil unit weight, lateral earth pressure, bearing capacity) used to complete the analyses are within a likely range for the soil units to be expected for this project, as presented in the Preliminary Geotechnical Assessment that accompanies this memo.

The first siting condition for which the concept layout was investigated was for the top of the vault to be flush with the 3<sup>rd</sup> runway apron and subject to direct aircraft loading (taxi weight of a 757-200). The second siting condition was for the vault located within the embankment slope such that 40 feet of the top surface would be exposed along the west edge. The dominant loading for this condition results from the approximately 38 feet deep triangular wedge of fill on top of the vault along east side and the accompanying highly differential lateral earth forces.

As a result of these specific structural investigations, HNTB engineers conclude that buried and partially buried rectangular vaults, of comparable order-of-magnitude size, are certainly feasible as a stormwater storage facility choice for this project.

SDN3A – These proposed vaults are of comparable size, and may be sited similar, to SDW1A for which specific structural investigations and affirmative feasibility conclusions were made, as discussed above.

## **PRELIMINARY GEOTECHNICAL ASSESSMENT OF CONSTRUCTING STORM WATER DETENTION VAULTS C<sub>1</sub>, C<sub>2</sub> AND G<sub>1</sub>, IN EMBANKMENT FILL**

Hart Crowser evaluated stability of proposed stormwater vaults C<sub>1</sub>, C<sub>2</sub> and G<sub>1</sub>, located as shown in the December 2000 Comprehensive Stormwater Management Plan, and including alternate locations on the top and bottom of the west facing embankment slope adjacent to Runway Stations 206+44 and near the toe of the embankment slope adjacent to Station 202+47.

1. Slope stability does not present a "critical flaw" for any of the proposed vault locations that we evaluated. Current feasibility level analyses were accomplished for both the 475- and 975-year seismic events. Ranges in factors of safety, with and without subgrade improvements, are slightly affected by vault location, but requirements for subgrade improvement (to provide slope stability) are more or less the same with and without vaults.
2. Embankment fill constructed to date has typically been compacted to minimum percentages of maximum, modified Proctor density of 90 to 92 percent, depending primarily on type of fill material. Actual densities achieved typically vary, but exceed specified minimums. Hart Crowser typically recommends that fill placed to support structures be compacted to at least 95 percent of maximum, modified Proctor density. Although some additional construction effort will be required, Third Runway construction experience to date indicates this can be accomplished within the area needed to support vaults.
3. Allowable bearing pressures for structures in the fill are anticipated to be around 4 kips per square foot and may be higher. This value is in addition to the weight of the fill itself, for embedded structures. Preliminary analyses indicated only nominal setback or embedment for foundations would be required relative to the sloping embankment surface to obtain the recommended allowable bearing. Additional setback or embedment may be appropriate to prevent any potential sloughing from exposing the footing.
4. Total and differential settlement of fill supporting the vaults will vary depending on location, size and sequence of construction. Potential differential settlement of pipes connected to the vault(s) and extending across fill zones compacted to different minimum densities may need to be addressed by settlement monitoring and construction



of piping after completion of primary fill settlement. Hart Crowser does not anticipate this will present any significant construction difficulty.

5. Nominal unit weight of the fill for conceptual design purposes is around 140 pounds per cubic foot (pcf). Lateral earth pressures acting on the vaults under static conditions are anticipated to be around 60 pcf (expressed as an equivalent fluid density) assuming backfill against non-yielding walls. Seismic surcharge on the embedded structures will be comparable to basement structures of comparable size, constructed per UBC.
6. Shallow piping (inlet or outlet) extending down the embankment face can be designed and constructed with conventional procedures, such as seepage collars. Published guidance used for outlet control structures on earth dams provides a good basis for design of the vaults and connecting piping for the Third Runway embankment.



The HNTB Companies

600 108th Ave., Suite 400, Bellevue, WA 98004

(425) 455-3555 • Fax (425) 453-9179 • Architectural Fax (425) 450-2597

# Memorandum

To Michael Cheyne, Program Manager

Date April 26, 2001

From Alan Black

Subject Port of Seattle – Stormwater Master Plan (SMP)  
Stormwater Detention Vaults – Feasibility of Maintenance

The HNTB engineering group was asked to address January 2001 King County comments, Group 5 checklist item 2 requesting additional information on the maintenance feasibility of above ground and deeper than standard detention vaults that are shown in the Stormwater Master Plan.

King County standard detention vaults are described in the King County Surface Water Design Manual as facilities having the following characteristics:

- Maximum depth finished grade to the vault invert shall be 20 feet.
- Access ports located within 50 feet of any point in the vault with round solid locking lids, 3-foot square, locking diamond plate covers.
- Facilities with >1250 square feet (SF) shall include 5' x 10' removable panel
- Meet OSHA confined space requirements including ladders with hand holds provided at inlet and outlet pipe locations and clearly marked entrances to confined space areas.
- Internal walls shall be provided with openings sufficient for maintenance access and sized and situated for access to maintenance "v" in the vault floor.
- Ventilation pipes (12-inch diameter minimum) provided at all four corners to allow for artificial ventilation prior to entry of maintenance personnel.

## Stormwater Master Plan Detention Vault Facilities

Vault Name	Exhibit	Depth*	Floor Area	Configuration
SDW2 Reserve Stormwater Release Vault	C131	16 ft	3600 SF	Below ground
SDS4	C139	20 ft	38,015 SF	Flush with ground at access road.
SDS7	C140	25 ft	47,250 SF	Above ground
SDS3	C141	41 ft	192,500 SF	Below ground
SDN3	C145	27 ft	56,100 SF	Mostly below ground
SDN2/SDN4	C146	23 ft	34,375 SF	Flush with ground at access road.
M6 (NEPL)	C147	28 ft	33,000 SF	Below ground
SDN1	C148	24 ft	21,905 SF	Mostly below ground
SDN6 (CARGO)	C149	18 ft	15,395 SF	Flush with ground
SDN3 (2 Vaults)	C150	25 ft	14,400 SF	Below ground
SDW1A	C151	38 ft	20,000 SF	Below ground

\* An allowance of 2 feet has been used for the top slab thickness to estimate depth where defined by the top of structure. Maximum potential access depth is shown; actual depths vary.

July 2001

556-2912-001 (28)

Michael Cheyne

April 26, 2001

Page 2 of 3

Based on currently available information and discussion with Port of Seattle maintenance staff, HNTB offers the following comments:

There is one facility, SDS7 Basin Vault (sheet C140 in appendix D) shown as an above ground facility. The SDS7 Basin Vault would have a top elevation at approximately 325. The adjacent land varies between elevation 312 and 300. Two maintenance options are available: external stair, ladder, or access ramp roadway. The stair or ladder options would provide person access for regular inspections but a crane would be required to lift pump units or other equipment for less frequent cleaning operations. The lift height of 15 feet would allow use of smaller cherry-picker cranes. An access ramp is also feasible for this location. The King County standard 15 percent maximum slope would result in an 87-foot long ramp grade along the tank wall. Site constraints would likely require that this ramp would be constructed as a 15-foot wide asphalt roadway on a mechanically stabilized earth wall and barrier type wall coping. The drawings illustrate the ramp option since it would require the largest facility footprint (to facilitate assessment of greatest likely impact), but selection of which option is used will be done during final design.

All others that protrude above ground do so in only a portion of the top area. These will be designed for direct HS20 loading to allow full vehicular access to any part of the top to allow maintenance using the same methods typical of an underground tank. Those facilities are accessed from the side that is buried.

Most facilities exceed the King County maximum depth of 20 feet between top of finished grade and the invert of the vault in at least a portion of the facility. Though the KCSWDM does not specifically note the reason for this guideline, the maximum depth is likely the result of limitations for existent King County maintenance equipment. Since these facilities would be Port maintained facilities, the King County standard are not considered as design constraints for these facilities. The following special provisions are anticipated to address the maintenance of these facilities.

- The Port of Seattle currently uses a vactor truck, truck mounted cranes and block and tackle on tripod for vault cleaning work. The Port has noted that they do not see the depth of these facilities as a concern, but has noted that they may require some new or modified equipment to accommodate the facilities as shown. HNTB conducted a web search and verified that vactor trucks are available in a wide range of configurations and capacities. The largest "heavy industrial vacuum (Guzzler/Vactor) truck available through United Pumping was noted to be "capable of removing [water] from remote or inaccessible locations as far away from the machine as 1000 feet or more through suction lines 8" in diameter and smaller." This is consistent with the Port's intent to test alternate suction line configurations to address the proposed conditions, but also gives HNTB confidence that there is a vactor truck available if the Port's existing truck proves to be inadequate. A local utility cleaning service that uses vactor trucks indicated that they have gone to 100-foot depths with their equipment but they noted that pumping time increases with depth. The Port has the option to replace the function of the vactor truck with new or existing portable pumps that could be selected to perform better. The existing truck mounted crane or block and tackle could be used to lower a pump into position. The designer will continue to work with the Port through final design to allow them adequate time to plan and acquire equipment as needed.
- Vault access ports will meet or exceed King County standards.

Michael Cheyne

April 26, 2001

Page 3 of 3

- The Port has noted that we should use the current maximum of 41 feet as the depth limit in the event that further design/model developments identify a need for larger facilities.
- OSHA requires that ladders greater than 25 feet in height include intermediate landing for personnel access safety. This tends to add undesirable inconvenience for lowering items through the access ports on space constrained facilities. The large size of these facilities would allow space for landings or internal stairways for personnel access separate from the 5' x 10' equipment access doors or panels.
- The County's suggested provisions for ventilation might be inadequate for the large facilities shown. A qualified engineer will work with the Port to ensure that ventilation is adequate to meet OSHA confined space requirements. Self-contained supplied air is readily available to maintenance worker if needed.
- Pretreatment on the airfield including 60-foot filter strips, catchbasins, and frequent inspection/removal of foreign objects (including trash) for the airport operation area will significantly reduce maintenance requirements relative to typical King County facilities.
- Internal walls with openings sufficient for maintenance and access to maintenance "v" in the vault floor and to outlet structure or sediment trap sumps. These details are not shown on the drawings but will be addressed as part of the final design.

The Port's experience in owning, operating, and maintaining the two large existing detention vault facilities gives HNTB confidence that maintenance of the proposed facilities will be feasible. Final design will include consideration of OSHA confined space requirements, and special Port needs to establish personnel and equipment access appropriate for each vault location.

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**APPENDIX E**

**BMP SIZING ESTIMATIONS PER UNIT PGIS AREA**

**APPENDIX E**  
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**AR 011077**

## 1. STORM FLOW AND VOLUME CALCULATIONS

Bioswale and filter strip size were based on the water quality design flow rate, specified as the 6-month, 24-hour design storm. Wetpool and treatment wetland pool size were based on the mean annual storm runoff volume.

### Water Quality Design Flow for Bioswales and Filter Strips

The following parameters were used in the StormSHED Hydrologic Modeling Program (Engenious Systems 1997):

**Area:** 1 acre

**Curve Number:** 98

**Time of Concentration:** 4 minutes

To estimate the 6-month storm flow rate, 64 percent of the 2-year, 24-hour precipitation of 2.0 inches was used. From StormSHED (output attached on following page), the 6-month peak flow rate was 0.32 cfs.

### Runoff Volume from Mean Annual Storm for Wetvaults

Wetvault volume was based on the mean annual storm runoff volume. The mean annual storm volume for a 1-acre impervious area was calculated using the simplified method described in the King County Manual (King County 1998). Because the area to be treated is 100 percent impervious, the runoff formula from the King County Manual was simplified to:

$$\text{Runoff Volume (ft}^3\text{), } V_r = 0.9 * A_i * (R/12)$$

where:  $A_i$  = impervious area (ft<sup>2</sup>)  
R = mean annual storm precipitation (inches)

The mean annual storm precipitation of 0.47 inch was obtained from the isopluvial map provided in the King County Manual. The mean annual runoff was calculated to be:

$$V_r = 0.9 * (43,560) * (0.47/12) = 1,535 \text{ ft}^3$$

**AR 011078**

**Drainage Area: 1AC**

Hyd Method: SBUH Hyd  
 Peak Factor: 484.00  
 Storm Dur: 24.00 hrs

Loss Method: SCS CN Number  
 SCS Abs: 0.20

	Area	CN	TC
Pervious	0.0000 ac	98.00	0.00 hrs
Impervious	1.0000 ac	98.00	0.07 hrs
Total	1.0000 ac		

**Supporting Data:****Impervious CN Data:**

Impervious 98.00 1.0000 ac

**Impervious TC Data:**

Flow type:	Description:	Length:	Slope:	Coeff:	Travel Time
Sheet	Sheet	200.00 ft	1.00%	0.0100	3.26 min
Channel	Storm Drain	200.00 ft	1.00%	42.0000	0.79 min

**1AC Event Summary:**

BasinID	Peak Q (cfs)	Peak T (hrs)	Peak Vol (ac-ft)	Area ac	Method /Loss	Raintype	Event
1AC	0.32	7.83	0.0887	1.00	SBUH/SCS	kc24hr	0.64*2y



## 2. BMP SIZE CALCULATIONS

BMP sizing was based on the 1998 King County Manual. Sizing requirements for the King County and Ecology manuals are approximately equivalent; however, the King County Manual's simpler sizing procedures were better suited for these generic calculations.

### 2.1 FILTER STRIP SIZING

Filter strips were sized for a 300-ft-wide by 145-ft-long (1-acre) section of runway. Although the typical maximum flow path across runways and taxiways is 140 ft across the pavement and shoulder of straightaway taxiways, the flow path may reach up to 300 feet across runway/taxiway intersections.

Note: For filter strips, as with bioswales, the *length* dimension is parallel to the direction of flow. Therefore, for filter strips, length is measured perpendicular to the runway, taxiway, or roadway centerline. Runway and taxiway length is measured parallel to the centerline.

The following assumptions were used for the filter strip sizing:

WQ design flow,  $Q = 0.32$  cfs  
Manning's roughness,  $n = 0.35$  (per King County Manual)  
Strip Width,  $W = 145$  ft  
Slope,  $s = 0.02^1$

The calculations are shown below:

$$\text{Flow depth, } d = \left[ \frac{Q * n}{1.49 * W * s^{0.5}} \right]^{0.6} = \left[ \frac{0.32 * 0.35}{1.49 * 145 * 0.02^{0.5}} \right]^{0.6} \approx 0.035 \text{ ft}$$

$$\text{Flow velocity, } V = \frac{Q}{A} = \frac{Q}{W * d} = \frac{0.32}{145 * 0.035} = 0.064 \text{ fps}$$

Per the King County Manual, the flow must have a minimum 9-minute (540-second) residence time.

$$\text{Strip Length, } L = 540 * V = 540 * 0.064 = 34.5 \text{ ft}$$

$$\Rightarrow \text{Area} = L * W = 145 * 34.5 \approx 5,000 \text{ ft}^2$$

### 2.2 BIOSWALE SIZING

The following assumptions were used for the bioswale sizing:

WQ design flow,  $Q = 0.32$  cfs

---

<sup>1</sup> Nominal STIA design specifications call for a slope of 1.5 percent. This assumed slope results in a conservative size estimate.

Manning's roughness,  $n = 0.2$  (per King County Manual)  
 Bottom width,  $b = 6$  ft  
 Flow depth,  $y = 2$  inches = 0.167 ft (regularly mowed)  
 H:V side slopes,  $Z = 3$   
 Slope,  $s = 0.02$

The calculations are shown below:

$$V = \frac{Q}{A} = \frac{Q}{by + Zy^2} = \frac{0.32}{(6 * 0.167) + (3 * 0.167^2)} = 0.30 \text{ fps}$$

Per the King County Manual, the flow must have a minimum 9-minute (540-second) residence time.

$$\text{Swale Length, } L = 540 * V = 540 * 0.30 = 160 \text{ ft}$$

$$\text{Swale Area} = L * W = 160 * 6 \approx 960 \text{ ft}^2$$

### 2.3 WETVAULT SIZING

The calculations for the wetvault sizing are shown below:

Basic wetpond size (per the King County Manual), therefore wetpool volume =  
 $3 * (\text{volume of mean annual 24-hour storm runoff})$

$$\text{Mean annual storm runoff volume} = 1,535 \text{ ft}^3$$

$$\text{Wetpool volume} = 3 * 1535 = 4,610 \text{ ft}^3$$

### 2.4 SUMMARY

Unit sizes of BMPs per acre are shown in Table E-2.

**Table E-2. BMP unit sizes per acre of PGIS.**

BMP	Approximate required size per acre of PGIS
Filter Strip	5,000 ft <sup>2</sup>
Bioswale <sup>1</sup>	960 ft <sup>2</sup>
Wetvault	4,610 ft <sup>3</sup>

<sup>1</sup> Area given is for bottom of bioswale. Including side slopes, bioswale area at 1.0 ft depth would be 1915 ft<sup>2</sup>. However, where the unit size (960 ft<sup>2</sup>) was used to evaluate existing bioswales, the bottom width of the existing bioswales was used, consistent with the area calculation used to estimate 960 ft<sup>2</sup>. Where this area figure is used to estimate future bioswale area, planners should be aware that an additional 6 ft of width is necessary (assuming  $Z = 3$ , 3 ft needed on each side of bottom).

**AR 011081**

### 3. REFERENCES

Ecology (Washington State Department of Ecology). 1992. Stormwater management manual for the Puget Sound basin, the technical manual. Washington Department of Ecology, Olympia, Washington.

Engenious Systems, Inc. 1997. WaterWorks hydrologic modeling program. Seattle, Washington.

King County Department of Natural Resources (King County DNR). 1998. King County surface water design manual. King County Department of Natural Resources, Water and Land Resources Division, Seattle, Washington.

**AR 011082**

**APPENDIX F**

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**FEASIBILITY OF STORMWATER INFILTRATION REPORT**

**AR 011083**

**Feasibility of Stormwater Infiltration  
Third Runway Project  
Sea-Tac International Airport  
SeaTac, Washington**

**Prepared for  
Port of Seattle**

**December 14, 2000  
J-4978-06**

**AR 011084**

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## **APPENDIX A EXPLORATION LOGS**

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# FEASIBILITY OF STORMWATER INFILTRATION THIRD RUNWAY PROJECT, SEA-TAC INTERNATIONAL AIRPORT SEATAC, WASHINGTON

## SUMMARY

Infiltration tests have been performed for selected sites on the west side of the proposed runway embankment to evaluate the feasibility of infiltration as part of the Stormwater Management Plan (SMP) for the Sea-Tac Third Runway project. The testing performed to date shows infiltration is feasible in two of the areas tested (Areas 1 and 3). Preliminary design infiltration rates have been developed from the field tests using methods stipulated by King County (1998) as listed in Table 1. Based on these results, potential infiltration capacities (in cubic feet per second [cfs]) at the individual sites have been developed for nominal 8-foot-wide infiltration trenches totaling 400 feet in length:

- ▶ Infiltration Area 1 can accommodate stormwater disposal at an average rate of 0.30 cfs; and
- ▶ Infiltration Area 3 can accommodate stormwater disposal at an average rate of 0.20 cfs.

Additional trenches may be located in these areas to increase infiltration capacity, depending on site logistics.

These data are suitable for conceptual infiltration facility design. The infiltration capacity of any site will depend on the detailed design and layout (i.e., area and elevation) of the infiltration facility, and the degree of variability in soil conditions beneath the facility. Additional infiltration tests and soil borings will be needed to meet all the requirements of the King County Surface Water Design Manual (1998) and should be completed once provisional footprints of the facilities are established.

This report summarizes design requirements for infiltration facilities, field data collection performed by Hart Crowser, and results of our work to date for Infiltration Areas 1 and 3.

## INTRODUCTION

As a result of increased stormwater storage capacity requirements in the SMP, Hart Crowser was tasked to investigate potential sites for infiltration of detained

stormwater on the west side of the proposed Third Runway project area (see Figure 1 for general location). Based on the location of detention ponds C, D, and G, three sites were identified as potential sites for infiltration of water discharged from detention ponds and/or vaults on the airfield. Additionally, the footprint of detention ponds C, D, and G were also considered for potential infiltration capacity. Locations of the detention ponds and Infiltration Areas 1, 2, and 3 are shown on Figures 2 and 3.

Infiltration testing was conducted along with the collection of soils and groundwater data that are needed to establish if infiltration can be implemented in each area in accordance with the requirements of the King County Surface Water Design Manual (KCSWDM – King County, 1998). The overall requirements for infiltration facilities are summarized in the following section.

## **INFILTRATION FACILITY REQUIREMENTS**

The following summary outlines the General Requirements (Section 5.4.1 of the KCSWDM) for infiltration facilities (ponds, tanks, and trenches) associated with the natural site conditions. Additional requirements identified below under “Other Engineering Considerations” need to be addressed by the engineering design team.

### **Soils**

- ▶ The basic requirement is a minimum of 3 feet of permeable soil below the bottom of the facility and at least 3 feet between the bottom of the facility and the maximum wet-season water table.
- ▶ A minimum of two test pits or soil borings per 10,000 ft<sup>2</sup> of infiltration area are required to characterize the site.
- ▶ Test pits or borings should extend at least 5 feet below the bottom of the infiltration facility, and at least one test hole should reach the water table.

### **Measured Infiltration Rates**

- ▶ The measured infiltration rate should be determined using either the double-ring infiltrometer test (ASTM Method D 3385, 2000) or the EPA falling head percolation test procedure (EPA, 1980).
- ▶ Sufficient tests should be performed to determine a representative infiltration rate but at least three tests shall be performed for each proposed infiltration facility.



### **Design Infiltration Rate**

- ▶ The design infiltration rate should be calculated by Equation 5-9 of the KCSWDM, using the correction factors listed in that Section 5.4.1.

### **Off-site Groundwater Impacts**

- ▶ The impacts of infiltration should be considered for the potential to provide increased water to landslide areas, increased groundwater resources available, increased water levels in closed depressions, and higher groundwater levels.

### **Groundwater Protection**

Groundwater protection requirements call for implementing one of the following actions when infiltrating water from pollution-generating surfaces:

- ▶ Provide water quality treatment prior to infiltration; or
- ▶ Demonstrate that the soil beneath the infiltration facility has properties which reduce the risk of groundwater contamination from typical stormwater runoff.

### **Other Engineering Considerations**

- ▶ 100-Year Overflow Conveyance
- ▶ Spill Control Devices
- ▶ Pre-settling
- ▶ Protection from Upstream Erosion
- ▶ Construction Guidelines.

This report by Hart Crowser provides a preliminary assessment of the soils, infiltration rates, and hydrology of each site to establish the feasibility of infiltration. Engineering aspects and site logistics will be addressed by the design team as part of final design.

## **APPROACH**

The type of infiltration test chosen at each location was dependent on the depth of the target soil strata or pond elevation. Generally, for tests less than 4 to 5 feet below ground surface, test pits were dug and the double-ring infiltrometer method was used. This method involved repeatedly measuring a small (< 1/4 inch) change in water level in both the inner and outer rings while consistently maintaining a head between 5.5 and 6 inches in both rings until a relatively

constant rate was obtained. Pre-soaking the test area is not required; however, to limit the amount of inconsistent readings at the beginning of the test, a pre-soaking period of approximately one hour was employed.

For testing depths below 5 feet, the EPA method was used in an augered hole with a 6-inch-diameter temporary casing inserted to prevent caving of the borehole walls. This method involved repeatedly measuring the water level drop from an initial head (6 inches above the base of the hole) over a given period until a relatively constant rate was obtained. At the end time interval the water level was adjusted back to the original head level prior to starting the next measurement. A minimum of four hours or overnight pre-soaking of the test zone was performed.

The seasonal high groundwater level was estimated by measuring current groundwater levels in existing or recently installed monitoring wells at each site and comparing these with longer records from existing nearby wells in similar hydrogeologic settings. Additionally, soil profile characteristics such as low chroma mottling were also reviewed to assess the seasonal high groundwater levels.

## RESULTS

We have completed infiltration tests and soil borings at one pond location and three potential infiltration areas:

- ▶ Pond G;
- ▶ Infiltration Area 1 (between Pond C and Pond G);
- ▶ Infiltration Area 2 (south of Pond G); and
- ▶ Infiltration Area 3 (northwest of Pond D).

Results of the double-ring infiltrometer tests are listed in Table 2; results of the EPA method falling head percolation tests are listed in Table 3.

Work on Pond D is still in progress. A third pond location (Pond C) was considered but the presence of groundwater seepage precluded further consideration of infiltration at Pond C. Infiltration in Pond G and Area 2 proved to be unfeasible due to low permeability soils and/or high groundwater levels. Logs of soil borings and test pits are included in Appendix A for Infiltration Areas 1 and 3.

In the following summaries, we include an estimate of the design infiltration rate for each area. This is currently based on the average values of the measured

infiltration rates for each area, factored by our estimate of the appropriate correction factors, as stipulated by King County (1998). However, given the variability of the soils encountered to date, the mean value may not be appropriate for the entire facility at each location. Final design would take into account the results of additional facility-specific testing, the actual geometry of the proposed facilities, and additional design adjustments to provide an adequate "factor of safety."

Final measured infiltration values will be recommended for the design of the proposed facilities after completion of the additional borings and tests needed to fulfill KCSWDM requirements.

### **Infiltration Area 1**

Investigative explorations show a consistent slightly silty fine to medium sand occurring across the site. The sand unit starts just below the surface and extends to depths of 8 feet (approximately 268 feet elevation) where deeper material increases in silt content.

The groundwater level measured in the new monitoring well HC00-B333, during November 2000, had an elevation of 268.5 feet. Table 4 lists the seasonal water level variations for two comparable wells east of Infiltration Area 1 with water level records that include last year's seasonal high. Based on the average seasonal fluctuation in these wells, and assuming currently observed water levels correspond to the seasonal low, the projected seasonal high water level for HC00-B333 is 273.1 feet (approximately 8 feet below ground surface).

The locations tested exhibited medium to high infiltration capacities ranging from 4.6 to 20.4 in./hr. Results are summarized in Table 1.

To illustrate the infiltration potential of this site, we have estimated the infiltration capacity of 400 lineal feet of 8-foot-wide infiltration trench(es). Using a design infiltration rate of 4.2 in./hr, such trenches in Area 1 may be expected to infiltrate 0.30 cfs of stormwater from SMP area SDW1A.

### **Infiltration Area 3**

Three test pits revealed varying shallow soil composition. The northern two test pits (HC00-TP338 and HC00-TP339) encountered silty fine to medium sand at elevations between 297 and 308 feet. Test pit HC00-TP337 in the southern portion of the site revealed dry silt from the surface at approximate elevation 309 feet, to the bottom of the test pit (approximate elevation 301 feet). Although not determined at this time, the groundwater level in Infiltration Area 3

is expected to be at a depth of at least 10 feet, based on the absence of seepage into the test pits. Local water table mapping by AESI (2000) suggests that the groundwater elevation in the shallow regional aquifer is around 230 to 240 feet at this location.

Double-ring infiltrometer tests were conducted in test pits approximately 3 to 4 feet below the ground surface (i.e., approximately 302 to 309 feet elevation). Two were located in a silty sand deposit and provided moderate infiltration rates of 7.5 and 5.0 in./hr. The third test was performed in finer-grained silty soil and gave an infiltration rate of 0.94 in./hr.

Using an estimated design infiltration rate of 2.7 in./hr and assuming overall trench dimensions of 400 feet by 8 feet, Area 3 should infiltrate approximately 0.2 cfs of stormwater from SMP area SDW1B. Additional trenches may be an option in this area; however, the proximity of the adjacent slope (greater than 15%) may require regrading to create benches. The KCSWDM indicates that a geotechnical assessment of slope stability would likely be required for construction of an infiltration facility in Area 3.

## CONCLUSIONS

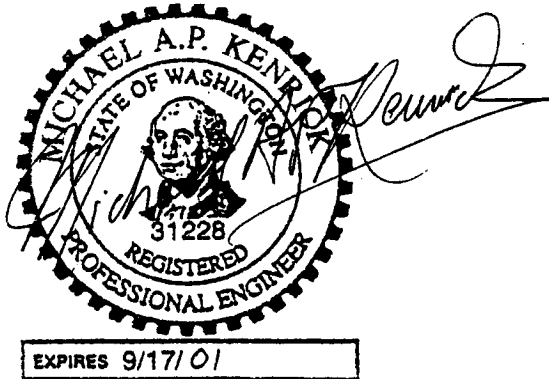
The results of our soil borings and infiltration tests show that Areas 1 and 3 are suitable for infiltration of detained stormwater. The infiltration capacities quoted in this report are provisional; the appropriate design infiltration rate for each area depends on the chosen location, layout, depth, and length of infiltration trenches. The implementation of infiltration facilities will necessitate full consideration of relevant engineering requirements as outlined in the KCSWDM.

Sincerely,

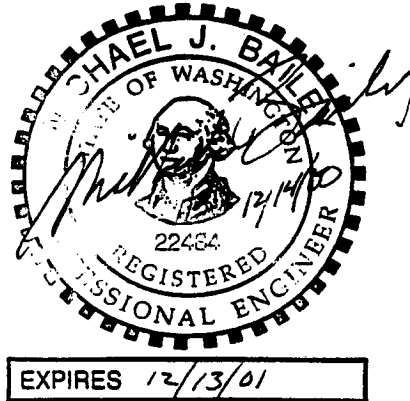
**HART CROWSER, INC.**



**ROBERT O. MIDDOUR**  
Project Hydrogeologist



**MICHAEL A.P. KENRICK, P.E.**  
Senior Associate Hydrogeologist



**MICHAEL J. BAILEY, P.E.**  
Project Manager

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King County, 1998. King County Surface Water Design Manual, King County and Department of Natural Resources.

EPA, 1980. EPA Falling Head Percolation Test Procedure, Design Manual - Onsite Wastewater Treatment and Disposal Systems, EPA.

ASTM, 2000. Double Ring Infiltrometer Test, ASTM Method D 3385, Annual Book of ASTM Standards, Soil and Rock (1): D 420 - D 5799.

AESI, 1999. Seattle-Tacoma International Airport Ground Water Study, Associated Earth Sciences, Inc. and S. S. Papadopoulos & Assoc.

**Table 1 - Summary of Infiltration Testing Results; West Side of Third Runway Embankment**

Location ID	Approximate Ground Surface Elevation in Feet (msl)	Measured Infiltration Rate in in./hr	Assumed Correction Factors		Design infiltration rate in in./hr	Estimated Infiltration Capacity in cfs using 200ft x 8ft trench(es)		
			F <sub>testing</sub>	F <sub>plugging</sub>		F <sub>geometry</sub>	1 (200 linear ft)	2 (400 linear ft)
<b>Infiltration Area 1 (SDW1A)</b>								
HC00-B327	276.1	20.40	0.3	0.9	1	0.20	0.41	0.61
HC00-B328	275.4	4.65	0.3	0.9	1	0.08	0.15	0.23
HC00-B329	280.8	18.45	0.3	0.9	1	0.18	0.37	0.55
<b>Infiltration Area 2</b>								
HC00-TP301	245.6	0.00	0.5	0.8	1	NA	NA	NA
HC00-TP302	244.2	0.33	0.5	0.8	1	NA	NA	NA
HC00-TP303	253.5	0.43	0.5	0.8	1	NA	NA	NA
<b>Infiltration Area 3 (SDW1B)</b>								
HC00-TP307	309.2	0.94	0.5	0.9	1	0.02	0.03	0.05
HC00-TP308	304.9	5.00	0.5	0.9	1	0.08	0.17	0.25
HC00-TP309	311.7	7.50	0.5	0.9	1	0.13	0.25	0.38
<b>Pond G</b>								
HC00-B310A	264.9	0.24	0.3	0.8	0.25	NA	NA	NA
HC00-B313A	260.2	1.68	0.3	0.8	0.25	NA	NA	NA

**Notes:**

(1) Infiltration rates determined by double-ring infiltrometer (ASTM method D 3385) or a modified EPA falling head percolation test procedure (Design Manual - Onsite Wastewater Treatment and Disposal Systems, EPA, 1980)

(2) Correction Factors: per King County Surface Water Design Manual (1998)

F<sub>testing</sub> = 0.5 for ASTM method D3385 and 0.3 for EPA method

F<sub>plugging</sub> = 0.8 for fine sands and loamy sands, 0.9 for medium sands

F<sub>geometry</sub> = 0.25 to 1.0, values for the trenches all exceeded 1.0 and Pond G was < 0.25

Design infiltration rate = measure rate x F<sub>testing</sub> x F<sub>plugging</sub> x F<sub>geometry</sub>

**Table 2 - Double Ring Infiltrometer Tests**

Test ID	Final Reading from Inner Ring in Inches	Time Increment in Minutes	Infiltration Rate in in./hr	Soil Description
<b>Infiltration Area 2</b>				
HC00-TP301A *	0.11	191	0.03	SILT
HC00-TP302A	0.25	46	0.33	Gravelly, silty SAND
HC00-TP303A	0.28	39	0.43	Slightly gravelly SAND
<b>Infiltration Area 3</b>				
HC00-TP307A	0.25	16	0.94	SILT
HC00-TP308A	0.50	6	5.00	Slightly silty, fine to medium SAND
HC00-TP309A	0.50	4	7.50	Slightly silty, fine to medium SAND

\* water seeping into test pit and pooling outside the rings



**Table 3 - Falling Head Percolation Tests**

Location ID	Test Number	Elapsed Time in min	Change in Head in feet	Percolation Rate in in./hr	Soil Type
<b>Infiltration Area 1</b>					
HC00-B327A	1	2	0.06	21.60	Slightly silty, fine to medium SAND
		5	0.15	21.60	
	2	2	0.06	21.60	
		5	0.14	20.16	
	3	2	0.06	21.60	
		5	0.14	20.16	
	4	2	0.05	18.00	
		5	0.14	20.16	
	5	2	0.05	18.00	
		5	0.14	20.16	
	6	2	0.06	21.60	
		5	0.14	20.16	
HC00-B328A	1	2	0.02	5.40	
		5	0.05	7.20	
		10	0.10	7.20	
	2	2	0.02	7.20	
		5	0.06	8.64	
		10	0.11	7.92	
	3	2	0.02	7.20	
		5	0.05	7.20	
		10	0.11	7.92	
	4	2	0.03	10.80	
		5	0.06	8.64	
		10	0.11	7.92	

**Table 3 - Falling Head Percolation Tests**

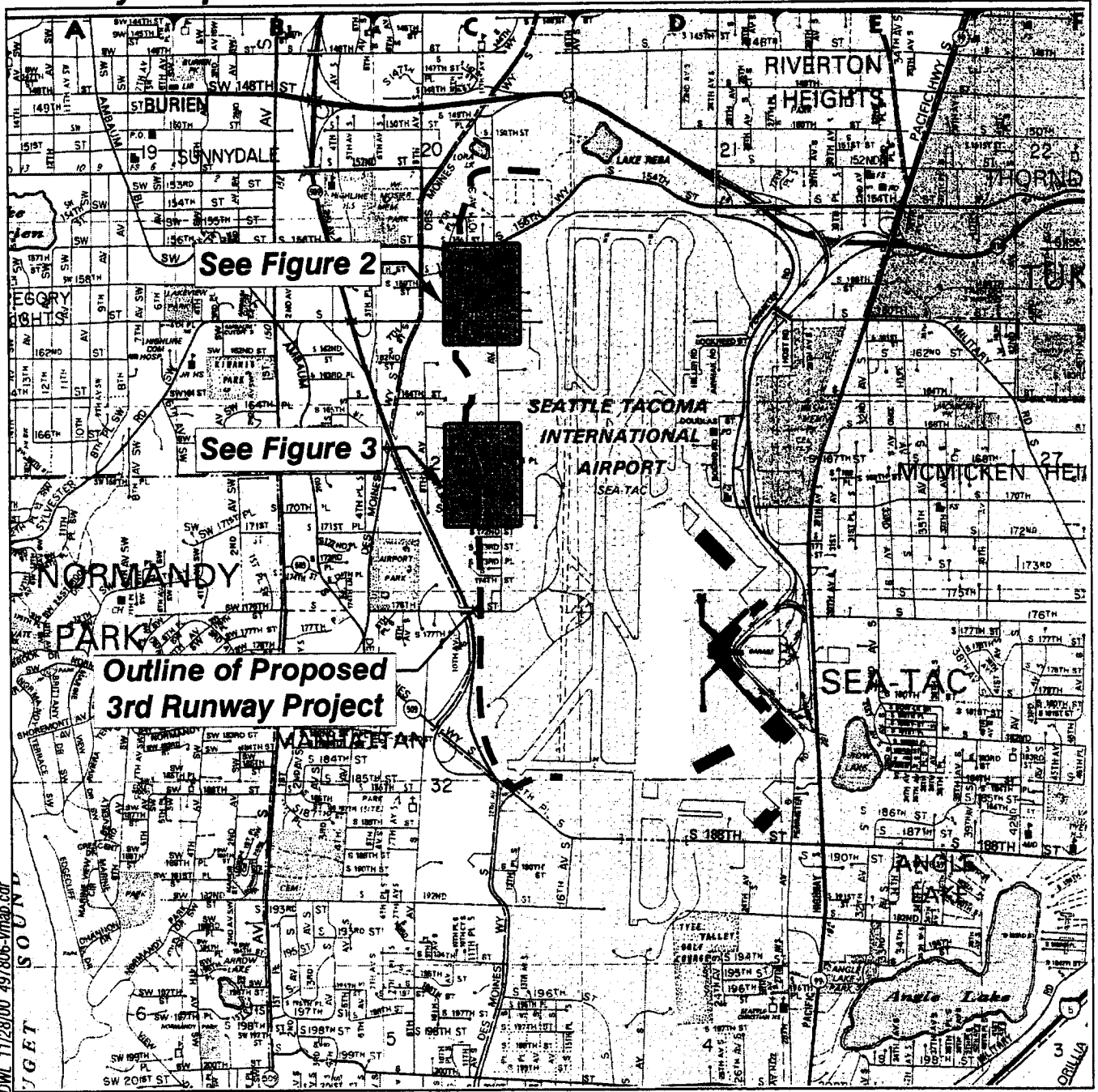
Location ID	Test Number	Elapsed Time in min	Change in Head in feet	Percolation Rate in in./hr	Soil Type
<b>Infiltration Area 1</b>					
HC00-B329A	1	2	0.05	16.20	Slightly silty, fine to medium SAND
		5	0.10	14.40	
		10	0.20	14.40	
		15	0.29	13.92	
		20	0.37	13.32	
	2	2	0.05	18.00	
		5	0.12	17.28	
		10	0.23	16.56	
		15	0.33	15.84	
		20	0.44	15.84	
	3	2	0.05	18.00	
		5	0.12	17.28	
		10	0.26	18.72	
		15	0.37	17.76	
		20	0.49	17.64	
	4	2	0.06	21.60	
		5	0.14	20.16	
		10	0.26	18.72	
		15	0.39	18.72	
	<b>Pond G</b>				
HC00-B310A	1	30	0.01	0.24	Slightly silty, fine to medium SAND
	2	30	0.01	0.24	
	3	30	0.01	0.24	
HC00-B313A	1	30	0.07	1.68	Silty, gravelly SAND
	2	30	0.06	1.44	
	3	30	0.07	1.68	
	4	30	0.07	1.68	

**Table 4 - Estimation of Seasonal High Water Level in Infiltration Area 1**

Monitoring Well ID	Ground Surface in Feet (msl)	Top of Casing in Feet (msl)	Seasonal Water Level Range		Date	Period of Record	Range of Fluctuation in Feet
			Minimum in Feet (msl)	Maximum in Feet (msl)			
HC99-B64	292	294.2	284.9	288.2	Nov-00	12/99 to 10/00	3.3
HC99-B73	291.7	293.80	283.42	289.3	Oct-99	10/99 to 10/00	5.88
HC00-B333	281	283.5	268.5		Nov-00		

Projected Seasonal High Groundwater Level in HC00-B333 =  $273.09 = 268.5 + (3.3+5.88)/2$

# Vicinity Map



DWL 1129100 497806-Vrmap.cdf

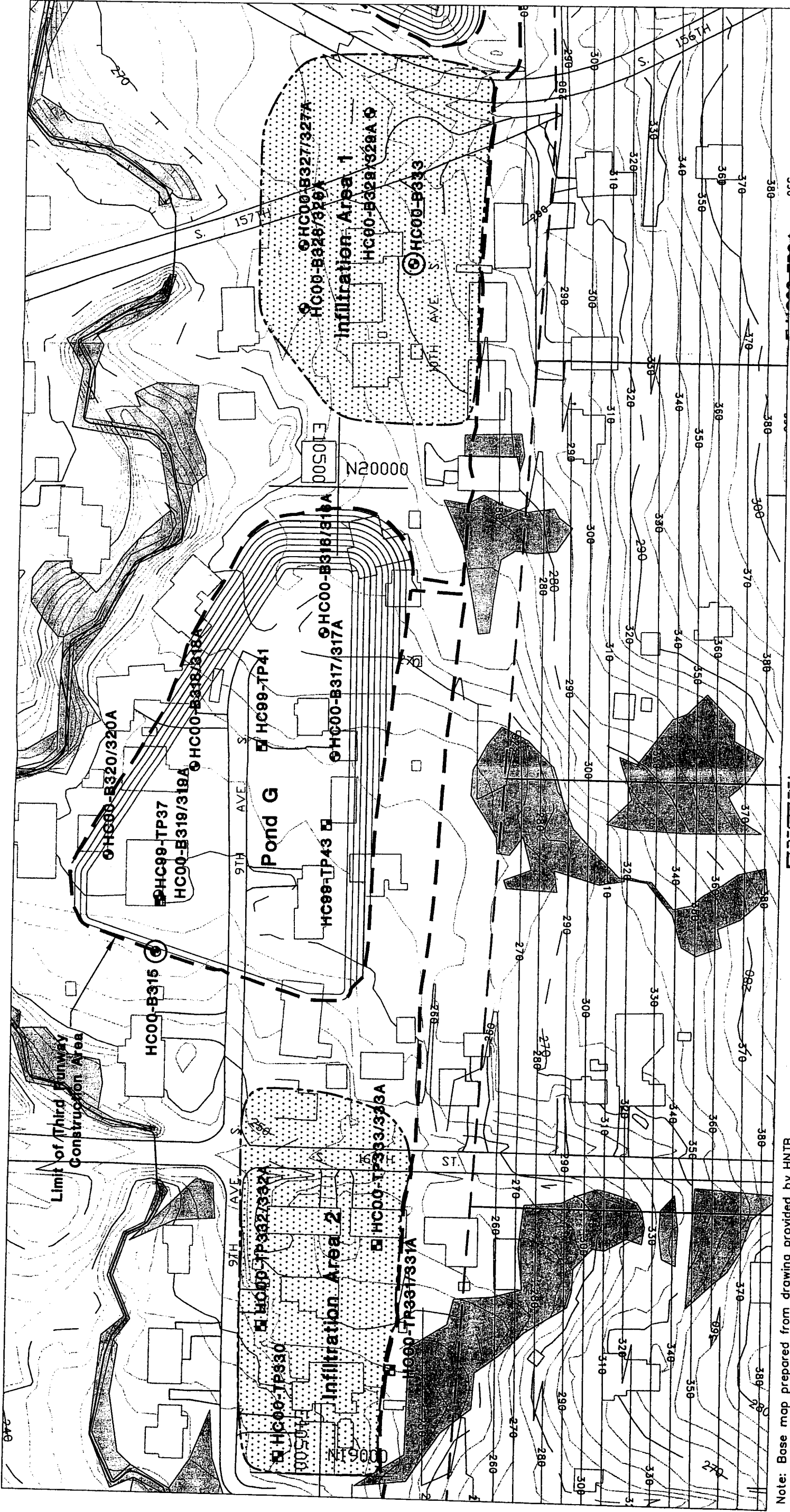


AR 011099

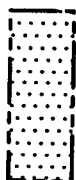



**HARTCROWSER**  
J-4978-06 11/00  
Figure 1

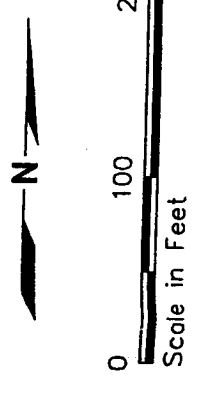
# Site and Exploration Plan

## Infiltration Testing



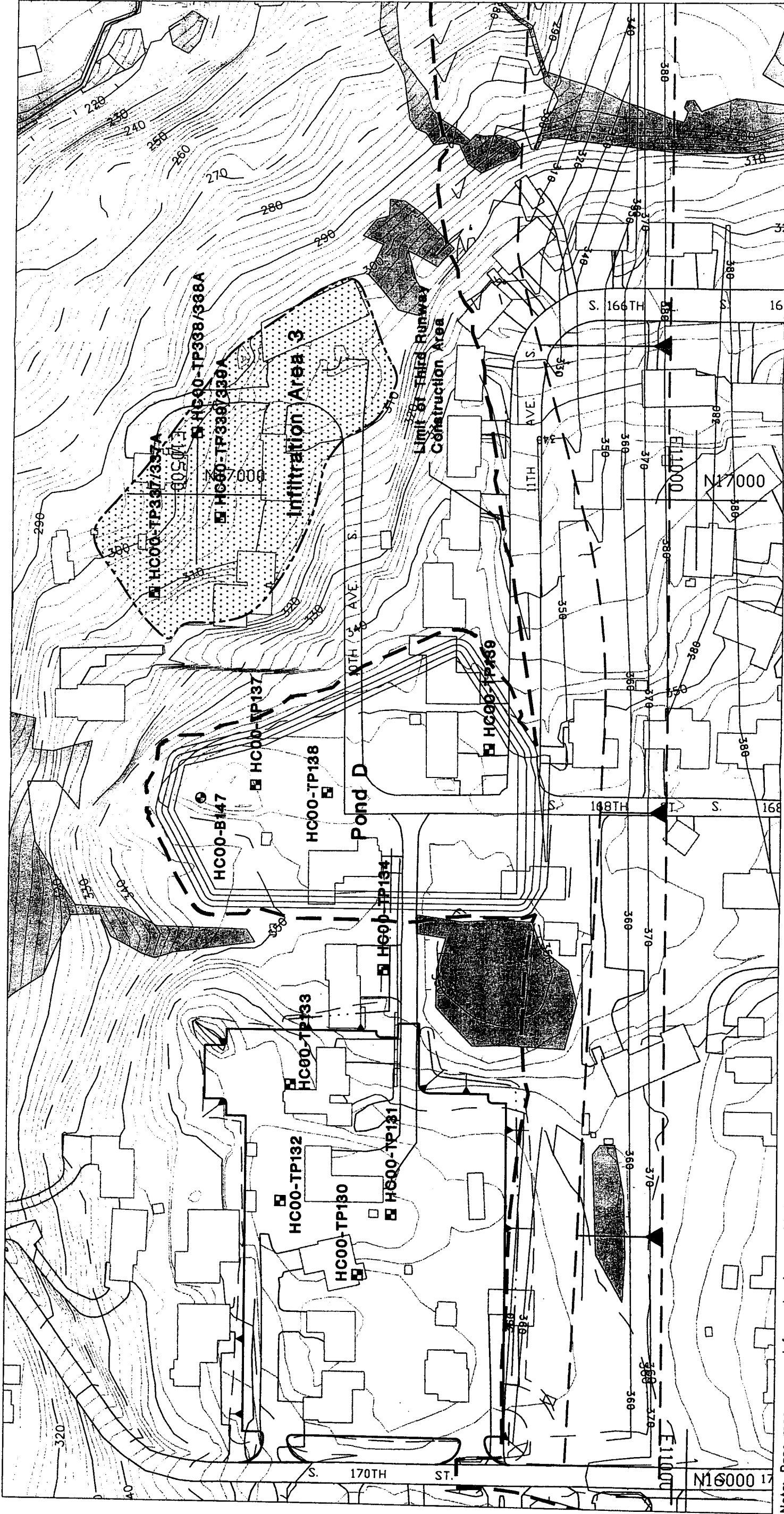
Note: Base map prepared from drawing provided by HNTB entitled "Topo\_Full.dwg", dated October 4, 1999. Wetland delineation prepared from drawing provided by Parametrix entitled, "W\_110800.dwg", dated November 8, 2000.

-  Infiltration Area
-  HC99-TP37 Test Pit
-  HC00-B311/311A Soil Boring
-  HC00-B315 Soil Boring Completed as Monitoring Well







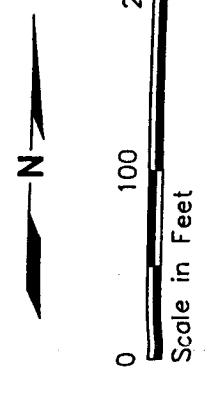
# Site and Exploration Plan

## Infiltration Testing



Note: Base map prepared from drawing provided by HNTB entitled "Topo\_Full.dwg", dated October 4, 1999. Wetland delineation prepared from drawing provided by Parametrix entitled, "W\_110800.dwg", dated November 8, 2000.

-  Infiltration Area
-  Exploration Location and Number
-  Test Pit
-  Soil Boring





# HARTCROWSER

Delivering smarter solutions

## MEMORANDUM

Anchorage

**DATE:** May 23, 2001

**TO:** Jim Thomson, HNTB

Boston

**FROM:** Michael Kenrick and Robert Middour, Hart Crowser Inc.

**RE:** **Sea-Tac Third Runway Project**  
**Infiltration Feasibility at Pond F (Walker Creek Basin)**  
4978-06

Chicago

**CC:** Tom Atkins, Parametrix

Denver

---

### Summary

This memo describes field investigations performed by Hart Crower at the site of stormwater detention Pond F to determine the feasibility of stormwater infiltration. Three borings were drilled to the proposed depth of the pond; the material encountered was dense glacial till. The holes appeared dry, so water was added (as prescribed in the EPA percolation test method) to presoak the ground overnight. However, measurements made the following morning indicated more water had accumulated in the holes, showing saturated conditions in the till at this depth. The infiltration tests were abandoned because stormwater infiltration directly from Pond F is not feasible in these conditions.

Fairbanks

Jersey City

Consideration also was given to the feasibility of shallow infiltration trenches constructed to the north of the pond. However, soil samples taken at a depth of 5 feet in each of the three borings revealed mottling indicative of shallow seasonal saturation, and weathered till overlying unweathered till was present within 5 to 8 feet of the surface. These conditions do not meet King County (1998) requirements for shallow infiltration facilities, so no further testing was performed.

Juneau

Long Beach

### Field Investigations

On April 25, 2001, investigative borings (designated HC01-B401, HC01-B402, and HC01-B403) were advanced east and northeast of the partially completed Pond F (see Figure 1). These three explorations revealed a surficial layer of silty, fine to medium sand approximately 5 feet thick overlying a dense to very dense, silty, gravelly, fine to coarse sand

Portland



(glacial till) that was consistent down to the end of the borings (approximately 25 to 30 feet below ground surface). Water-bearing zones were not observed during drilling.

Down-hole percolation tests (EPA Falling Head Percolation Test Procedure - EPA 1980) were set up in three borings to test the infiltration capacity of soils at the base elevation of the proposed Pond F. The infiltration testing apparatus was set up in accordance with the EPA procedure. Four gallons of clean water were added to presoak the soils at the base of each hole. After presoaking the boreholes overnight, measured water levels were higher than those from the previous day.

Subsequent water-level monitoring revealed stabilized water levels (Table 1) that indicate saturated conditions within the glacial till. Note that the measured water levels do not form a consistent water table as such, but appear to be dependent on the depth of the individual borehole. These observations are consistent with a steep downward hydraulic gradient within the thick layer of low-permeability glacial till in this area. This hydraulic gradient is in equilibrium with natural infiltration and is not a true perched groundwater condition. Regional groundwater levels in the shallow regional (Qva) aquifer are around 260 feet elevation, well below the static water levels measured in the Pond F boreholes (AESI, 1999).

### **Conclusions**

Three factors negate the feasibility of stormwater infiltration at or in the vicinity of Pond F:

- The presence of thick glacial till at the base elevation of Pond F;
- Variably saturated conditions within the till; and
- Mottled appearance of shallow till soils beneath the surficial sand layer, suggesting seasonally high water levels.

Based on these findings, infiltration of stormwater at Pond F, either via the pond bottom or through shallow infiltration trenches located adjacent to the pond, is not feasible.

### **References**

King County, 1998. King County Surface Water Design Manual, King County and Department of Natural Resources.

EPA, 1980. EPA Falling Head Percolation Test Procedure, Design Manual - Onsite Wastewater Treatment and Disposal Systems, EPA.





HNTB  
May 23, 2001

4978-06  
Page 3

AESI, 1999. Seattle-Tacoma International Airport Ground Water Study, Associated Earth Sciences, Inc. and S. S. Papadopulos & Assoc.

**Attachments:**

Table 1 - Boreholes for Infiltration Feasibility at Pond F  
Figure 1 - Site and Exploration Plan, Pond F  
Appendix A - Exploration Logs

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*July 2001*  
556-2912-001 (28)

**AR 011104**

**Table 1– Boreholes for Infiltration Feasibility at Pond F**

	GS Elevation in Feet	Borehole Depth in Feet	Borehole Bottom Elevation in Feet	Water Level Elevation in Feet
HC01-B401	349.6	19.8	329.8	331.4
HC01-B402	355.7	19.8	335.9	336.7
HC01-B403	361.7	24.8	336.9	340.6

July 2001  
556-2912-001 (28)

Hart Crowser  
497806/PondBoreholes.xls

**AR 011105**



**APPENDIX A  
EXPLORATION LOGS**

# Key to Exploration Logs

## Sample Description

Classification of soils in this report is based on visual field and laboratory observations which include density/consistency, moisture condition, grain size, and plasticity estimates and should not be construed to imply field nor laboratory testing unless presented herein. Visual-manual classification methods of ASTM D 2488 were used as an identification guide.

Soil descriptions consist of the following:

Density/consistency, moisture, color, minor constituents, MAJOR CONSTITUENT, additional remarks.

## Density/Consistency

Soil density/consistency in borings is related primarily to the Standard Penetration Resistance.

Soil density/consistency in test pits is estimated based on visual observation and is presented parenthetically on the test pit logs.

SAND or GRAVEL	Standard Penetration Resistance (N) in Blows/Foot	SILT or CLAY	Standard Penetration Resistance (N) in Blows/Foot	Approximate Shear Strength in TSF
Density		Consistency		
Very loose	0 - 4	Very soft	0 - 2	<0.125
Loose	4 - 10	Soft	2 - 4	0.125 - 0.25
Medium dense	10 - 30	Medium stiff	4 - 8	0.25 - 0.5
Dense	30 - 50	Stiff	8 - 15	0.5 - 1.0
Very dense	>50	Very stiff	15 - 30	1.0 - 2.0
		Hard	>30	>2.0

## Moisture

Dry	Little perceptible moisture
Damp	Some perceptible moisture, probably below optimum
Moist	Probably near optimum moisture content
Wet	Much perceptible moisture, probably above optimum

## Minor Constituents

Estimated Percentage

Not identified in description	0 - 5
Slightly (clayey, silty, etc.)	5 - 12
Clayey, silty, sandy, gravelly	12 - 30
Very (clayey, silty, etc.)	30 - 50

## Legends

### Sampling Test Symbols



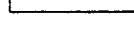
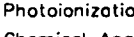
#### BORING SAMPLES

- Split Spoon
- Shelby Tube
- Cuttings
- Core Run
- \* No Sample Recovery
- P Tube Pushed, Not Driven

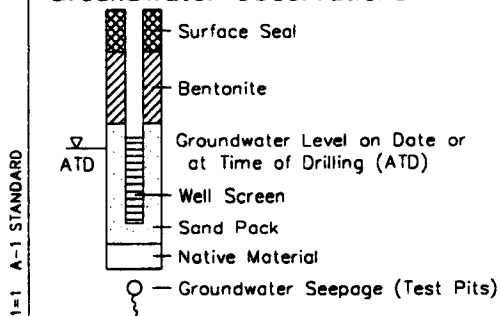
#### TEST PIT SAMPLES

- Grab (Jar)
- Bag
- Shelby Tube

## Test Symbols

- GS Grain Size Classification
- CN Consolidation
- UU Unconsolidated Undrained Triaxial
- CU Consolidated Undrained Triaxial
- CD Consolidated Drained Triaxial
- QU Unconfined Compression
- DS Direct Shear
- K Permeability
- PP Pocket Penetrometer  
Approximate Compressive Strength in TSF
- TV Torvane  
Approximate Shear Strength in TSF
- CBR California Bearing Ratio
- MD Moisture Density Relationship
- AL Atterberg Limits
  -  Water Content in Percent
  -  Liquid Limit
  -  Natural
  -  Plastic Limit
- PID Photoionization Detector Reading
- CA Chemical Analysis
- DT In Situ Density Test

## Groundwater Observations



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Figure A-1 July 2001  
556-2912-001 (28)

AR 011108

# Boring Log HC01-B401

## Soil Descriptions

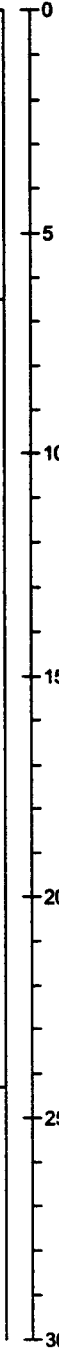
Approximate Ground Surface Elevation in Feet: 349.6

Loose, moist to wet, gray and red-brown, mottled, gravelly, silty SAND. (WEATHERED TILL)

Very dense, moist, gray, gravelly, silty SAND with slightly silty zone near top. (TILL)

Bottom of Boring at 24.3 Feet.  
Completed 04/25/01.

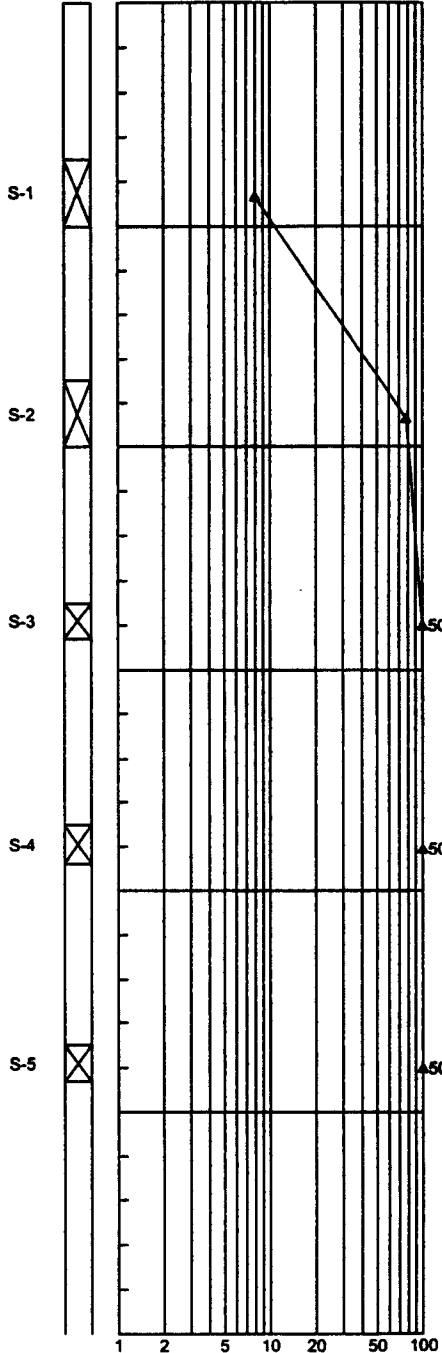
Depth  
in Feet



## STANDARD PENETRATION RESISTANCE

LAB TESTS

Sample ▲ Blows per Foot



BORING LOG 4978068.GPJ HC CORP.GDT 5/23/01

1. Refer to Figure A-1 for explanation of descriptions and symbols.
2. Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.



J-4978-06

04/01

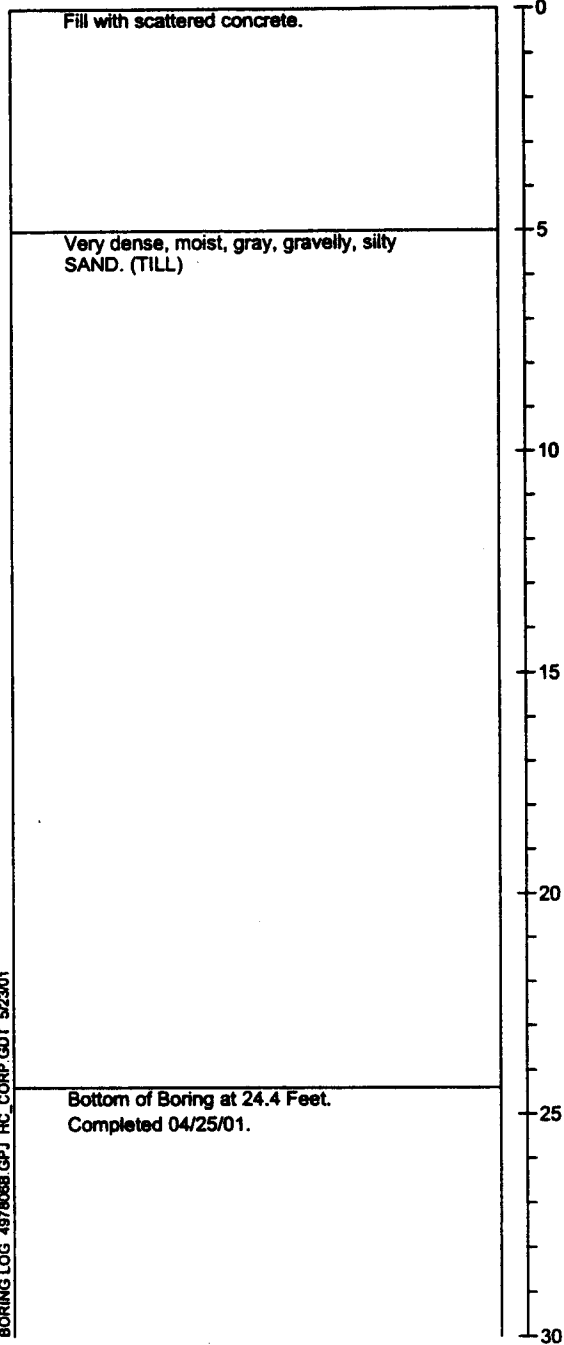
Figure A-2

July 2001  
556-2912-001 (28)

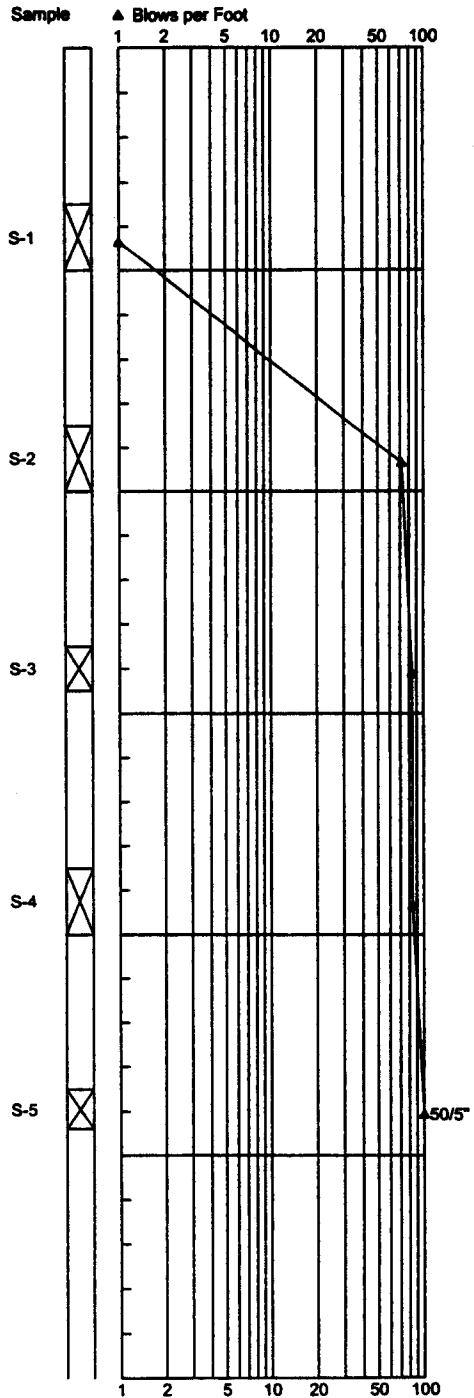
AR 011109

# Boring Log HC01-B402

Soil Descriptions  
 Approximate Ground Surface Elevation in Feet: 355.7



## STANDARD PENETRATION RESISTANCE



LAB TESTS

1. Refer to Figure A-1 for explanation of descriptions and symbols.
2. Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.



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Figure A-3

July 2001

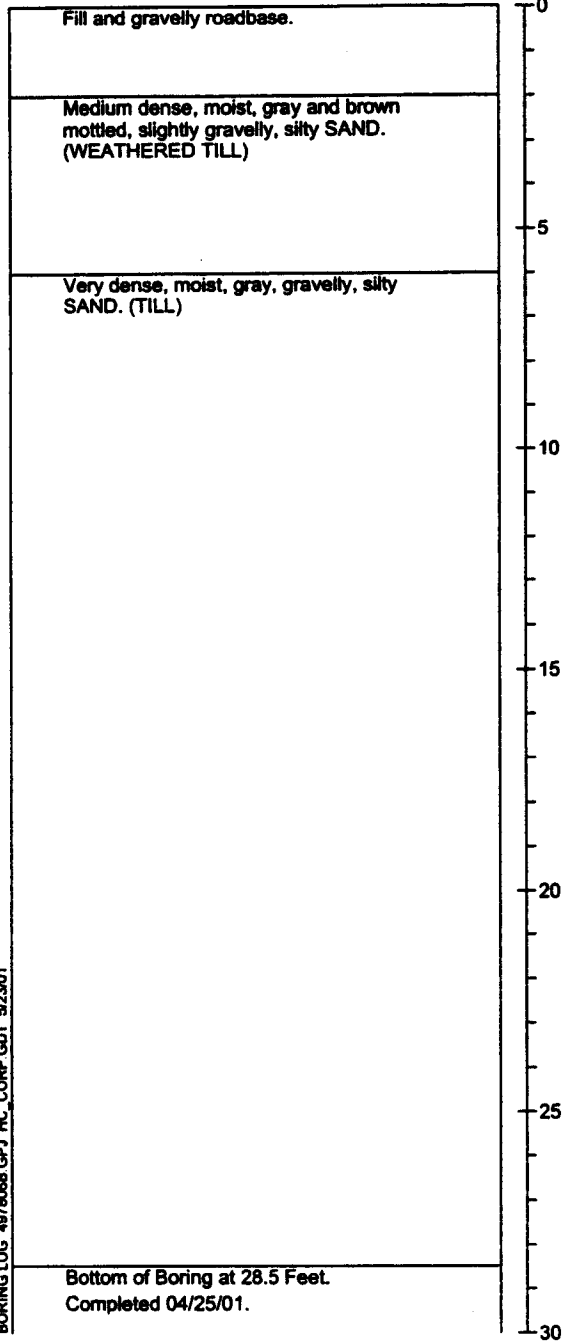
556-2912-001 (28)

AR 011110

# Boring Log HC01-B403

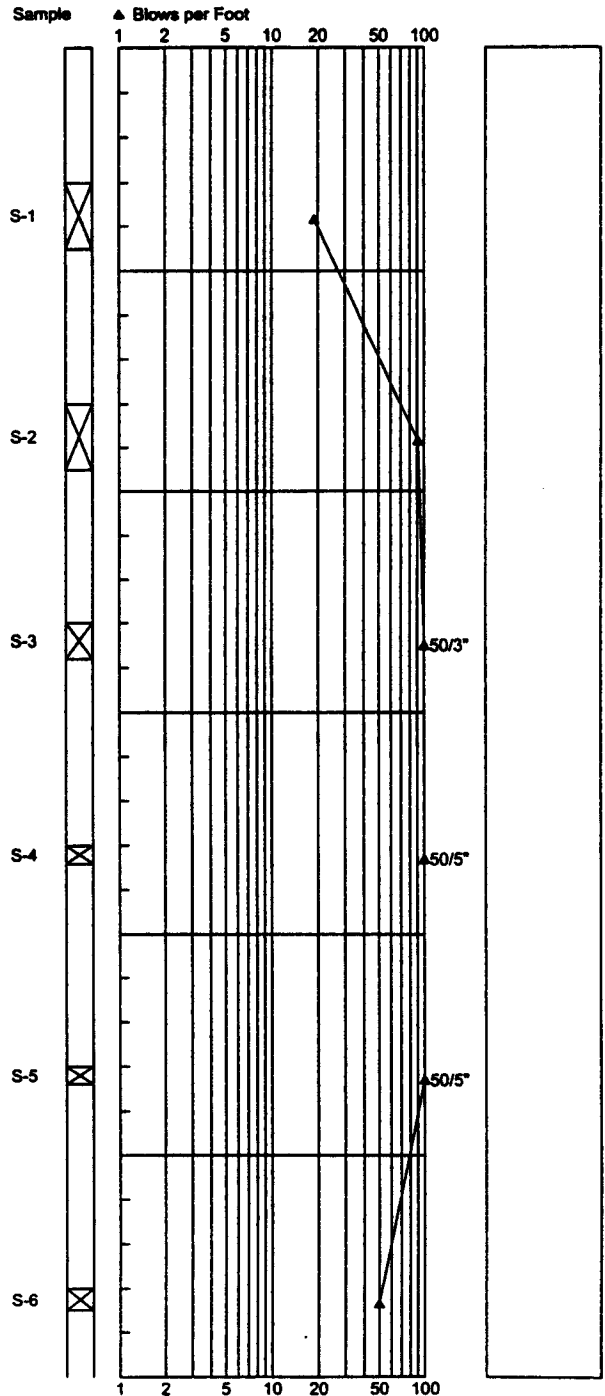
## Soil Descriptions

Approximate Ground Surface Elevation in Feet: 361.7



## STANDARD PENETRATION RESISTANCE

## LAB TESTS



1. Refer to Figure A-1 for explanation of descriptions and symbols.
2. Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

**HARTCROWSER**

J-4978-06

04/01

Figure A-4

July 2001

556-2912-001 (28)

AR 011111



**APPENDIX H**

**WATER QUALITY TREATMENT FACILITY SIZING CALCULATIONS  
FOR SASA, STEP, AND THIRD RUNWAY**

**APPENDIX H**

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## 1. WATER QUALITY DESIGN STORM PEAK FLOW CALCULATIONS FOR BIOSWALE AND FILTER STRIP SIZING

Bioswale and filter strip size was based on the water quality design flow rate, specified as the 6-month, 24-hour peak flow rate.

The Santa Barbara Urban Hydrograph (SBUH) method of the StormSHED hydrologic modeling program was used to estimate the 6-month storm flow rate, using 64 percent of the STIA 2-year, 24-hour precipitation of 2.0 inches. The StormSHED calculated flows for the SASA parking lot (17.76 acres), a 300-ft flowpath across a runway-taxiway intersection (assumed to be 145 ft long, for a total of 1.0 acre), and the North Cargo Area (4.30 acres) are provided in Table H-1. The StormSHED output files for these areas are provided in Tables H-2, H-3, and H-4.

Table H-1. Water quality design storm peak flows.

Area	6-month peak flow (64% of 2-yr) (cfs)
SASA Parking Lot	5.03
Taxiway	0.31
North Cargo Area	1.29

**Drainage Area: SASA Parking**

Hyd Method: SBUH Hyd  
 Peak Factor: 484.00  
 Storm Dur 24.00 hrs

Loss Method: SCS CN Number  
 SCS Abs: 0.20

Area CN  
 Pervious 0.0000 ac 0.00  
 Impervious 17.7600 ac 98.00  
 Total 17.7600 ac

TC  
 0.00 hrs  
 0.13 hrs

**Supporting Data:**

**Impervious CN Data:**

Imp Area 98.00 17.7600 ac

**Impervious TC Data:**

Flow type: Description:  
 Sheet sheet  
 Channel pipes

Length:	Slope:	Coeff:	Travel Time
250.00 ft	1.00%	0.0110	4.44 min
800.00 ft	2.00%	42.0000	2.24 min

<b>SASA Parking Event Summary:</b>							
BasinID	Peak Q	Peak T	Peak Vol	Area	Method	Raintype	Event
-----	(cfs)	(hrs)	(ac-ft)	ac	/Loss		
SASA Parking	5.03	8.00	1.5746	17.76	SBUH/SCS	kc24hr	0.64*2y

**Drainage Area: RW/TW Intersection**

Hyd Method:	SBUH Hyd	Loss Method:	SCS CN Number
Peak Factor:	484.00	SCS Abs:	0.20
Storm Dur	24.00 hrs		
	Area	CN	TC
Pervious	0.0000 ac	78.00	0.00 hrs
Impervious	1.0000 ac	98.00	0.08 hrs
Total	1.0000 ac		

**Supporting Data:**

**Impervious CN Data:**

Impervious 98.00 1.0000 ac

**Impervious TC Data:**

Flow type:	Description:	Length:	Slope:	Coeff:	Travel Time
Sheet	Sheet	300.00 ft	1.50%	0.0100	5.06 min

**RW/TW Intersection Event Summary:**

BasinID	Peak Q	Peak T	Peak Vol	Area	Method	Raintype	Event
-----	(cfs)	(hrs)	(ac-ft)	ac	/Loss		
RW/TW Intersection	0.31	7.83	0.0887	1.00	SBUH/SCS	kc24hr	0.64*2y

**Drainage Area: North Cargo**

Hyd Method: SBUH Hyd  
 Peak Factor: 484.00  
 Storm Dur 24.00 hrs

Loss Method: SCS CN Number  
 SCS Abs: 0.20

Area CN  
 Pervious 0.0000 ac 78.00  
 Impervious 4.3000 ac 98.00  
 Total 4.3000 ac

TC  
 0.00 hrs  
 0.09 hrs

**Supporting Data:**

**Impervious CN Data:**

Impervious 98.00 4.3000 ac

**Impervious TC Data:**

Flow type: Description:  
 Sheet Sheet  
 Channel Pipe

Length: Slope: Coeff: Travel Time  
 200.00 ft 2.00% 0.0100 3.26 min  
 800.00 ft 2.00% 42.0000 2.24 min

**North Cargo Event Summary:**

BasinID	Peak Q (cfs)	Peak T (hrs)	Peak Vol (ac-ft)	Area ac	Method /Loss	Raintype	Event
North Cargo	1.29	7.83	0.3812	4.30	SBUH/SCS	kc24hr	0.64*2y

## 2. MEAN ANNUAL STORM RUNOFF VOLUME CALCULATIONS FOR WETVAULT SIZING

Wetvault volume was based on the mean annual storm volume. The mean annual storm volume for the STEP service area and Air Cargo Road was calculated using the simplified method described in the King County Manual (King County, 1998). Because the area to be treated is 100 percent impervious, the runoff formula from the King County Manual was simplified to:

$$\text{Runoff Volume (ft}^3\text{), } V_r = 0.9 * A_i * (R/12)$$

where:  $A_i$  = impervious area (ft<sup>2</sup>)  
R = mean annual storm precipitation (inches)

The STEP service area and Air Cargo Road are approximately 9.05 acres (394,300 ft<sup>2</sup>). The mean annual storm precipitation of 0.47 inch was obtained from the isopluvial map provided in the King County Manual. Therefore, the mean annual runoff was calculated to be:

$$V_r = 0.9 * (394,300) * (0.47/12) = 13,900 \text{ ft}^3.$$

### 3. TAXIWAY FILTER STRIP SIZE CALCULATIONS

Calculations were performed for a 300-ft flow path across a runway-taxiway intersection (assumed to be 145-ft long). 300 ft is approximately the longest flow path dimension among taxiways and runways, chosen for conservative BMP calculations.

Note: length measurements for runways and taxiways are perpendicular to length measurements for filter strips. For taxiways and runways, length is measured parallel to the direction of aircraft landing and takeoff; for filter strips, length is the dimension parallel to the direction of flow.

Assumptions:

WQ design flow,  $Q = 0.31$  cfs (see Section 1 in this appendix)  
Manning's roughness,  $n = 0.35$  (per the King County Manual)  
Strip width,  $W = 145$  ft (assumed)  
Longitudinal slope,  $s = 0.02^1$

Calculations:

$$\text{Flow depth, } d = \left[ \frac{Q * n}{1.49 * W * s^{0.5}} \right]^{0.6} = \left[ \frac{0.31 * 0.35}{1.49 * 145 * 0.02^{0.5}} \right]^{0.6} \approx 0.034 \text{ ft}$$

$$\text{Flow velocity, } V = \frac{Q}{W * d} = \frac{0.31}{145 * 0.034} \approx 0.063 \text{ fps}$$

Per the King County Manual, the strip must have a minimum 9-minute (540-second) residence time.

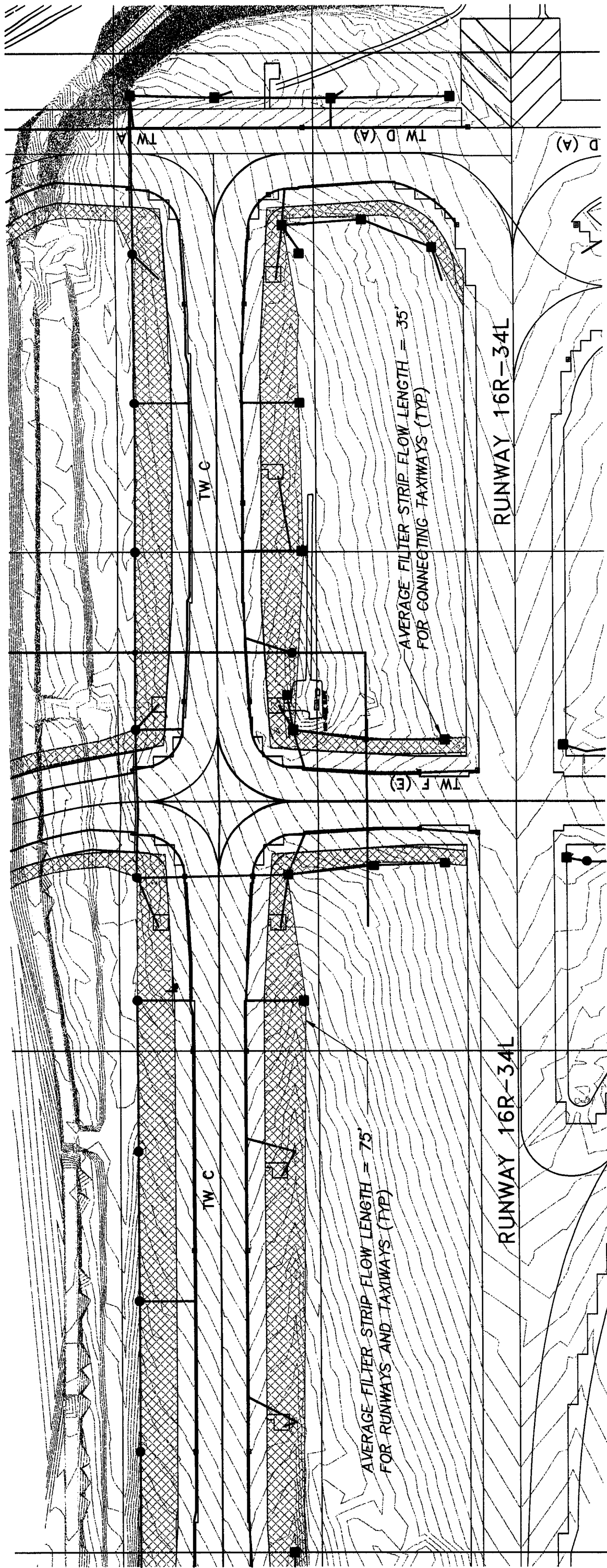
$$\text{Strip Length, } L = 540 * V = 540 * 0.063 = 34 \text{ ft}$$

The plan view for a typical segment of filter strip drain is shown in Figure H-1, and the typical filter strip detail is shown in Figure H-2. As shown, the typical filter strip width will exceed the minimum required width in most locations, and the filter strips will meet the minimum required width in all locations.

---

<sup>1</sup> Nominal STIA design specifications call for a slope of 1.5 percent as shown on Figure H-2. This assumed slope results in a conservative size estimate.










FILE: 16R-1  
DATE: 08/09/00

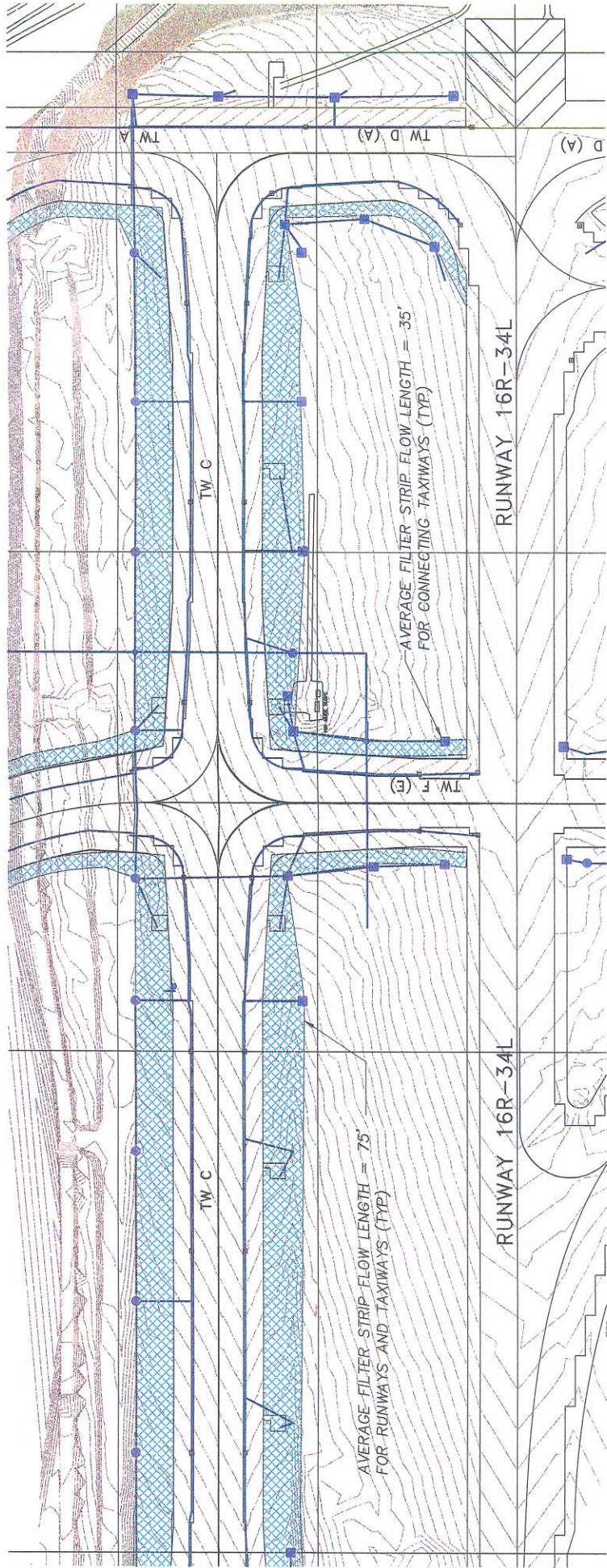


0 100 200  
SCALE IN FEET

LEGEND

-  FILTER STRIP
-  MANHOLE/CATCHBASIN
-  CATCHBASIN/INLET
-  UNDER DRAIN
-  DRAINAGE PIPES

**Figure H-1**  
**Third Runway Taxiway C**  
**Typical Segment, Filter Strip and Drainage**  
**Plan View**



**Figure H-1**  
**Third Runway Taxiway C**  
**Typical Segment, Filter Strip and Drainage**  
**Plan View**

AR 011120

**LEGEND**

-  FILTER STRIP
-  MANHOLE/CATCHBASIN
-  CATCHBASIN/INLET
-  UNDER DRAIN
-  DRAINAGE PIPES



FILE: H-1  
 DATE: 06/05/00

#### 4. SASA BIOSWALE SIZE CALCULATIONS

The following assumptions were used for the SASA bioswale calculations:

Flow depth,  $y = 2$  inches = 0.167 ft (per the King County Manual for regularly mowed swale)

H:V side slopes,  $Z = 3$  (chosen value)

Manning's roughness,  $n = 0.20$  (per the King County Manual)

Longitudinal slope,  $s = 0.05$  (chosen value)

The SASA parking area was divided into four subareas (A through D). Bioswale sizes were calculated for each subarea. To calculate bioswale sizes, the total discharge of 5.03 cfs was divided among the subareas (weighted by area), as shown in Table H-5.

Table H-5. The SASA SDS subareas (conceptual).

Subarea	Area (acres)	% of SASA PGIS	Q (cfs) <sup>a</sup>
A	2.82	35%	1.76
B	3.06	38%	1.91
C	1.17	14%	0.70
D	1.02	13%	0.65
<b>TOTAL</b>	<b>8.07</b>	<b>100%</b>	<b>5.03</b>

<sup>a</sup> For this feasibility analysis, the total SASA water quality flow rate of 5.03 cfs was divided among the subareas (prorated by area). This is a conservative estimate of flows for each subarea—flows modeled individually for the subareas would be smaller.

An example calculation for Area B is shown below:

$$\text{Bottom width, } b = \frac{Q * n}{1.49 * y^{1.67} * s^{0.5}} = \frac{1.91 * 0.20}{1.49 * (0.167)^{1.67} * (0.05)^{0.5}} = 22.9 \text{ ft}$$

Bottom width cannot be greater than 10 ft. The swale was therefore divided into three flow channels, each taking one-third of the flow (= 0.64 cfs).

$$\text{Bottom width, } b = \frac{Q * n}{1.49 * y^{1.67} * s^{0.5}} = \frac{0.64 * 0.20}{1.49 * (0.167)^{1.67} * (0.05)^{0.5}} = 7.7 \text{ ft}$$

$$V = \frac{Q}{A} = \frac{Q}{by + Zy^2} = \frac{0.64}{(7.7 * 0.167) + (3 * 0.167^2)} = 0.47 \text{ ft/s}$$

The flow must have a minimum 9-minute (540-second) residence time.

$$\text{Swale Length, } L = 540 * V = 540 * 0.47 \approx 254 \text{ ft}$$

The complete bioswale calculations for the four subareas are summarized in Table H-6. Subareas A and B required splitting the flow.

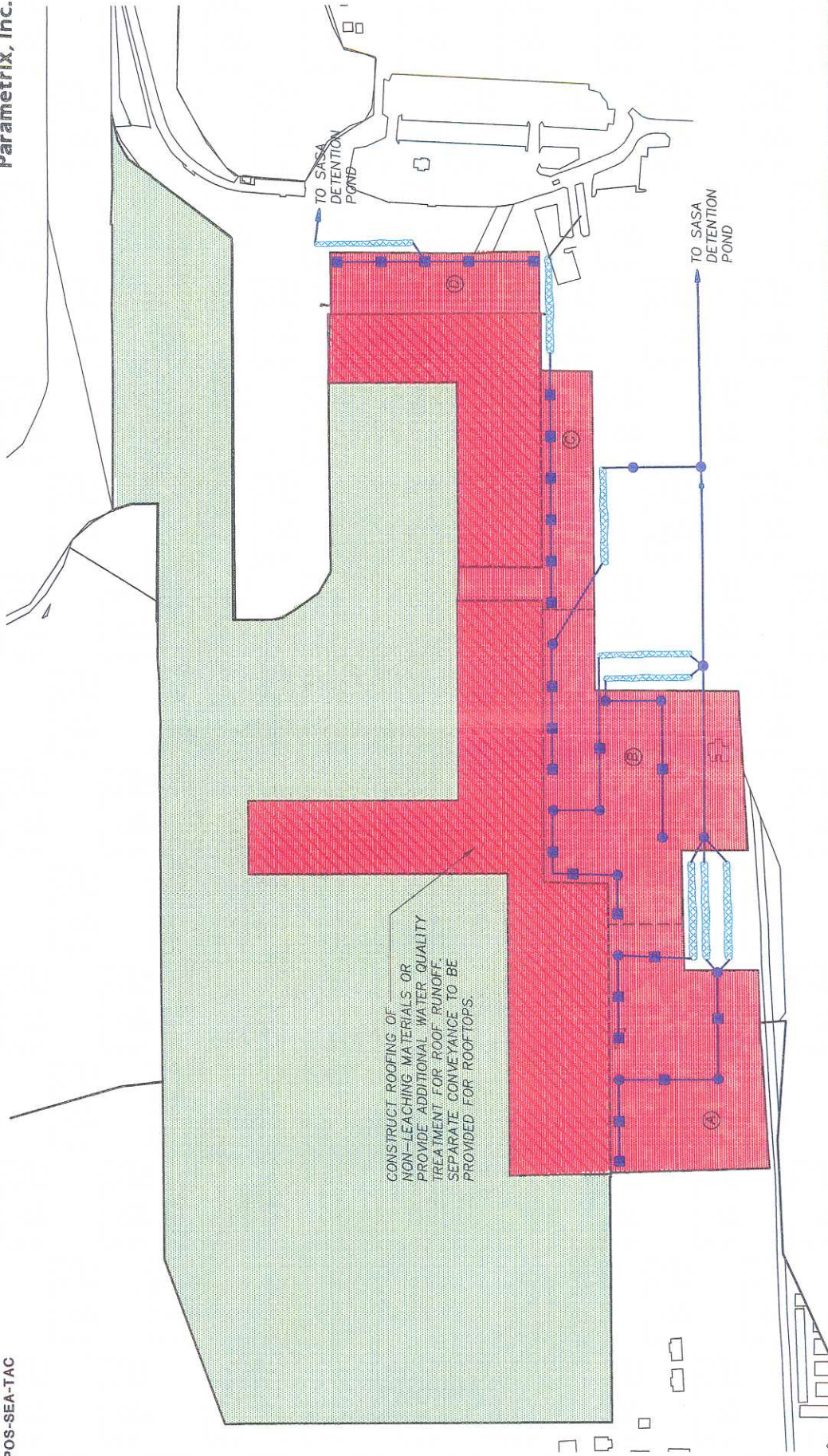
**Table H-6. Bioswale sizing calculations for the SASA SDS (conceptual).**

Subarea		Q	Z	n	y	s	b	V	L
A		1.76	3	0.20	0.17	0.05	21.1 <sup>a</sup>		
	1.76 ÷ 3 =	0.59	3	0.20	0.17	0.05	7.0	0.47	253
B		1.91	3	0.20	0.17	0.05	22.9 <sup>a</sup>		
	1.91 ÷ 3 =	0.64	3	0.20	0.17	0.05	7.7	0.47	254
C		0.70	3	0.20	0.17	0.05	8.4	0.47	256
D		0.65	3	0.20	0.17	0.05	7.8	0.47	254

<sup>a</sup> *b* must be less than 10 ft; therefore, divide flow into three equal-width swales.

The SASA parking lot conceptual bioswale layout is shown in Figure H-3, and the typical bioswale detail is shown in Figure H-4.





DATE: 02/20/16  
DATE: 02/25/16



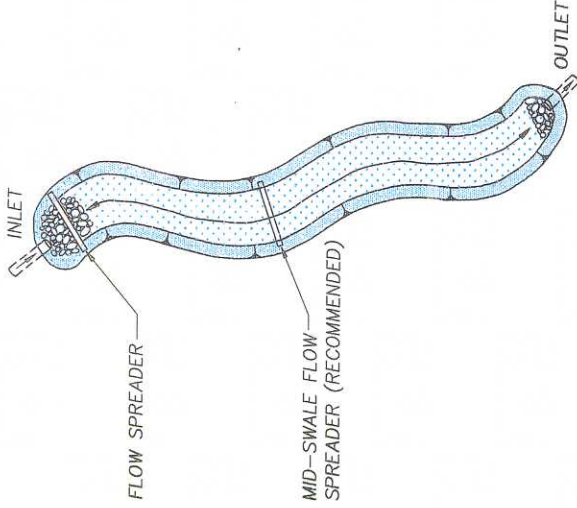
0 125 250  
SCALE IN FEET

LEGEND

- IWS AREA
- SDS POLLUTION GENERATING IMPERVIOUS SURFACE
- SDS ROOFTOP AREA
- BIOFILTRATION SWALE

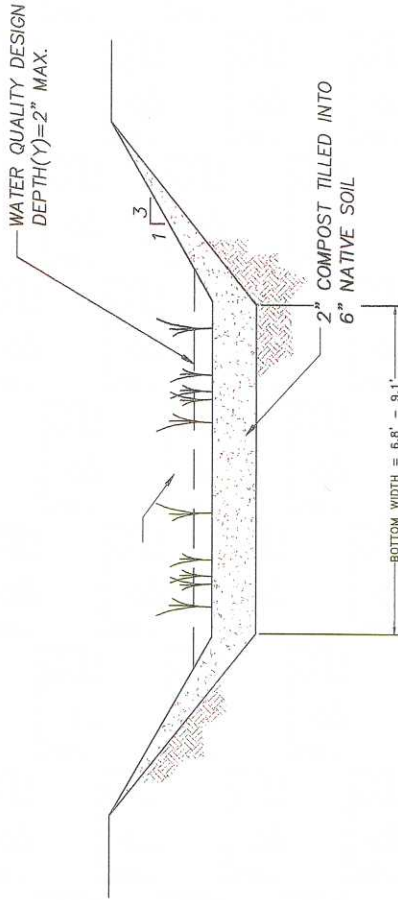
- PGIS SUBAREA BOUNDARY
- PGIS SUBAREA
- MANHOLE/CATCHBASIN
- CATCHBASIN/INLET

Figure H-3  
SASA Parking Lot  
Conceptual Bioswale Layout  
(Based on Existing Topography)



NOTES:  
 LONGITUDINAL SLOPES 1-6%  
 PROVIDE UNDERDRAIN FOR SLOPES < 1.5%

BIOFILTRATION SWALE  
 PLAN  
 NTS



TYPICAL SWALE  
 SECTION  
 NTS

Figure H-4  
 Typical SASA Parking Lot  
 Bioswale Detail

AR 011124

## 5. NORTH CARGO AREA BIOSWALE CALCULATIONS

The following assumptions were used for the North Cargo Area bioswale calculations:

WQ design flow,  $Q = 1.29$  cfs (see Section 1 in this appendix)

Flow depth,  $y = 2$  inches = 0.167 ft (per the King County Manual for regularly mowed swale)

H:V side slopes,  $Z = 3$  (chosen value)

Manning's roughness,  $n = 0.20$  (per the King County Manual)

Longitudinal slope,  $s = 0.05$  (chosen value)

The calculations are shown below:

$$\text{Bottom width, } b = \frac{Q * n}{1.49 * y^{1.67} * s^{0.5}} = \frac{1.29 * 0.20}{1.49 * (0.167)^{1.67} * (0.05)^{0.5}} = 15.4 \text{ ft}$$

The swale width of 15.4 ft is greater than the 10-ft minimum; therefore, the swale will be divided in the center to form two 7.7-ft wide swales. Each swale will treat half the flow (0.65 cfs each).

$$V = \frac{Q}{A} = \frac{Q}{by + Zy^2} = \frac{0.65}{(7.7 * 0.167) + (3 * 0.167^2)} = 0.47 \text{ ft/s}$$

The flow must have a minimum 9-minute (540-second) residence time.

$$\text{Swale Length, } L = 540 * V = 540 * 0.47 = 254 \text{ ft}$$

The bioswale calculations are summarized in Table H-6.

**Table H-6. Bioswale sizing calculations for the North Cargo Area (conceptual).**

No. of Swales	Q (each swale)	Z	n	y	s	b	V	L
2	0.65	3	0.20	0.17	0.05	7.7	0.47	254

## 6. STEP WETVAULT SIZE CALCULATIONS

The following assumption was used for STEP Wetvault size calculations:

Basic Wetpond size (per the King County Manual), therefore:

Wetpool volume = 3 \* [Volume of mean annual 24-hour storm runoff]

The calculation is shown below:

From Section 2 of this appendix, the mean annual storm runoff volume from STEP and Air Cargo Road = 13,900 ft<sup>3</sup>.

Wetvault volume = 3 \* 13,900 = 41,700 ft<sup>3</sup>.

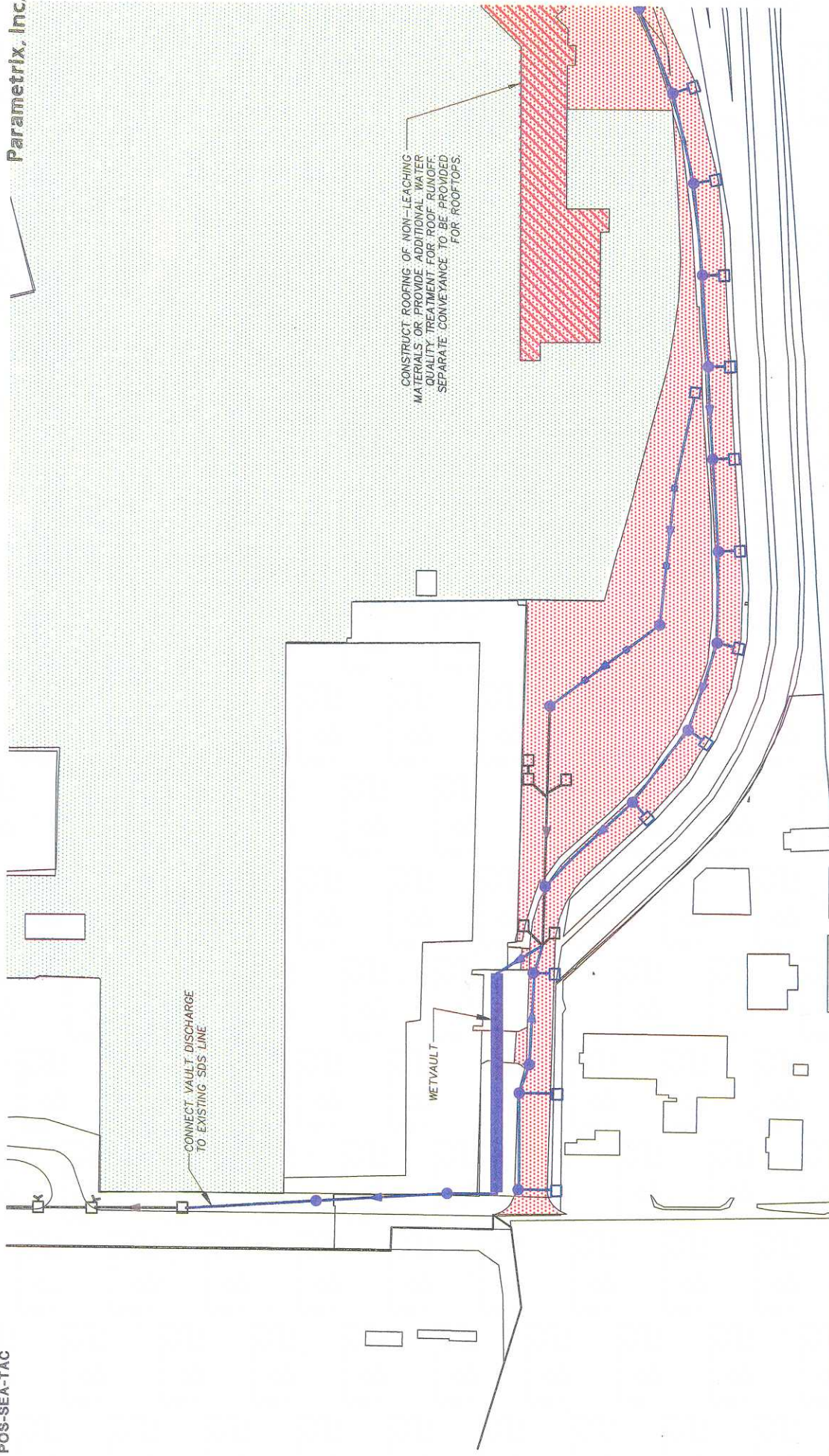
The dimension criteria are shown below:

- The wetvault shall be divided into two cells by a baffle;
- First cell shall have minimum sediment storage of at least 1 ft;
- Second cell shall be minimum 3 ft deep; and
- First cell shall be sized for 25 percent to 35 percent of wetvault volume.

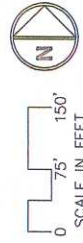
A wetvault 350-ft-long by 10.5-ft-deep by 15-ft-wide would meet the above criteria. The first cell would be 87 ft long, the second cell would be 263 ft long. The 10.5-ft depth includes 2 ft of sediment storage provided in both cells of the vault.

The STEP drainage layout and wetvault location are shown in Figure H-5, and the wetvault details are shown in Figure H-6.





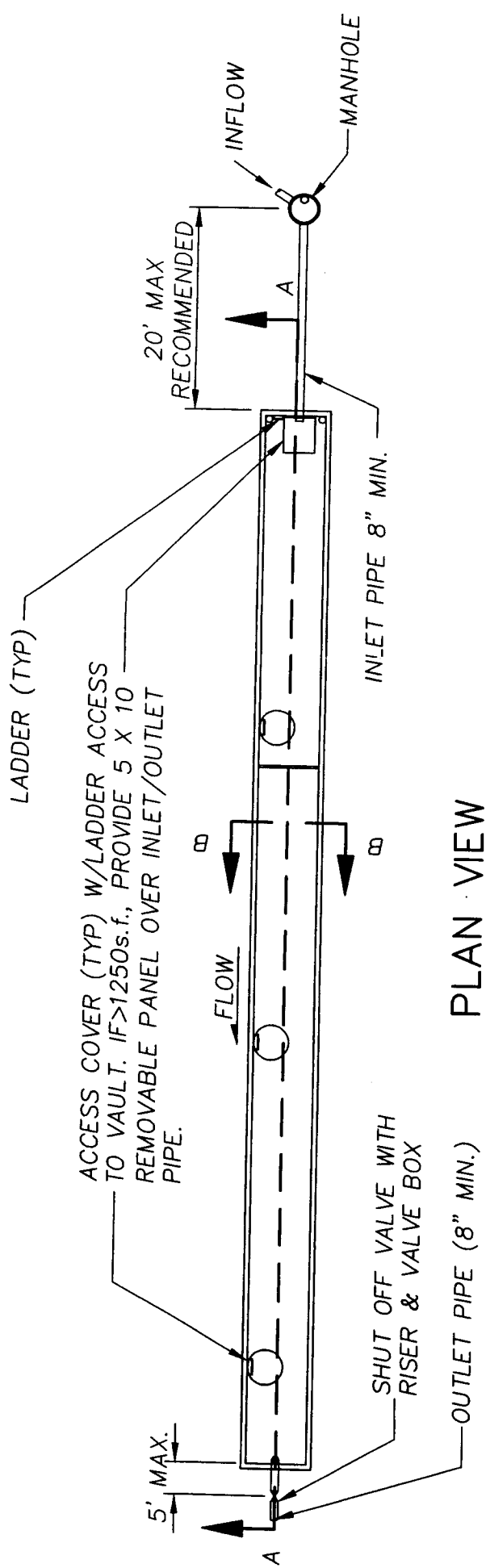
FILE: PG4-5  
DATE: 06/09/00



LEGEND

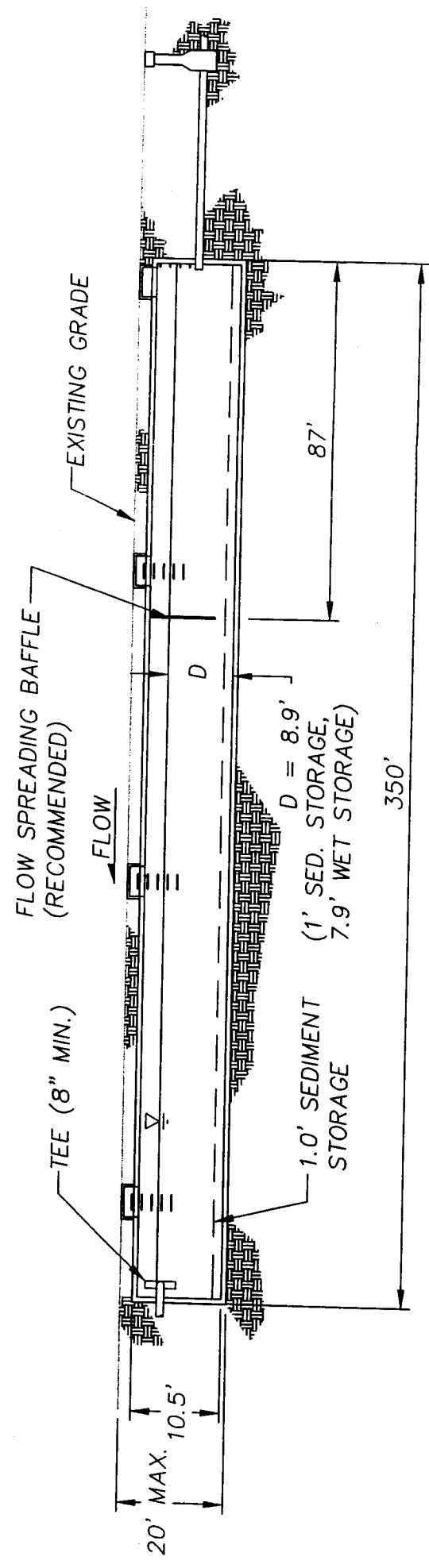
-  SDS POLLUTION GENERATING IMPERVIOUS SURFACE
-  SDS ROOFTOP AREA
-  MANHOLE/CATCHBASIN
-  CATCHBASIN/INLET
-  DRAINAGE PIPE
-  IWS AREA

Figure H-5  
STEP Drainage Layout  
and Wetvaul Location



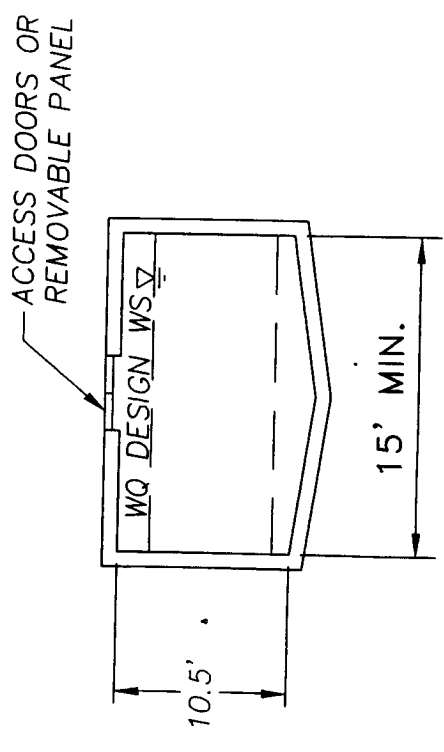
PLAN VIEW

NOT TO SCALE



SECTION A-A

NOT TO SCALE



SECTION B-B

NOT TO SCALE  
ROTATED 90° CCW

FILE: 08-11-00  
DATE: 08/17/00

## **7. NORTH EMPLOYEE PARKING LOT BIOSWALE CALCULATIONS**

For information on the North Employee Parking Lot bioswale calculations, see the attached excerpt from the technical information report prepared by David Evans and Associates, Inc.

## Water Quantity Control

Detention facilities will be sized to limit peak rate runoff control to be at or below the existing 2, 10 and 100-year, 24-hour design storm event. Detention will be accomplished with an underground concrete detention vault located at the west end of the site. The outlet structure will consist of a flow restrictor/oil pollution control device with two orifices and a notch weir to control the 2, 10 and 100 year design storm events.

On-site water quantity calculations are contained in Appendix A.

Existing Flow (cfs)			Developed Flow (cfs)		
2 yr	10yr	100 yr	2 yr	10 yr	100 yr
2.99	8.14	15.84	19.00	28.84	41.07

## Water Quality Control

On-site water quality control will be provided through the use of a grass-lined swale.

The water quality facility will be placed downstream of the detention facility which allows for the use of existing condition flow sizing for the biofiltration swale. Swale bottom width was determined using peak flow from a 2-year, 24-hour storm with existing conditions, 6.0% slope, 3:1 side slopes, Mannings "n" of 0.35 and a flow depth of 4 inches. The schematic layout for the water quantity and water quality facilities are shown in Figure 5.

On-site water quality calculations are contained in Appendix B.

## CHANNEL DESIGN FORM

PROJECT: Port of Seattle - North Employee Parking Lot

DESCRIPTION: Biofiltration Swale

BEGIN LOCATION:

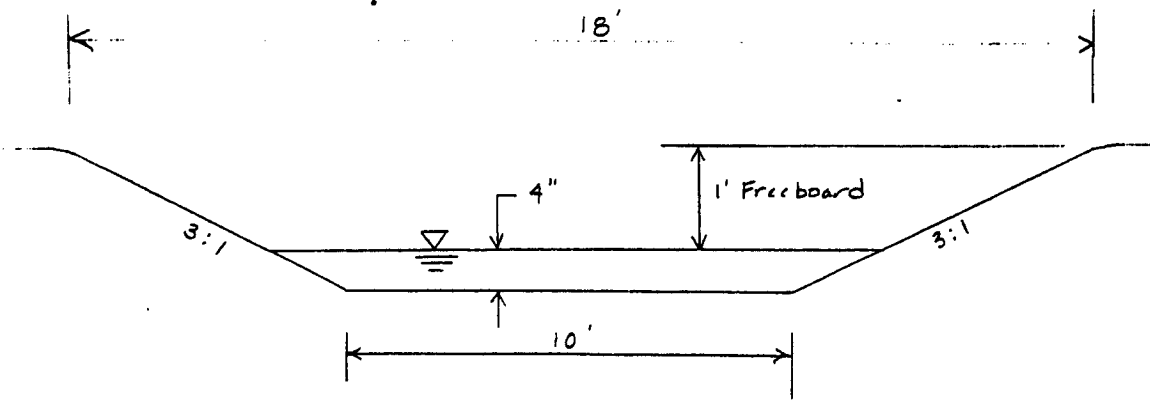
END LOCATION:

LENGTH 200 LF

	Input	Output
FREEBOARD DEPTH (FT)	f = 1	Velocity = 0.82
WATER DEPTH (FT)	y = 0.33	Flow, CFS = 2.96
SIDE SLOPE 1 = (1/H)	H1 = 3	Top Width = 17.98
SIDE SLOPE 2 = (1/H)	H2 = 3	
BOTTOM WIDTH IN FEET	b = 10	
MANNINGS VALUE	n = 0.2	
SLOPE OF CHANNEL FT/FT	s = 0.06	

The above indicates the design for biofiltration swale function for the 2-year event for existing conditions.

Per KCSWM, a swale slope of up to 6 % and Mannings "n" value of 0.20 shall be used when designing for swale function.



TYPICAL SECTION

## CHANNEL DESIGN FORM

PROJECT: Port of Seattle - North Employee Parking Lot

DESCRIPTION: Biofiltration Swale

BEGIN LOCATION:

END LOCATION:

LENGTH 200 LF

	Input	Output
FREEBOARD DEPTH (FT)	f = 1	Velocity = 4.94
WATER DEPTH (FT)	y = 0.33	Flow, CFS = 17.93
SIDE SLOPE 1 = (1/H)	H1 = 3	Top Width = 17.98
SIDE SLOPE 2 = (1/H)	H2 = 3	
BOTTOM WIDTH IN FEET	b = 10	
MANNINGS VALUE	n = 0.033	
SLOPE OF CHANNEL FT/FT	s = 0.06	

The above indicates the swale conveyance and stability calculations.

The actual swale slope and Mannings "n" value are used.

The swale has the capacity to convey the 100-year design flow of 15.84 cfs and maintain a freeboard of 1 foot with a velocity of less than 5 ft/sec.

SEE PREVIOUS SHEET FOR TYPICAL SECTION

TYPICAL SECTION





**8. SOUTH 154<sup>TH</sup> STREET/156<sup>TH</sup> WAY RELOCATION BIOSWALE CALCULATIONS  
AND STORMWATER DRAINAGE PLANS**

For information on the South 154<sup>th</sup> Street/156<sup>th</sup> Way relocation bioswale calculations and stormwater drainage plans, see the attached excerpt from the technical information report prepared by Kato and Warren, Inc.



---

*Surface Water  
Technical Information Report  
Appendix B  
Biofiltration Swale Calculations*

*for*

*Runways 16L/16R Safety Area  
Improvement Project  
South 154<sup>th</sup> Street/156<sup>th</sup> Way Relocation*

*Presented to:  
Port of Seattle  
Sea-Tac International Airport*

*Presented by:  
Kato & Warren, Inc.*

*August 1998*

---

**AR 011135**

<b>KATO &amp; WARREN, INC.</b> 2003 - WESTERN AVENUE SUITE 555 • MARKET PLACE ONE SEATTLE, WASHINGTON 98121 (206) 448-4200 FAX (206) 728-5608	CLIENT <u>PORT OF SEATTLE</u>	By <u>KRS</u>	sheet <u>    </u> of <u>    </u>
	PROJECT <u>S. 154<sup>th</sup> St. / 156<sup>th</sup> Way</u>	Date <u>7/21/98</u>	
	CONTACT <u>                    </u>	Chkd <u>          </u>	
	PHONE ( <u>    </u> )	Date <u>          </u>	96-382 Job No.

BIOFILTRATION SWALE SIZING

## DESIGN CRITERIA

draft 1997 King County Surface Water Design Manual (KCSWDM)  
 flow depth = 4 inches (unless otherwise noted to achieve  $W_{min} = 2'$ )

$Q_{w0} = 60\% (Q_2)$  [15-minute time step]

$n = 0.20$  sizing for biofiltration

$n = 0.027$  100-yr conveyance check

$V_{max} = 1 \text{ ft/s}$  @  $Q_{w0}$

$V_{max} = 5 \text{ ft/s}$  @  $Q_{100}$

BIOSWALE #1	89+00 to 90+75
BIOSWALE #2	90+75 to 115+50
BIOSWALE #3	115+50 to 148+70
BIOSWALE #4	PERIM. ROAD 140+00 to E. END

2 & 100 yr. peak flow rates ( $Q_2$  &  $Q_{100}$ )

KCRTS

Rainfall region / scale factor : SeaTac 1.0

Time step 15-minute

Swale bottom width

sized using Manning's equation (FlowMaster software) rather than eqn's (6-1)-(6-4)

Swale Length

9-min residence time:  $L = V(540)$  (eqn. 6-5)

**KATO & WARREN, INC.**  
 2003 - WESTERN AVENUE  
 SUITE 555 - MARKET PLACE ONE  
 SEATTLE, WASHINGTON 98121  
 (206) 448-4200 • FAX (206) 728-5608  
 E-MAIL: katwar@nwlink.com

CLIENT PORT OF SEATTLE

PROJECT S. 154<sup>th</sup> St / 156<sup>th</sup> Way

CONTACT \_\_\_\_\_

PHONE ( ) \_\_\_\_\_

By KRS

Date 7/21/88

Chkd \_\_\_\_\_

Date \_\_\_\_\_

sheet \_\_\_\_\_ of \_\_\_\_\_

96-382

Job No.

BIOSWALE #1 88+82 to 90+75

Impervious area:

11,549 S.F. → 0.27 AC

Pervious area:

13,060 - 11549 = 1511 S.F. → 0.04 AC

2 & 100-yr peak runoff (KCRTS)

Flow paths:

Impervious - 21.5' @ 2%

Pervious - 8' @ 2%

$Q_2 = 0.19$  cfs

$Q_{wa} = 0.6(Q_2) = 0.11$  cfs

$Q_{100} = 0.39$  cfs

Swale geometry options:

SLOPE	WIDTH	DEPTH	VELOCITY	LENGTH
1.5%	2'	0.18'	0.25 fps	135'
2%	2'	0.16'	0.27 fps	146'
2.5%	2'	0.15'	0.30 fps	162'

← selection

Swale design:

Bottom width = 2'

Bottom slope = 1.5%

Length = 135'

100-yr conveyance check

$d_{100} = 0.35'$  ✓ OK

$V_{100} = 0.36$  fps ✓ OK

BIOSWALE #1

Flow Frequency Analysis  
Time Series File:pos-bs1.tsf  
Project Location:Sea-Tac

---Annual Peak Flow Rates---  
Flow Rate Rank Time of Peak  
(CFS)  
0.201 4 8/27/ 1 18:00  
0.128 8 1/ 6/ 2 1:00  
0.343 2 12/ 8/ 2 17:15  
0.137 7 8/25/ 4 23:45  
0.200 5 10/28/ 4 16:00  
0.207 3 10/22/ 5 10:00  
0.190 6 10/25/ 6 22:45  
0.393 1 1/ 9/ 8 6:30  
Computed Peaks

-----Flow Frequency Analysis-----  
- - Peaks - - Rank Return Prob  
(CFS) Period  
0.393 1 100.00 0.990  
0.343 2 25.00 0.960  
0.207 3 10.00 0.900  
0.201 4 5.00 0.800  
0.200 5 3.00 0.667  
0.190 6 2.00 0.500  
0.137 7 1.30 0.231  
0.128 8 1.10 0.091  
0.377 50.00 0.980

**Biofiltration Swale #1**  
**Worksheet for Trapezoidal Channel**

---

<b>Project Description</b>	
Project File	c:\fm\96382.fm2
Worksheet	Biofiltration sizing
Flow Element	Trapezoidal Channel
Method	Manning's Formula
Solve For	Channel Depth

---

---

<b>Input Data</b>		
Mannings Coefficient	0.200	
Channel Slope	0.015000	ft/ft
Left Side Slope	3.00	H : V
Right Side Slope	3.00	H : V
Bottom Width	2.00	ft
Discharge	0.11	ft <sup>3</sup> /s

---

---

<b>Results</b>		
Depth	0.18	ft
Flow Area	0.44	ft <sup>2</sup>
Wetted Perimeter	3.11	ft
Top Width	3.05	ft
Critical Depth	0.04	ft
Critical Slope	1.693355	ft/ft
Velocity	0.25	ft/s
Velocity Head	0.96e-3	ft
Specific Energy	0.18	ft
Froude Number	0.11	
Flow is subcritical.		

---

**Biofiltration Swale #1  
Worksheet for Trapezoidal Channel**

<b>Project Description</b>	
Project File	c:\fm\96382.fm2
Worksheet	Biofiltration 100-year conveyance
Flow Element	Trapezoidal Channel
Method	Manning's Formula
Solve For	Channel Depth

<b>Input Data</b>		
Mannings Coefficient	0.200	
Channel Slope	0.015000	ft/ft
Left Side Slope	3.00	H : V
Right Side Slope	3.00	H : V
Bottom Width	2.00	ft
Discharge	0.39	ft <sup>3</sup> /s

<b>Results</b>		
Depth	0.35	ft
Flow Area	1.07	ft <sup>2</sup>
Wetted Perimeter	4.22	ft
Top Width	4.10	ft
Critical Depth	0.10	ft
Critical Slope	1.328946	ft/ft
Velocity	0.36	ft/s
Velocity Head	0.21e-2	ft
Specific Energy	0.35	ft
Froude Number	0.13	
Flow is subcritical.		

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	PROJECT <u>S. 154<sup>th</sup> St. / 156<sup>th</sup> Way</u>	Date <u>7/2/98</u>	
	CONTACT <u>                    </u>	Chkd <u>            </u>	
	PHONE ( <u>    </u> ) <u>            </u>	Date <u>            </u>	96-382 Job No.

BIOSWALE #2 90+75 to 115+50

Impervious area:

41' (115+50 - 90+75) = 1,01,475 S.F. } 2.35 AC  
 Widening @ W. end : 1017 S.F.

Pervious area:

16' (115+50 - 90+75) = 0.91 AC (till grass)

2 & 100 yr. peak runoff (KCRS)

Flow paths:

- Impervious - 16' @ 2%
- Pervious - 8' @ 2%

$Q_2 = 1.67 \text{ cfs}$        $Q_{wo} = 0.6(Q_2) = 1.0 \text{ cfs}$

$Q_{100} = 3.72 \text{ cfs}$

Swale geometry options:

SLOPE	WIDTH	VELOCITY	LENGTH	ELEVATIONAL DROP	
1%	8.2'	0.33 fps	178'	1.8'	← selection
1.5%	6.6'	0.40 fps	216'	3.2'	
2%	5.7'	0.45 fps	243'	4.9'	

Swale design

Bottom width = 9'

Bottom slope = 1%

Length = 180'

100-yr conveyance check

$d_{100} = 0.67'$  ✓OK

$V_{100} = 0.50 \text{ fps}$  ✓OK

BIDSWALE #2

Flow Frequency Analysis  
Time Series File:pos-bs2.tsf  
Project Location:Sea-Tac

---Annual Peak Flow Rates---  
Flow Rate Rank Time of Peak  
(CFS)  
1.76 5 8/27/ 1 18:00  
1.20 8 1/ 6/ 2 1:00  
3.24 2 12/ 8/ 2 17:15  
1.22 7 8/25/ 4 23:45  
2.00 3 11/17/ 4 5:00  
1.82 4 10/22/ 5 10:00  
1.67 6 10/25/ 6 22:45  
3.72 1 1/ 9/ 8 6:30  
Computed Peaks

-----Flow Frequency Analysis-----  
- - Peaks - - Rank Return Prob  
(CFS) Period  
3.72 1 100.00 0.990  
3.24 2 25.00 0.960  
2.00 3 10.00 0.900  
1.82 4 5.00 0.800  
1.76 5 3.00 0.667  
1.67 6 2.00 0.500  
1.22 7 1.30 0.231  
1.20 8 1.10 0.091  
3.56 50.00 0.980



**Biofiltration Swale #2**  
**Worksheet for Trapezoidal Channel**

<b>Project Description</b>	
Project File	c:\fm\96382.fm2
Worksheet	Biofiltration sizing
Flow Element	Trapezoidal Channel
Method	Manning's Formula
Solve For	Bottom Width

<b>Input Data</b>		
Mannings Coefficient	0.200	
Channel Slope	0.010000	ft/ft
Depth	0.33	ft
Left Side Slope	3.00	H : V
Right Side Slope	3.00	H : V
Discharge	1.00	ft <sup>3</sup> /s

<b>Results</b>		
Bottom Width	8.22	ft
Flow Area	3.04	ft <sup>2</sup>
Wetted Perimeter	10.30	ft
Top Width	10.20	ft
Critical Depth	0.08	ft
Critical Slope	1.390935	ft/ft
Velocity	0.33	ft/s
Velocity Head	0.17e-2	ft
Specific Energy	0.33	ft
Froude Number	0.11	
Flow is subcritical.		

**Biofiltration Swale #2**  
**Worksheet for Trapezoidal Channel**

<b>Project Description</b>	
Project File	c:\fm\96382.fm2
Worksheet	Biofiltration 100-year conveyance
Flow Element	Trapezoidal Channel
Method	Manning's Formula
Solve For	Channel Depth

<b>Input Data</b>		
Mannings Coefficient	0.200	
Channel Slope	0.010000 ft/ft	
Left Side Slope	3.00	H : V
Right Side Slope	3.00	H : V
Bottom Width	9.00	ft
Discharge	3.72	ft <sup>3</sup> /s

<b>Results</b>		
Depth	0.67	ft
Flow Area	7.39	ft <sup>2</sup>
Wetted Perimeter	13.24	ft
Top Width	13.03	ft
Critical Depth	0.17	ft
Critical Slope	1.076273	ft/ft
Velocity	0.50	ft/s
Velocity Head	0.39e-2	ft
Specific Energy	0.67	ft
Froude Number	0.12	
Flow is subcritical.		

**KATO & WARREN, INC.**  
 2003 - WESTERN AVENUE  
 SUITE 555 - MARKET PLACE ONE  
 SEATTLE, WASHINGTON 98121  
 (206) 448-4200 • FAX (206) 728-5608  
 E-MAIL: katwar@nwlink.com

CLIENT PART OF SEATTLE  
 PROJECT S. 154<sup>th</sup> St. / 156<sup>th</sup> Way  
 CONTACT \_\_\_\_\_  
 PHONE ( ) \_\_\_\_\_

By KRS  
 Date 7/16/08  
 Chkd \_\_\_\_\_  
 Date \_\_\_\_\_

sheet \_\_\_\_\_ of \_\_\_\_\_  
96-382  
 Job No.

BIOSWALE #3 115+50 to 148+70

Impervious area:

$41' (148+70 - 115+50) = 136,120 \text{ S.F.}$

Widening @ E. end: 1011 SF.

Vehicle turn-outs/driveways:  $450 + 4471 \text{ S.F.} = 4,921 \text{ S.F.}$

} 3.26 AC

Pervious area:

$16' (145+00 - 115+50) = 47,200 \text{ S.F.}$

$5' (148+70 - 145+00) = 1,850 \text{ S.F.}$

} 1.13 AC (till grass)

2 @ 100 yr. peak runoff (KCRTS)

Flow paths

Impervious - 16' @ 2%

Pervious - 8' @ 2%

$Q_2 = 2.34 \text{ cfs}$

$Q_{we} = 0.6 Q_2 = 1.40 \text{ cfs}$

$Q_{100} = 5.21 \text{ cfs}$

swale geometry options:

SLOPE	WIDTH	VELOCITY	LENGTH
1.5%	9.5'	0.41 fps	221'
2.5%	7.2'	0.52 fps	281'
3.0%	6.6'	0.56 fps	300'
3.5%	6.1'	0.60 fps	324'

← selection (avg. swale slope)

Swale design:

Bottom width = 8'

Bottom slope = avg 3% (min 2.5%, max 3.5%)

Length = 300'

100yr conveyance check:

$d_{100} = 0.86' \quad \checkmark \text{OK}$

$V_{100} = 0.57 \text{ fps} \quad \checkmark \text{OK}$

BIOSWALE #3

Flow Frequency Analysis  
Time Series File:pos-bs3.tsf  
Project Location:Sea-Tac

---Annual Peak Flow Rates---  
Flow Rate Rank Time of Peak  
(CFS)  
2.46 5 8/27/ 1 18:00  
1.66 8 1/ 6/ 2 1:00  
4.49 2 12/ 8/ 2 17:15  
1.70 7 8/25/ 4 23:45  
2.69 3 11/17/ 4 5:00  
2.57 4 10/22/ 5 10:00  
2.34 6 10/26/ 6 0:45  
5.21 1 1/ 9/ 8 6:30  
Computed Peaks

-----Flow Frequency Analysis-----  
- - Peaks - - Rank Return Prob  
(CFS) Period  
5.21 1 100.00 0.990  
4.49 2 25.00 0.960  
2.69 3 10.00 0.900  
2.57 4 5.00 0.800  
2.46 5 3.00 0.667  
2.34 6 2.00 0.500  
1.70 7 1.30 0.231  
1.66 8 1.10 0.091  
4.97 50.00 0.980

**Biofiltration Swale #3**  
**Worksheet for Trapezoidal Channel**

<b>Project Description</b>	
Project File	c:\fm\96382.fm2
Worksheet	Biofiltration sizing
Flow Element	Trapezoidal Channel
Method	Manning's Formula
Solve For	Bottom Width

<b>Input Data</b>		
Mannings Coefficient	0.200	
Channel Slope	0.030000	ft/ft
Depth	0.33	ft
Left Side Slope	3.00	H : V
Right Side Slope	3.00	H : V
Discharge	1.40	ft <sup>3</sup> /s

<b>Results</b>		
Bottom Width	6.57	ft
Flow Area	2.49	ft <sup>2</sup>
Wetted Perimeter	8.65	ft
Top Width	8.55	ft
Critical Depth	0.11	ft
Critical Slope	1.242869	ft/ft
Velocity	0.56	ft/s
Velocity Head	0.49e-2	ft
Specific Energy	0.33	ft
Froude Number	0.18	
Flow is subcritical.		

**Biofiltration Swale #3**  
**Worksheet for Trapezoidal Channel**

---

<b>Project Description</b>	
Project File	c:\fm\96382.fm2
Worksheet	Biofiltration 100-year conveyance
Flow Element	Trapezoidal Channel
Method	Manning's Formula
Solve For	Channel Depth

---

---

<b>Input Data</b>	
Mannings Coefficient	0.200
Channel Slope	0.010000 ft/ft
Left Side Slope	3.00 H : V
Right Side Slope	3.00 H : V
Bottom Width	8.00 ft
Discharge	5.21 ft <sup>3</sup> /s

---

---

<b>Results</b>	
Depth	0.86 ft
Flow Area	9.10 ft <sup>2</sup>
Wetted Perimeter	13.44 ft
Top Width	13.16 ft
Critical Depth	0.23 ft
Critical Slope	0.987059 ft/ft
Velocity	0.57 ft/s
Velocity Head	0.01 ft
Specific Energy	0.86 ft
Froude Number	0.12
Flow is subcritical.	

---

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 (206) 448-4200 • FAX (206) 728-5608  
 E-MAIL: karwar@nwlink.com

CLIENT PORT OF SEATTLE

PROJECT S. 154<sup>th</sup> St. / 156<sup>th</sup> Way

CONTACT \_\_\_\_\_

PHONE ( ) \_\_\_\_\_

By KRS

Date 7/16/98

Chkd \_\_\_\_\_

Date \_\_\_\_\_

sheet \_\_\_\_\_ or \_\_\_\_\_

96-382

Job No.

BIOSWALE #4

PERIM. RD. 140+00 to E. END

Impervious area:

$$20' (147+00 - 140+00) = 14,000 \text{ S.F.} \rightarrow 0.32 \text{ AC}$$

Pervious area

$$[\text{Basin E1}] - [\text{Area downstream of bioswale}] - [\text{Impervious area}]$$

$$[1.54 \text{ AC}] - (25' \times 236') - [0.32 \text{ AC}] = 1.08 \text{ AC (till grass)}$$

2 & 100-yr peak runoff (KCRTS)

flow paths

Impervious - 20' @ 2%

Pervious - 130' @ 16%

$$Q_2 = 0.27 \text{ cfs}$$

$$Q_{wa} = 0.6(Q_2) = 0.16 \text{ cfs}$$

$$Q_{100} = 1.02 \text{ cfs}$$

Swale geometry options:

SLOPE	WIDTH	DEPTH	VELOCITY	LENGTH
4%	3'	0.13'	0.36 fps	194'
4.5%	3'	0.13'	0.37 fps	200' ← selection
6%	3'	0.12'	0.41 fps	221'

Swale design:

Bottom width = 3'

Bottom slope = 4.5%

\*NOTE - Since the adjacent road slope is 6.5%, 1'-high check dams will be placed every 50' to reduce slope to 4.5%.

Length = 200'

\*NOTE - The design criteria for "continuous inflow bioswale" was not used because the majority of runoff enters at the end, and the road receives little travel.

100-yr conveyance check

$$d_{100} = 0.38' \text{ } \checkmark \text{OK}$$

$$V_{100} = 0.69 \text{ fps } \checkmark \text{OK}$$

BIOSWALE #4

Flow Frequency Analysis  
Time Series File:pos-bs4.tsf  
Project Location:Sea-Tac

---Annual Peak Flow Rates---  
Flow Rate Rank Time of Peak  
(CFS)  
0.301 4 2/ 9/ 1 12:30  
0.235 7 1/ 6/ 2 1:00  
0.666 2 12/ 8/ 2 17:15  
0.183 8 8/26/ 4 0:45  
0.621 3 11/17/ 4 5:00  
0.266 6 10/27/ 5 10:45  
0.281 5 11/24/ 6 1:00  
1.02 1 1/ 9/ 8 6:30  
Computed Peaks

-----Flow Frequency Analysis-----  
- - Peaks - - Rank Return Prob  
(CFS) Period  
1.02 1 100.00 0.990  
0.666 2 25.00 0.960  
0.621 3 10.00 0.900  
0.301 4 5.00 0.800  
0.281 5 3.00 0.667  
0.266 6 2.00 0.500  
0.235 7 1.30 0.231  
0.183 8 1.10 0.091  
0.899 50.00 0.980



**Biofiltration Swale #4**  
**Worksheet for Trapezoidal Channel**

---

<b>Project Description</b>	
Project File	c:\-fm\96382.fm2
Worksheet	Biofiltration sizing
Flow Element	Trapezoidal Channel
Method	Manning's Formula
Solve For	Channel Depth

---

---

<b>Input Data</b>		
Mannings Coefficient	0.200	
Channel Slope	0.045000 ft/ft	
Left Side Slope	3.00	H : V
Right Side Slope	3.00	H : V
Bottom Width	3.00	ft
Discharge	0.16	ft <sup>3</sup> /s

---

---

<b>Results</b>		
Depth	0.13	ft
Flow Area	0.43	ft <sup>2</sup>
Wetted Perimeter	3.81	ft
Top Width	3.77	ft
Critical Depth	0.04	ft
Critical Slope	1.684690 ft/ft	
Velocity	0.37	ft/s
Velocity Head	0.21e-2	ft
Specific Energy	0.13	ft
Froude Number	0.19	
Flow is subcritical.		

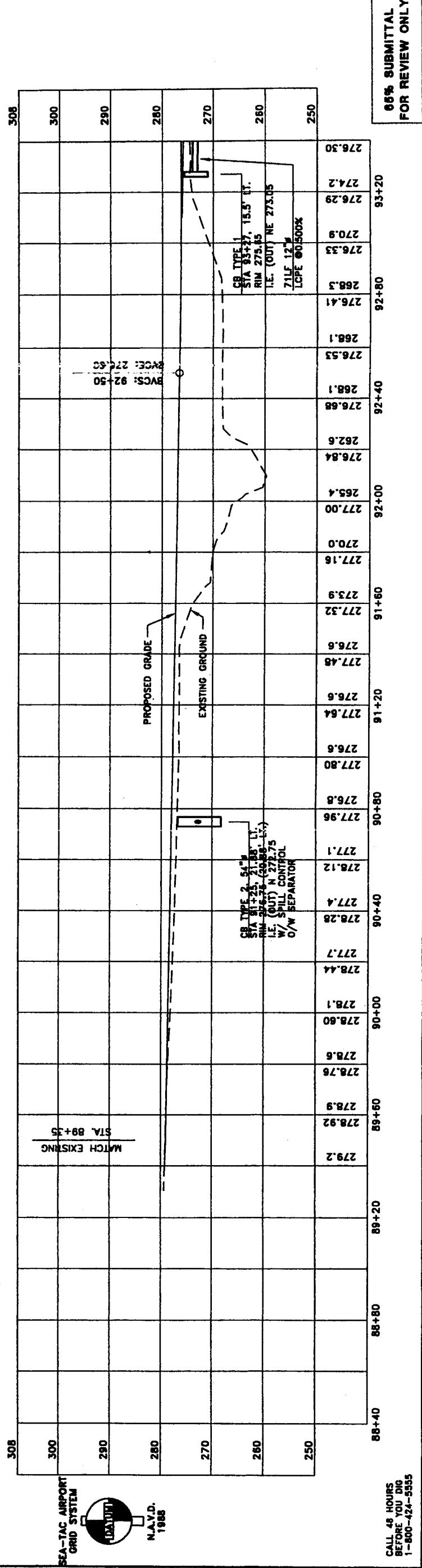
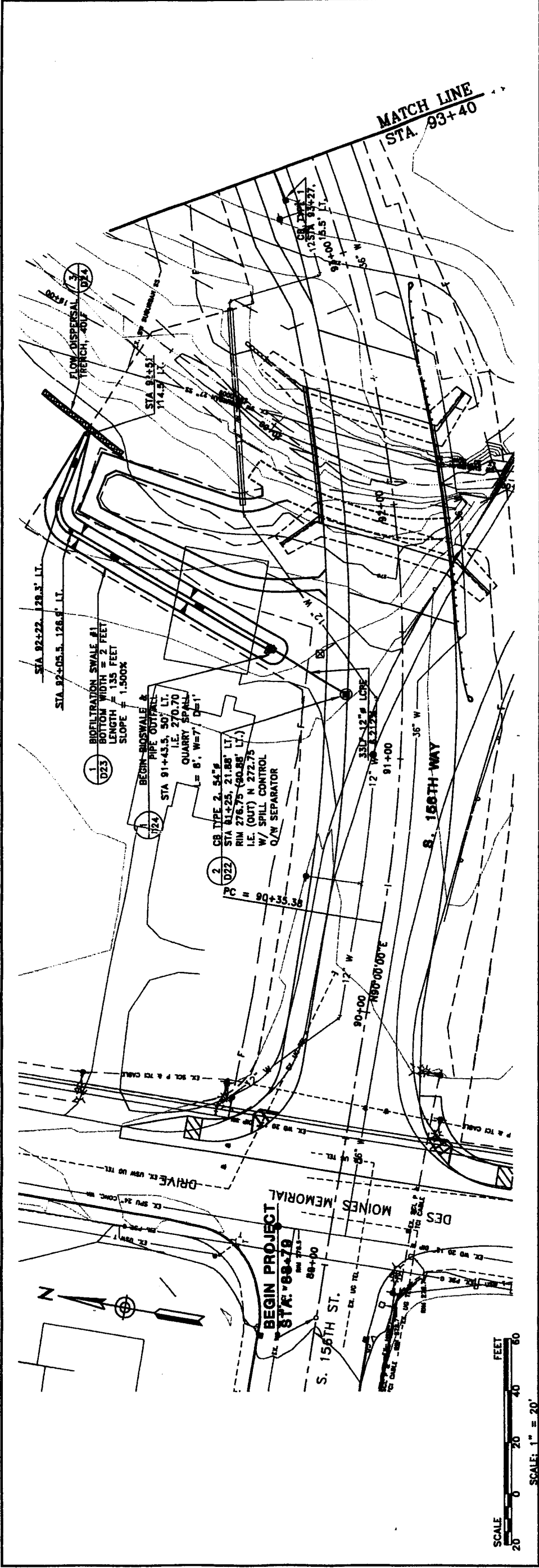
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**Biofiltration Swale #4**  
**Worksheet for Trapezoidal Channel**

<b>Project Description</b>	
Project File	c:\-fm\96382.fm2
Worksheet	Biofiltration 100-year conveyance
Flow Element	Trapezoidal Channel
Method	Manning's Formula
Solve For	Channel Depth

<b>Input Data</b>		
Mannings Coefficient	0.200	
Channel Slope	0.045000 ft/ft	
Left Side Slope	3.00	H : V
Right Side Slope	3.00	H : V
Bottom Width	3.00	ft
Discharge	1.02	ft <sup>3</sup> /s

<b>Results</b>		
Depth	0.37	ft
Flow Area	1.50	ft <sup>2</sup>
Wetted Perimeter	5.32	ft
Top Width	5.20	ft
Critical Depth	0.15	ft
Critical Slope	1.171746 ft/ft	
Velocity	0.68	ft/s
Velocity Head	0.01	ft
Specific Energy	0.37	ft
Froude Number	0.22	
Flow is subcritical.		



**KATOG WARREN INCORPORATED**  
2003 Western Avenue  
555 Market Place One  
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(206) 448-4200

**PROJECT:** SEA-TAC INTERNATIONAL AIRPORT  
SOUTH 156TH STREET/156TH WAY/  
MILLER CREEK RELOCATION  
**STORM DRAINAGE PLAN & PROFILE**  
STA. 88+79 TO STA. 93+40

**DATE:** 11/20/00  
**SCALE:** 1" = 20'  
**PROJECT NO.:** 96-382  
**DRAWN BY:** STA-9806-D1

**REVISIONS**

NO.	DATE	BY	DESCRIPTION

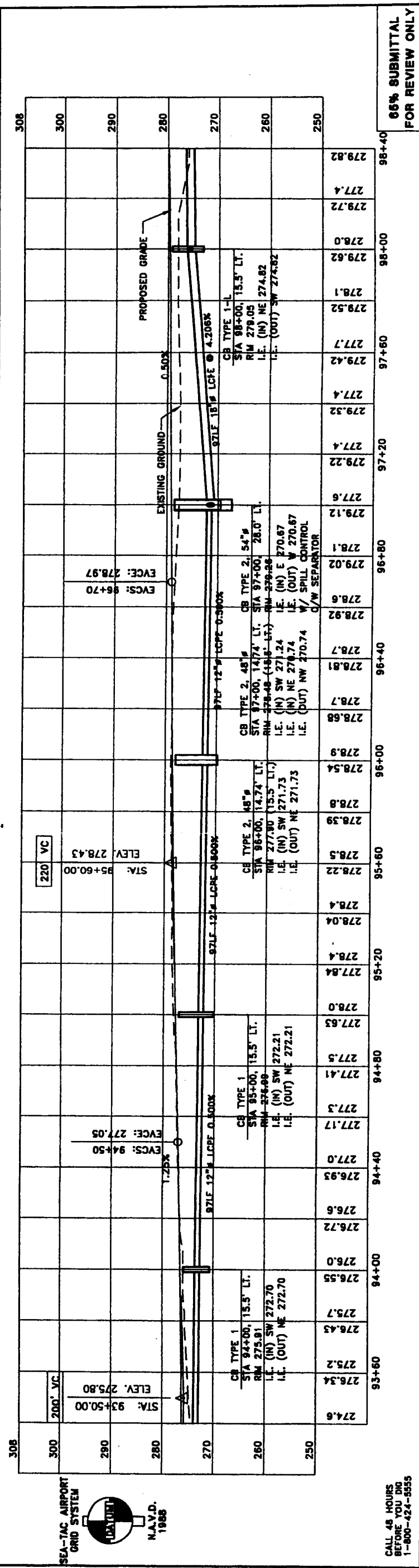
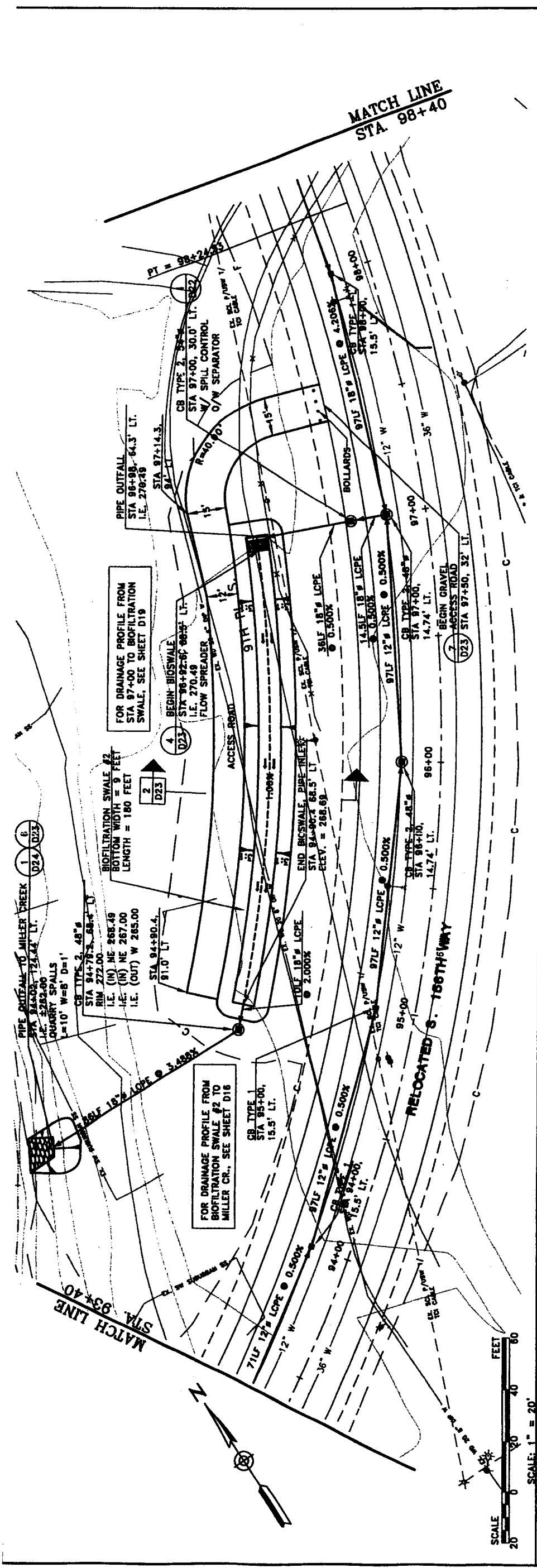
**PROJECT ENGINEER:** \_\_\_\_\_  
**CHECKED BY:** \_\_\_\_\_  
**DESIGNED BY:** \_\_\_\_\_  
**DATE:** \_\_\_\_\_  
**PROJECT NO.:** \_\_\_\_\_

**APP'S**

NO.	DATE	BY	DESCRIPTION

**BASEMAP\CONTOUR1.DWG**  
**\1997PRES\MODBASE.DWG**  
**\PLANSET\PH1PH2.DWG**  
**\UTIL\UTILITY.DWG**

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**SEA-TAC AIRPORT GRID SYSTEM**  
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 (206) 448-4200

Part of Sheets  
**SEA-TAC INTERNATIONAL AIRPORT**  
 SOUTH 154TH STREET/186TH WAY/  
 MILLER CREEK RELOCATION  
**STORM DRAINAGE PLAN & PROFILE**  
 STA. 93+40 TO STA. 98+40

**66% SUBMITTAL FOR REVIEW ONLY**

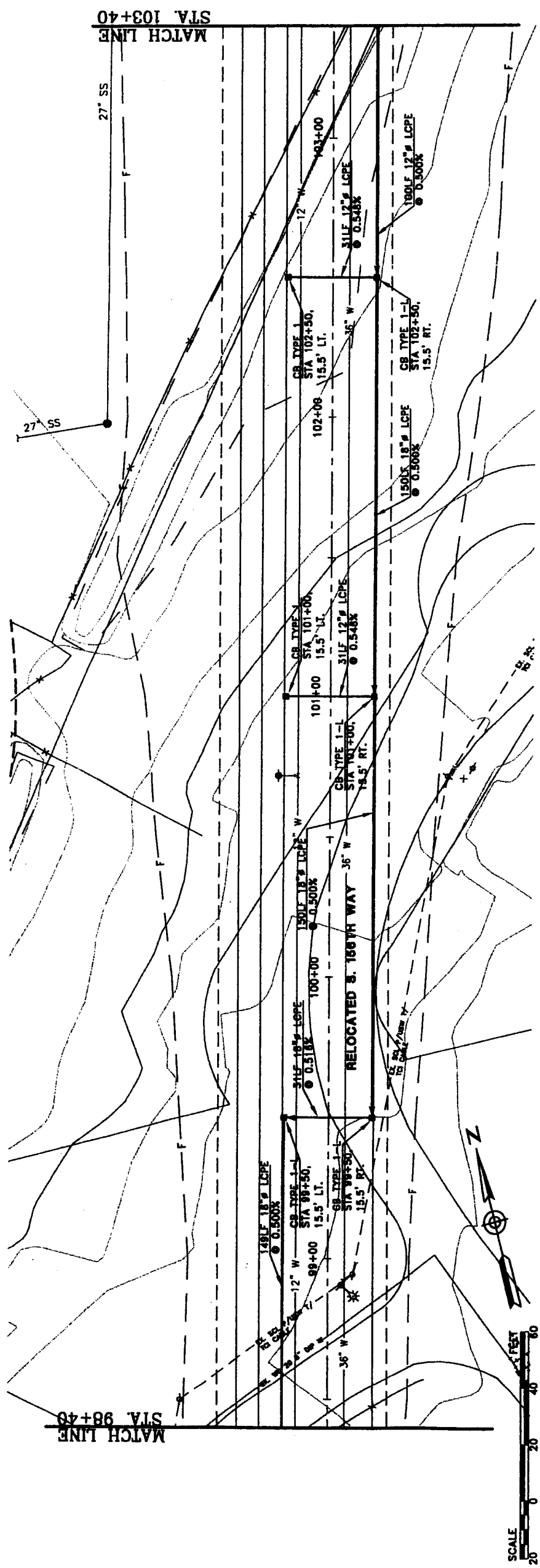
NO.	DATE	BY	DESCRIPTION	REVISIONS

PROJECT NO. C-3496  
 CONTRACTOR'S NO. 98-382  
 SHEET NO. 98-382  
 SHEET OF 98-382

PROJECT ENGINEER: [Signature]  
 CHECKED BY: [Signature]  
 DATE: [Date]  
 DRAWN BY: [Signature]  
 APPROVED BY: [Signature]

\1997\area\MOBASE.DWG  
 \planner\J\_PH1PH2.DWG  
 \base\map\CONTOUR1.DWG

STA. 93+50.00 ELEV. 275.80  
 STA. 94+50 EVCES: 277.05  
 STA. 95+60.00 ELEV. 278.43  
 STA. 96+70 EVCES: 278.97



SCALE: 1" = 20'



Station	Proposed Grade	Existing Ground	Notes
98+40	276.0	276.0	
98+42	276.6	276.6	
98+44	276.8	276.8	
98+46	276.9	276.9	
98+48	276.9	276.9	
98+50	275.9	275.9	
98+52	275.5	275.5	
98+54	274.9	274.9	
98+56	274.7	274.7	
98+58	274.0	274.0	
98+60	273.8	273.8	
98+62	273.5	273.5	
98+64	272.8	272.8	
98+66	271.0	271.0	
98+68	267.9	267.9	
98+70	267.0	267.0	
98+72	266.4	266.4	
98+74	265.9	265.9	
98+76	264.7	264.7	
98+78	262.02	262.02	
98+80	264.2	264.2	
98+82	262.12	262.12	
98+84	262.22	262.22	
98+86	263.3	263.3	
98+88	262.52	262.52	

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NO.	DATE	BY	DESCRIPTION

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NO.	DATE	BY	DESCRIPTION

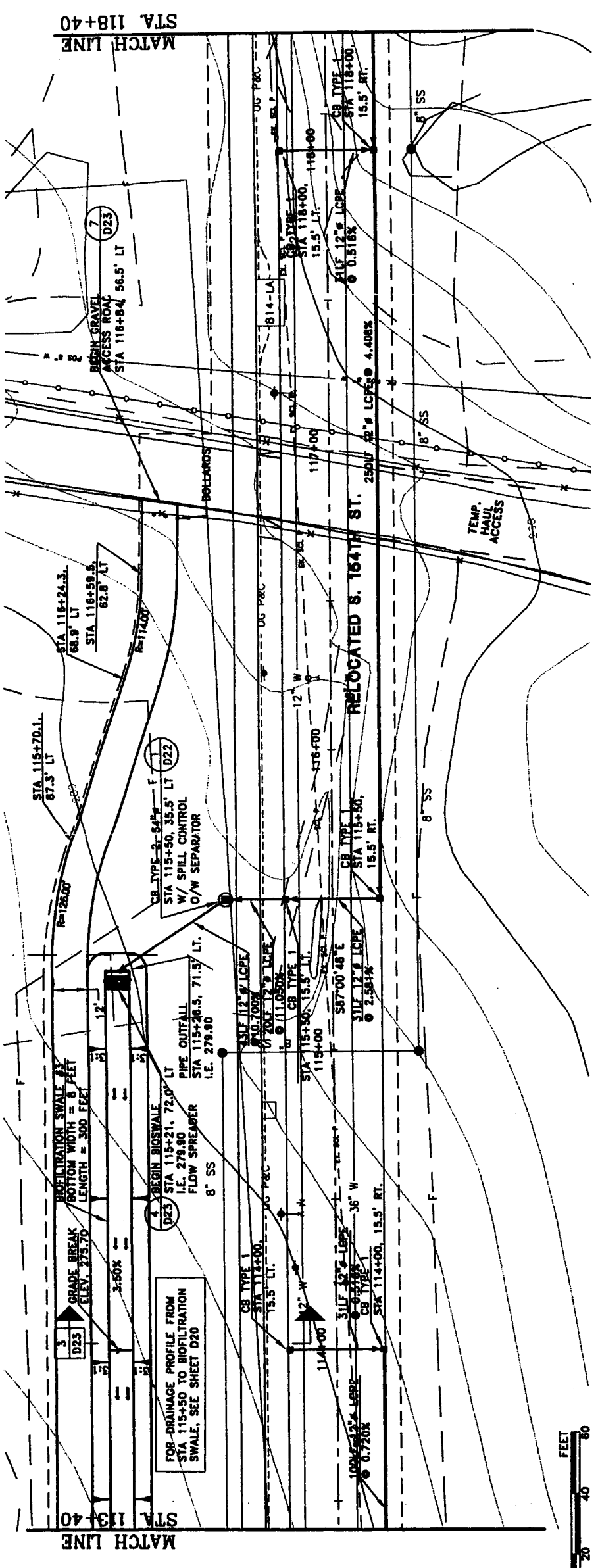
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**Part of Seattle**  
**SEA-TAC INTERNATIONAL AIRPORT**  
SOUTH 164TH STREET/166TH WAY/  
MILLER CREEK RELOCATION  
**STORM DRAINAGE PLAN & PROFILE**  
STA. 98+40 TO STA. 103+40

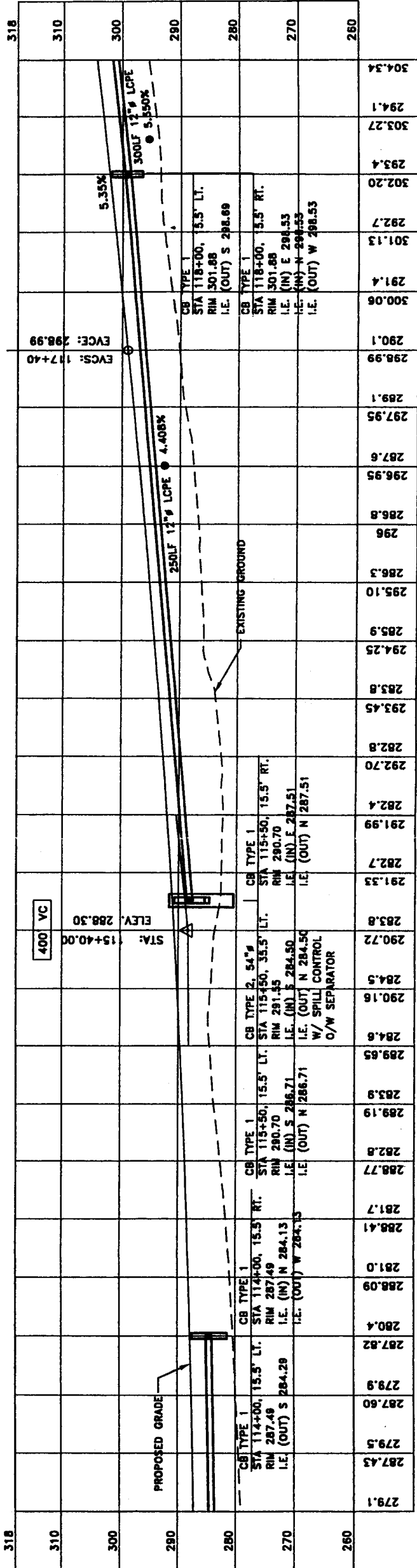
PROJECT NO. C3496  
COMMITTEE'S NO. 96-382  
SHEET NO. STA-9806-D3







SCALE: 1" = 20'



STATION	ELEVATION	DESCRIPTION
113+40	279.1	PROPOSED GRADE
113+40	287.43	CB TYPE 1 STA 114+00, 15.5' LT. RIM 287.49 I.E. (IN) N 284.13 I.E. (OUT) S 284.29
114+00	287.82	CB TYPE 1 STA 114+00, 15.5' LT. RIM 287.49 I.E. (IN) N 284.13 I.E. (OUT) S 284.29
114+00	288.09	CB TYPE 1 STA 114+00, 15.5' LT. RIM 287.49 I.E. (IN) N 284.13 I.E. (OUT) S 284.29
114+00	288.41	CB TYPE 1 STA 114+00, 15.5' LT. RIM 287.49 I.E. (IN) N 284.13 I.E. (OUT) S 284.29
114+40	288.77	CB TYPE 1 STA 115+50, 15.5' LT. RIM 290.70 I.E. (IN) S 286.71 I.E. (OUT) N 286.71
114+80	289.19	CB TYPE 1 STA 115+50, 15.5' LT. RIM 290.70 I.E. (IN) S 286.71 I.E. (OUT) N 286.71
115+20	289.65	CB TYPE 2, 54" STA 115+50, 35.5' LT. RIM 291.35 I.E. (IN) S 284.50 I.E. (OUT) N 284.50
115+50	290.16	CB TYPE 2, 54" STA 115+50, 35.5' LT. RIM 291.35 I.E. (IN) S 284.50 I.E. (OUT) N 284.50
115+50	290.72	CB TYPE 2, 54" STA 115+50, 35.5' LT. RIM 291.35 I.E. (IN) S 284.50 I.E. (OUT) N 284.50
115+50	291.53	CB TYPE 1 STA 115+50, 15.5' RT. RIM 290.70 I.E. (IN) E 287.51 I.E. (OUT) N 287.51
115+80	291.99	CB TYPE 1 STA 115+50, 15.5' RT. RIM 290.70 I.E. (IN) E 287.51 I.E. (OUT) N 287.51
116+00	292.70	CB TYPE 1 STA 115+50, 15.5' RT. RIM 290.70 I.E. (IN) E 287.51 I.E. (OUT) N 287.51
116+00	292.8	EXISTING GROUND
116+00	293.45	EXISTING GROUND
116+40	294.25	EXISTING GROUND
116+40	295.10	EXISTING GROUND
116+80	296.8	EXISTING GROUND
117+20	297.95	EXISTING GROUND
117+20	299.1	EXISTING GROUND
117+20	299.99	EXISTING GROUND
117+20	298.99	EXISTING GROUND
117+60	298.1	EXISTING GROUND
117+60	299.06	EXISTING GROUND
117+60	299.1	EXISTING GROUND
117+60	299.27	EXISTING GROUND
117+60	299.4	EXISTING GROUND
117+60	299.77	EXISTING GROUND
117+60	300.06	EXISTING GROUND
118+00	300.20	EXISTING GROUND
118+00	300.34	EXISTING GROUND
118+00	300.4	EXISTING GROUND
118+00	300.54	EXISTING GROUND

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 2003 Western Avenue  
 555 Market Place One  
 Seattle, WA 98121  
 (206) 448-4200

DATE: 7/23/00  
 TIME: 11:25  
 USER: J.PH2  
 PROJECT: 96382

NO.	DATE	BY	DESCRIPTION

PROJECT: 96382  
 SHEET NO.: 06  
 SHEET TITLE: STORM DRAINAGE PLAN & PROFILE STA. 113+40 TO STA. 118+40

Part of Seattle SEA-TAC INTERNATIONAL AIRPORT SOUTH 164TH STREET/196TH WAY/ MILLER CREEK RELOCATION STORM DRAINAGE PLAN & PROFILE STA. 113+40 TO STA. 118+40

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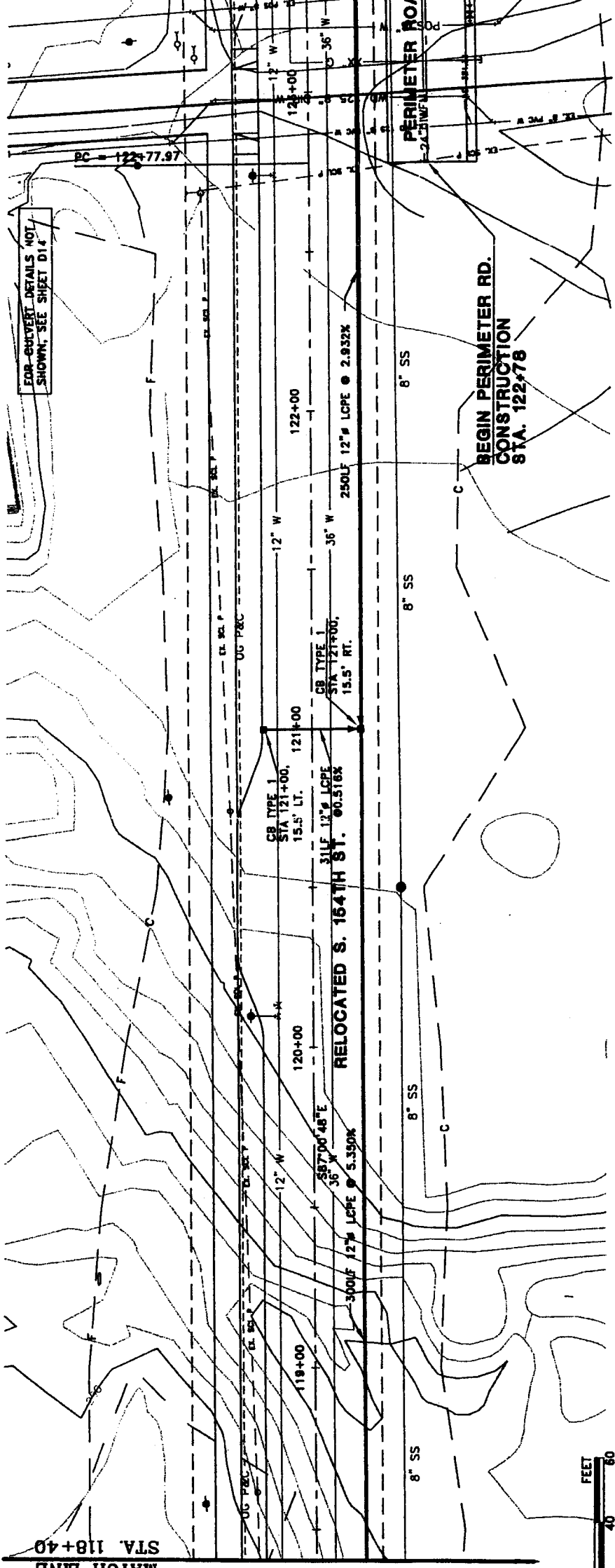
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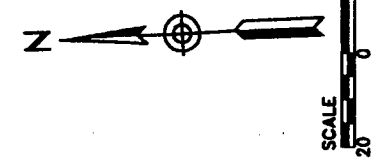
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MATCH LINE STA. 118+40



MATCH LINE STA. 123+40



SCALE: 1" = 20'

Station	Elevation	Notes
118+40	295.4	
118+40	297.3	
118+40	306.48	
118+80	299.6	
118+80	307.55	
118+80	308.62	
119+20	297.5	
119+20	308.69	
119+20	304.2	
119+60	310.76	
119+60	307.7	
119+60	311.63	
120+00	310.8	
120+00	312.90	
120+00	311.8	
120+00	313.97	
120+40	315.04	
120+40	312.6	
120+40	316.11	
120+80	315.0	
120+80	317.18	
120+80	315.4	
120+80	318.25	
121+00	315.5	
121+00	318.25	
121+00	315.5	
121+20	319.29	
121+20	315.8	
121+20	320.30	
121+60	315.7	
121+60	321.26	
121+60	315.9	
121+60	322.18	
122+00	316.0	
122+00	323.05	
122+00	316.8	
122+00	323.88	
122+40	317.5	
122+40	324.67	
122+40	317.8	
122+40	325.41	
122+40	318.7	
122+40	326.11	
122+80	319.2	
122+80	326.76	
122+80	319.2	
123+20	327.58	
123+20	318.1	
123+20	327.95	

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 2003 Western Avenue  
 555 Market Place One  
 Seattle, WA 98121  
 (206) 448-4200

DATE: 11/23/00  
 TIME: 10:00 AM  
 DRAWN BY: JAW  
 CHECKED BY: JAW

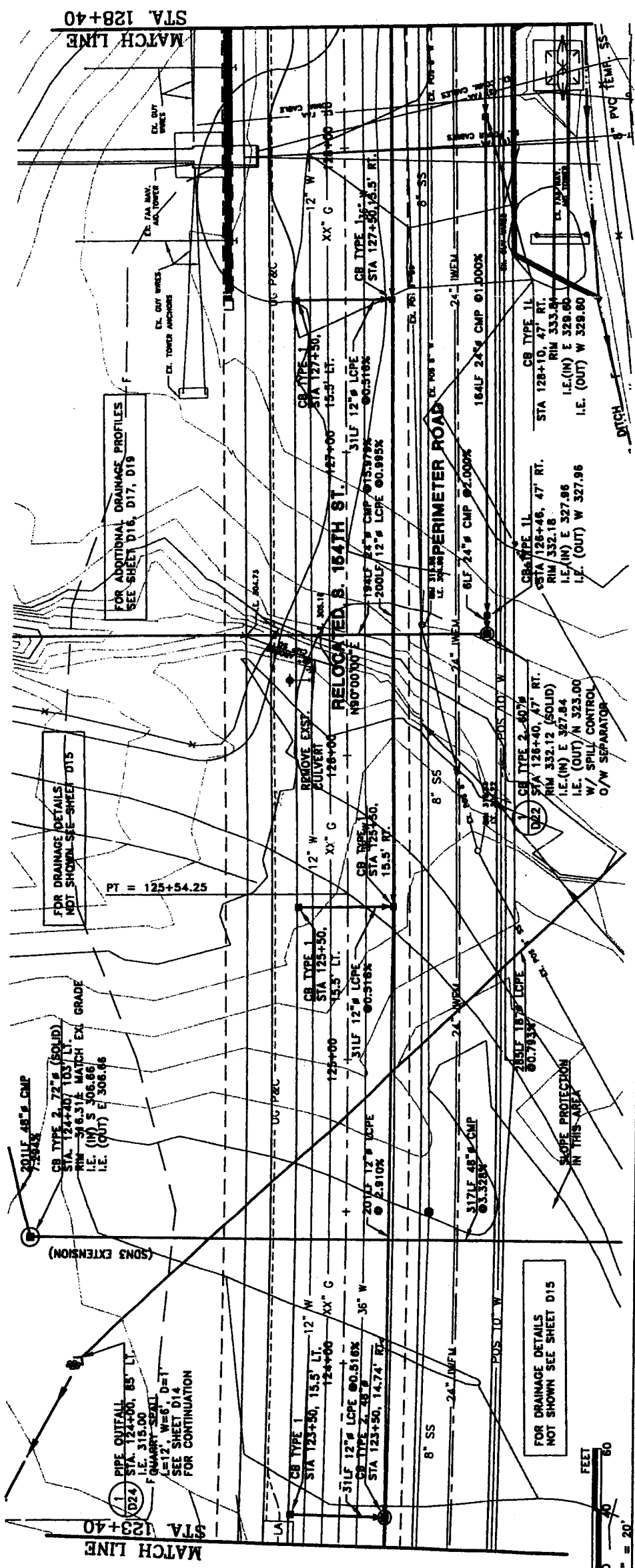
NO.	DATE	BY	DESCRIPTION

NO.	DATE	BY	DESCRIPTION

PROJECT NO.	DATE	BY	DESCRIPTION

**Part of Seattle SEA-TAC INTERNATIONAL AIRPORT**  
 SOUTH 164TH STREET/166TH WAY/  
 MILLER CREEK RELOCATION  
**STORM DRAINAGE PLAN & PROFILE**  
 STA. 118+40 TO STA. 123+40

PROJECT NO. C3498  
 COMMUNITY NO. 96-382  
 PART OF MAPLE ST. STA. 9806-07



Station	Proposed Grade	Existing Ground
123+60	319.2	319.2
123+70	320.7	320.7
123+80	321.8	321.8
123+90	329.59	329.59
124+00	320.7	320.7
124+10	329.79	329.79
124+20	320.5	320.5
124+30	320.14	320.14
124+40	322.0	322.0
124+50	350.45	350.45
124+60	319.9	319.9
124+70	350.71	350.71
124+80	317.8	317.8
124+90	350.93	350.93
125+00	315.7	315.7
125+10	351.13	351.13
125+20	314.3	314.3
125+30	331.33	331.33
125+40	313.2	313.2
125+50	331.73	331.73
125+60	311.7	311.7
125+70	351.93	351.93
125+80	311.4	311.4
125+90	332.13	332.13
126+00	307.8	307.8
126+10	332.53	332.53
126+20	311.8	311.8
126+30	332.55	332.55
126+40	315.0	315.0
126+50	332.72	332.72
126+60	317.0	317.0
126+70	332.92	332.92
126+80	318.6	318.6
126+90	333.12	333.12
127+00	319.6	319.6
127+10	333.52	333.52
127+20	320.0	320.0
127+30	333.52	333.52
127+40	320.3	320.3
127+50	333.72	333.72
127+60	320.1	320.1
127+70	333.92	333.92
127+80	320.3	320.3
127+90	334.12	334.12
128+00	320.0	320.0
128+10	334.32	334.32
128+20	320.0	320.0
128+30	334.52	334.52
128+40	320.0	320.0

SEA-TAC AIRPORT GRID SYSTEM  
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**KATOS WARREN INCORPORATED**  
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(206)448-4200

Port of Seattle  
SEA-TAC INTERNATIONAL AIRPORT  
SOUTH 164TH STREET/168TH WAY/  
MILLER CREEK RELOCATION  
**STORM DRAINAGE PLAN & PROFILE**  
STA. 123+40 TO STA. 128+40

PROJECT NO. C3486  
SHEET NO. 96-382  
DATE: 08/20/00  
DRAWN BY: J. W. HARRIS  
CHECKED BY: J. W. HARRIS

DATE: 08/20/00  
SCALE: 1"=20'  
PROJECT NO. C3486  
SHEET NO. 96-382  
DATE: 08/20/00  
DRAWN BY: J. W. HARRIS  
CHECKED BY: J. W. HARRIS

65% SUBMITTAL FOR REVIEW ONLY

1897area\MODBASE.DWG  
1897area\PHI1PH2.DWG  
1897area\CONTOUR1.DWG

H-15









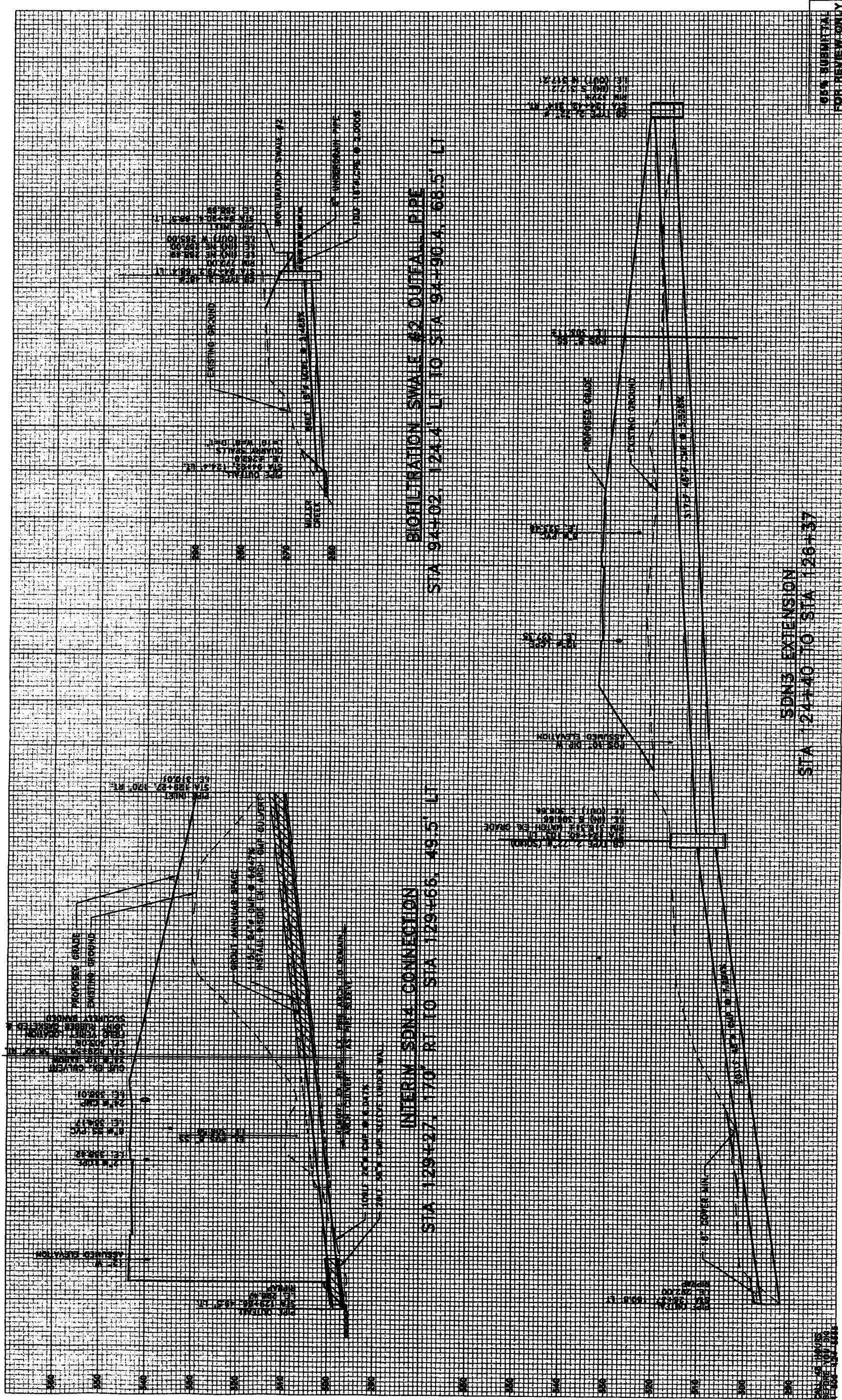












DATE: 05/14/03  
 DRAWN BY: JWG  
 CHECKED BY: JWG  
 1-800-393-1288

Part of Seattle  
**SEA-TAC INTERNATIONAL AIRPORT**  
 SOUTH 164TH STREET/168TH WAY/  
 MILLER CREEK RELOCATION  
**DRAINAGE PROFILES**

PROJECT NUMBER: C3498  
 COMMUNITY NO.: 96-382  
 SHEET TITLE: STA-9806-D16

DATE: 05/14/03  
 DRAWN BY: JWG  
 CHECKED BY: JWG  
 1-800-393-1288

REVISIONS		DATE	BY	DESCRIPTION

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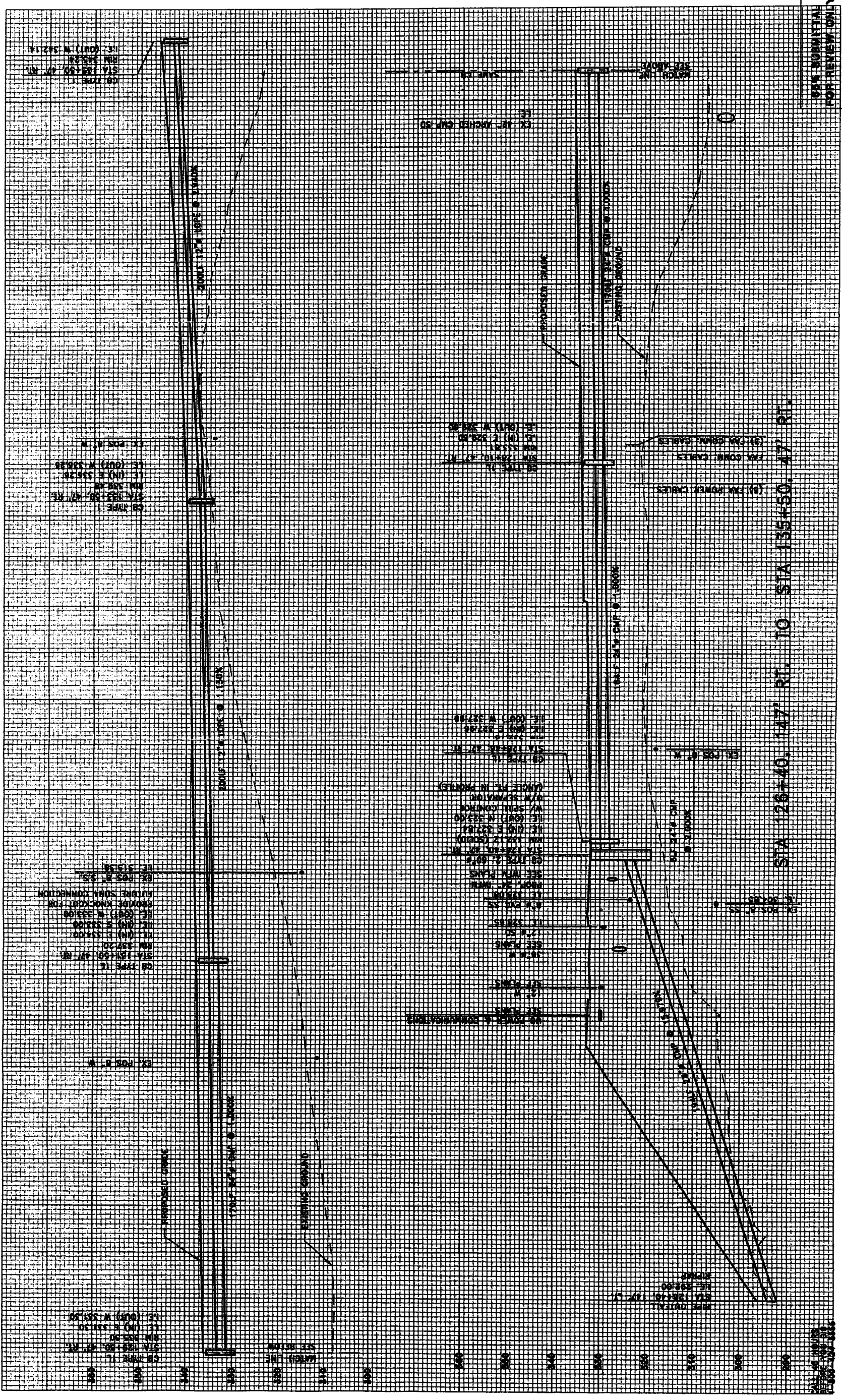
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 WARREN  
 INCORPORATED**

XREFS:

H-23

AR 011168





SEA-T/C INTERNATIONAL AIRPORT  
 PERIMETER ROAD  
 DRAINAGE PROFILES  
 STA 125+00 TO STA 150+00

DATE: 10/20/00  
 DRAWN BY: J. W. WILSON  
 CHECKED BY: J. W. WILSON  
 PROJECT NO: 96-382  
 SHEET NO: 10 OF 10  
 STA: 9606-D17

Part of Seattle  
**SEA-T/C INTERNATIONAL AIRPORT**  
 SOUTH 164TH STREET/169TH WAY/  
 MILLER CREEK RELOCATION  
**DRAINAGE PROFILES**  
 PERIMETER ROAD

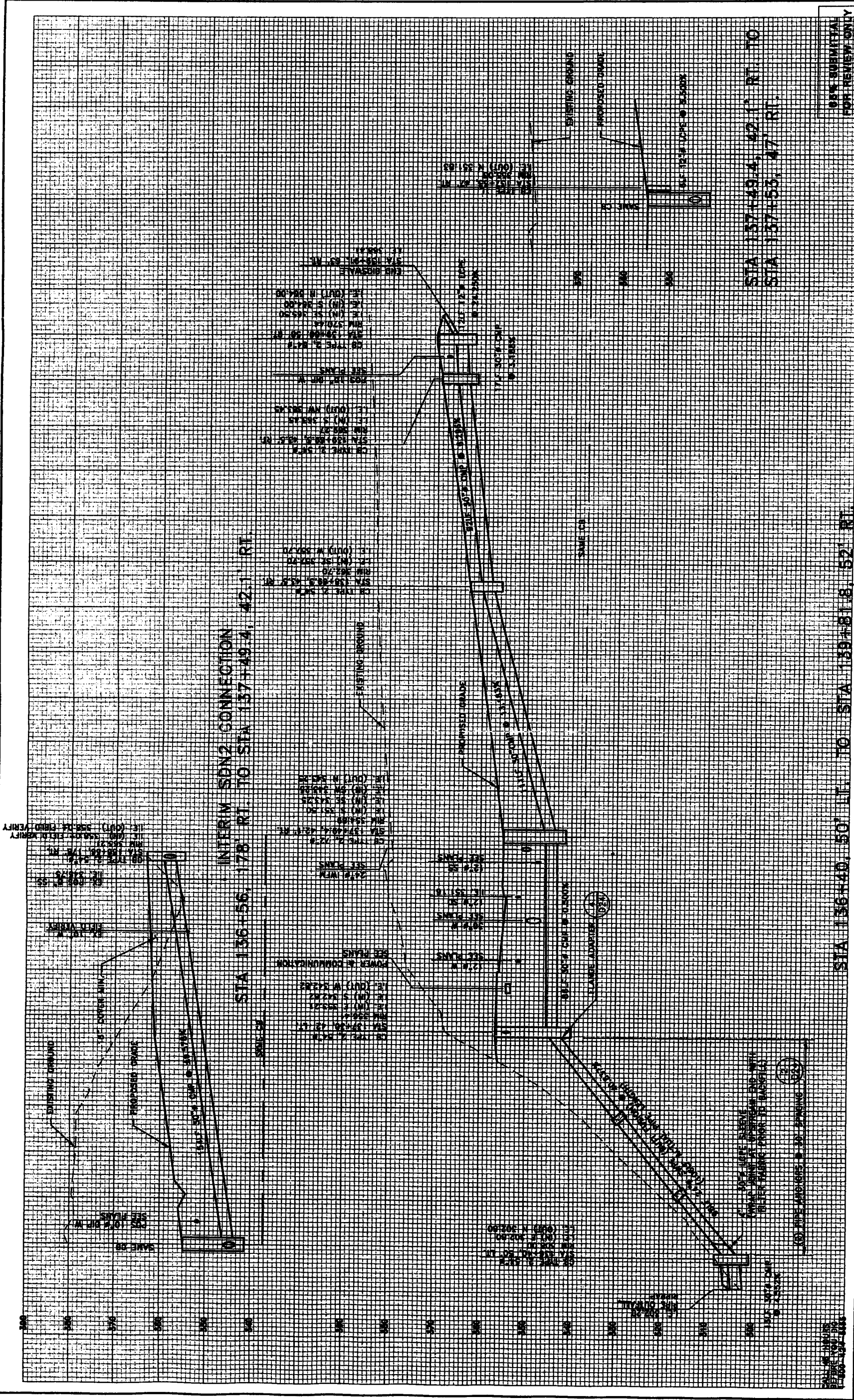
NO.	DATE	BY	DESCRIPTION

NO.	DATE	BY	DESCRIPTION

DATE: 10/20/00  
 SCALE: 1" = 20' & V. 1" = 10'  
 SHEET: 10 OF 10  
 PROJECT: 96-382  
 STA: 9606-D17

**KATOG WARREN**  
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 2003 Western Avenue  
 560 Market Place One  
 Seattle, WA 98121  
 (206) 448-4200





INTERIM SDN2 CONNECTION  
 STA 136+56, 178, RT. TO STA 137+49.4, 42.1, RT.

STA 137+49.4, 42.1, RT. TO  
 STA 137+53, 47, RT.

STA 136+40, 50, LT. TO STA 139+81.8, 52, RT.

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 FOR REVIEW ONLY

**KATOW WARREN**  
INCORPORATED

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Port of Seattle  
 SEA-TAC INTERNATIONAL AIRPORT  
 SOUTH 164TH STREET/186TH WAY/  
 MILLER CREEK RELOCATION  
 DRAINAGE PROFILES

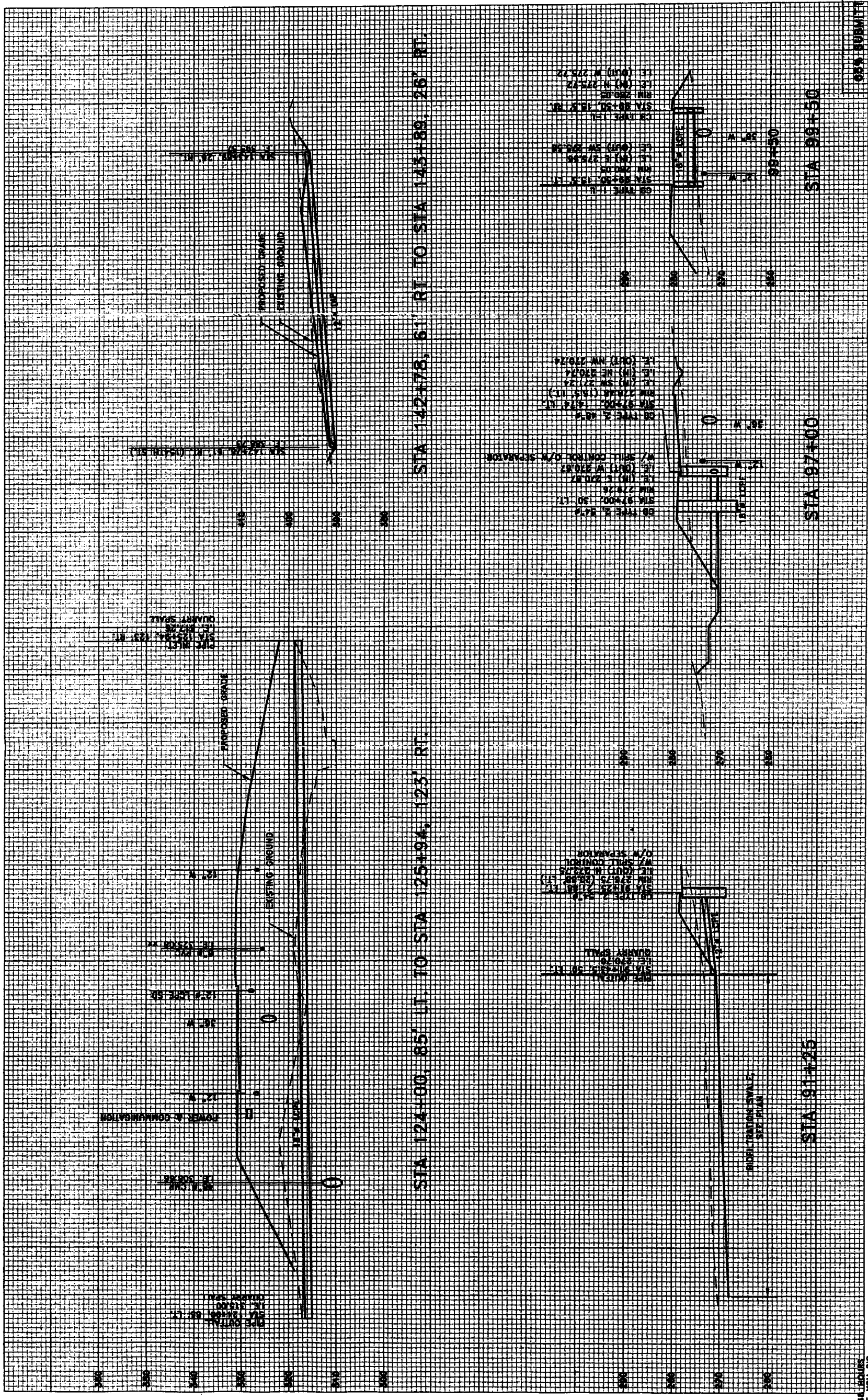
PROJECT NUMBER		DATE		DESCRIPTION		BY		CHECKED	

REVISIONS		DATE		DESCRIPTION	

DATE OF DRAWING: 08/08/05  
 DRAWN BY: J. W. [unreadable]

SCALE: 1" = 20' & V. 1" = 10'  
 DATE: 08/08/05  
 PROJECT NO: 96-382  
 SHEET NO: 018





039 SUBMITTAL  
FOR REVIEW ONLY

PROJECT NUMBER: C-3496  
CONTRACTOR'S NO.: 96-362  
PART OF SHEET NO.: STIA-9806-D19

Part of Seattle  
SEA-TAC INTERNATIONAL AIRPORT  
SOUTH 184TH STREET/188TH WAY/  
MILLER CREEK RELOCATION  
PROJECT TITLE: DRAINAGE PROFILES

PROJECT NUMBER:  
ISSUED BY:  
SCALE:  
DATE:  
DESIGNED BY:  
PROJECT NO.:

NO.	DATE	BY	DESCRIPTION	APP'D	DATE	BY

PROJECT DATA/FILES  
DATE: 12/1/00  
SCALE: H<sub>1</sub> = 1" = 20' & V<sub>1</sub> = 1" = 10'  
DATE: SEP 2000  
PROJECT NO.: STIA-9806-D19

2003 Western Avenue  
555 Market Place One  
Seattle, WA 98121  
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WARREN  
INCORPORATED**

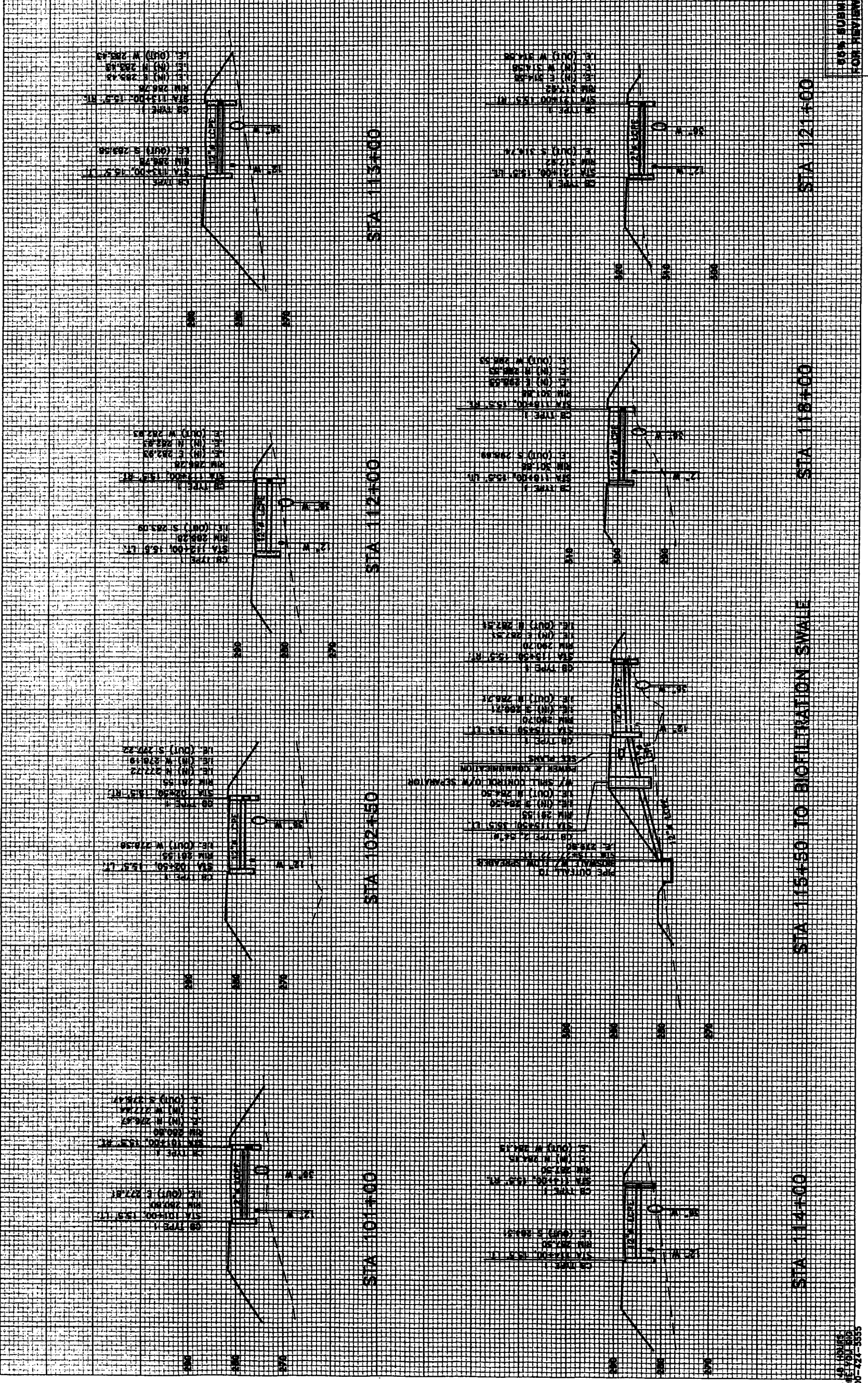
CALL 48 HOURS  
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XREFS:

H-26

AR 011171





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FORM REVIEW ONLY

PROJECT NO. C3496  
CONTRACTOR'S NO. 96-382  
PART OF SHEET NO. STIA-9808-D20

Part of Seattle  
SEA-TAC INTERNATIONAL AIRPORT  
SOUTH 164TH STREET/166TH WAY/  
MILLER CREEK RELOCATION  
DRAINAGE PROFILES

PROJECT NUMBER:  
SHEET NO.  
SCALE:  
DATE:  
DESIGNED BY:  
APPROVED BY:

NO.	DATE	BY	DESCRIPTION	APP'D	DATE	BY	DESCRIPTION

PROJECT TITLE:  
SHEET NO.:

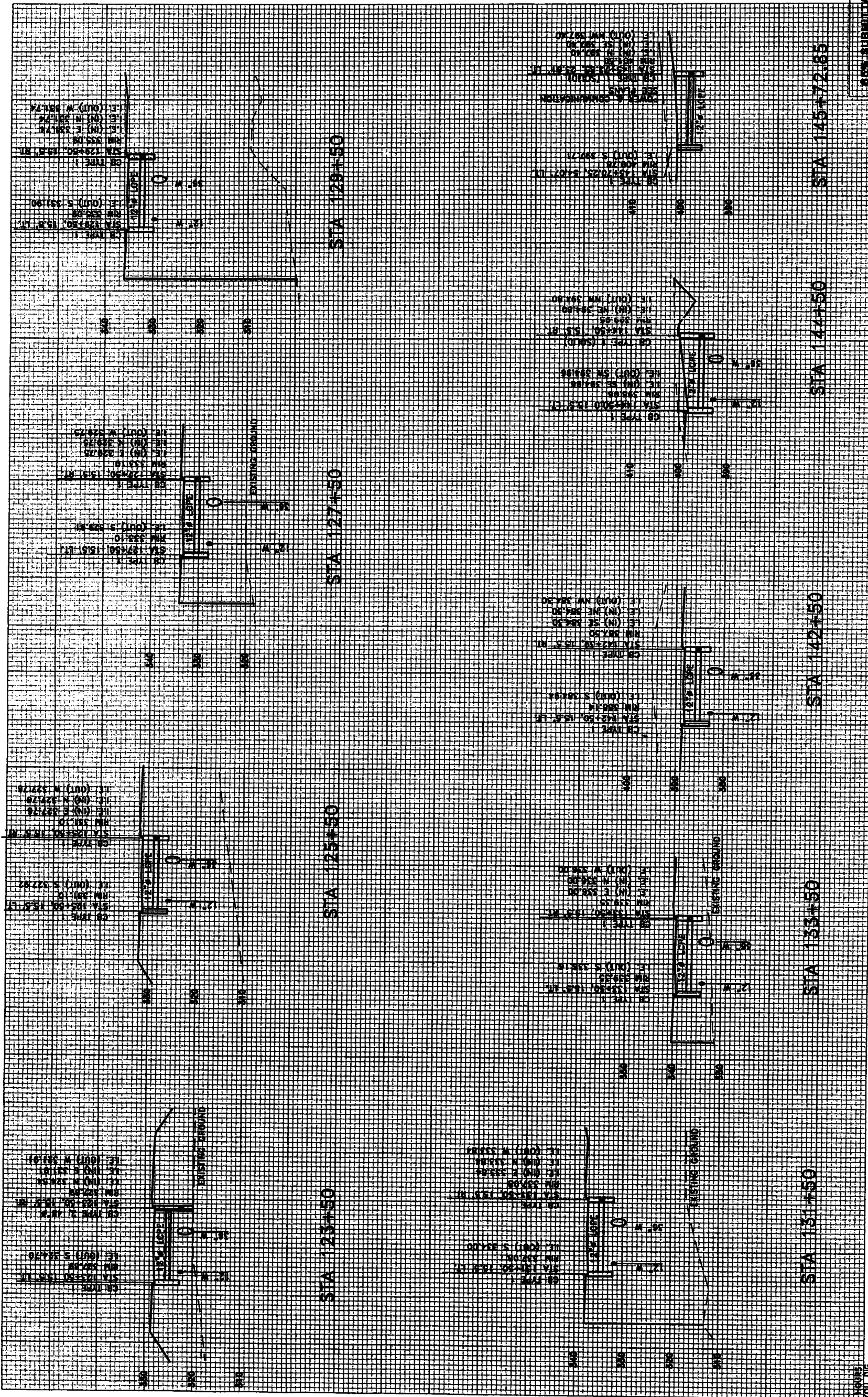
SCALE: HORIZ. = 1" = 20' & V. = 1" = 10'  
DATE: 08/20/00  
DRAWN BY:  
CHECKED BY:

2003 Western Avenue  
555 Market Place One  
Seattle, WA 98121  
(206) 448-4200

KATON  
WARREN  
INCORPORATED

CALL 48 HOURS  
BEFORE PLS END  
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CALL 48 HOURS BEFORE YOU DIG  
1-800-424-5555

60% SUBMITTAL  
FOR REVIEW ONLY



2003 Western Avenue  
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Seattle, WA 98121  
(206) 448-4200

DATE	BY	DESCRIPTION

REVISIONS		
NO.	DATE	DESCRIPTION

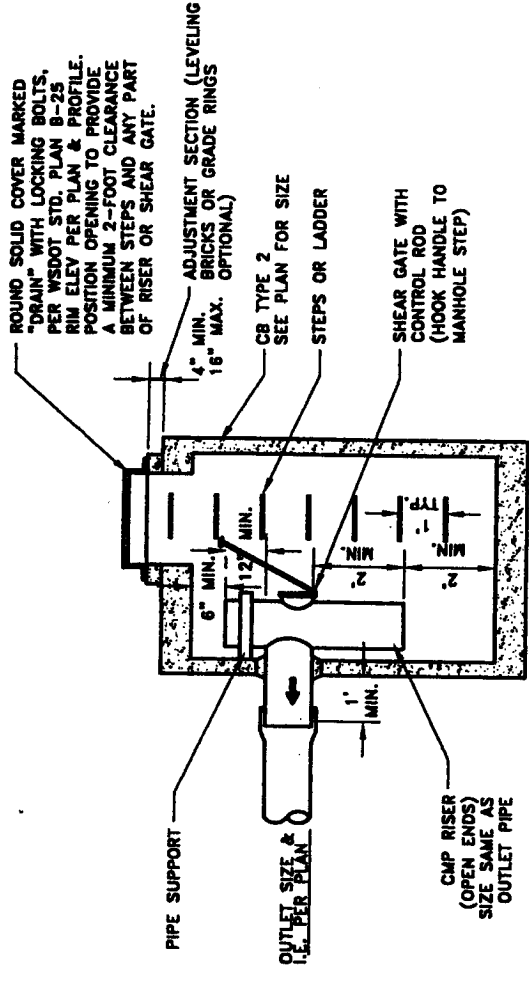
PROJECT NO.	C3496
DATE	96-382
DRAWN BY	
CHECKED BY	

Port of Seattle  
SEA-TAC INTERNATIONAL AIRPORT  
SOUTH 164TH STREET/166TH WAY/  
MILLER CREEK RELOCATION  
PROJECT  
SHEET TITLE: DRAINAGE PROFILES  
SHEET NO. STA1A-8806-021

XREFS:

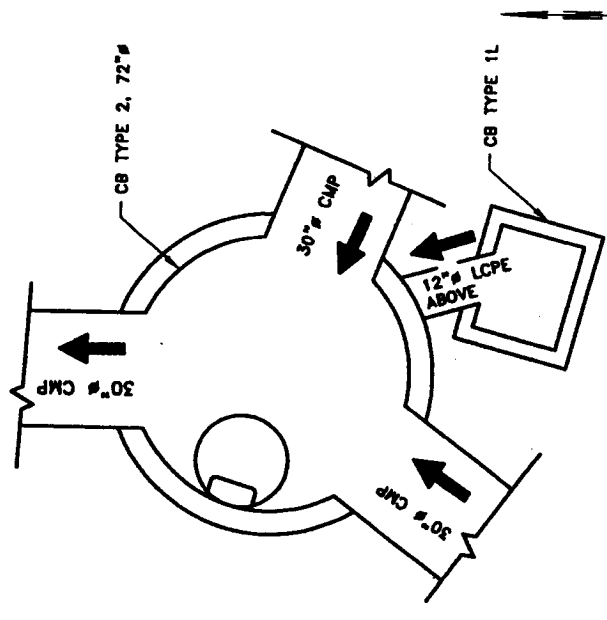
H-28

AR 011173



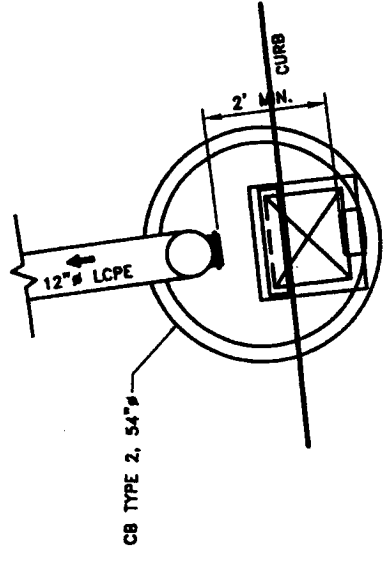
NOTE:  
CONSTRUCT PER WSDOT  
STANDARD PLAN B-3, EXCEPT  
AS NOTED OTHERWISE.

**ELEVATION**  
SPILL CONTROL O/W SEPARATOR  
STA. 97+00, 28.0' LT.  
STA. 115+50, 35.5' LT.  
STA. 128+40, 47' RT.  
SCALE: N.T.S.

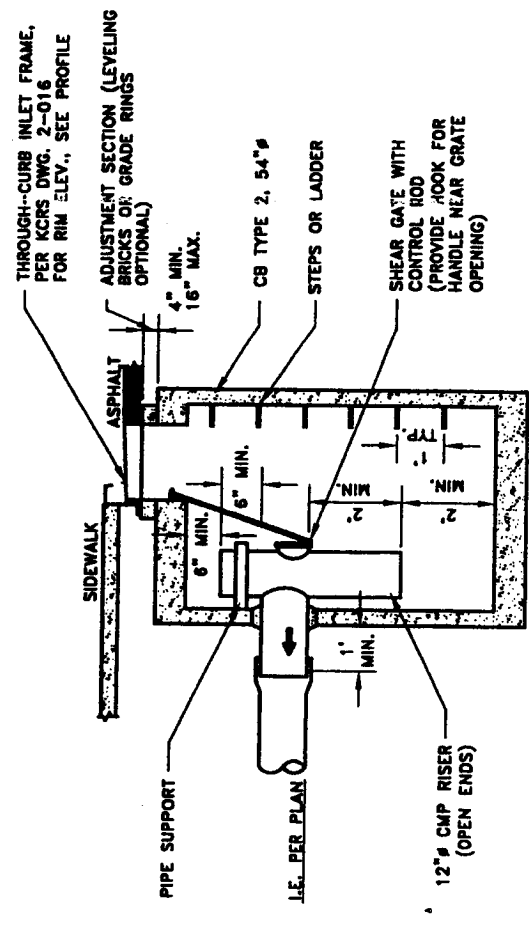


NOTE:  
1. INVERT ELEVATIONS, SEE PLAN

**PLAN**  
CB TYPE 1L & CB TYPE 2, STA 137+50  
SCALE: 1" = 2'



**PLAN**



NOTE:  
CONSTRUCT PER WSDOT  
STANDARD PLAN B-3, EXCEPT  
AS NOTED OTHERWISE.

**ELEVATION**

**PLAN & ELEVATION 2**  
SPILL CONTROL O/W SEPARATOR  
STA. 91+25, 21.68' LT.  
SCALE: N.T.S.

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REVISIONS		DATE	BY	DESCRIPTION

Part of Seattle  
SEA-TAC INTERNATIONAL AIRPORT  
SOUTH 184TH STREET/188TH WAY/  
MILLER CREEK RELOCATION  
DRAINAGE DETAILS

65% SUBMITTAL  
FOR REVIEW ONLY

PROJECT NUMBER: C3486  
CONSULTANT'S NO.: 96-382  
PART OF SHEET NO.: STIA-9805-D22







**APPENDIX M**

**WATER QUALITY BMP COST ESTIMATES FOR AREAS  
DETERMINED TO BE NON-PRACTICABLE FOR RETROFITTING**

**ENGINEER'S ESTIMATE**

<b>Parametrix, Inc.</b>	Job No. 556-2912-001-28	Date 6/8/00	Sheet 1 of 1
Project Title SDE-4 WETVAULT			
Location Seattle-Tacoma International Airport			
Owner Port of Seattle			
Estimated By P. McAvoy		Checked By G. McDonald	Approved By

Item No.	Description	Estimated Quantity	Unit	Unit Price Mat. & Lab.	Estimated Amount
	<b>SDE-4 VAULT</b>				
1	MOBILIZATION (10 %)	LUMP SUM	L.S.	\$405,830	\$405,830
2	SDE-4 VAULT (454'x151'x8')	12,631	Acre Feet	\$250,000	\$3,157,750
3	SAW CUTTING ASPHALT CONC. PAVEMENT	300	L.F.	\$5.00	\$1,500
4	UTILITY ADJUSTMENT (STRUCTURES)	9	EACH	\$5,000.00	\$45,000
5	CONC. STORM DRAIN PIPE	200	L.F.	\$50.00	\$10,000
6	REMOVING CEM. CONC. PAVEMENT	0	C.Y.	\$25.00	\$0
7	REMOVING ASPH. CONC. PAVEMENT	555	S.Y.	\$10.00	\$5,550
8	REMOVE AND REPLACE SOD	1,820	C.Y.	\$10.00	\$18,200
9	STRUCTURE EXCAVATION	55,250	C.Y.	\$10.00	\$552,500
10	GRAVEL BACKFILL	8,000	C.Y.	\$20.00	\$160,000
11	CSBC -ROADWAY	175	TON	\$20.00	\$3,500
12	CSTC -ROADWAY	115	TON	\$20.00	\$2,300
13	APHALT CONC. PVMT -ROADWAY	200	TON	\$45.00	\$9,000
14	LANDSCAPING(SOD)-ROADWAY	18,000	S.F.	\$1.00	\$18,000
15	TRAFFIC CONTROL/DETOURING-ROADWAY	LUMP SUM	L.S.	\$75,000	\$75,000
			<b>Subtotal Items</b>		\$4,464,130
			<b>Total Items</b>		\$4,464,130
			<b>Sales Tax 8.4%</b>		\$374,987
			<b>Subtotal</b>		\$4,839,117
			<b>Contingency (50%)</b>		\$2,419,558
	<b>TOTAL</b>				<b>\$7,258,675</b>

**ENGINEER'S ESTIMATE**

<b>Parametrix, Inc.</b>	Job No. 556-2912-001-28	Date 6/8/00	Sheet 1 of 2
Project Title SDS-3 WETVAULTS			
Location Seattle-Tacoma International Airport			
Owner Port of Seattle			
Estimated By P. McAvoy		Checked By G. McDonald	Approved By

Item No.	Description	Estimated Quantity	Unit	Unit Price Mat. & Lab.	Estimated Amount
<b>SDS-3 VAULT 1</b>					
1	MOBILIZATION (10 %)	LUMP SUM	L.S.	\$108,350	\$108,350
2	SDS-3 VAULT 1 (217'x72'x8')	2.870	Acre Feet	\$250,000	\$717,500
3	SAW CUTTING CEMENT CONC. PAVEMENT	225	L.F.	\$16.00	\$3,600
4	UTILITY ADJUSTMENT (STRUCTURES)	2	EACH	\$5,000.00	\$10,000
5	REMOVING CEM. CONC. PAVEMENT	6300	S.F.	\$12.00	\$75,600
6	CONSTRUCT NEW TAXIWAY (16" TH CONC)	6300	S.F.	\$15.00	\$94,500
7	STRUCTURE EXCAVATION	11,800	C.Y.	\$10.00	\$118,000
8	GRAVEL BACKFILL	2,090	C.Y.	\$20.00	\$41,800
9	LANDSCAPING(SOD)	22,500	S.F.	\$1.00	\$22,500
<b>Subtotal Items</b>					<b>\$1,191,850</b>
<b>SDS-3 VAULT 2</b>					
1	MOBILIZATION (10 %)	LUMP SUM	L.S.	\$28,400	\$28,400
2	SDS-3 VAULT 2 (109'x36'x8')	0.730	Acre Feet	\$250,000	\$182,500
3	SAW CUTTING CEMENT CONC. PAVEMENT	0	L.F.	\$16.00	\$0
4	UTILITY ADJUSTMENT (STRUCTURES)	2	EACH	\$5,000.00	\$10,000
5	CONC. STORM DRAIN PIPE	325	L.F.	\$50.00	\$16,250
6	REMOVING CEM. CONC. PAVEMENT	250	S.F.	\$12.00	\$3,000
7	CONSTRUCT NEW TAXIWAY (16" TH CONC)	250	S.F.	\$15.00	\$3,750
8	STRUCTURE EXCAVATION	4,900	C.Y.	\$10.00	\$49,000
9	GRAVEL BACKFILL	625	C.Y.	\$20.00	\$12,500
10	LANDSCAPING(SOD)	7,000	S.F.	\$1.00	\$7,000
<b>Subtotal Items</b>					<b>\$312,400</b>
<b>SDS-3 VAULT 3</b>					
1	MOBILIZATION (10 %)	LUMP SUM	L.S.	\$195,336	\$195,336
2	SDS-3 VAULT 3 (230'x77'x8')	3.253	Acre Feet	\$250,000	\$813,250
3	SAW CUTTING CEMENT CONC. PAVEMENT	810	L.F.	\$16.00	\$12,960
4	UTILITY ADJUSTMENT (STRUCTURES)	1	EACH	\$5,000.00	\$5,000
5	CONC. STORM DRAIN PIPE	150	L.F.	\$50.00	\$7,500
6	REMOVING CEM. CONC. PAVEMENT	34,750	S.F.	\$12.00	\$417,000
7	CONSTRUCT NEW TAXIWAY (16" TH CONC)	34,750	S.F.	\$15.00	\$521,250
8	STRUCTURE EXCAVATION	12,840	C.Y.	\$10.00	\$128,400
9	GRAVEL BACKFILL	2,400	C.Y.	\$20.00	\$48,000
10	LANDSCAPING(SOD)	0	S.F.	\$1.00	\$0
<b>Subtotal Items</b>					<b>\$2,148,696</b>
<b>SDS-3 VAULT 4</b>					
1	MOBILIZATION (10 %)	LUMP SUM	L.S.	\$63,005	\$63,005
2	SDS-3 VAULT 4 (166'x55'x8')	1.692	Acre Feet	\$250,000	\$423,000
3	SAW CUTTING CEMENT CONC. PAVEMENT (RUNWAY)	500	L.F.	\$25.00	\$12,500
4	UTILITY ADJUSTMENT (STRUCTURES)	2	EACH	\$5,000.00	\$10,000
5	CONC. STORM DRAIN PIPE	400	L.F.	\$50.00	\$20,000
6	REMOVING CEM. CONC. PAVEMENT (RUNWAY)	1,250	S.F.	\$15.00	\$18,750
7	CONSTRUCT NEW RUNWAY (24" TH CONC)	1,250	S.F.	\$20.00	\$25,000
8	STRUCTURE EXCAVATION	8,200	C.Y.	\$10.00	\$82,000
9	GRAVEL BACKFILL	1,290	C.Y.	\$20.00	\$25,800
10	LANDSCAPING(SOD)	13,000	S.F.	\$1.00	\$13,000
<b>Subtotal Items</b>					<b>\$693,055</b>

continued on next page

**ENGINEER'S ESTIMATE**

<b>Parametrix, Inc.</b>	Job No. 556-2912-001-28	Date 6/8/00	Sheet 2 of 2
Project Title SDS-3 WETVAULTS			
Location Seattle-Tacoma International Airport			
Owner Port of Seattle			
Estimated By P. McAvoy		Checked By G. McDonald	Approved By

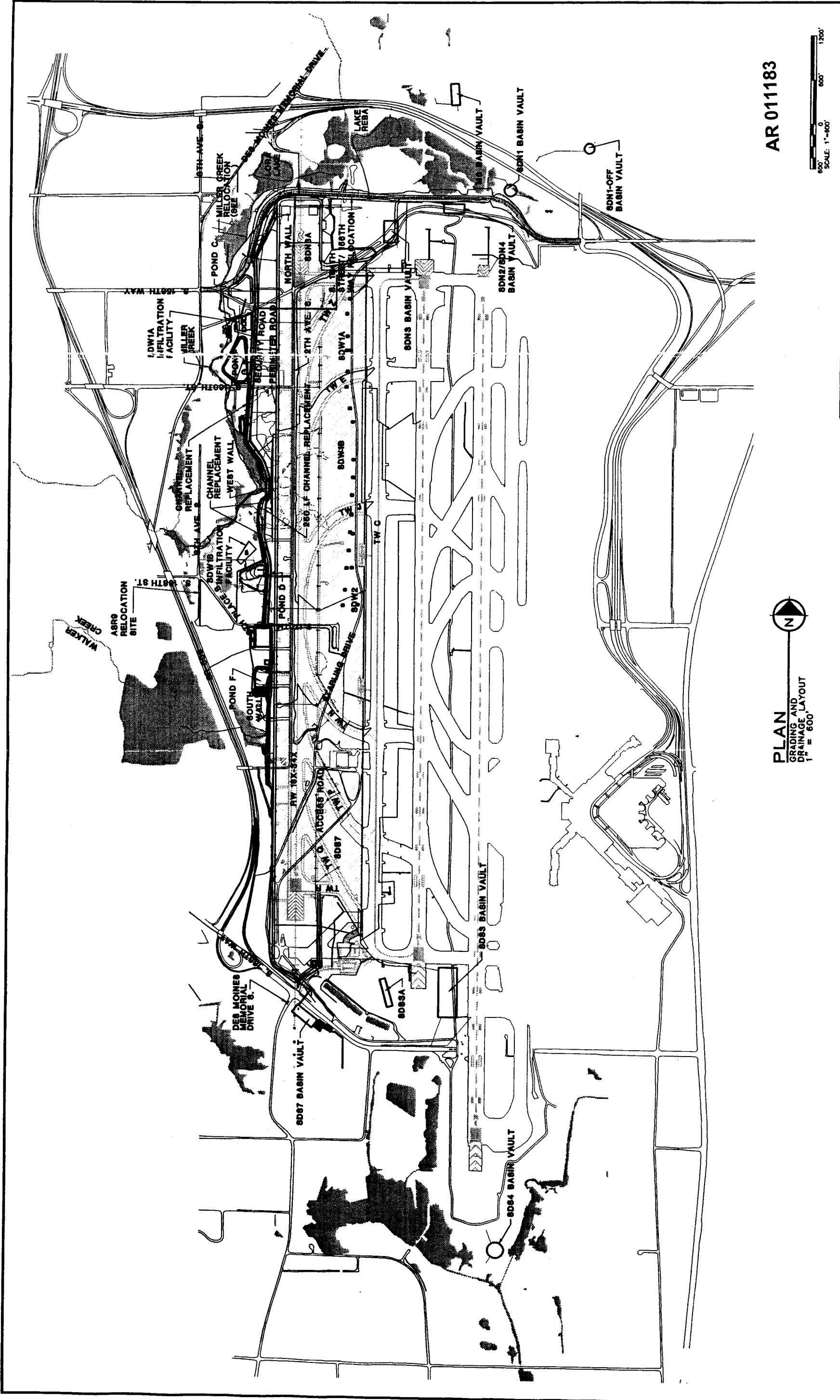
Item No.	Description	Estimated Quantity	Unit	Unit Price Mat. & Lab.	Estimated Amount
	<b>10 INCH. DIA. CONC. STORM DRAIN PIPE</b>				
1	MOBILIZATION (10 %)	LUMP SUM	L.S.	\$23,218	\$23,218
2	UTILITY ADJUSTMENT (STRUCTURES)	2	EACH	\$5,000.00	\$10,000
3	10 INCH. DIA. CONC. STORM DRAIN PIPE	2,580	L.F.	\$30.00	\$77,400
4	STRUCTURE EXCAVATION	1,500	C.Y.	\$10.00	\$15,000
5	GRAVEL BACKFILL FOR DRAIN	1,500	C.Y.	\$25.00	\$37,500
6	LANDSCAPING(REMOVE & REPLACE SOD)	10,450	S.F.	\$1.00	\$10,450
7	SAW CUTTING CEMENT CONC. PAVEMENT	980	L.F.	\$16.00	\$15,680
8	REMOVING CEM. CONC. PAVEMENT	2,450	S.F.	\$12.00	\$29,400
9	CONSTRUCT NEW TAXIWAY (16" TH CONC)	2,450	S.F.	\$15.00	\$36,750
	<b>Subtotal Items</b>				\$255,398
	<b>18 INCH. DIA. CONC. STORM DRAIN PIPE</b>				
1	MOBILIZATION (10 %)	LUMP SUM	L.S.	\$11,224.50	\$11,225
2	UTILITY ADJUSTMENT (STRUCTURES)	2	EACH	\$5,000.00	\$10,000
3	18 INCH. DIA. CONC. STORM DRAIN PIPE	575	L.F.	\$40.00	\$23,000
4	STRUCTURE EXCAVATION	320	C.Y.	\$10.00	\$3,200
5	GRAVEL BACKFILL FOR DRAIN	320	C.Y.	\$25.00	\$8,000
6	LANDSCAPING(REMOVE & REPLACE SOD)	1,150	S.F.	\$1.00	\$1,150
7	SAW CUTTING CEMENT CONC. PAVEMENT (TAXIWAY)	370	L.F.	\$16.00	\$5,920
8	SAW CUTTING CEMENT CONC. PAVEMENT (RUNWAY)	320	L.F.	\$25.00	\$8,000
9	REMOVING CEM. CONC. PAVEMENT (TAXIWAY)	925	S.F.	\$12.00	\$11,100
10	REMOVING CEM. CONC. PAVEMENT (RUNWAY)	800	S.F.	\$15.00	\$12,000
11	CONSTRUCT NEW TAXIWAY (16" TH CONC)	925	S.F.	\$15.00	\$13,875
12	CONSTRUCT NEW RUNWAY (24" TH CONC)	800	S.F.	\$20.00	\$16,000
	<b>Subtotal Items</b>				\$123,470
	<b>Total Items</b>				\$4,724,869
	<b>Sales Tax 8.4%</b>				\$396,889
	<b>Subtotal</b>				\$5,121,757
	<b>Contingency (50%)</b>				\$2,560,879
	<b>TOTAL</b>				<b>\$7,682,636</b>

**APPENDIX Q**  
**PROPOSED STORMWATER CONVEYANCE FIGURES**

**GRADING AND DRAINAGE PLAN**

<b>Plan Ref. No.</b>	<b>Sheet Title</b>
C109	GRADING AND DRAINAGE LAYOUT
C110	GRADING AND DRAINAGE PLAN
C111	GRADING AND DRAINAGE PLAN
C112	GRADING AND DRAINAGE PLAN
C113	GRADING AND DRAINAGE PLAN
C114	GRADING AND DRAINAGE PLAN
C115	GRADING AND DRAINAGE PLAN
C116	GRADING AND DRAINAGE PLAN
C117	GRADING AND DRAINAGE PLAN
C118	GRADING AND DRAINAGE PLAN
C119	GRADING AND DRAINAGE PLAN
C120	GRADING AND DRAINAGE PLAN
C121	GRADING AND DRAINAGE PLAN
C122	GRADING AND DRAINAGE PLAN
C123	GRADING AND DRAINAGE PLAN
C124	GRADING AND DRAINAGE PLAN
C125	GRADING AND DRAINAGE PLAN
C126	GRADING AND DRAINAGE PLAN
C127	GRADING AND DRAINAGE PLAN
C128	GRADING AND DRAINAGE PLAN
C129	GRADING AND DRAINAGE PLAN
C130	GRADING AND DRAINAGE PLAN



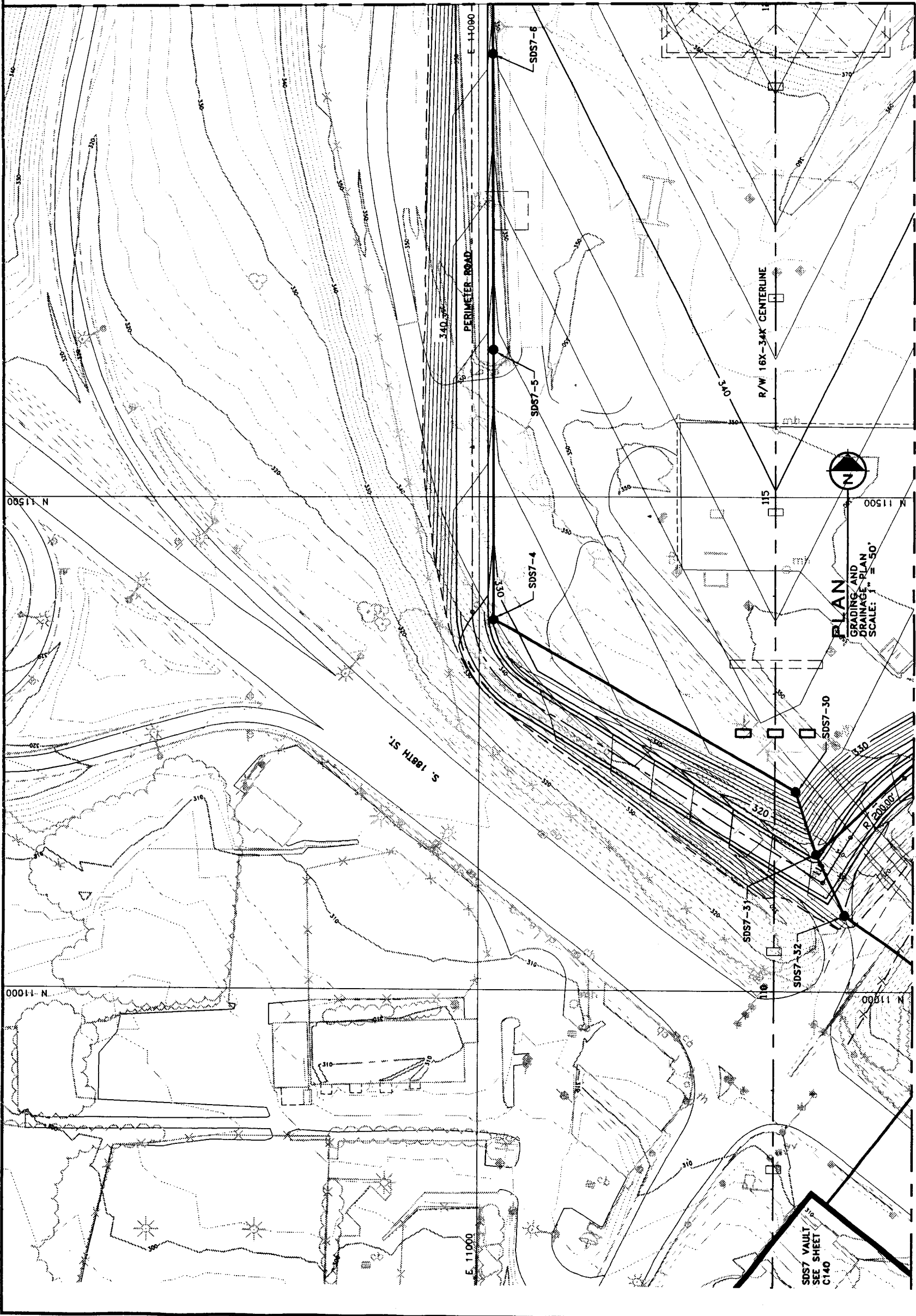


PLAN  
 GRADING AND  
 DRAINAGE LAYOUT  
 1" = 600'

SCALE: 1"=600'  
 0 600' 1200'

AR 011183

	DATE	DEC. 15, 2000
	PROJECT	SEA-TAC INTERNATIONAL AIRPORT
	CONTRACTOR'S NO.	
	SHEET TITLE	THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6 <b>GRADING AND      DRAINAGE LAYOUT</b>
	PORT OF SEATTLE NO.	
	EXHIBIT	C109



NOTES:

LEGEND:

- STORM DRAIN PIPE
- ▨ WETLANDS



KEY PLAN

110	111	112	113	114	115	116	117	118	119
128	129	130	121	122	123	124	125	126	127

MATCHLINE SEE SHEET C-111

MATCHLINE SEE SHEET C-119

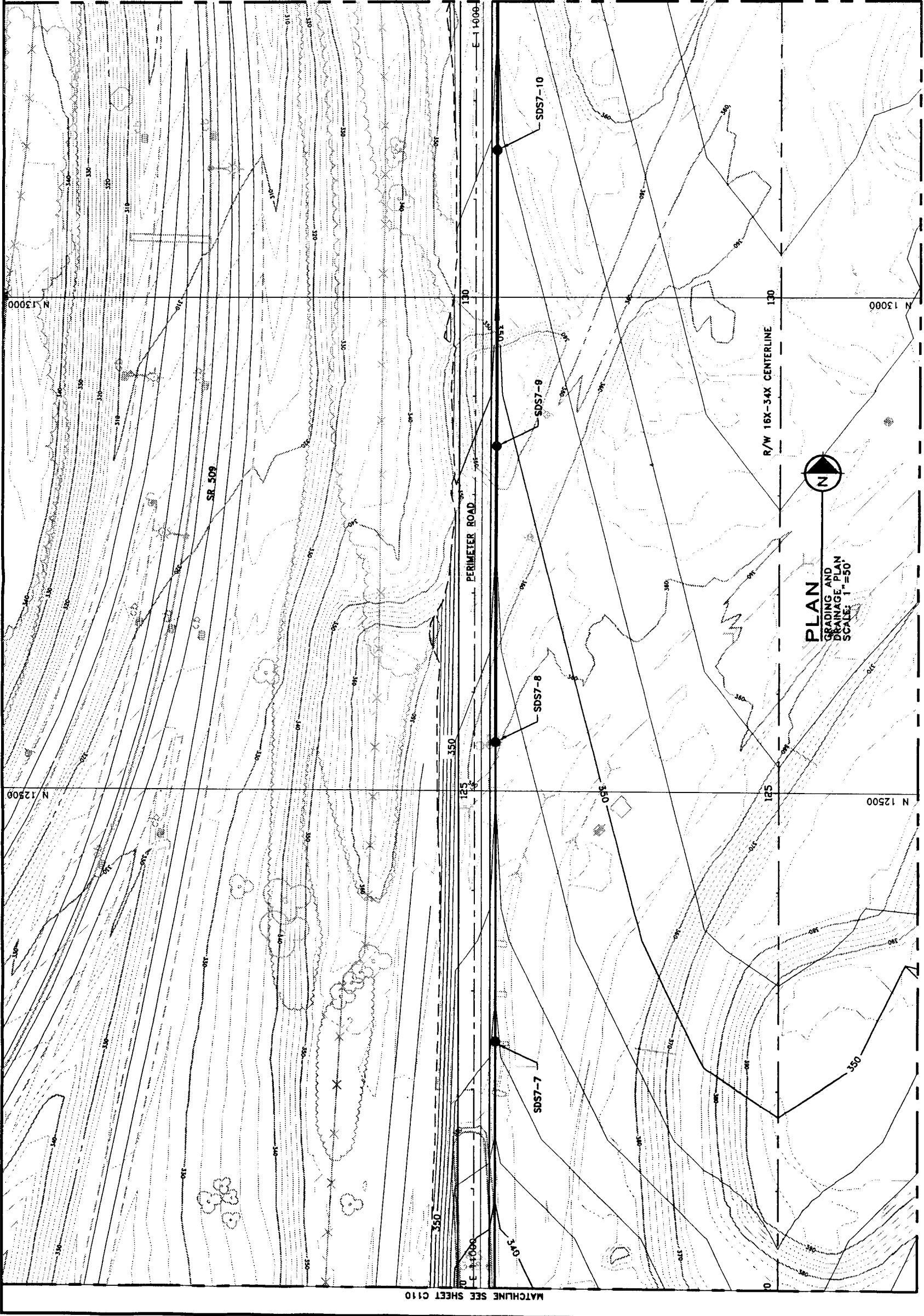
PLAN  
GRADING AND DRAINAGE PLAN  
SCALE: 1" = 50'

SDS7 VAULT  
SEE SHEET  
C140

Part of Seattle  
SEA-TAC INTERNATIONAL AIRPORT  
PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6  
SHEET TITLE: GRADING AND DRAINAGE PLAN

DATE: DEC. 15, 2000  
CONSULTANT'S NO.  
PART OF SEATTLE NO.  
EXHIBIT: C110

AR 011184



NOTES:

LEGEND:

- STORM DRAIN PIPE
- ▨ WETLANDS



KEY PLAN

110	111	112	113	114	115	116	117	118
119	120	121	122	123	124	125	126	127
128	129	130						

MATCHLINE SEE SHEET C110

MATCHLINE SEE SHEET C112



PLAN  
GRADING AND  
DRAINAGE PLAN  
SCALE: 1"=50'

MATCHLINE SEE SHEET C120

**Port of Seattle**  
SEA-TAC INTERNATIONAL AIRPORT

PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 5

SHEET TITLE: **GRADING AND DRAINAGE PLAN**

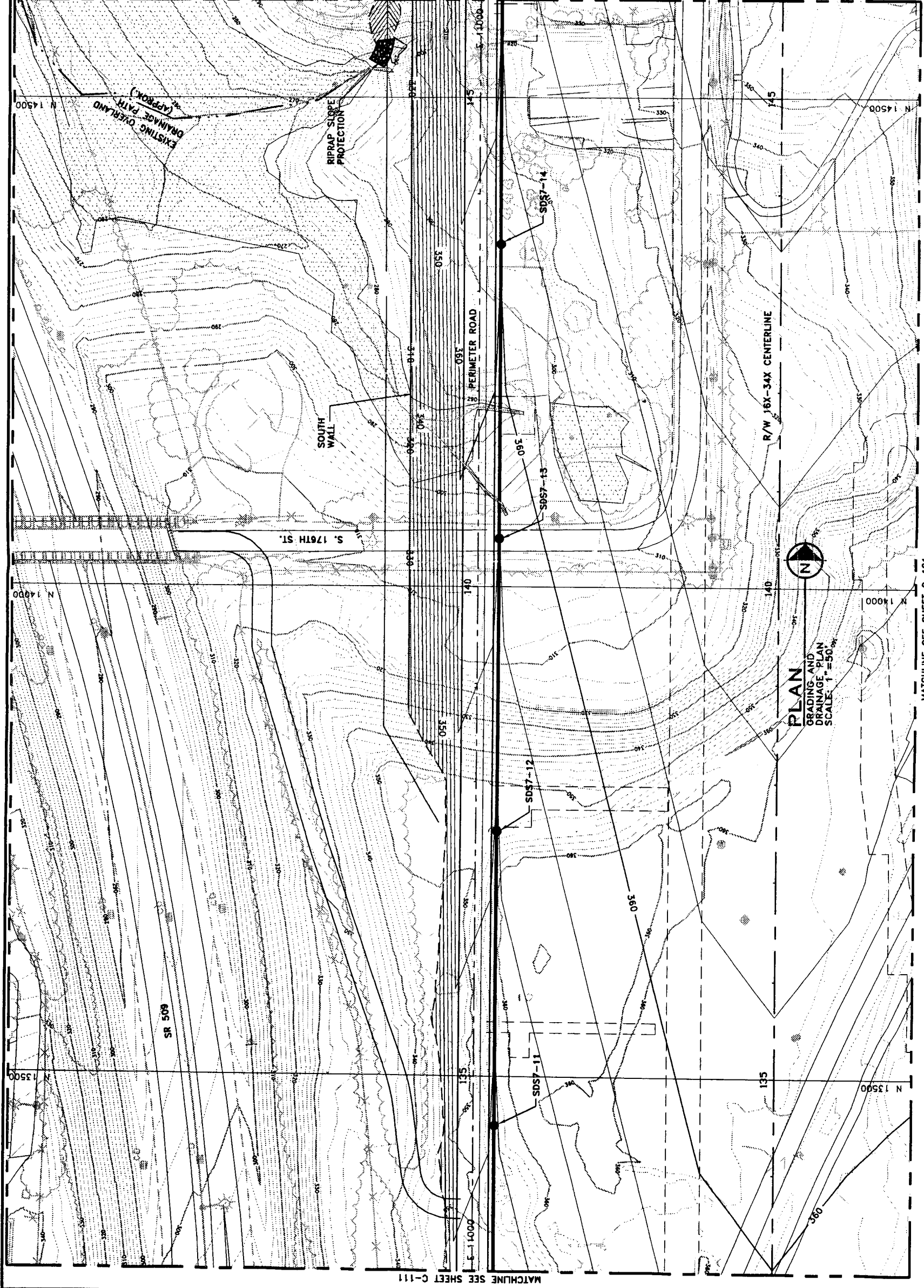
DATE: DEC. 15, 2000

CONSULTANT'S NO.

PORT OF SEATTLE NO.

EXHIBIT\_ C111

AR 011185



NOTES:

LEGEND:

- STORM DRAIN PIPE
- ▨ WETLANDS



110	111	112	113	114	115	116	117	118	119	120	121	122	123	124	125	126	127	128	129	130
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KEY PLAN

**PLAN**  
 GRADING AND DRAINAGE PLAN  
 SCALE: 1"=50.00'



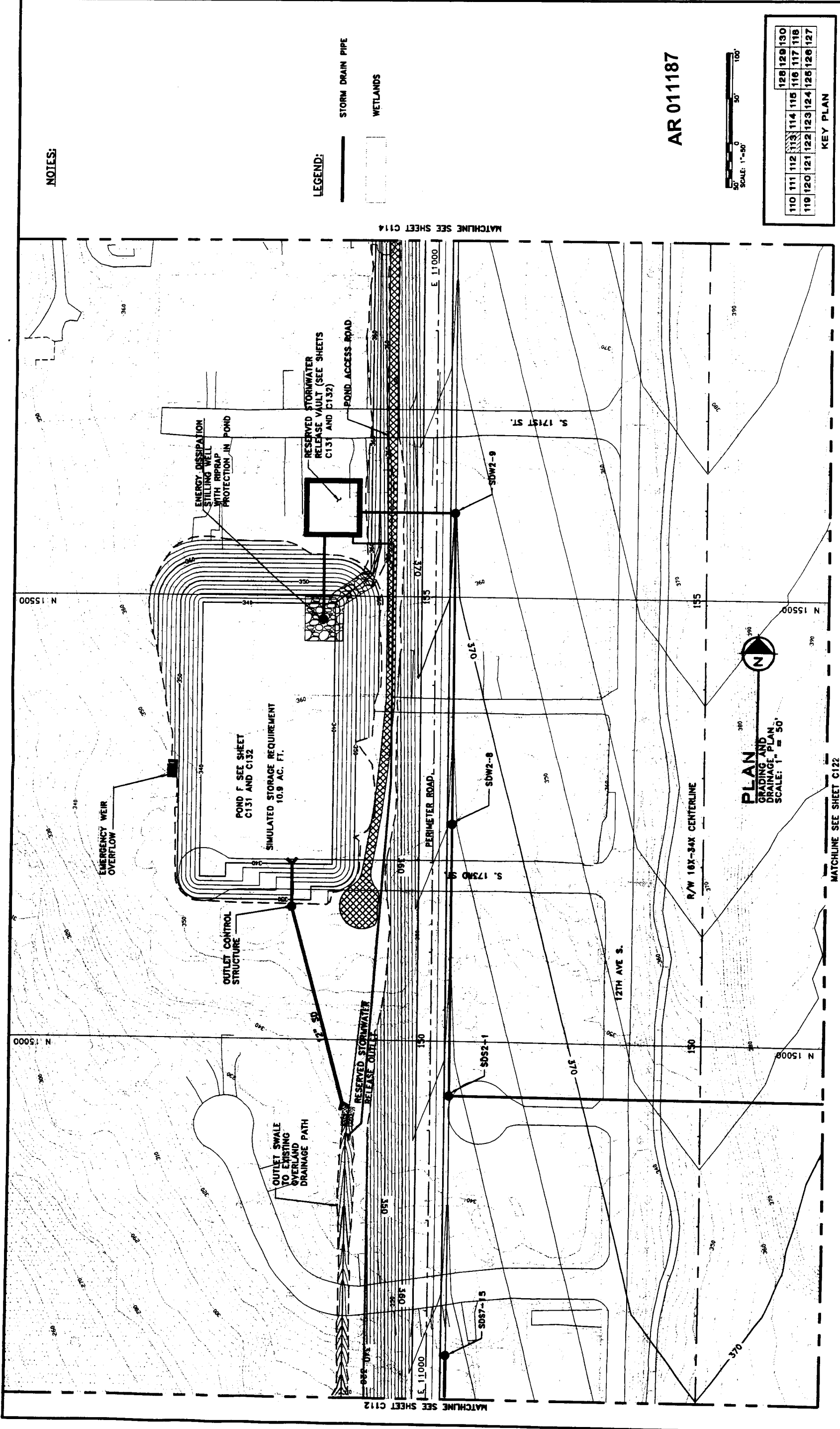
MATCHLINE SEE SHEET C-111

MATCHLINE SEE SHEET C-121

**Port of Seattle**  
 SEA-TAC INTERNATIONAL AIRPORT  
 PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 5  
 SHEET TITLE: **GRADING AND DRAINAGE PLAN**

DATE: DEC. 15, 2000  
 CONSULTANT'S NO.:  
 PART OF BEATTLE NO.:  
 EXHIBIT: C112

AR 011186



NOTES:

LEGEND:



AR 011187



KEY PLAN

110	111	112	113	114	115	116	117	118
119	120	121	122	123	124	125	126	127
						128	129	130

**Port of Seattle**  
**SEA-TAC INTERNATIONAL AIRPORT**  
 PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6  
 SHEET TITLE: **GRADING AND DRAINAGE PLAN**

July 2001  
 556-2912-001 (28)  
 DATE: MAY 10, 2001  
 CONSULTANT'S FILE:  
 PORT OF SEATTLE FILE:  
 EXHIBIT: C113

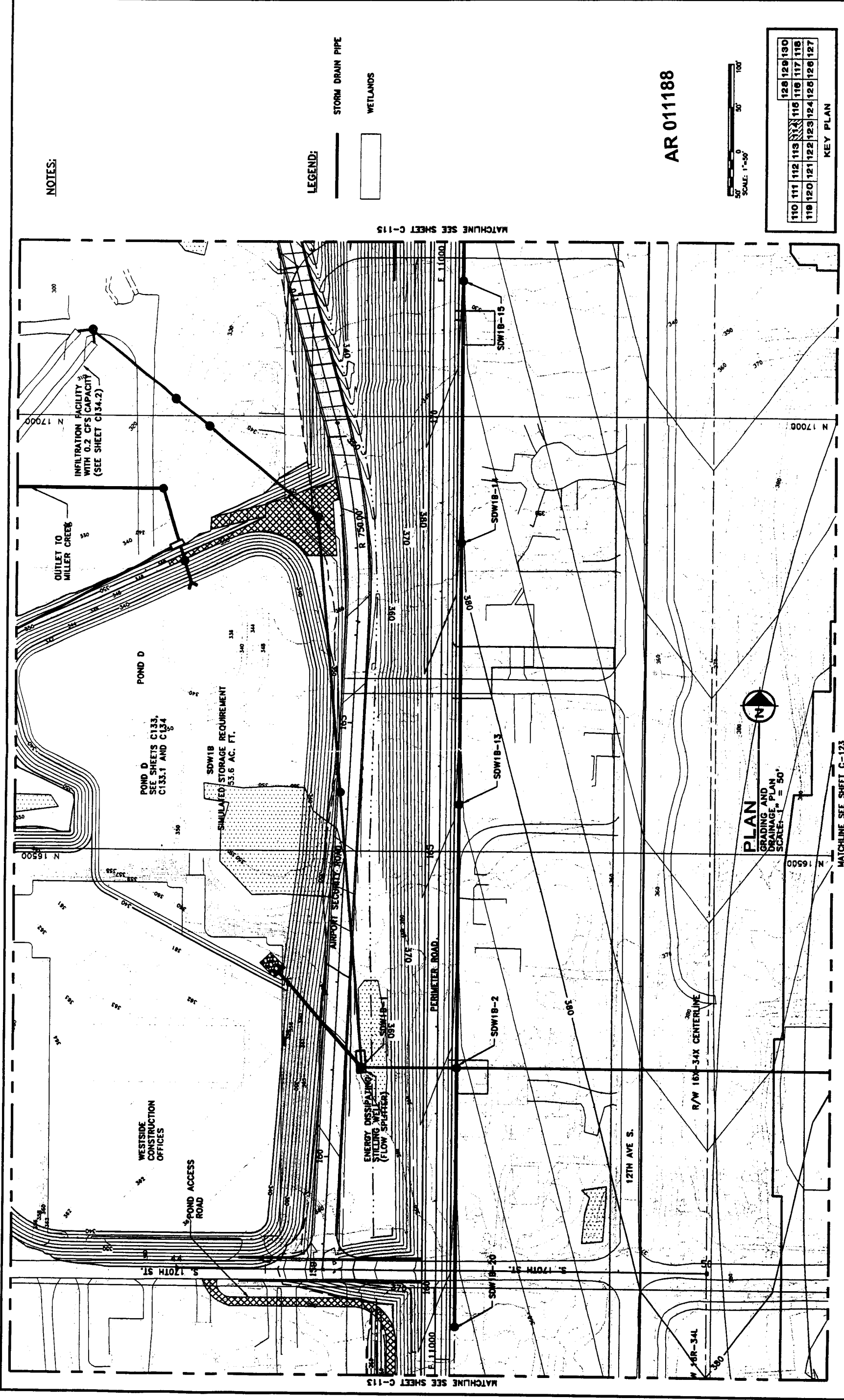
**PLAN**  
 GRADING AND  
 DRAINAGE PLAN  
 SCALE: 1" = 50'

MATCHLINE SEE SHEET C112

MATCHLINE SEE SHEET C114

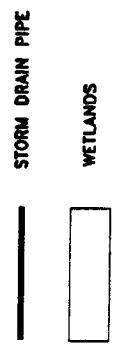
MATCHLINE SEE SHEET C122





NOTES:

LEGEND:

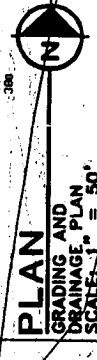


AR 011188



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128	129	130						

KEY PLAN



PLAN  
GRADING AND  
DRAINAGE PLAN  
SCALE: 1" = 50'

MATCHLINE SEE SHEET C-115

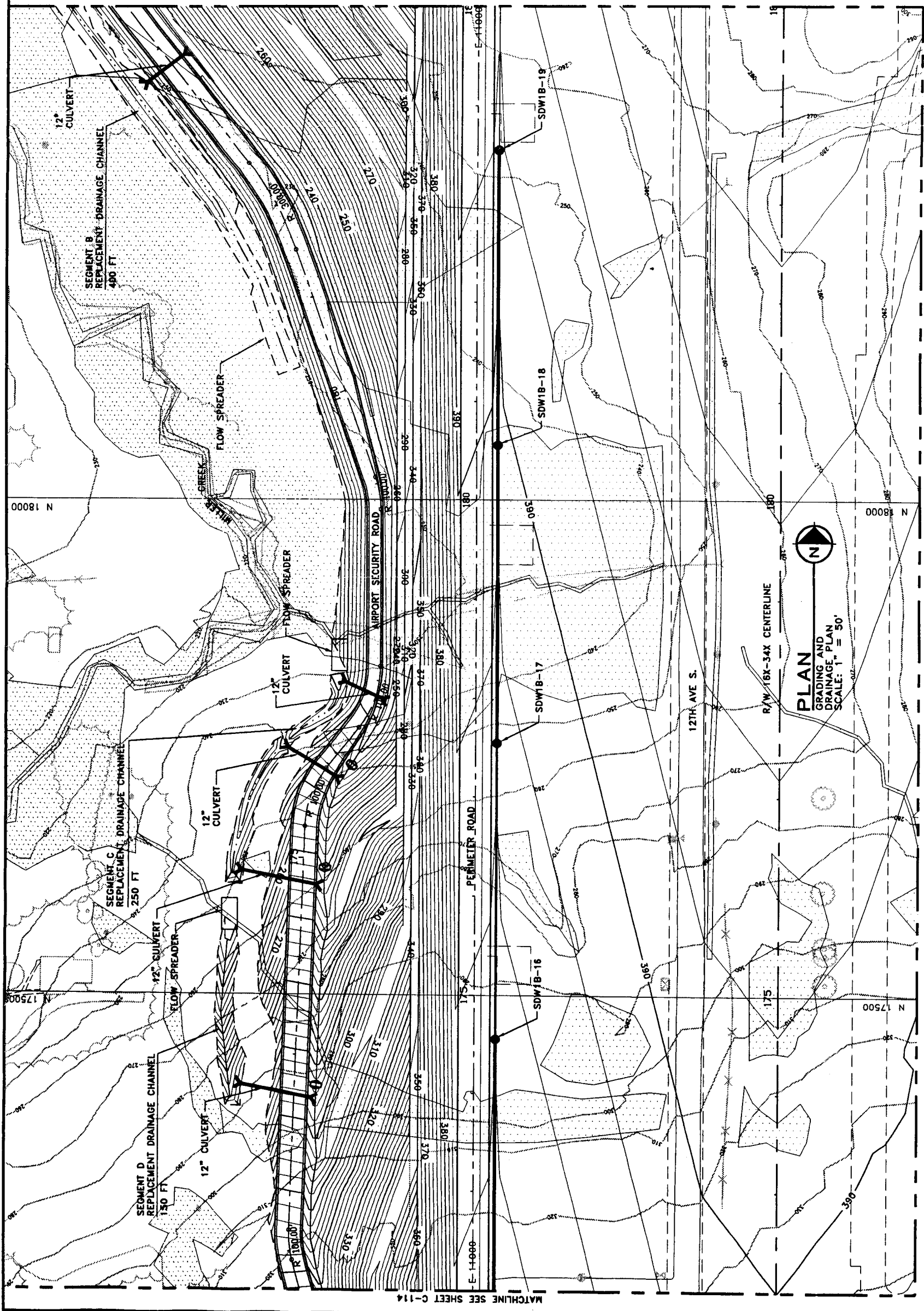
MATCHLINE SEE SHEET C-113

MATCHLINE SEE SHEET C-123

**Port of Seattle**  
SEA-TAC INTERNATIONAL AIRPORT  
PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6  
SHEET TITLE: **GRADING AND DRAINAGE PLAN**

DATE: JUNE 28, 2001  
DESIGNER: [Redacted]  
DRAWN BY: [Redacted]  
EXHIBIT: C114

July 2001  
556-2912-001 (28)



NOTES:

LEGEND:

- STORM DRAIN PIPE
- ▨ WETLANDS

AR 011189



KEY PLAN

110	111	112	113	114	115	116	117	118	119	120	121	122	123	124	125	126	127	128	129	130
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MATCHLINE SEE SHEET C-128

MATCHLINE SEE SHEET C-116

PLAN  
GRADING AND  
DRAINAGE PLAN  
SCALE: 1" = 50'



MATCHLINE SEE SHEET C-124



Port of Seattle  
SEA-TAC INTERNATIONAL AIRPORT

PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6

SHEET TITLE: GRADING AND DRAINAGE PLAN

DATE  
DEC. 15, 2000

CONSULTANT'S NO.

PART OF SEATTLE NO.  
EXHIBIT\_ C115

NOTES:

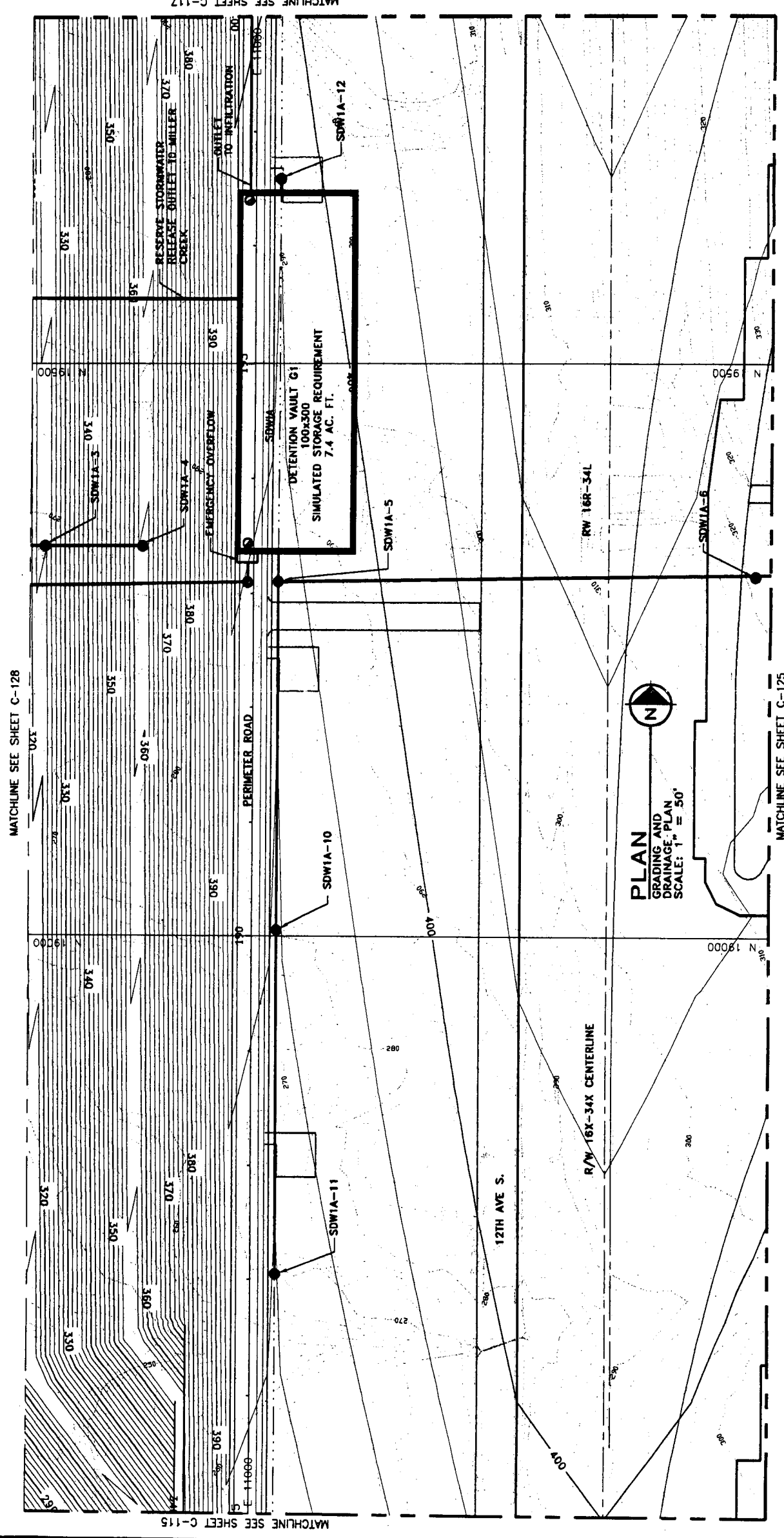
LEGEND:  
 — STORM DRAIN PIPE  
 □ WETLANDS

AR 011190




KEY PLAN

110	111	112	113	114	115	116	117	118
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128	129	130	131	132	133	134	135	136



PLAN  
 GRADING AND  
 DRAINAGE PLAN  
 SCALE: 1" = 50'




**Port of Seattle**  
 SEA-TAC INTERNATIONAL AIRPORT  
 PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6  
 SHEET TITLE: **GRADING AND DRAINAGE PLAN**

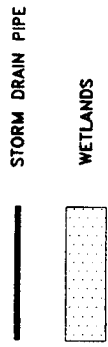
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 PORT OF SEATTLE NO.:  
 EXHIBIT: C116

July 2001  
 556-2912-001 (28)



NOTES:

LEGEND:

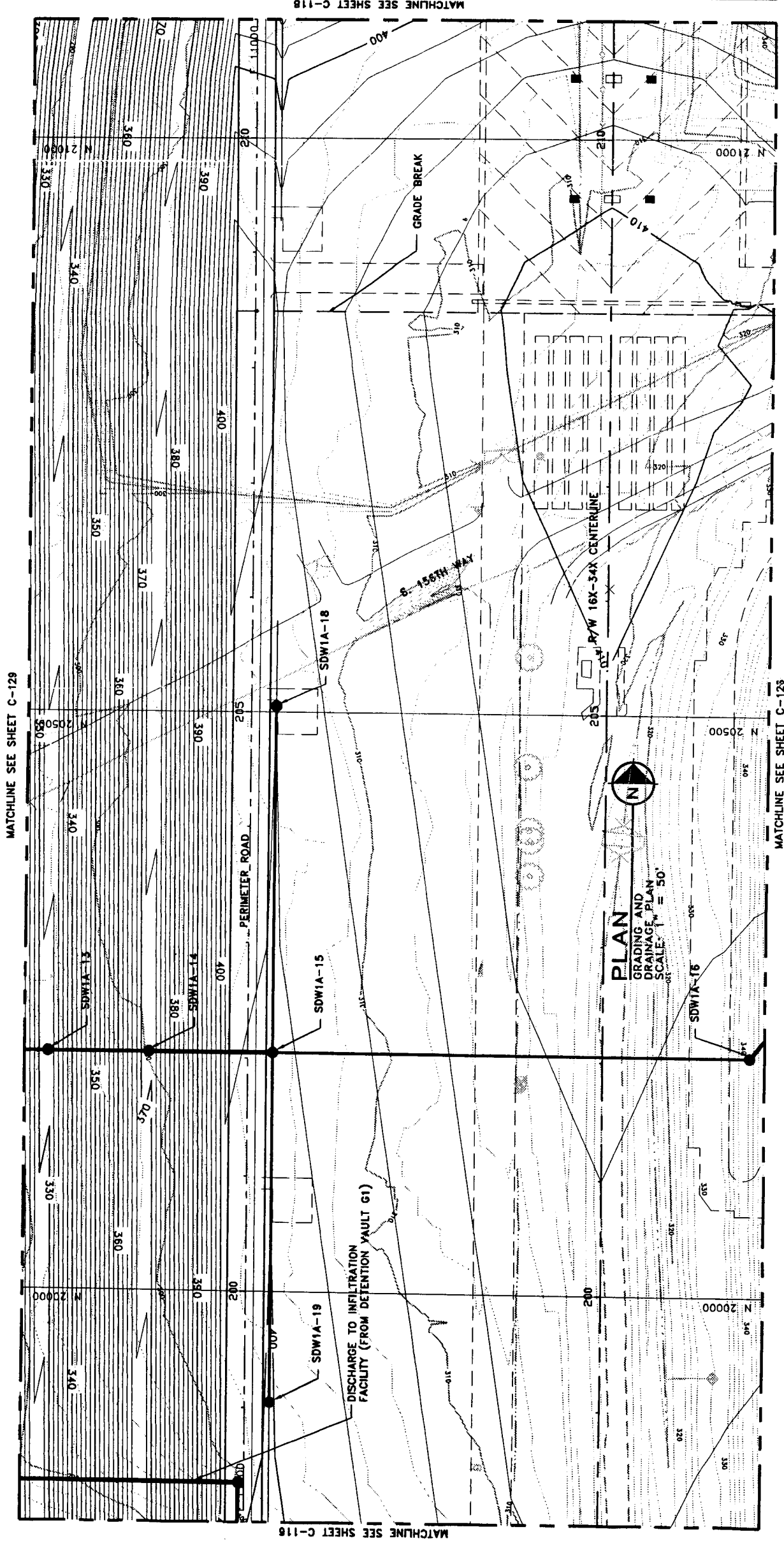


AR 011191



128	129	130						
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KEY PLAN



**Port of Seattle**  
SEA-TAC INTERNATIONAL AIRPORT

PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6

SHEET TITLE: GRADING AND DRAINAGE PLAN

DATE: DEC. 15, 2000

CONTRACTOR'S NO.

PORT OF SEATTLE NO.

EXHIBIT\_ C117

NOTES:

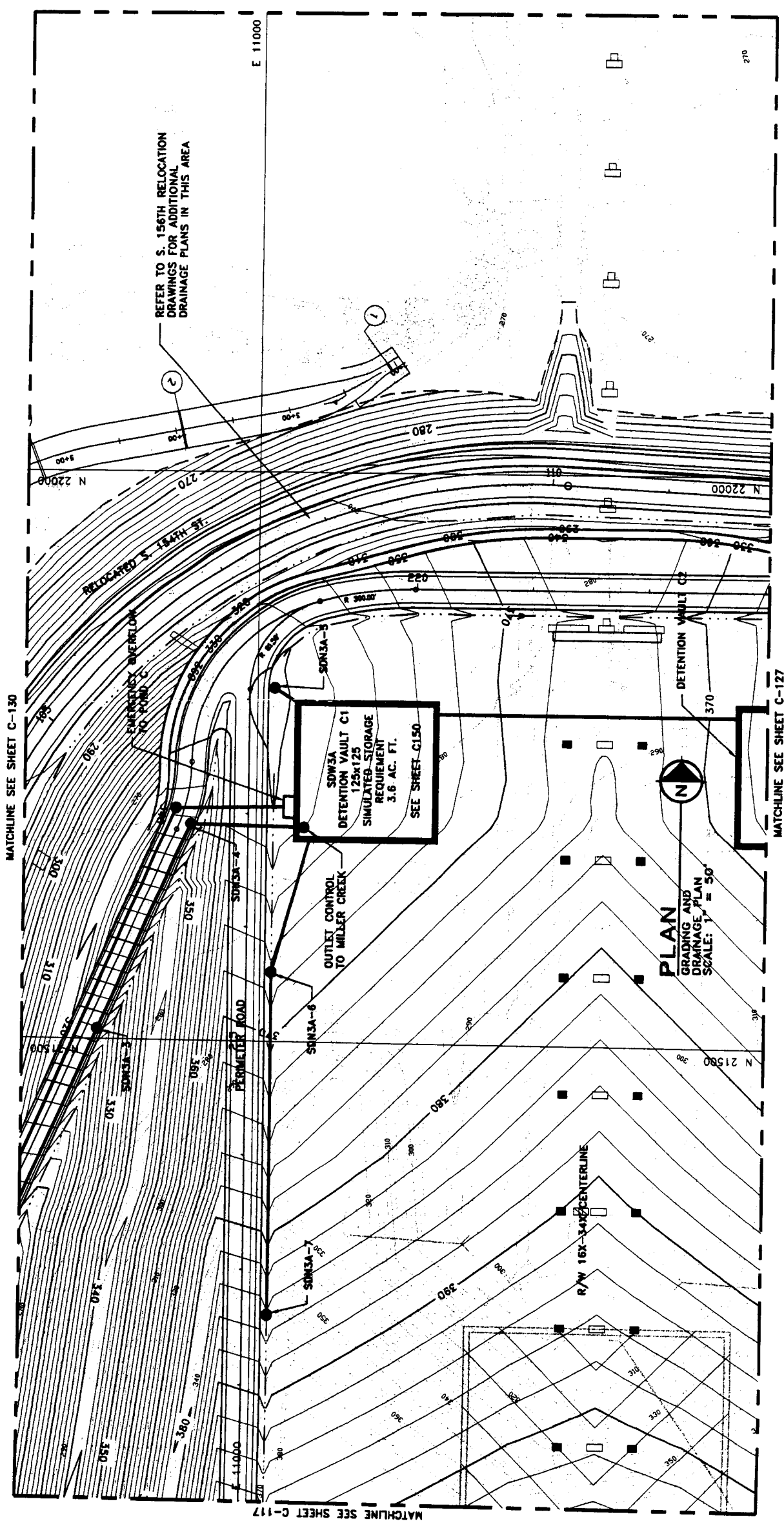
LEGEND:  
 ——— STORM DRAIN PIPE  
 - - - - WETLANDS

AR 011192



KEY PLAN

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128	129	130						



Port of Seattle  
 SEA-TAC INTERNATIONAL AIRPORT  
 PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6  
 SHEET TITLE: GRADING AND DRAINAGE PLAN

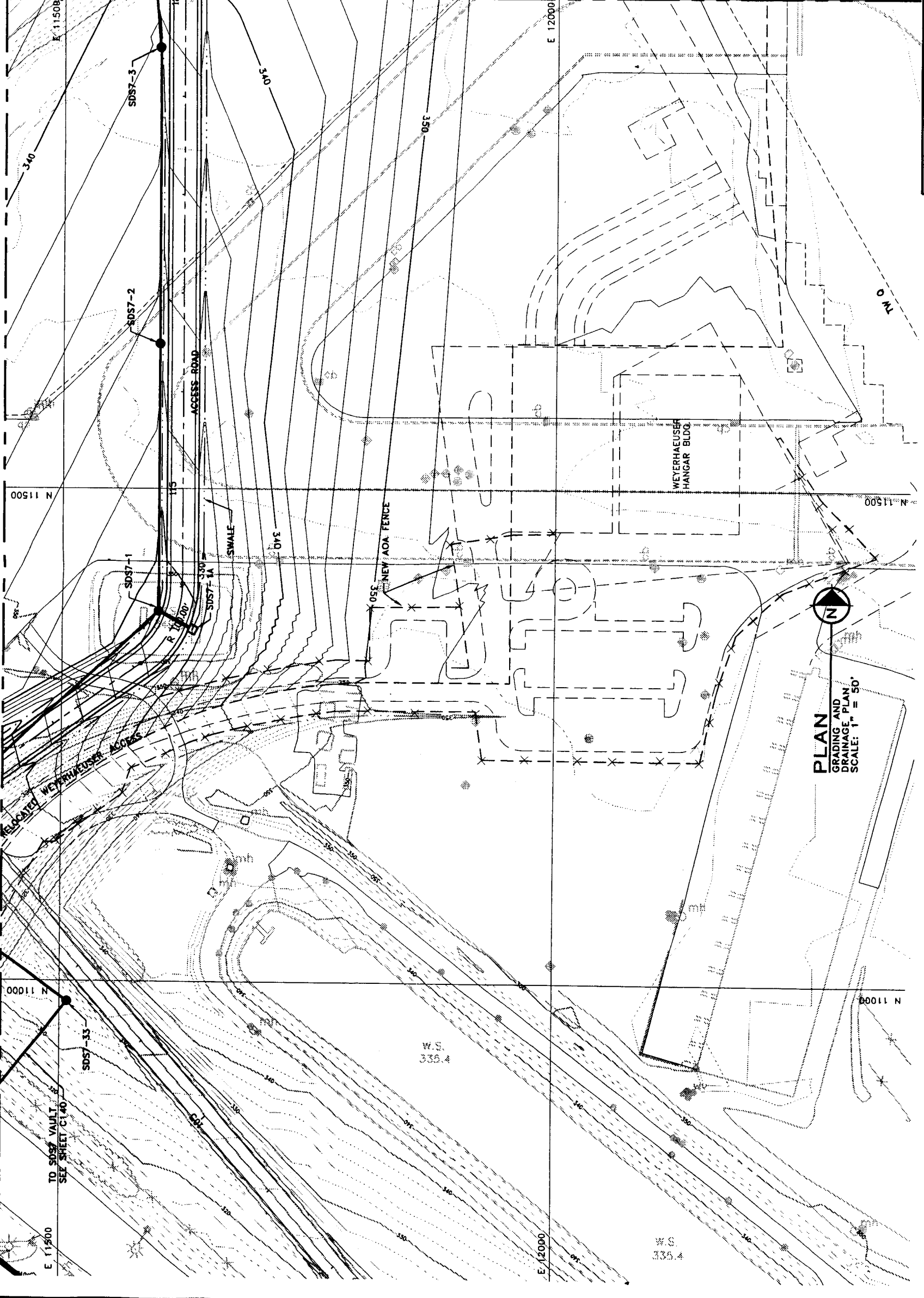
DATE: JUNE 11, 2001  
 CONSULTANT'S NO.  
 PART OF BIDDING NO.  
 EXHIBIT: C118

July 2001  
 556-2912-001 (28)

**NOTES:**

**LEGEND:**

- STORM DRAIN PIPE
- ▨ WETLANDS



MATCHLINE SEE SHEET C-120

MATCHLINE SEE SHEET C-110

TO SDS7 VAULT  
SEE SHEET C-140

**PLAN**  
GRADING AND  
DRAINAGE PLAN  
SCALE: 1" = 50'



KEY PLAN

110	111	112	113	114	115	116	117	118
119	120	121	122	123	124	125	126	127
128	129	130						

AR 011193

Part of Seattle  
SEA-TAC INTERNATIONAL AIRPORT

PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6

SHEET TITLE: GRADING AND DRAINAGE PLAN

DATE: DEC. 15, 2000

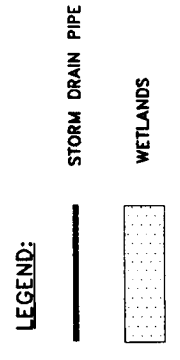
CONSULTANT'S NO.

PART OF SHEET NO.

EXHIBIT: C119



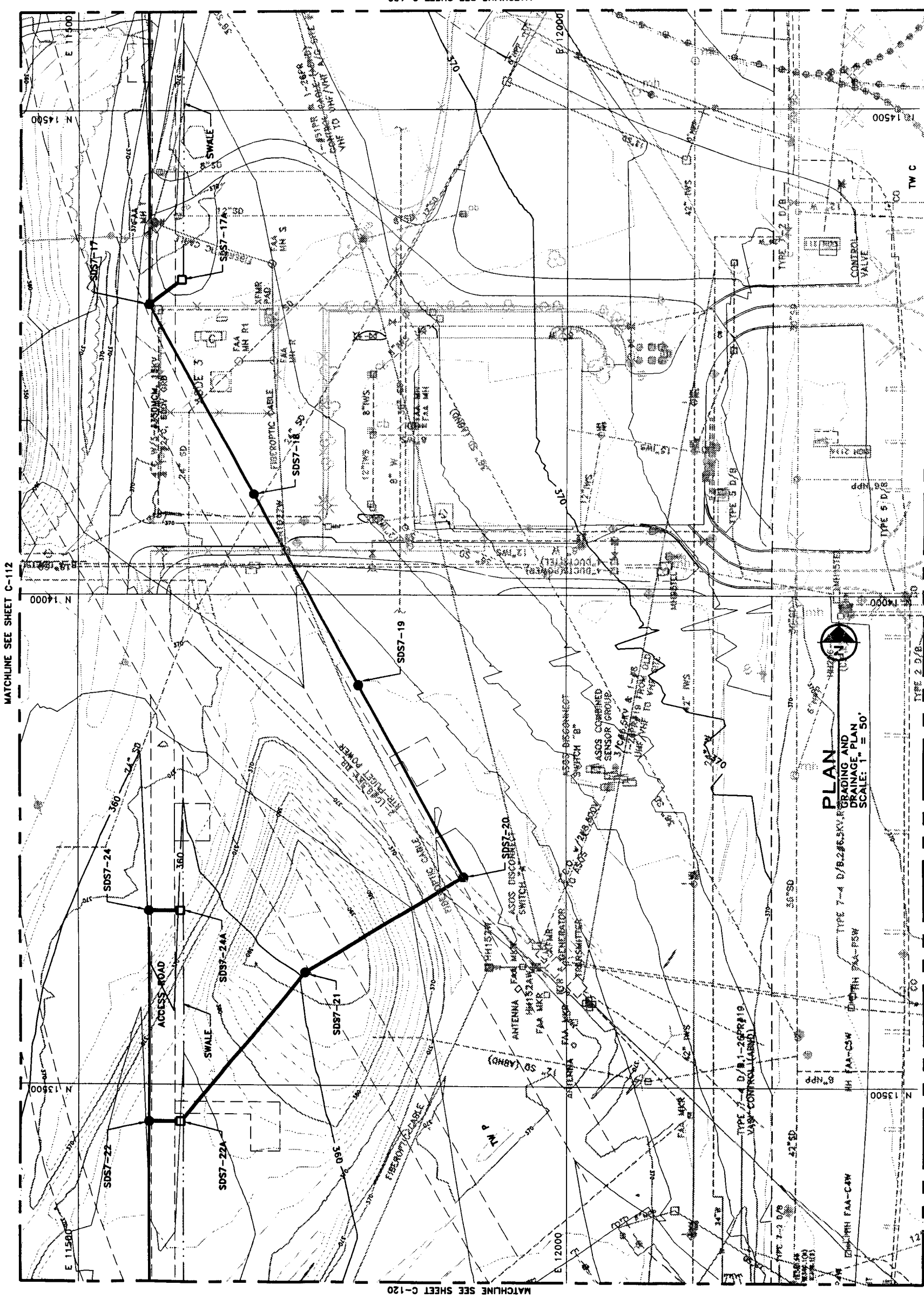
NOTES:



KEY PLAN

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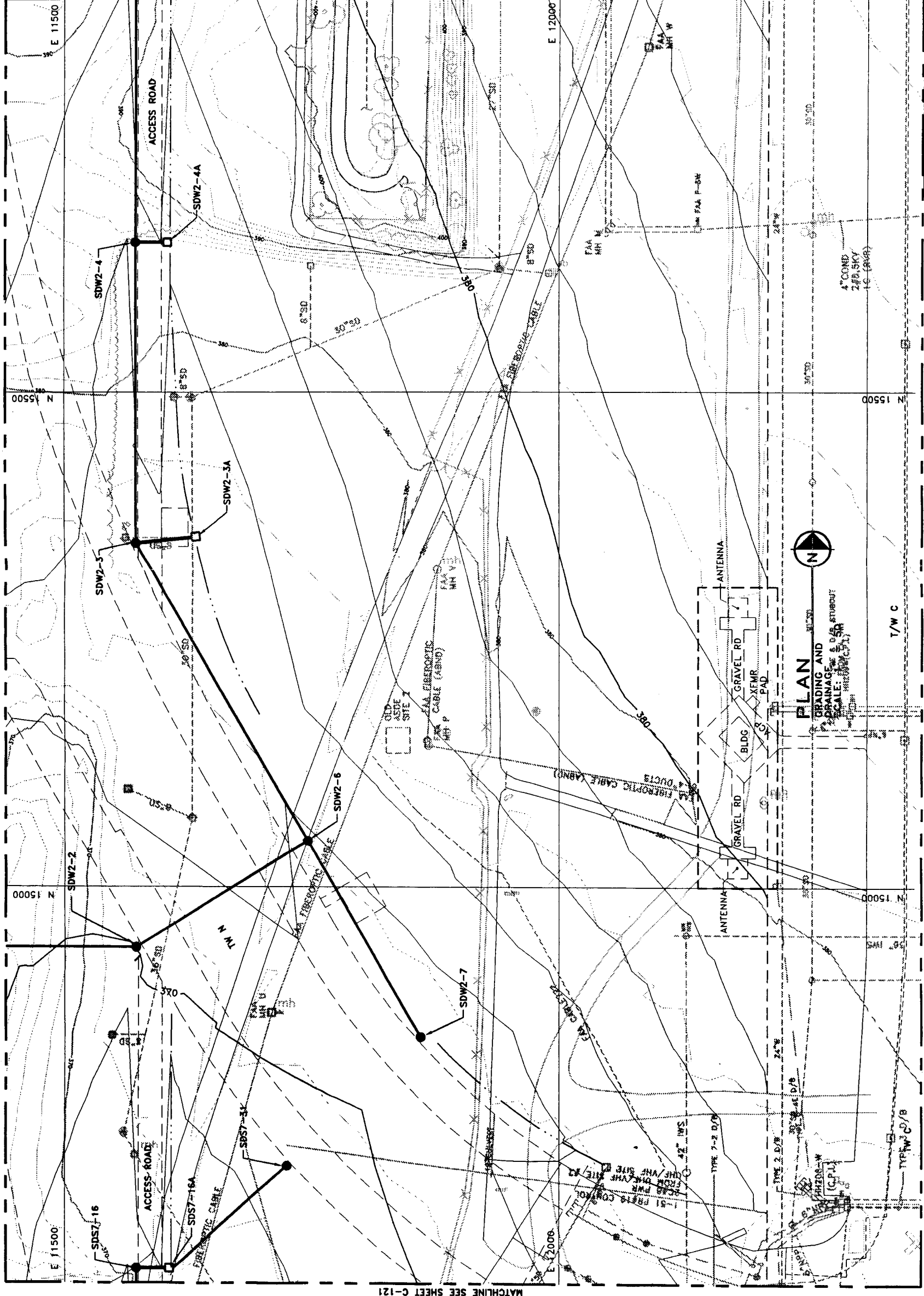
AR 011195



Port of Seattle  
SEA-TAC INTERNATIONAL AIRPORT  
PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6  
SHEET TITLE: GRADING AND DRAINAGE PLAN

DATE: DEC. 15, 2000  
CONTRACT NO.:  
PORT OF SEATTLE NO.:  
EXHIBIT: C121





**LEGEND:**  
 — STORM DRAIN PIPE  
 [Hatched Box] WETLANDS

AR 011196



KEY PLAN

126	129	130
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111	114	115
116	117	118
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122	123	124
125	126	127

MATCHLINE SEE SHEET C-123

MATCHLINE SEE SHEET C-121



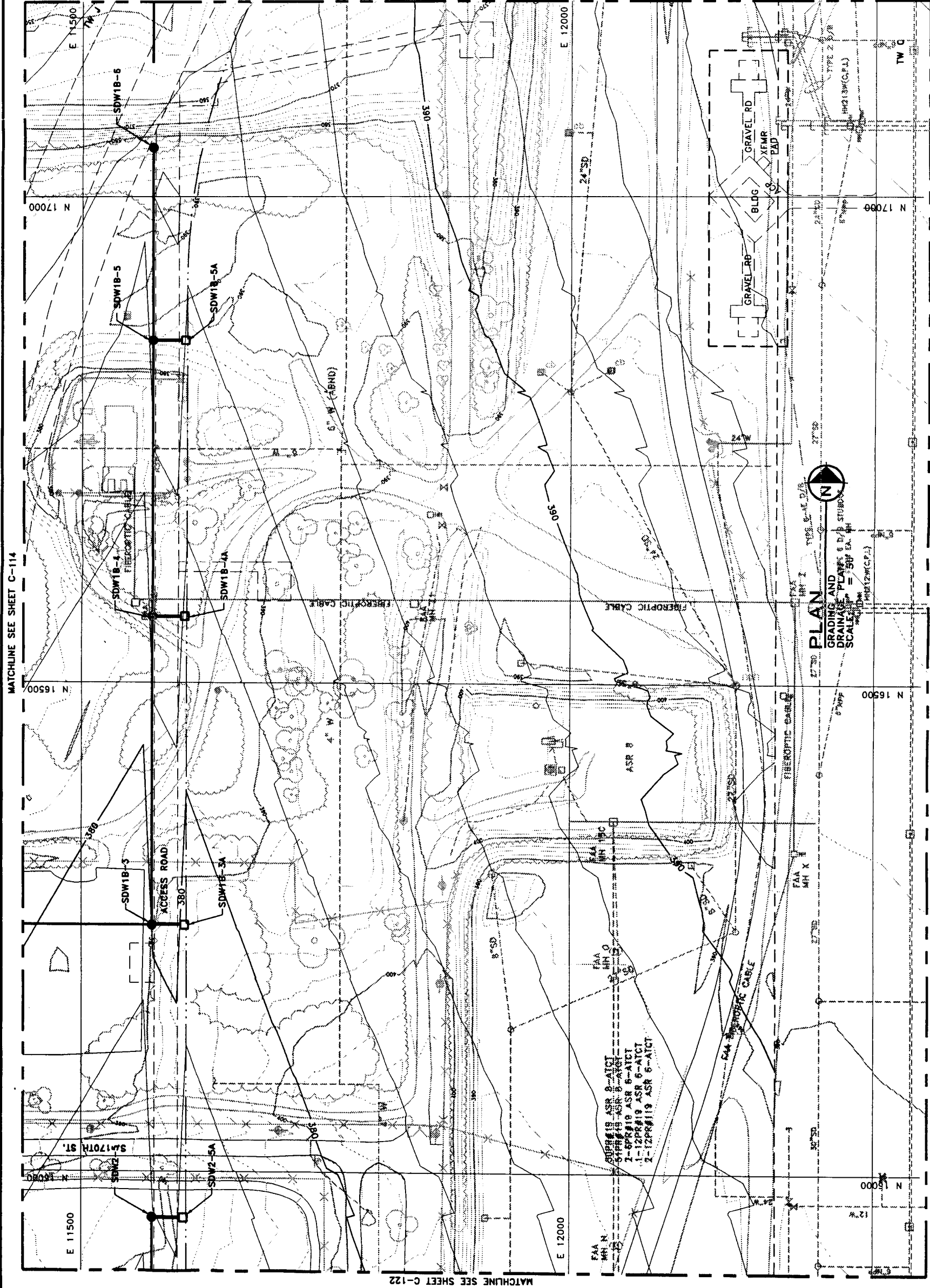
**PLAN**

GRADING AND DRAINAGE & UTILITIES  
 SCALE: 1"=50'

T/W C

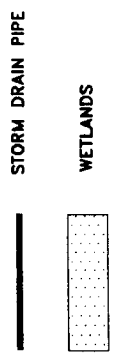
**Port of Seattle**  
 SEA-TAC INTERNATIONAL AIRPORT  
 PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6  
 SHEET TITLE: GRADING AND DRAINAGE PLAN

DATE: DEC. 15, 2000  
 CONSULTANT'S NO.:  
 PART OF SEATTLE NO.:  
 EXHIBIT: C122



NOTES:

LEGEND:



AR 011197



KEY PLAN

128	129	130
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113	114	115
116	117	118
119	120	121
122	123	124
125	126	127

Port of Seattle  
SEA-TAC INTERNATIONAL AIRPORT  
THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6  
PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6  
SHEET TITLE: GRADING AND DRAINAGE PLAN

DATE: DEC. 15, 2000  
CONTRACT NO.:  
PORT OF SEATTLE NO.:  
EXHIBIT: C123

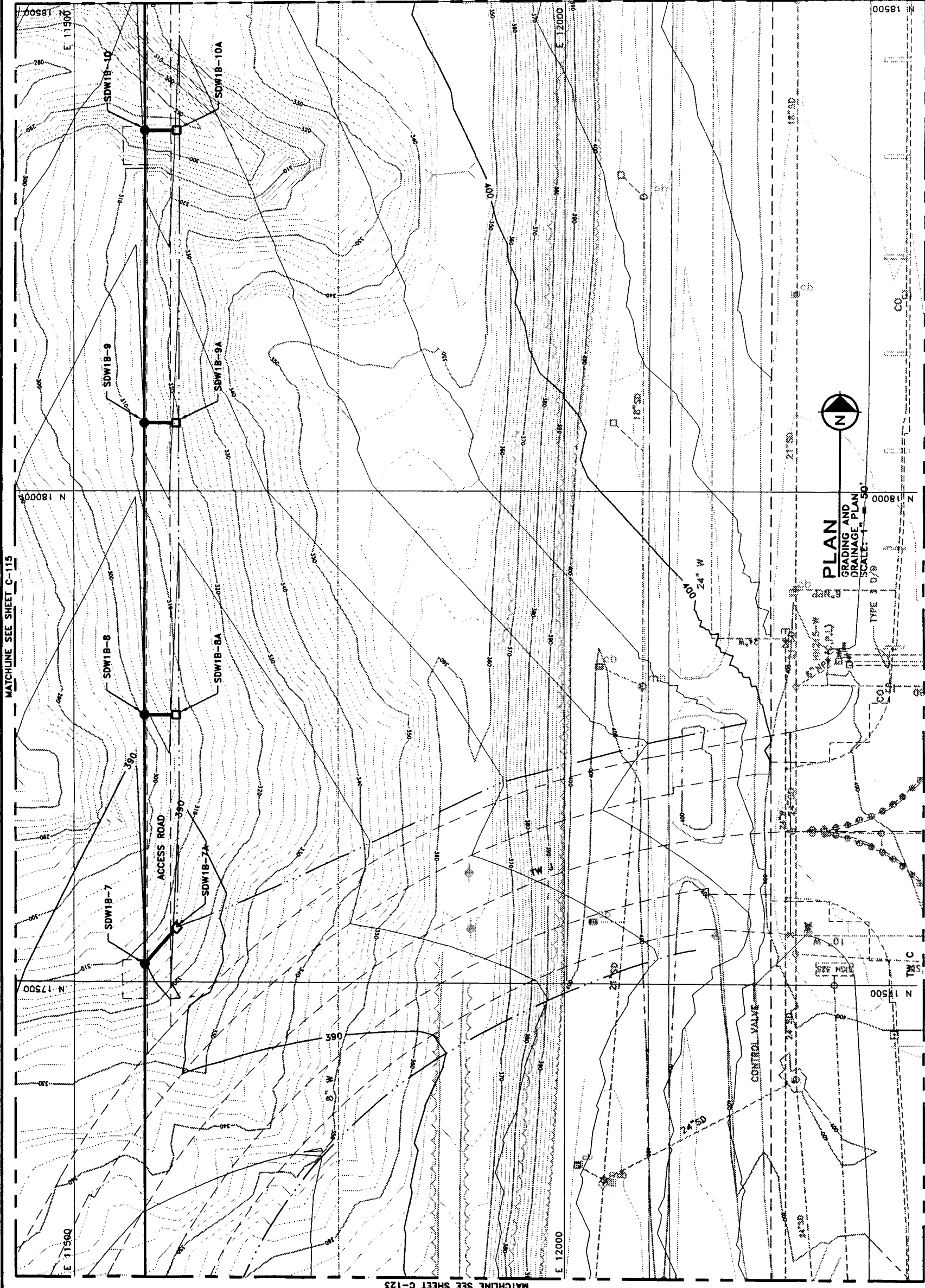


ASR 8  
1-12PR#19 ASR 8-ATCT  
2-12PR#19 ASR 8-ATCT  
1-6PR#18 ASR 8-ATCT  
2-6PR#18 ASR 8-ATCT  
1-12PR#19 ASR 5-ATCT  
2-12PR#19 ASR 5-ATCT

MATCHLINE SEE SHEET C-114

MATCHLINE SEE SHEET C-122

MATCHLINE SEE SHEET C-124



NOTES:

LEGEND:

- STORM DRAIN PIPE
- ▨ WETLANDS

MATCHLINE SEE SHEET C-125

MATCHLINE SEE SHEET C-123

AR 011198



PLAN  
GRADING AND  
DRAINAGE PLAN  
SCALE: 1" = 50'



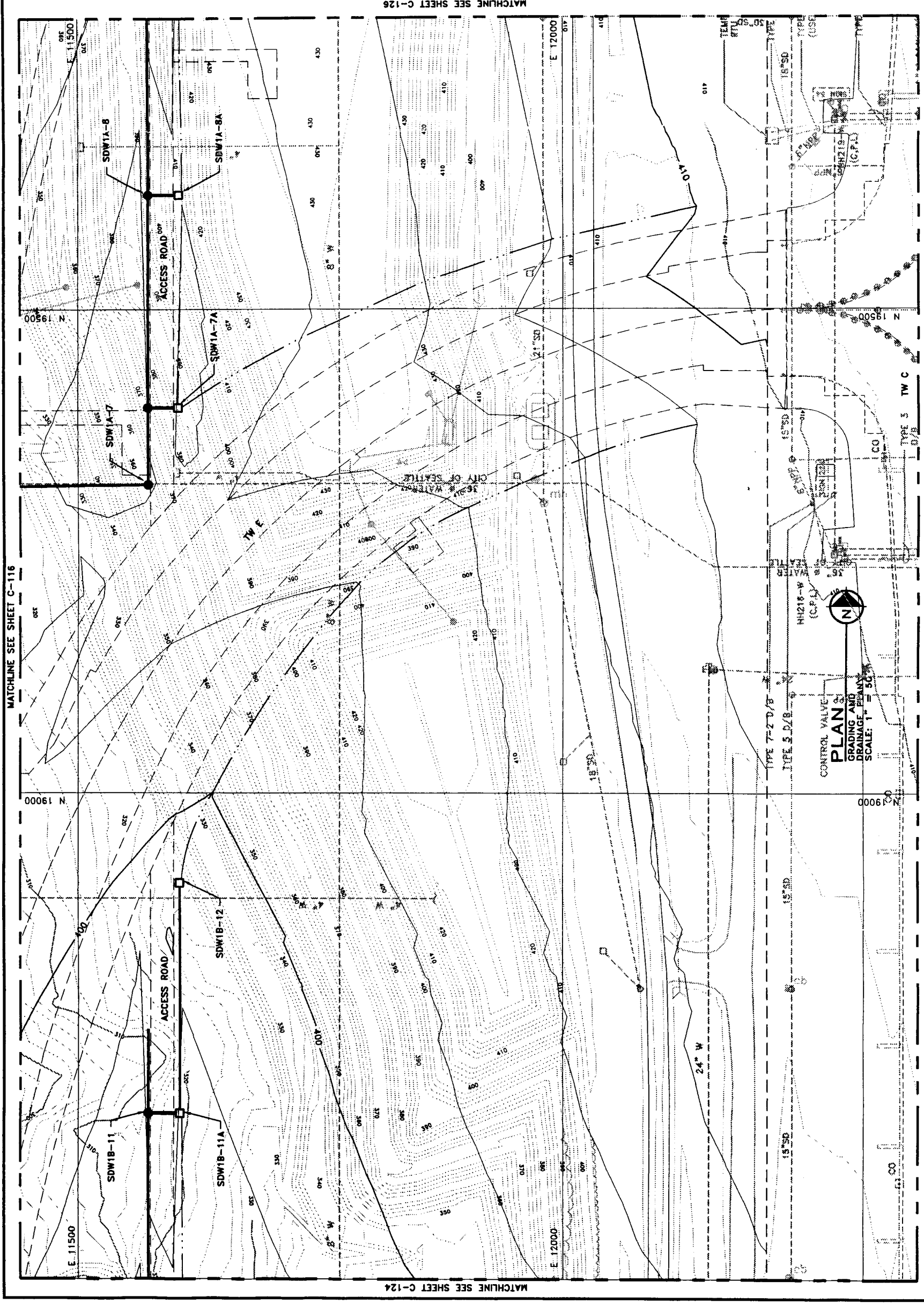
KEY PLAN

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116	117	118
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122	123	124
125	126	127


**Port of Seattle**  
 SEA-TAC INTERNATIONAL AIRPORT  
 PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6  
 SHEET TITLE: GRADING AND DRAINAGE PLAN

DATE: DEC. 15, 2000  
 CONTRACT NO.:  
 PART OF SHEET NO.:  
 EXHIBIT: C124





NOTES:

LEGEND:

- STORM DRAIN PIPE
- ▨ WETLANDS

AR 011199



KEY PLAN			
110	111	112	113
114	115	116	117
118	119	120	121
122	123	124	125
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130	131	132	133



CONTROL VALVE  
**PLAN**  
 GRADING AND  
 DRAINAGE DETAILS  
 SCALE: 1" = 50'

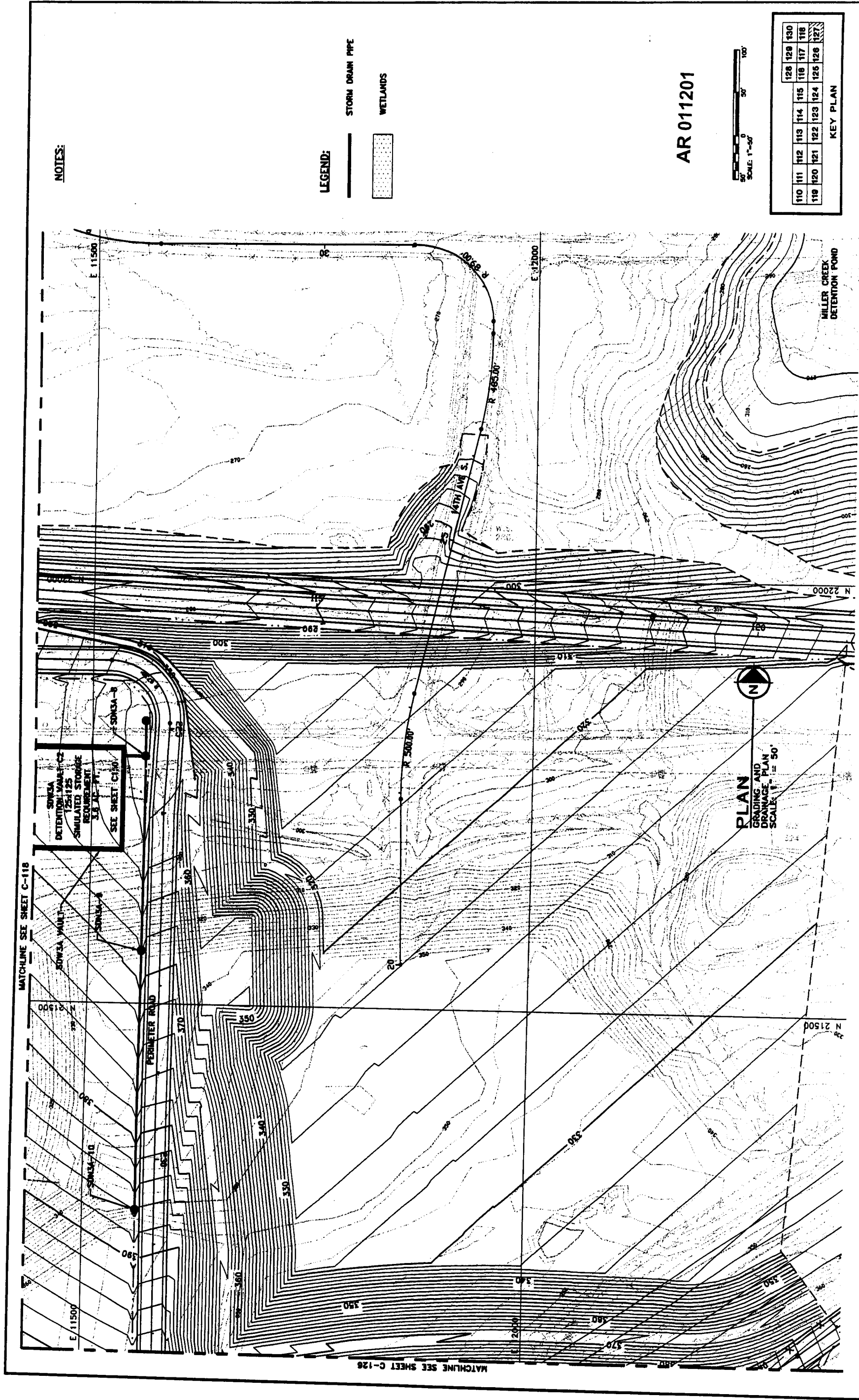
MATCHLINE SEE SHEET C-126

MATCHLINE SEE SHEET C-124

**Part of Seattle**  
**SEA-TAC INTERNATIONAL AIRPORT**  
 PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 5  
 SHEET TITLE: **GRADING AND DRAINAGE PLAN**

DATE: DEC. 15, 2000  
 CONSULTANT'S NO.  
 PART OF SEATTLE NO.  
 EXHIBIT: C125





NOTES:

LEGEND:  
 ——— STORM DRAIN PIPE  
 [Patterned Box] WETLANDS

AR 011201



KEY PLAN														
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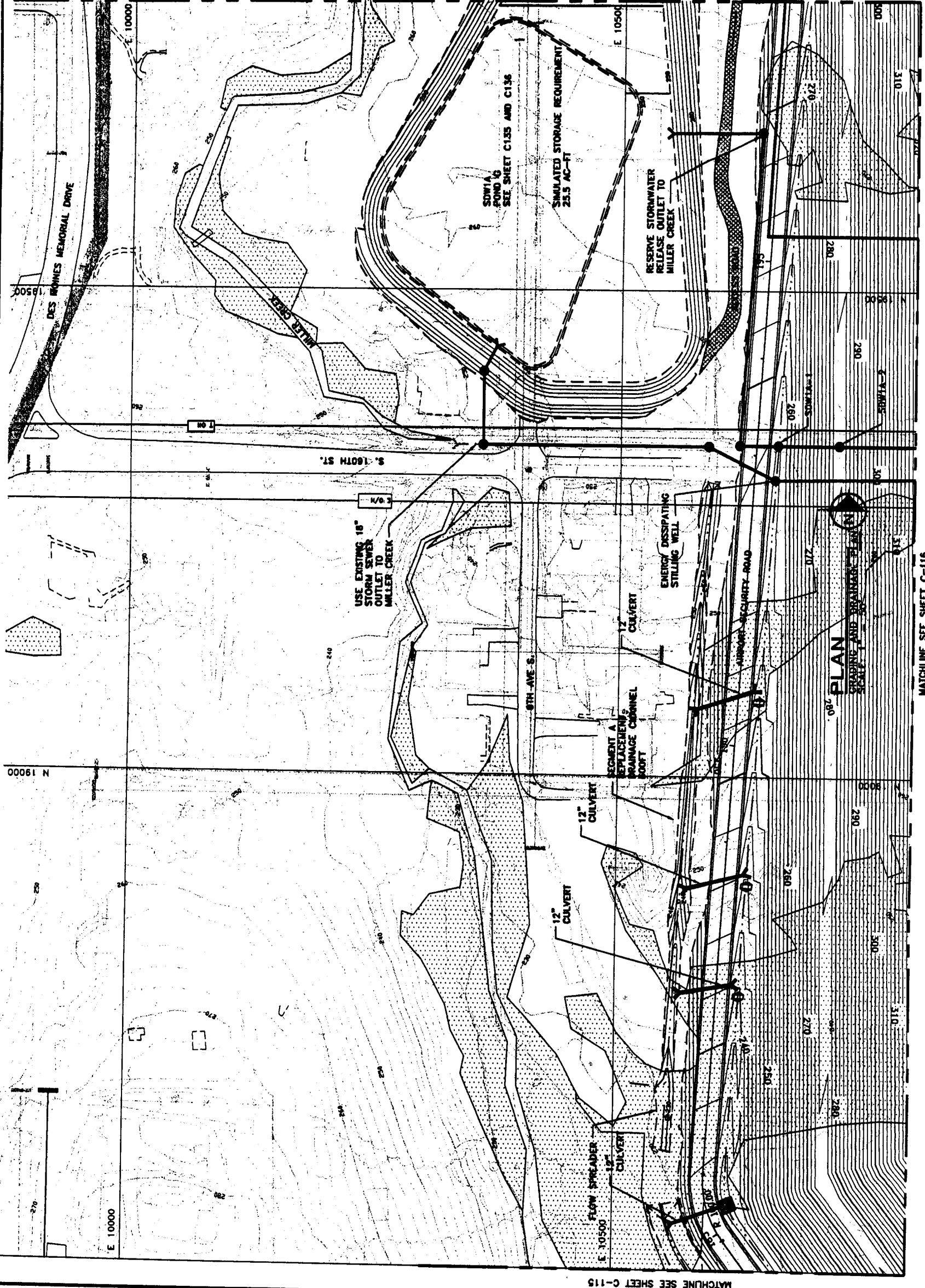
**Port of Seattle**  
 SEA-TAC INTERNATIONAL AIRPORT  
 PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6  
 SHEET TITLE: **GRADING AND DRAINAGE PLAN**  
 DATE: JUNE 11, 2001  
 DRAWN BY: [Name]  
 CHECKED BY: [Name]  
 EXHIBIT: C127

July 2001  
 556-2912-001 (28)

PLAN  
 GRADING AND  
 DRAINAGE PLAN  
 SCALE: 1" = 50'

MATCHLINE SEE SHEET C-118

MATCHLINE SEE SHEET C-126



NOTES:

LEGEND:

- STORM DRAIN PIPE
- ▨ WETLANDS

AR 011202



KEY PLAN

128	129	130
110	111	112
113	114	116
118	120	121
122	123	124
126	127	

**Port of Seattle**  
 SEA-TAC INTERNATIONAL AIRPORT  
 PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6  
 SHEET TITLE: **GRADING AND DRAINAGE PLAN**

July 2001  
 556-2912-001 (28)

MATCHLINE SEE SHEET C-116

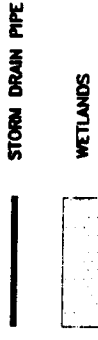
**PLAN**  
 GRADING AND DRAINAGE PLAN  
 SCALE: 1"=50'

DATE: JUNE 28, 2001  
 CONSULTANT'S SEAL  
 PART OF RECORD PLAN  
 EXHIBIT C.128




NOTES:

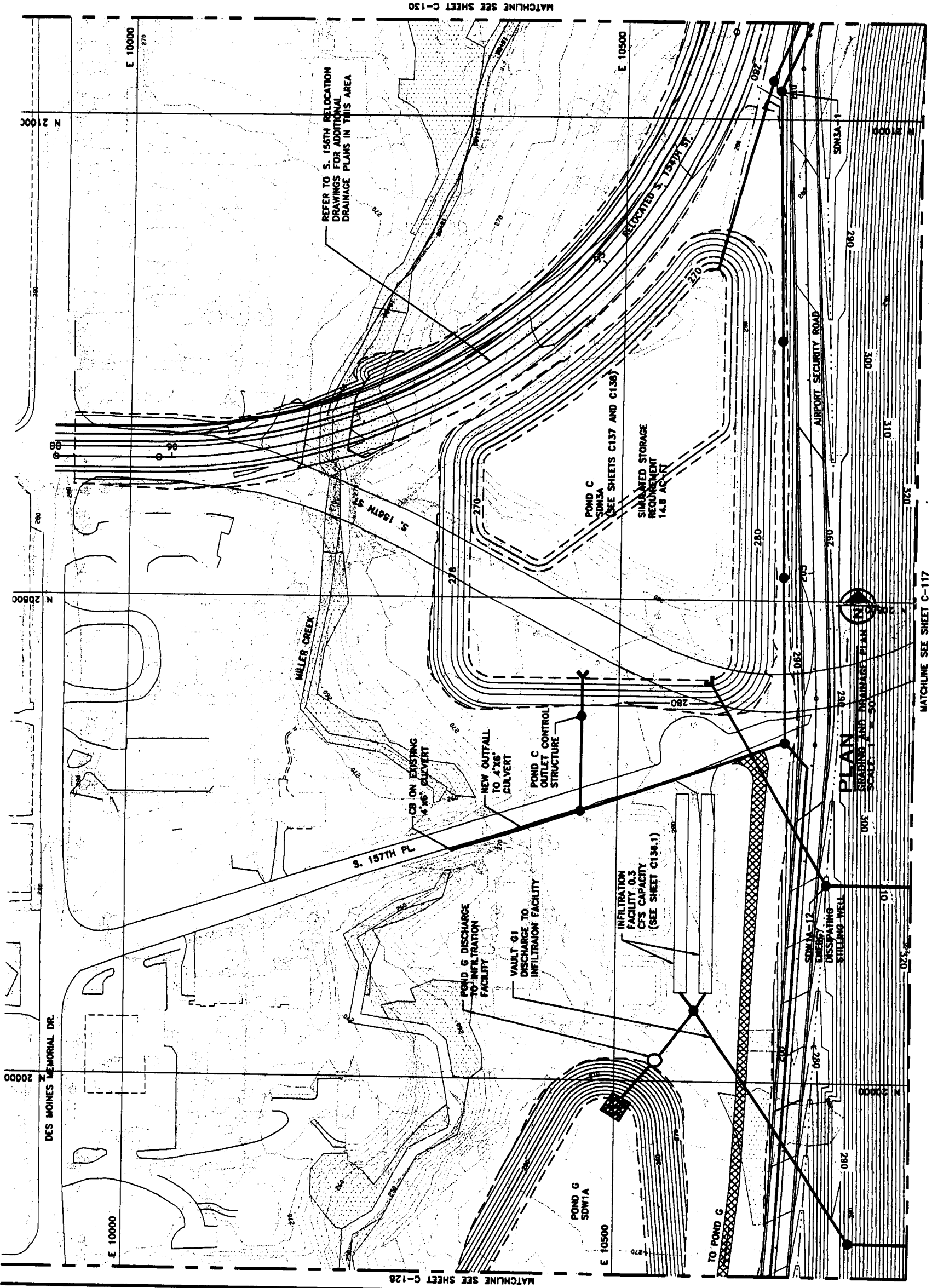
LEGEND:



AR 011203


**Port of Seattle**  
 SEA-TAC INTERNATIONAL AIRPORT  
 PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6  
 SHEET TITLE: GRADING AND DRAINAGE PLAN  
 DATE: JUNE 11, 2001  
 CONTRACTOR'S NO.:  
 PART OF MAPLE NO.:  
 EXHIBIT: C 129

July 2001  
 556-2912-001 (28)



REFER TO S. 155TH RELOCATION DRAWINGS FOR ADDITIONAL DRAINAGE PLANS IN THIS AREA

POND C  
SDW13A  
(SEE SHEETS C137 AND C138)

SIMULATED STORAGE REQUIREMENT 14.8 AC FT

MILLER CREEK

S. 157TH PL

PLAN  
SCALE: 1"=50'

POND G  
SDW1A

POND G DISCHARGE TO INFILTRATION FACILITY

VAULT G1 DISCHARGE TO INFILTRATION FACILITY

INFILTRATION FACILITY 0.3 CFS CAPACITY (SEE SHEET C136.1)

SDW12-12  
EMERGENCY  
DISPATCHING  
STREETING WELL

MATCHLINE SEE SHEET C-130

MATCHLINE SEE SHEET C-117

DES MOINES MEMORIAL DR.

E 10000

E 10000

E 10500

E 10500

N 21000

N 28500

N 20000

E 10000

E 10500

N 21000

N 28500

N 20000

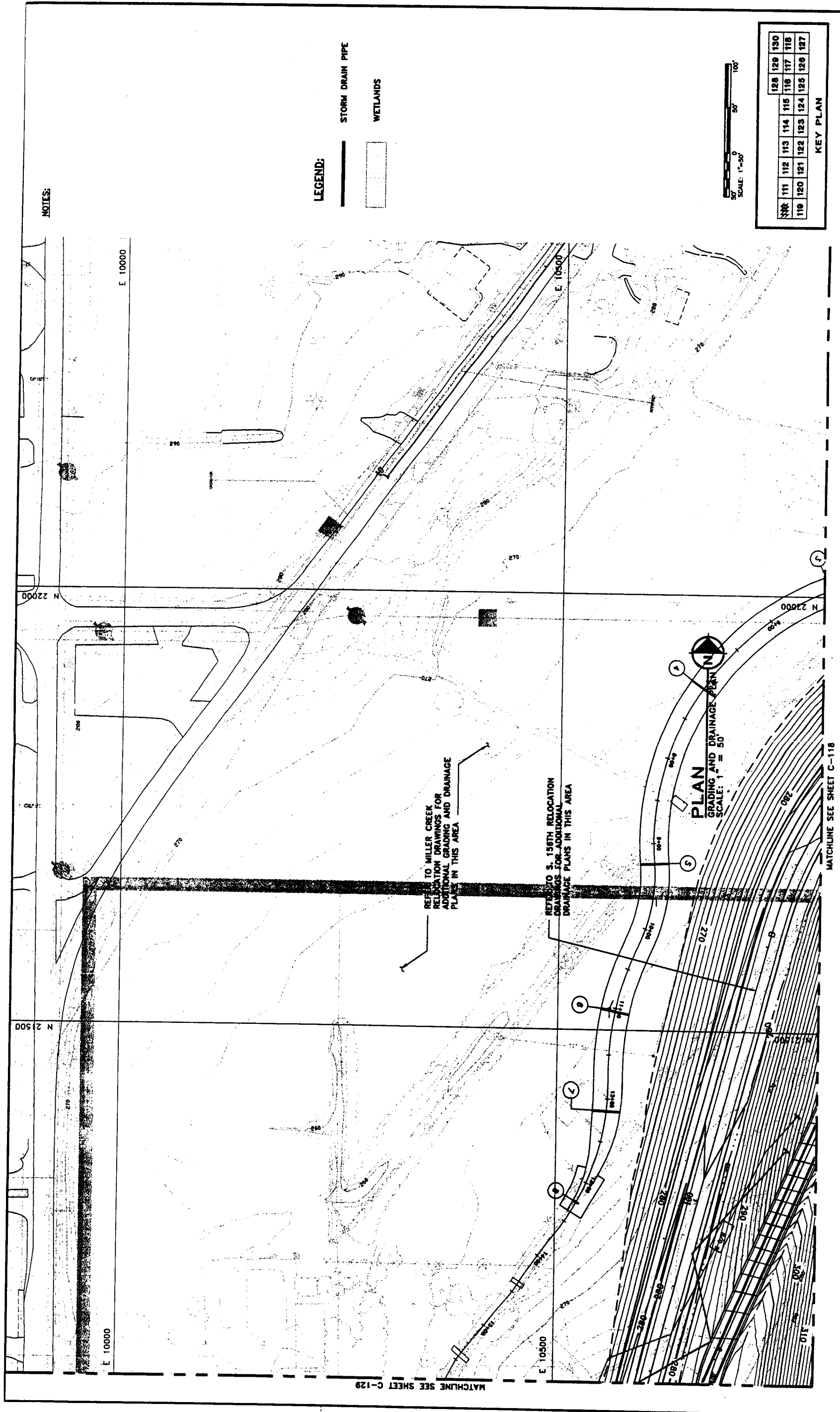
E 10000

E 10500

N 21000

N 28500

N 20000



NOTES:

**LEGEND:**

— STORM DRAIN PIPE

▨ WETLANDS

SCALE: 1" = 50'

50' 100'

**KEY PLAN**

109	111	112	113	114	115	116	117	118
119	120	121	122	123	124	125	126	127
						128	129	130

**Port of Seattle**  
SEA-TAC INTERNATIONAL AIRPORT  
PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6  
SHEET TITLE: **GRADING AND DRAINAGE PLAN**

DATE: JUNE 11, 2001  
DESIGNER: GRIFFIN & CO.  
DRAWN BY: M. WATKINS, INC.  
EXHIBIT: C130

July 2001  
556-2912-001 (28)

**APPENDIX W**  
**ENERGY DISSIPATION DESIGN CRITERIA AND PARAMETERS**

# Seattle Tacoma International Airport Third Dependent Runway Energy Dissipation

## Design Criteria and Parameters

### Background and History

The subject of energy dissipation at the bottom of slopes is a very important issue for the Port of Seattle for both the long-term stability and maintenance of the embankment and for protection of the adjacent natural resources. Erosion at the toe of the slope will be avoided through a design approach that is based on national and regional experience with these facilities.

The design has been developed following the guidelines in the U.S. Department of Transportation, Federal Highway Administration, Hydraulic Engineering Circular No 14 titled "Hydraulic Design of Energy Dissipaters for Culverts and Channels" September 1983 (referenced as "Circular No.14" below). This is a current energy dissipation design reference containing 18 possible alternatives to address a wide range of design conditions. These alternatives draw from the wide range of modeling and operation experience of the Corps of Engineers, U.S. Bureau of Reclamation, U.S. Forest Service, Colorado State University, and others around the nation.

Though storm sewer designs are typically based on the peak flow for the 10-year, 24-hour design storm, the design of the permanent energy dissipation will be based on the 100-year, 24-hour design storm.

Phased construction of the runway embankment included provisions for energy dissipation in the form of corrugated, high density polyethylene pipe anchored to the surface of the embankment down to a short run on a flat slope before discharging to a riprap lined ditch. Drops have been down 2 (horizontal) to 1 (vertical) slopes as high as 90 feet. At least one design 10-year recurrent storm event storm was exceeded during the time that those pipes have been in place and demonstrated satisfactory performance of those temporary conveyance systems.

### Proposed Measures

Circular No.14 primarily focuses on dissipating energy in culvert to channel transitions and in steep channels. The embankment project has steep slope storm sewer discharges, down the typically 2:1 embankment slopes, either directly to detention facilities or to flat sloped closed systems at the toe of the slope. Refer to the attached table for a summary of the design data relative to the stormwater discharge points.

The general approach has evolved into having a closed stormwater conveyance system that carries the flow down the slope for energy dissipation. The down-slope pipe will meet the guidelines as provided in the 1998 King County Surface Water Design Manual (Table 4.2.1.A Maximum Slopes and Velocities).



Where the storm sewer discharges directly to detention facilities, erosion at the bottom of the earthen pond will be prevented through the following design approaches:

- Natural or Forced Hydraulic Jumps
- Impact Basins
- Stilling Wells
- Riprap
- Roughness Elements to Increase Pipe Resistance Near the Outlet  
(combined with one of the measures above)

Where the steep storm sewer pipe discharges to flat sloped closed systems, manhole lid blow-off and erosion will be prevented through the following design approaches:

- Impact Basins
- Stilling Wells
- Roughness Elements to Increase Pipe Resistance Near the Outlet  
(combined with one of the measures above)

Each of these design approaches is feasible and constructable using the Circular No. 14 guidelines. Selection will be based on the physical space available, cost and ease of maintenance for each location.

#### Hydrology

Peak discharge rates for the 100-year, 24-hour design storm has been calculated based on the proposed pavement, airfield grading, and storm sewer layout. The calculated peak discharge rates are set and not likely to change more than 5 percent in final design. The Governor certified drainage boundaries set the total contributing areas for most of the basins.

Two subbasins in Millar Creek (SDW1A and SDN3A) are combined in the calculations. This is the worst case scenario for the outlet to Pond G. The likely alternate design would have runoff from subbasin SDN3A conveyed to Pond C within a closed stormwater conveyance system that will be located within the Security Road with a slope of 15 percent. If this conveyance alternate is selected the flow rate would reduce in the Pond G system and the slope transition within the Security Road is gradual enough that energy dissipation is not seen a significant issue.

## Conclusion

For the purposes of permit review, calculation are attached to demonstrate that the Corp of Engineers Stilling Well, as described in Chapter X-B of Circular No. 14 (attached for the reviewer's convenience), will work in the three locations where energy dissipation is a concern. This approach was selected for its simplicity, satisfactory operation for a wide range of flows (up to  $Q/D^{5/2} < 10.0$ ), and ease of construction. King County has also indicated that they have had good experience with this system in this region. The Port may further refine or substitute another energy dissipation concept during final design to provide an equal or superior energy dissipation system.

Existing construction methods are operating satisfactorily and future construction contracts will include energy dissipation features. Construction staging could potentially place these features earlier to ensure that they are in place at the base of the embankment to enhance the corrugated HDPE pipe and ditch lining provisions for higher slope heights. Circular No. 14 notes that the Corp of Engineers Stilling Well "may also be useful as a temporary erosion control device during construction" and that it "will operate with moderate to high concentrations of sand and silt" (p. X-B-1). This ability for service as a temporary construction device will be a factor in the selection of the energy-dissipating device.

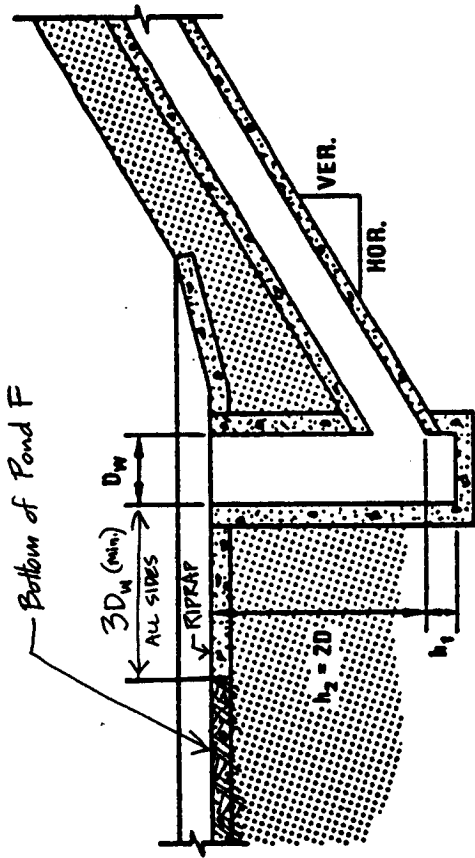
**SEA-TAC THIRD RUNWAY EMBANKMENT CONSTRUCTION  
STORMWATER DISCHARGE DOWN SLOPE**

Basin	Receiving Waters	Pond Name	Runway Baseline	Contributing Area (Ac)	100-year Height of		Slope (Ft/Ft)	Downstream Condition
					Flowrate (cfs)	Drop (ft)		
SDS7	Des Moines Creek	SDS7	106	91.3	41.7	13	0.155	Closed system (180' flat slope) to detention vault. No energy dissipation issues anticipated.
SDW2	Walker Creek	F	153	39.8	17.5	18	0.133	Direct discharge to detention pond. Energy dissipation issues potential addressed at bottom of pond.
SDW1B	Miller Creek	D	167	82.2	36.3	26	0.500	Closed system (100' flat slope) to detention pond. Energy dissipation issues potential address at base of 2:1 slope.
SDW1A & SDN3A	Miller Creek	G & C	197	64.0	32.8	140	0.500	Closed system (430' flat slope) to detention pond. Energy dissipation issues potential addressed at base of 2:1 slope for worst case scenario single down-slope discharge.

SDWZ

STILLING WELL - Corps of Engineers

10-B



D	Q	Slope	D <sub>W</sub>	h <sub>1</sub> /D <sub>W</sub>	h <sub>1</sub>	h <sub>2</sub>	h <sub>W</sub>
24"	17.5	0.133	2D use 48"	0.15	7" use 12"	48"	5'

WITH 24"  $\phi$ , ENTRANCE CONTROLS BACKWATER

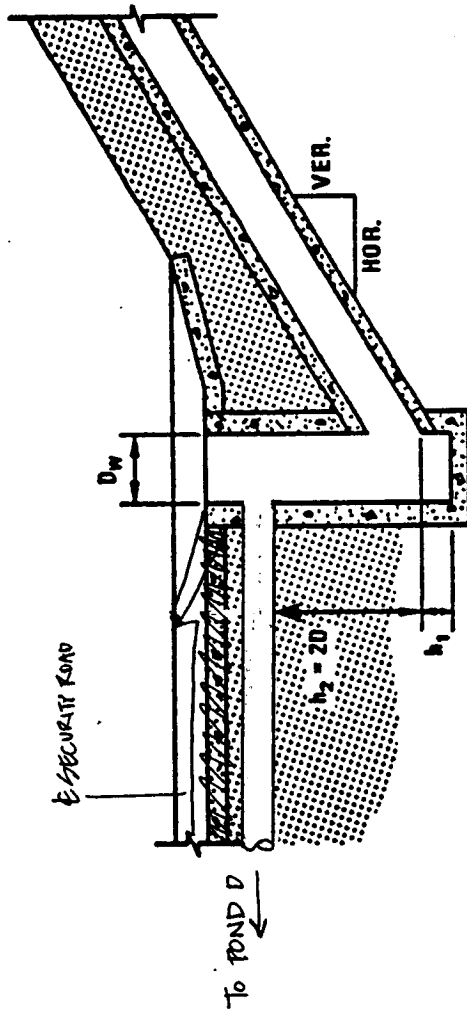
PER 1998 SWDM FIGURE 4.3.1.B,  $HW/D = 1.27 \Rightarrow HW = 2.5 \text{ ft.}$

CHECK  $Q/\phi^{5/2} < 10.0$ :  $17.5/\phi^{5/2} = 3.09 < 10.0 \therefore$  WITHIN DESIGN LIMITS

SDWIB

STILLING WELL - Corps of Engineers

10-B



D	Q	Slope	DW	$h_1/D_w$	$h_1$	$h_2$	$h_w$
30"	36.3	2:1 0.500	2.5D 6.25' use 6.5'φ	0.42	2.73' use 3'	5'	8'

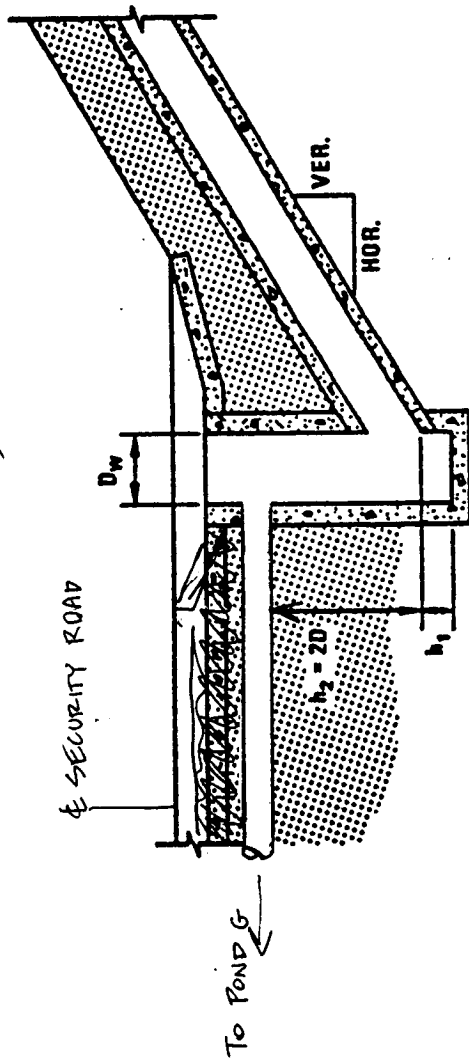
WITH 30" φ, ENTRANCE CONTROLS BACKWATER  
 PER 1998 SWDM FIGURE 4.3.1.8,  $HW/D = 1.6 \Rightarrow HW = 3.75'$

CHECK CRITICAL DEPTH IN PIPE: 2.1' < 2.5' ✓

PIPE OUT  $Q = 36.3$   $V = 3$  fps min.  
 USE 36" @  $S = 0.005$ : Capacity = 47.2 cfs  $V = 6.67$   
 USE STANDARD RIPRAP TREATMENT AT DISCHARGE TO POND D

COMBINED SDWIA & SDN3A STILLING WELL - Corps of Engineers

(INDIVIDUAL DISCHARGES WOULD BE SMALLER)



D	Q	Slope	DW	$h_1/D_w$	$h_1$	$h_2$	$h_w$
30"	32.8	2:1 0.500	2D 5'	0.42	2.1'	2.5'	4.6'

WITH 30"  $\phi$ , ENTRANCE CONTROLS BACKWATER  
 PER 1998 SWDM FIGURE 4.3.1.B,  $HW/D = 1.45 \Rightarrow HW = 3.63'$

CHECK CRITICAL DEPTH IN PIPE:  $1.9' < 2.5' \checkmark$

PIPE OUT  $Q = 32.8 \quad v = 3 \text{ fps min.}$

USE 36" @ 5 = 0.005 : CAPACITY = 47.2 cfs  $v = 6.67$   
 USE STANDARD RIP RAP TREATMENT AT DISCHARGE TO POND G

<b>HNTB</b> The HNTB Companies	Made by <i>A.D. Black</i>	Date <i>8/10/00</i>	Job Number <i>20764-DS-001</i>
	Checked by <i>RCE/Slp</i>	Date <i>8/16/00</i>	Sheet Number
Calculations For <i>SEA 3rd Runway</i>	Backchecked by	Date	<i>1</i>

SDS2 Total Area = 3975027 sq ft = 91.25 Ac. (CADD measure)

Runway Pavement = 1553770 - 107239 = ~~1446531 sq ft = 33.20 Ac.~~  
 + 121120 = 1,567,651 sq ft.

Perimeter Road Pmt =  $[(1200 - 11000) + (14935 - 10900)] 32 = 173920 \text{ sq. ft.}$   
 Access Rd Pmt 1650' x 24' = 39,840

Total Proposed Impervious = 1567651 + 173920 = ~~1741571 sq ft = 40 Ac~~  
 1781411 40.9 Ac

T<sub>c</sub> calculation

Path through inlet SDS7-29A

150' sheet flow on TW-N	S = 0.01	250
100' sheet flow on infield	S = 0.0150	370
330' intermittent on infield	S = 0.0150	
3500' strm. swr conveyance	S = 0.0077	3250

Path through inlet SDS7-18

160' sheet flow on runway	S = 0.01
100' sheet flow on w. grass	S = 0.015
90' intermittent " "	S = 0.015
200' ditch conveyance	S = 0.0077
4150' strm. swr. conveyance	S = 0.0077

Seattle Tacoma International Airport

Third Runway Hydrology / Hydraulics

Energy Dissipation

=====

BASIN SUMMARY

BASIN ID: SDS7                    NAME:

SBUH METHODOLOGY

TOTAL AREA.....:	92.15 Acres	BASEFLOWS:	0.00 cfs	
RAINFALL TYPE.....:	TYPE1A		PERV	IMP
PRECIPITATION.....:	4.00 inches	AREA...:	51.25 Acres	40.90 Acres
TIME INTERVAL.....:	10.00 min	CN.....:	85.00	98.00
		TC.....:	48.22 min	36.61 min

ABSTRACTION COEFF: 0.20

TcReach - Sheet L: 250.00 ns:0.1500 p2yr: 2.00 s:0.0150

TcReach - Shallow L: 370.00 ks:11.00 s:0.0150

TcReach - Channel L:3250.00 kc:42.00 s:0.0077

impTcReach - Sheet L: 150.00 ns:0.0110 p2yr: 2.00 s:0.0100

impTcReach - Sheet L: 100.00 ns:0.1500 p2yr: 2.00 s:0.0150

impTcReach - Shallow L: 330.00 ks:11.00 s:0.0150

impTcReach - Channel L:3500.00 kc:42.00 s:0.0077

PEAK RATE: 41.71 cfs VOL: 23.33 Ac-ft TIME: 490 min



<b>HNTB</b> The HNTB Companies	Made by <u>A. D. Black</u>	Date <u>8/10/00</u>	Job Number <u>20764-DS-001</u>
	Checked by <u>RC Elslip</u>	Date <u>8/16/00</u>	Sheet Number
Calculations For <u>SEA 3<sup>rd</sup> Runway</u>	Backchecked by	Date	<u>2</u>

SDW2 Total Area =  $\frac{194,898 \text{ sq ft}}{1735.838 \text{ sq ft}} = \frac{44.6 \text{ Ac.}}{39.8 \text{ Ac.}}$  (CADD measure)

Runway Pmt =  $412 \times 228 \text{ sq ft}$   
 Perimeter Rd Pmt =  $(14950 - 14935) \times 32 = 32,480 \text{ sq ft}$

Total Imp. =  $412,228 + 32,480 = 444,708 \text{ sq ft} = 40.2 \text{ Ac.}$   
 + Access Rd Pmt =  $900 \times 24' = 21,600$   
 $\rightarrow 466,308 \text{ sq ft} = 10.7 \text{ Ac.}$

Tc Calculation - through SDW2-SA

Pervious Area  
 200' sheet flow on infield  $S = 0.015$   
 470' intermittent flow on infield  $S = 0.015$   
 265' ditch flow e. of access road  $S = 0.0077$   
 2030' stm swr conveyance  $S = 0.005$

Impervious Area

200' sheet flow on runway  $S = 0.01$   
 190' intermittent flow on infield  $S = 0.015$   
 250' ditch flow w. of access road  $S = 0.0077$   
 2000' stm swr conveyance  $S = 0.005$

Seattle Tacoma International Airport

Third Runway Hydrology / Hydraulics

Energy Dissipation

=====
BASIN SUMMARY

BASIN ID: SDW2 NAME:
SBUH METHODOLOGY
TOTAL AREA.....: 39.80 Acres BASEFLOWS: 0.00 cfs
RAINFALL TYPE....: TYPE1A PERV IMP
PRECIPITATION....: 4.00 inches AREA...: 29.10 Acres 10.70 Acres
TIME INTERVAL....: 10.00 min CN.....: 85.00 98.00
TC.....: 44.38 min 18.94 min
ABSTRACTION COEFF: 0.20
TcReach - Sheet L: 200.00 ns:0.1500 p2yr: 2.00 s:0.0150
TcReach - Shallow L: 470.00 ks:11.00 s:0.0150
TcReach - Channel L: 265.00 kc:17.00 s:0.0077
TcReach - Channel L:2030.00 kc:42.00 s:0.0050
impTcReach - Sheet L: 200.00 ns:0.0110 p2yr: 2.00 s:0.0100
impTcReach - Shallow L: 190.00 ks:11.00 s:0.0150
impTcReach - Channel L: 250.00 kc:17.00 s:0.0177
impTcReach - Channel L:2000.00 kc:42.00 s:0.0050
PEAK RATE: 17.51 cfs VOL: 9.32 Ac-ft TIME: 480 min

<b>HNTB</b> The HNTB Companies	Made by <i>A.D. Black</i>	Date <i>8/10/00</i>	Job Number <i>20764-DS-001</i>
	Checked by <i>MC Elship</i>	Date <i>8/16/00</i>	Sheet Number
Calculations For <i>SEA 3rd Runway</i>	Backchecked by	Date	<i>3</i>

SDW1B

Total Area = ~~4,219,143 sq ft~~ = ~~96.9 Ac.~~  
~~3,579,820 sq ft~~ = 82.2 Ac. (plateau only)

Runway Pavt. = 242428 + 809733 = 1052161 sq ft ~~2142~~

Perimeter Rd Pavt = (18720 - 1595) 32 = 88640 sq ft

~~Total Pavt~~

Access Rd Pavt = (1450 + 820) 24 = 54480

Total Imp. Area = 1,195,281 sq ft = 27.4 Ac.

Ic calculation

Pervious Area (through SDW1B-12)

250' sheet flow on infield	S = 0.015
300' intermittent shallow on infield	S = 0.015
2720' stm swr conveyance	S = 0.0077
700' stm swr conveyance	S = 0.005

Imperious Area

300' sheet flow on TW E	S = 0.02
650' ditch flow s. of TW E	S = 0.02
2720' stm swr $\beta$ to Runway	S = 0.0077
700' stm swr $\ominus$	S = 0.005

=====

BASIN SUMMARY

BASIN ID: SDW1B                    NAME:

SBUH METHODOLOGY

TOTAL AREA.....:	82.20 Acres	BASEFLOWS:	0.00 cfs	
RAINFALL TYPE....:	TYPE1A		PERV	IMP
PRECIPITATION....:	4.00 inches	AREA...:	54.80 Acres	27.40 Acres
TIME INTERVAL....:	10.00 min	CN.....:	85.00	98.00
		TC.....:	48.88 min	24.43 min

ABSTRACTION COEFF: 0.20

TcReach - Sheet L: 250.00 ns:0.1500 p2yr: 2.00 s:0.0150

TcReach - Shallow L: 300.00 ks:11.00 s:0.0150

TcReach - Channel L:2720.00 kc:42.00 s:0.0077

TcReach - Channel L: 700.00 kc:42.00 s:0.0050

impTcReach - Sheet L: 300.00 ns:0.0110 p2yr: 2.00 s:0.0200

impTcReach - Channel L: 650.00 kc:17.00 s:0.0200

impTcReach - Channel L:2720.00 kc:42.00 s:0.0077

impTcReach - Channel L: 700.00 kc:42.00 s:0.0050

PEAK RATE: 36.31 cfs VOL: 19.82 Ac-ft TIME: 490 min

<b>HNTB</b> The HNTB Companies	Made by A.D. Black	Date 8/10/00	Job Number 20764-DS-001
	Checked by <i>RC Elslip</i>	Date 8/16/00	Sheet Number
Calculations For <i>SEA 3rd Runway</i>	Backchecked by	Date	4

SDWIA + SDN3A

Total Area = 2073640 + 716341  
2789981 sq ft = 64.0 Ac

SDWIA Runway Prmt 945093 - 106818 = 838,275 sq ft

SDN3A Runway Prmt. 142825 sq ft.

Perimeter Rd Prmt.  $(20850 - 18720) \cdot 52 = 68,160 \text{ sq ft}$

$(29500 - 20850) \cdot 32 = 78,400 \text{ sq ft}$

Access Rd. Prmt.  $1000' \cdot 24' = 24,000 \text{ sq ft.}$

Total Imp. Area = 1,151,660 sq ft = 26.4 Ac.

To Calc Path through SDN3A-7

<u>Infiltration</u>	
210' sheet flow on Blast Pad	8%
40' sheet flow off Blast Rd on grass	8%
260' Shallow flow on infield	8%
240' Ditch (grass) to SDN3A-7	7%
3,230 pipe flow	0.77%

Permeability

<del>350</del> 250	Sheet flow	8%
70	Shallow flow	8%
120	Ditch flow	8%
1080	Pipe flow	0.5%
2450	Pipe flow	0.77%





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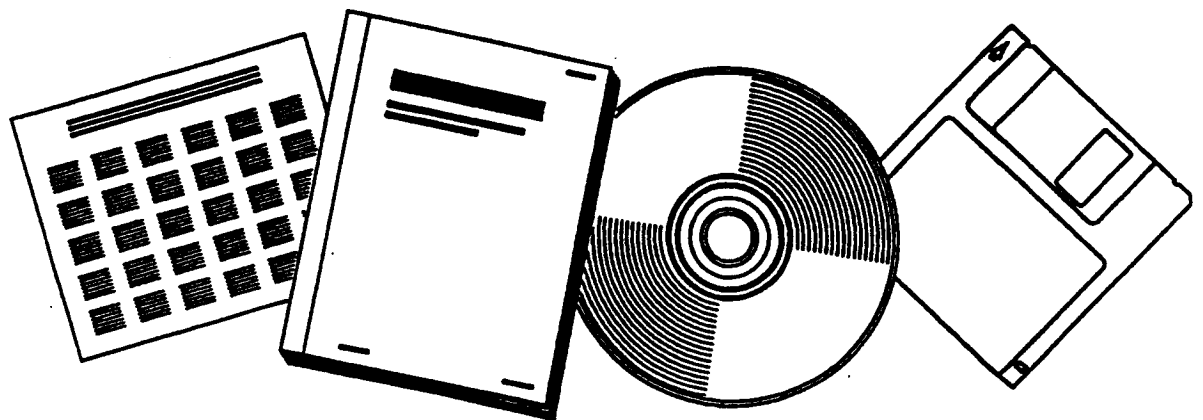
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# HYDRAULIC DESIGN OF ENERGY DISSIPATORS FOR CULVERTS AND CHANNELS

FEDERAL HIGHWAY ADMINISTRATION  
WASHINGTON, DC

SEP 83



U.S. DEPARTMENT OF COMMERCE  
National Technical Information Service

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AR 011221

## X-B. CORPS OF ENGINEERS STILLING WELL

The design of this type of stilling well energy dissipator is based on model tests conducted by the Corps of Engineers. (X-B-1 and 2)

The dissipator has application where debris is not a serious problem. It will operate with moderate to high concentrations of sand and silt but is not recommended for areas where quantities of large floating or rolling debris is expected unless suitable debris-control structures are utilized. Its greatest potential, as far as highways are concerned, is at the outfalls of storm drains, median, and pipe down drains where little debris is expected. It may also be useful as a temporary erosion control device during construction.

### Design Recommendations

The design is straightforward. Once the size and discharge of the incoming pipe are determined, figure X-B-1 is used to select the stilling well diameter ( $D_w$ ). The model tests indicated that satisfactory performance can be maintained for  $Q/D^{5/2}$  ratios as large as 10.0, with stilling well diameters of one, two, three, and five times that of the incoming conduits. These ratios were used to define the curves shown in figure X-B-1.

The tests also indicated that there is an optimum depth of stilling well below the invert of the incoming pipe. This depth is determined by entering figure X-B-2 with the slope of the incoming pipe and using the stilling well diameter ( $D_w$ ) previously obtained from figure X-B-1.

The height of the stilling well above the invert is fixed at twice the diameter of the incoming pipe ( $2D$ ). This dimension results in satisfactory operation and is practical from a cost standpoint; however, if increased, greater efficiency will result.

Tailwater also increases the efficiency of the stilling well. Whenever possible, it should be located in a sump or depressed area.

It is recommended that riprap or other types of channel protection be provided around the stilling well outlet and for a distance of at least  $3D_w$  downstream.

X-B-1



The outlet may also be covered with a screen or grate for safety. However, the screen or grate should have a clear opening area of at least 75 percent of the total stilling well area and be capable of passing small floating debris such as cans and bottles.

#### Design Procedures

- (1) Select approach pipe diameter (D) and discharge (Q).
- (2) Obtain well diameter ( $D_w$ ) from figure X-B-1.
- (3) Calculate the culvert slope = (Vertical/horizontal distance). The depth of the well below the culvert invert ( $h_1$ ) is determined from figure X-B-2.
- (4) The depth of the well above the culvert invert ( $h_2$ ) is equal to 2(D) as a minimum but may be greater if the site permits.
- (5) The total height of the well ( $h_w$ ) =  $h_1+h_2$ .

#### Example Problem

Given: 24" CMP downdrain on a 2:1 slope carrying a  
 $Q = 15$  cfs

Find: Stilling well dimensions

Solution:

- (1)  $D=2$  ft.,  $Q=15$  cfs
- (2) From figure X-B-1  $D_w=1.5D=3$  ft.
- (3) Slope= $1/2=.5$ ,  $h_1/D_w=.42$  from figure X-B-2  
 $h_1=.42(3.0)=1.26$  ft., Use  $h_1=1.3$  ft.
- (4)  $h_2=2(D)=2(2)=4$  ft.
- (5)  $h_w=h_1+h_2=1.3+4=5.3$  ft.

X-B-1. IMPACT-TYPE ENERGY DISSIPATOR FOR STORM-DRAINAGE  
OUTFALLS STILLING WELL DESIGN, U. S. Army Corps of  
Engineers, Technical Report No. 2-620 March 1963,  
WES, Vicksburg, Mississippi.

X-B-2. Grace, J. L., Pickering, G. A., EVALUATION OF  
THREE ENERGY DISSIPATORS FOR STORM DRAIN OUTLETS,  
U.S. Army WES, HRB 1971, Washington, D.C.

X-B-2

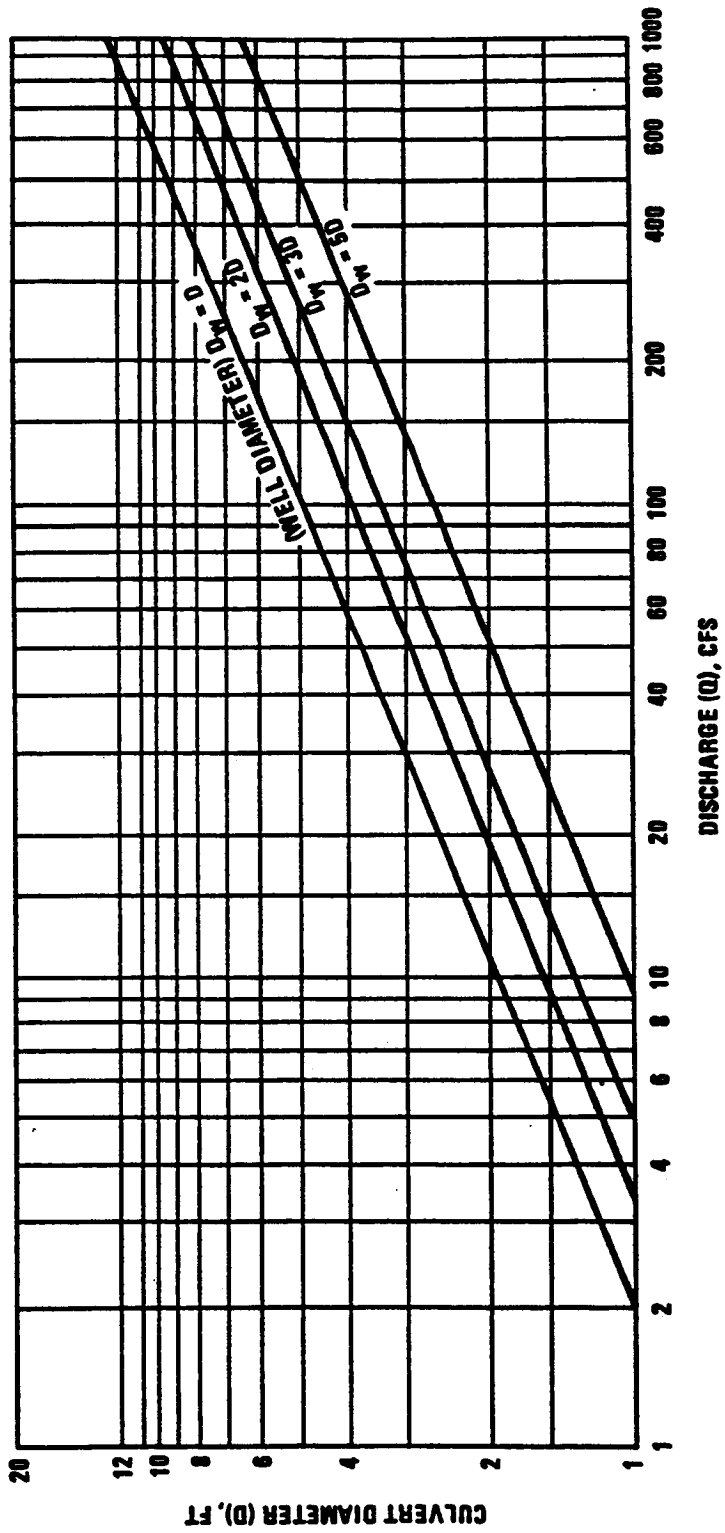


FIGURE X-B-1. STILLING WELL DIAMETER (D<sub>w</sub>) FROM REFERENCE X-B-1

X-B-3

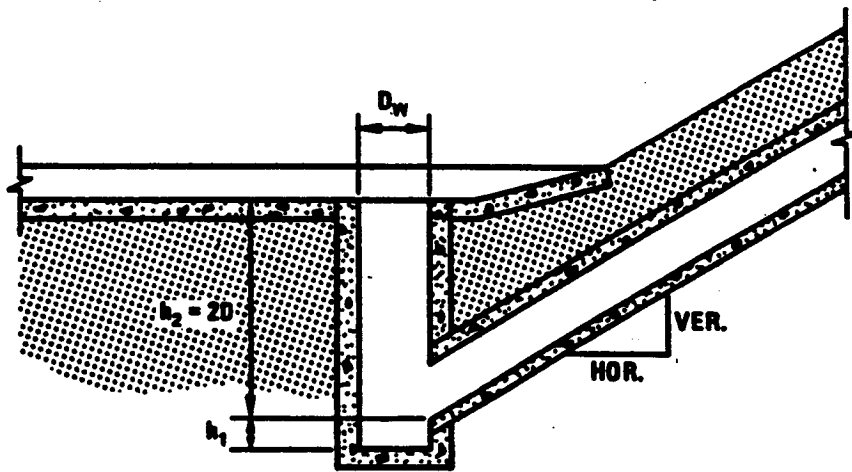
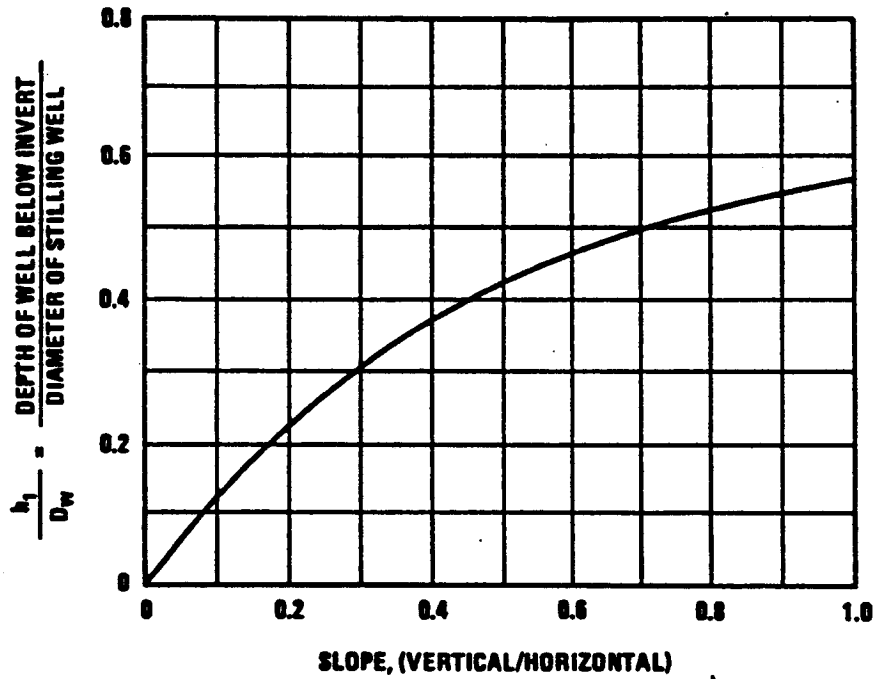


FIGURE X-B-2. STILLING WELL HEIGHT FROM REFERENCE X-B-1

X-B-4

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**APPENDIX Z**  
**IWS LAGOON STORAGE CAPACITY MODELING**

**TABLE OF CONTENTS**

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## 1. IWS CONTINUOUS SIMULATION

### 1.1 INTRODUCTION

A continuous simulation of the Industrial Waste System (IWS) was performed using the King County Runoff Time Series (KCRTS) to:

- Confirm that overflows will not occur under the future configuration of the IWS (simulated over the 50-year KCRTS period of record);
- Estimate the treatment rate at which one overflow would occur when the 50-year KCRTS period of record is simulated; and
- Estimate the treatment rate at which two overflows would occur.

### 1.2 APPROACH AND ASSUMPTIONS

The following assumptions were used for the analyses:

- With recent improvements to the Industrial Waste Water Treatment Plant (IWTP), the maximum IWS treatment and discharge rate exceeds 4.0 mgd (6.2 cfs).
- The total lagoon storage (for Lagoons 1, 2, and 3) will be increased to approximately 76.9 million gallons by the end of 2001. Lagoon 3 is currently being enlarged as an element of the All Known Available and Reasonable Treatment (AKART) alternative, which is required the Port's National Pollutant Discharge Elimination System (NPDES) Permit. The final storage of Lagoon 3 will be no less than the design volume of 72 million gallons. Upon completion, the effective volume is expected to exceed the design volume, with a final volume of 72 to 76 million gallons. The design volume of 72 million gallons was used for this analysis.
- The hydraulic capacity of the existing 18-inch outfall was determined to be at least 6.3 mgd (see Appendix O, Case 1). This exceeds the current and proposed future maximum treatment rate of 4 mgd. Additionally, approximately 75 feet of effluent line upstream of the effluent manhole was replaced in 1996 (Kennedy/Jenks 1998) and a portion of the 18-inch effluent line under Lagoon 3 is scheduled to be replaced in 2001. These improvements will increase the capacity of the outfall to 7.1 mgd (see Appendix O, Case 2).
- Land use for the IWS in 2006 is summarized in Table Z-1. This land use is conservative because pump stations were modeled to direct 100 percent of flows to the IWS; flows greater than the 6-month flow rate actually overflow from some of these facilities to the Storm Drain System (SDS). Although this analysis accounts for all major planned additions to the IWS, approximately 16 acres of additional impervious area were included in this analysis to allow for future unplanned area additions.

**Table Z-1. IWS configuration (land use, storage, treatment rate, and discharge rate).**

<b>Parameter</b>	<b>Value</b>
<b>Land Use</b>	
Till Grass	16.53 acres
Outwash Grass	8.16 acres
Airport Fill	0.01 acres
Wetland	0.01 acres
Impervious Area	<u>410.00 acres</u>
<b>TOTAL</b>	<b>434.71 acres</b>
<b>Storage Volume</b>	
Lagoon 1	1.6 mg
Lagoon 2	3.3 mg
Lagoon 3	<u>72.0 mg</u>
<b>TOTAL</b>	<b>76.9 mg = 236.0 ac-ft</b>
<b>Treatment Rate</b>	<b>4.0 mgd</b>
<b>Outfall Discharge Capacity</b>	<b>7.1 mgd</b>

## 2. IWS LAGOON STORAGE CAPACITY MODELING

### 2.1 CONFIRMATION OF ZERO OVERFLOWS

A single-outlet reservoir file was set up in KCRTS representing the total lagoon storage and treatment rate. In the reservoir file, the processing rate linearly increases to the maximum of 6.2 cfs, at which time Lagoon 1 is full (this assumption is conservative, as the maximum treatment rate is normally attained as soon as the treatment facility is started, when Lagoon 1 is less than full). After Lagoon 1 is full, the maximum processing rate was held constant at 6.2 cfs.

The IWS 2006 time series was routed through the reservoir to confirm that the peak reservoir storage was not exceeded. As shown in the attached KCRTS reservoir setup, the peak storage attained for the 50-year KCRTS period of record (208 ac-ft) does not exceed the total IWS lagoon storage capacity of 236 ac-ft, thus no overflows occur.

### 2.2 TREATMENT RATE AT WHICH ONE OVERFLOW OCCURS

To determine the treatment rate at which one overflow would occur, a double-outlet reservoir was used. The treatment rate was represented as a constant discharge from the first outlet (ramped up as described above). Overflow was represented by the second outlet's discharge, which did not occur until the pond reached full volume. The overflow discharge rate was arbitrarily set at 1.0 cfs to indicate that overflow was occurring (only a positive-negative indicator of overflow was required).

The IWS 2006 time series file was routed through the two-outlet reservoir. A trial-and-error process was performed, examining the KCRTS reservoir routing output (attached) to bracket the flow rate at which overflow would occur. This treatment rate was approximately 3.05 mgd (1.97 cfs), with overflow occurring in water year (WY) 1997.

### **2.3 TREATMENT RATE AT WHICH TWO OVERFLOWS OCCUR**

To determine the treatment rate at which two overflows would occur, a double-outlet reservoir was used. The treatment rate was represented as a constant discharge from the first outlet. Overflow was represented by the second outlet's discharge, which did not occur until the pond reached full volume. Then the overflow discharge rate was arbitrarily set at 1.0 cfs to indicate that overflow was occurring (only a positive-negative indicator of overflow was required). The KCRTS reservoir input and reservoir routing results are attached.

The IWS 2006 time series file was routed through the two-outlet reservoir. A trial-and-error flow-frequency analysis was performed on the second outlet's time series to determine the treatment rate at which overflow would occur in only one year (but not in two or more different years). This treatment rate was approximately 2.68 mgd (4.15 cfs), with overflow occurring only in WY 1956 (see attached KCRTS output and hydrograph). The hydrograph from WY 1956 was extracted and analyzed to determine how many overflows occurred during that year (only one overflow occurred).

Using flow frequency analysis and extracted hydrographs, the trial-and-error process was extended to determine the rate at which two overflows would occur in 1956, or at which overflows would occur in two separate years. Two overflows occurred during the 50-year KCRTS period of record at a treatment rate of approximately 2.35 mgd (4.10 cfs). Both overflows occurred in WY 1956 (see attached KCRTS output and hydrograph).



KCRTS TIME SERIES INPUT: IWS 2006 LAND USE

+-Land Use Summary-			
Till Forest	0.00	acres	
Till Pasture	0.00	acres	
Till Grass	16.53	acres	
Airport Fill	0.01	acres	
Outwash Forest	0.00	acres	
Outwash Pasture	0.00	acres	
Outwash Grass	8.16	acres	
Wetland	0.01	acres	
Impervious	410.00	acres	
-----			
Total Area :	434.71	acres	
Scale Factor :	1.00	Hourly	Historic
-----			
Time Series: iws2006			
-----			

**KCRTS ONE-OUTLET RESERVOIR SETUP AND RESERVOIR ROUTING RESULTS  
IWS 2006 LAGOON STORAGE AND TREATMENT RATE**

One Outlet Reservoir Routing File

Stage (Ft)	Discharge (CFS)	Storage (Cu-Ft)	Perm-Area (Sq-Ft)
0.00	0.000	0.	0.
0.10	6.190	213875.	0.
10.00	6.190	10279374.	0.

0.00 Ft : Base Reservoir Elevation  
0.0 Minutes/Inch: Average Perm-Rate

KCRTS Command

-----  
Route through a SINGLE (1) outlet Reservoir  
-----

Loading Reservoir File:iwsres.RS1 :  
Loading Time Series File:iws2006.tsf 50

Reservoir Routing [Single Outlet]

Computing Series:iws1.tsf  
Years Complete: 50

Inflow/Outflow Analysis

-----  
Peak Inflow Discharge: 168.40 CFS at 6:00 on Jan 9 in 1990  
Peak Outflow Discharge: 6.19 CFS at 2:00 on Jan 3 in 1997  
Peak Reservoir Stage: 8.81 Ft  
Peak Reservoir Elev: 8.81 Ft  
Peak Reservoir Storage: 9065779. Cu-Ft  
: 208.122 Ac-Ft

Storing Time Series File:iws1.tsf 50

Routing Complete

**KCRTS TWO-OUTLET RESERVOIR ROUTING RESULTS  
TO DETERMINE ONE OVERFLOW  
IWS 2006 LAGOON STORAGE WITH 3.03 MGD TREATMENT RATE**

KCRTS Command

-----  
Route through a DOUBLE (2) outlet Reservoir  
-----

Loading Reservoir File:iwsres2.RS2 :  
Loading Time Series File:iws2006.tsf 50

Reservoir Routing [Double Outlet]  
Computing Series:iwstreat.tsf  
and Series:iwsover.tsf  
Years Complete: 50

Inflow/Outflow Analysis  
-----

Peak Inflow Discharge:	168.40 CFS at 6:00 on Jan 9 in 1990
Peak A-Outflow Discharge:	4.70 CFS at 2:00 on Jan 3 in 1997
Peak B-Outflow Discharge:	1.00 CFS at 2:00 on Jan 3 in 1997
Peak Reservoir Stage:	10.02 Ft
Peak Reservoir Elev:	10.02 Ft
Peak Reservoir Storage:	9635228. Cu-Ft
:	221.194 Ac-Ft
	Storing Time Series File:iwstreat.tsf 50
	Storing Time Series File:iwsover.tsf 50

Routing Complete

**KCRTS TWO-OUTLET RESERVOIR ROUTING RESULTS  
TO DETERMINE ONE OVERFLOW  
IWS 2006 LAGOON STORAGE WITH 3.06 MGD TREATMENT RATE**

KCRTS Command

-----  
Route through a DOUBLE (2) outlet Reservoir  
-----

Loading Reservoir File:iwsres2.RS2 :  
Loading Time Series File:iws2006.tsf 50

Reservoir Routing [Double Outlet]

Computing Series:iwstreat.tsf  
and Series:iwsover.tsf  
Years Complete: 50

Inflow/Outflow Analysis  
-----

Peak Inflow Discharge:	168.40 CFS at 6:00 on Jan 9 in 1990
Peak A-Outflow Discharge:	4.75 CFS at 3:00 on Jan 3 in 1997
Peak B-Outflow Discharge:	0.000 CFS at 3:00 on Jan 3 in 1997
Peak Reservoir Stage:	10.00 Ft
Peak Reservoir Elev:	10.00 Ft
Peak Reservoir Storage:	9620638. Cu-Ft
:	220.859 Ac-Ft
	Storing Time Series File:iwstreat.tsf 50
	Storing Time Series File:iwsover.tsf 50

Routing Complete

**KCRTS TWO-OUTLET RESERVOIR SETUP AND RESERVOIR ROUTING RESULTS  
TO DETERMINE TWO OVERFLOWS  
IWS 2006 LAGOON STORAGE WITH 2.68 MGD TREATMENT RATE**

Two Outlet Reservoir Routing File

Stage (Ft)	Discharge (CFS)		Storage (Cu-Ft)	Perm-Area (Sq-Ft)
	A	B		
0.00	0.000	0.000	0.	0.
0.10	4.150	0.000	213875.	0.
10.00	4.150	0.000	10279374.	0.
10.01	4.150	1.000	10279375.	0.
20.00	4.150	1.000	20000000.	0.

0.00 Ft : Base Reservoir Elevation  
0.0 Minutes/Inch: Average Perm-Rate

-----  
KCRTS Command  
-----

Route through a DOUBLE (2) outlet Reservoir  
-----

Loading Reservoir File:iwsres2.RS2 :  
Loading Time Series File:iws2006.tsf 50

Reservoir Routing [Double Outlet]  
Computing Series:iwstreat.tsf  
and Series:iwsover.tsf  
Years Complete: 50

Inflow/Outflow Analysis  
-----

Peak Inflow Discharge: 168.40 CFS at 6:00 on Jan 9 in 1990  
Peak A-Outflow Discharge: 4.15 CFS at 14:00 on Jan 6 in 1956  
Peak B-Outflow Discharge: 1.00 CFS at 14:00 on Jan 6 in 1956  
Peak Reservoir Stage: 10.16 Ft  
Peak Reservoir Elev: 10.16 Ft  
Peak Reservoir Storage:10429023. Cu-Ft  
: 239.417 Ac-Ft

Storing Time Series File:iwstreat.tsf 50  
Storing Time Series File:iwsover.tsf 50

Routing Complete

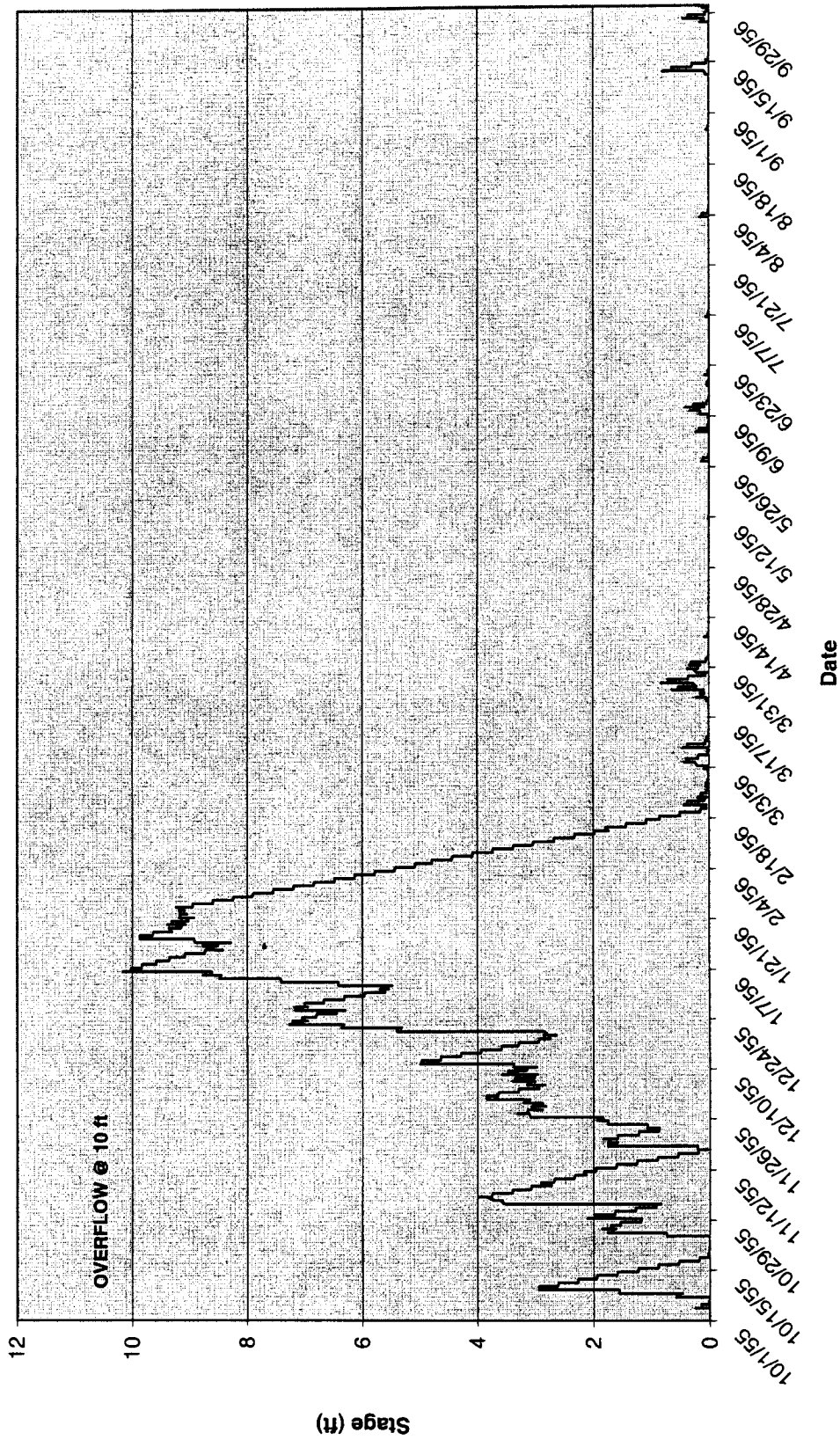
**KCRTS FLOW FREQUENCY ANALYSIS  
FOR IWS 2006 LAGOON STORAGE WITH 2.68 MGD TREATMENT RATE  
OVERFLOW IN YEAR 1956 ONLY  
(ONE OVERFLOW IN 1956; SEE THE FOLLOWING STAGE CHART)**

Flow Frequency Analysis  
Time Series File:iwsover.tsf  
Project Location:Sea-Tac DMoines

---Annual Peak Flow Rates---				-----Flow Frequency Analysis-----				
Flow Rate (CFS)	Rank	Time of Peak		- - Peaks - - (CFS)	Rank	Return Period	Prob	
0.000	42	2/22/49	22:00	1.00	10.16	1	89.50	0.989
0.000	21	1/27/50	3:00	0.000	9.59	2	32.13	0.969
0.000	4	2/10/51	20:00	0.000	9.18	3	19.58	0.949
0.000	47	2/04/52	7:00	0.000	8.92	4	14.08	0.929
0.000	5	2/07/53	9:00	0.000	8.85	5	10.99	0.909
0.000	36	1/23/54	1:00	0.000	8.69	6	9.01	0.889
0.000	38	2/08/55	7:00	0.000	8.41	7	7.64	0.869
1.00	1	1/06/56	14:00	0.000	8.19	8	6.63	0.849
0.000	31	3/10/57	4:00	0.000	7.10	9	5.86	0.829
0.000	30	1/17/58	9:00	0.000	6.91	10	5.24	0.809
0.000	40	11/24/58	8:00	0.000	6.43	11	4.75	0.789
0.000	9	12/15/59	15:00	0.000	6.31	12	4.34	0.769
0.000	19	11/24/60	18:00	0.000	6.14	13	3.99	0.749
0.000	46	12/24/61	6:00	0.000	5.88	14	3.70	0.729
0.000	26	11/30/62	20:00	0.000	5.75	15	3.44	0.709
0.000	14	11/19/63	19:00	0.000	5.75	16	3.22	0.690
0.000	12	12/01/64	10:00	0.000	5.59	17	3.03	0.670
0.000	23	1/14/66	1:00	0.000	5.44	18	2.85	0.650
0.000	16	1/28/67	8:00	0.000	5.29	19	2.70	0.630
0.000	34	1/20/68	21:00	0.000	5.26	20	2.56	0.610
0.000	33	12/11/68	10:00	0.000	4.90	21	2.44	0.590
0.000	17	1/27/70	5:00	0.000	4.85	22	2.32	0.570
0.000	28	12/10/70	20:00	0.000	4.73	23	2.22	0.550
0.000	7	3/14/72	7:00	0.000	4.73	24	2.13	0.530
0.000	10	12/27/72	21:00	0.000	4.58	25	2.04	0.510
0.000	20	12/27/73	21:00	0.000	4.50	26	1.96	0.490
0.000	41	12/27/74	8:00	0.000	4.07	27	1.89	0.470
0.000	35	12/04/75	3:00	0.000	4.04	28	1.82	0.450
0.000	43	8/26/77	8:00	0.000	4.04	29	1.75	0.430
0.000	24	12/15/77	20:00	0.000	4.04	30	1.70	0.410
0.000	50	11/19/78	10:00	0.000	3.98	31	1.64	0.390
0.000	3	12/22/79	4:00	0.000	3.95	32	1.59	0.370
0.000	27	12/30/80	23:00	0.000	3.82	33	1.54	0.350
0.000	18	10/08/81	22:00	0.000	3.64	34	1.49	0.330
0.000	29	1/08/83	6:00	0.000	3.60	35	1.45	0.310
0.000	45	11/24/83	9:00	0.000	3.55	36	1.41	0.291
0.000	37	11/11/84	9:00	0.000	3.48	37	1.37	0.271
0.000	25	1/19/86	1:00	0.000	3.42	38	1.33	0.251
0.000	13	11/27/86	1:00	0.000	3.36	39	1.30	0.231
0.000	32	12/10/87	8:00	0.000	3.34	40	1.27	0.211
0.000	39	11/25/88	2:00	0.000	3.26	41	1.24	0.191
0.000	11	1/09/90	18:00	0.000	2.93	42	1.21	0.171
0.000	8	11/24/90	19:00	0.000	2.77	43	1.18	0.151

0.000	22	2/01/92	0:00	0.000	2.68	44	1.15	0.131
0.000	48	3/23/93	9:00	0.000	2.57	45	1.12	0.111
0.000	49	2/17/94	22:00	0.000	2.52	46	1.10	0.091
0.000	15	12/27/94	21:00	0.000	2.44	47	1.08	0.071
0.000	6	2/09/96	7:00	0.000	2.33	48	1.05	0.051
0.000	2	1/03/97	3:00	0.000	1.83	49	1.03	0.031
0.000	44	10/30/97	12:00	0.000	1.67	50	1.01	0.011

IWS Overflow Analysis WY 1956  
 Treatment Rate 2.68 mgd



AR 011238



**KCRTS TWO-OUTLET RESERVOIR SETUP AND RESERVOIR ROUTING RESULTS  
TO DETERMINE TWO OVERFLOWS  
IWS 2006 LAGOON STORAGE WITH 2.65 MGD TREATMENT RATE**

Two Outlet Reservoir Routing File

Stage (Ft)	Discharge (CFS)		Storage (Cu-Ft)	Perm-Area (Sq-Ft)
	A	B		
0.00	0.000	0.000	0.	0.
0.10	4.100	0.000	213875.	0.
10.00	4.100	0.000	10279374.	0.
10.01	4.100	1.000	10279375.	0.
20.00	4.100	1.000	20000000.	0.

0.00 Ft : Base Reservoir Elevation  
0.0 Minutes/Inch: Average Perm-Rate

-----  
KCRTS Command  
-----

Route through a DOUBLE (2) outlet Reservoir  
-----

Loading Reservoir File:iwsres2.RS2 :  
Loading Time Series File:iws2006.tsf 50

Reservoir Routing [Double Outlet]  
Computing Series:iwstreat.tsf  
and Series:iwsover.tsf  
Years Complete: 50

Inflow/Outflow Analysis  
-----

Peak Inflow Discharge: 168.40 CFS at 6:00 on Jan 9 in 1990  
Peak A-Outflow Discharge: 4.10 CFS at 14:00 on Jan 6 in 1956  
Peak B-Outflow Discharge: 1.00 CFS at 14:00 on Jan 6 in 1956  
Peak Reservoir Stage: 10.43 Ft  
Peak Reservoir Elev: 10.43 Ft  
Peak Reservoir Storage:10687904. Cu-Ft  
: 245.361 Ac-Ft  
Storing Time Series File:iwstreat.tsf 50  
Storing Time Series File:iwsover.tsf 50

Routing Complete

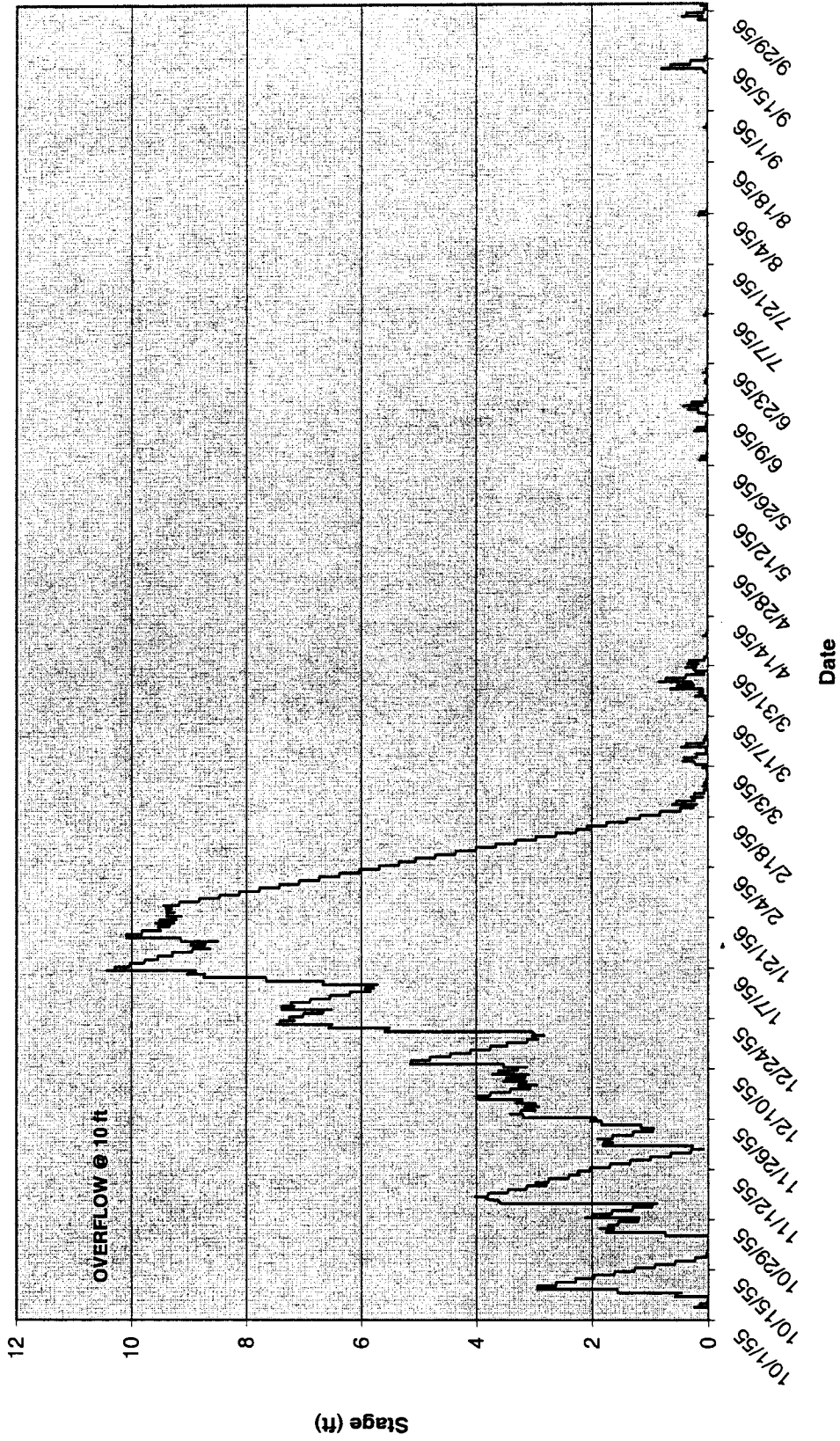
**KCRTS FLOW FREQUENCY ANALYSIS  
FOR IWS 2006 LAGOON STORAGE WITH 2.65 MGD TREATMENT RATE  
OVERFLOW IN YEAR 1956 ONLY  
(TWO OVERFLOWS IN 1956; SEE THE FOLLOWING STAGE CHART)**

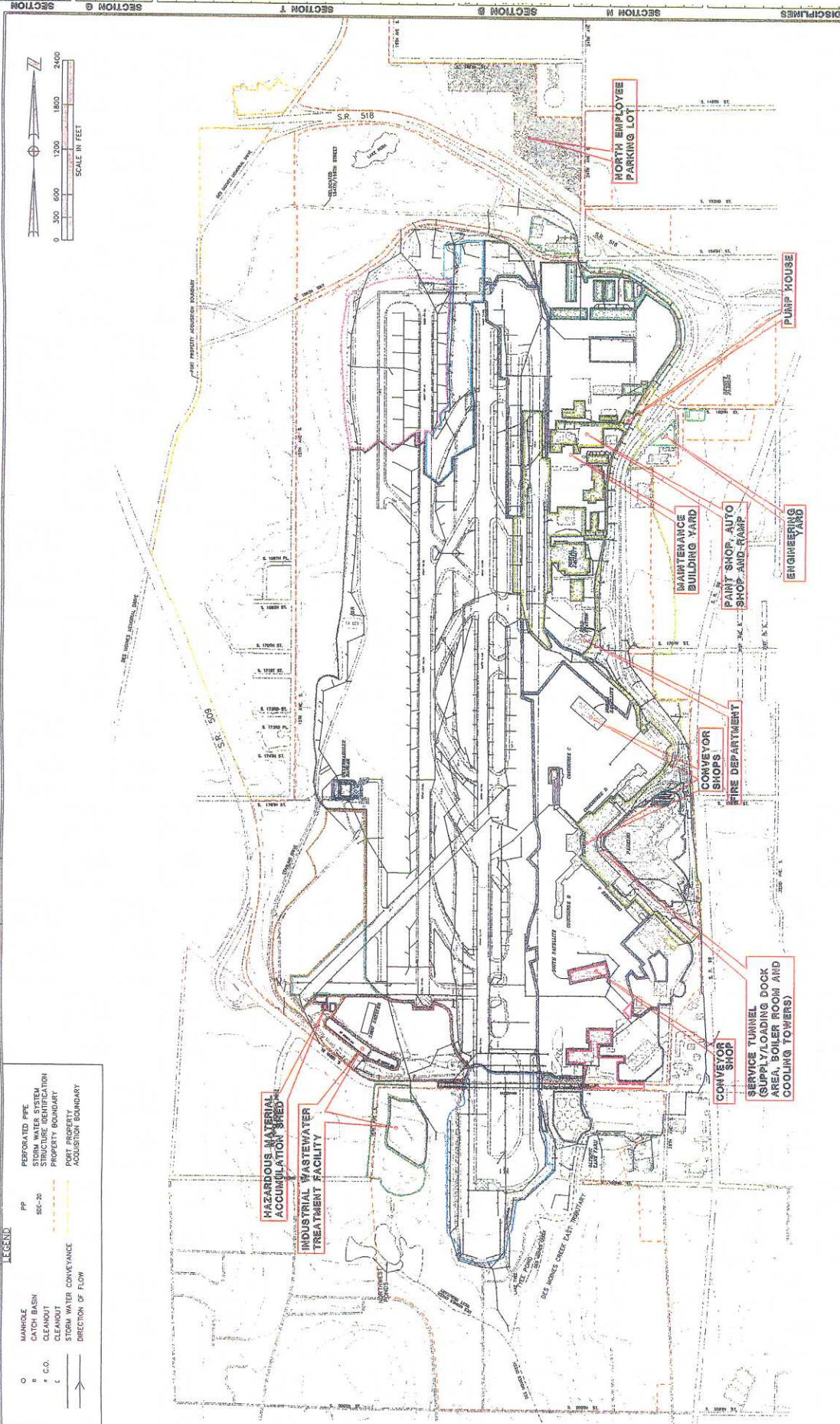
Flow Frequency Analysis  
Time Series File:iwsover.tsf  
Project Location:Sea-Tac DMoines

---Annual Peak Flow Rates---			-----Flow Frequency Analysis-----					
Flow Rate (CFS)	Rank	Time of Peak	- - Peaks - - (CFS)	Rank (ft)	Return Period	Prob		
0.000	42	2/22/49 22:00	1.00	10.43	1	89.50	0.989	
0.000	21	1/27/50 3:00	0.000	9.61	2	32.13	0.969	
0.000	3	2/10/51 20:00	0.000	9.28	3	19.58	0.949	
0.000	47	2/04/52 7:00	0.000	9.22	4	14.08	0.929	
0.000	6	2/07/53 9:00	0.000	9.09	5	10.99	0.909	
0.000	35	1/23/54 1:00	0.000	9.01	6	9.01	0.889	
0.000	39	2/08/55 7:00	0.000	8.48	7	7.64	0.869	
1.00	1	1/06/56 14:00	0.000	8.26	8	6.63	0.849	
0.000	31	3/10/57 4:00	0.000	7.22	9	5.86	0.829	
0.000	28	1/17/58 9:00	0.000	6.96	10	5.24	0.809	
0.000	40	11/24/58 8:00	0.000	6.45	11	4.75	0.789	
0.000	9	12/15/59 15:00	0.000	6.35	12	4.34	0.769	
0.000	20	2/25/61 3:00	0.000	6.18	13	3.99	0.749	
0.000	46	12/24/61 6:00	0.000	6.08	14	3.70	0.729	
0.000	26	11/30/62 20:00	0.000	6.01	15	3.44	0.709	
0.000	15	11/19/63 19:00	0.000	5.80	16	3.22	0.690	
0.000	12	12/01/64 10:00	0.000	5.65	17	3.03	0.670	
0.000	23	1/14/66 1:00	0.000	5.49	18	2.85	0.650	
0.000	14	1/28/67 8:00	0.000	5.47	19	2.70	0.630	
0.000	34	1/20/68 21:00	0.000	5.37	20	2.56	0.610	
0.000	33	12/11/68 10:00	0.000	5.05	21	2.44	0.590	
0.000	17	1/27/70 5:00	0.000	4.88	22	2.32	0.570	
0.000	29	12/10/70 21:00	0.000	4.82	23	2.22	0.550	
0.000	7	3/14/72 7:00	0.000	4.75	24	2.13	0.530	
0.000	10	12/27/72 21:00	0.000	4.59	25	2.04	0.510	
0.000	19	12/27/73 21:00	0.000	4.56	26	1.96	0.490	
0.000	41	12/27/74 8:00	0.000	4.11	27	1.89	0.470	
0.000	36	12/04/75 3:00	0.000	4.07	28	1.82	0.450	
0.000	43	8/26/77 8:00	0.000	4.07	29	1.75	0.430	
0.000	24	12/15/77 20:00	0.000	4.06	30	1.70	0.410	
0.000	50	11/19/78 10:00	0.000	4.04	31	1.64	0.390	
0.000	4	12/22/79 4:00	0.000	3.99	32	1.59	0.370	
0.000	27	12/30/80 23:00	0.000	3.87	33	1.54	0.350	
0.000	18	10/08/81 22:00	0.000	3.69	34	1.49	0.330	
0.000	30	1/08/83 6:00	0.000	3.63	35	1.45	0.310	
0.000	45	11/24/83 9:00	0.000	3.62	36	1.41	0.291	
0.000	37	11/11/84 9:00	0.000	3.53	37	1.37	0.271	
0.000	25	1/19/86 1:00	0.000	3.46	38	1.33	0.251	
0.000	13	11/27/86 1:00	0.000	3.43	39	1.30	0.231	
0.000	32	12/10/87 8:00	0.000	3.43	40	1.27	0.211	
0.000	38	11/25/88 2:00	0.000	3.26	41	1.24	0.191	
0.000	11	1/09/90 18:00	0.000	2.96	42	1.21	0.171	
0.000	8	11/24/90 19:00	0.000	2.78	43	1.18	0.151	

0.000	22	2/01/92	0:00	0.000	2.69	44	1.15	0.131
0.000	48	3/23/93	9:00	0.000	2.67	45	1.12	0.111
0.000	49	2/17/94	22:00	0.000	2.55	46	1.10	0.091
0.000	16	12/27/94	21:00	0.000	2.46	47	1.08	0.071
0.000	5	2/09/96	7:00	0.000	2.34	48	1.05	0.051
0.000	2	1/03/97	3:00	0.000	1.85	49	1.03	0.031
0.000	44	10/30/97	12:00	0.000	1.68	50	1.01	0.011

IWS Overflow Analysis: WY 1956  
Treatment Rate 2.65 mgd





**LEGEND**

○	MANHOLE	PP	PERFORATED PIPE
□	CATCH BASIN	SSC-SB	STORM WATER SYSTEM
•	C.O.	---	STRUCTURE IDENTIFICATION
┌	STORM WATER CLEANOUT	---	PROPERTY BOUNDARY
→	STORM WATER CONVEYANCE	---	PORT PROPERTY
→	DIRECTION OF FLOW	---	ACQUISITION BOUNDARY

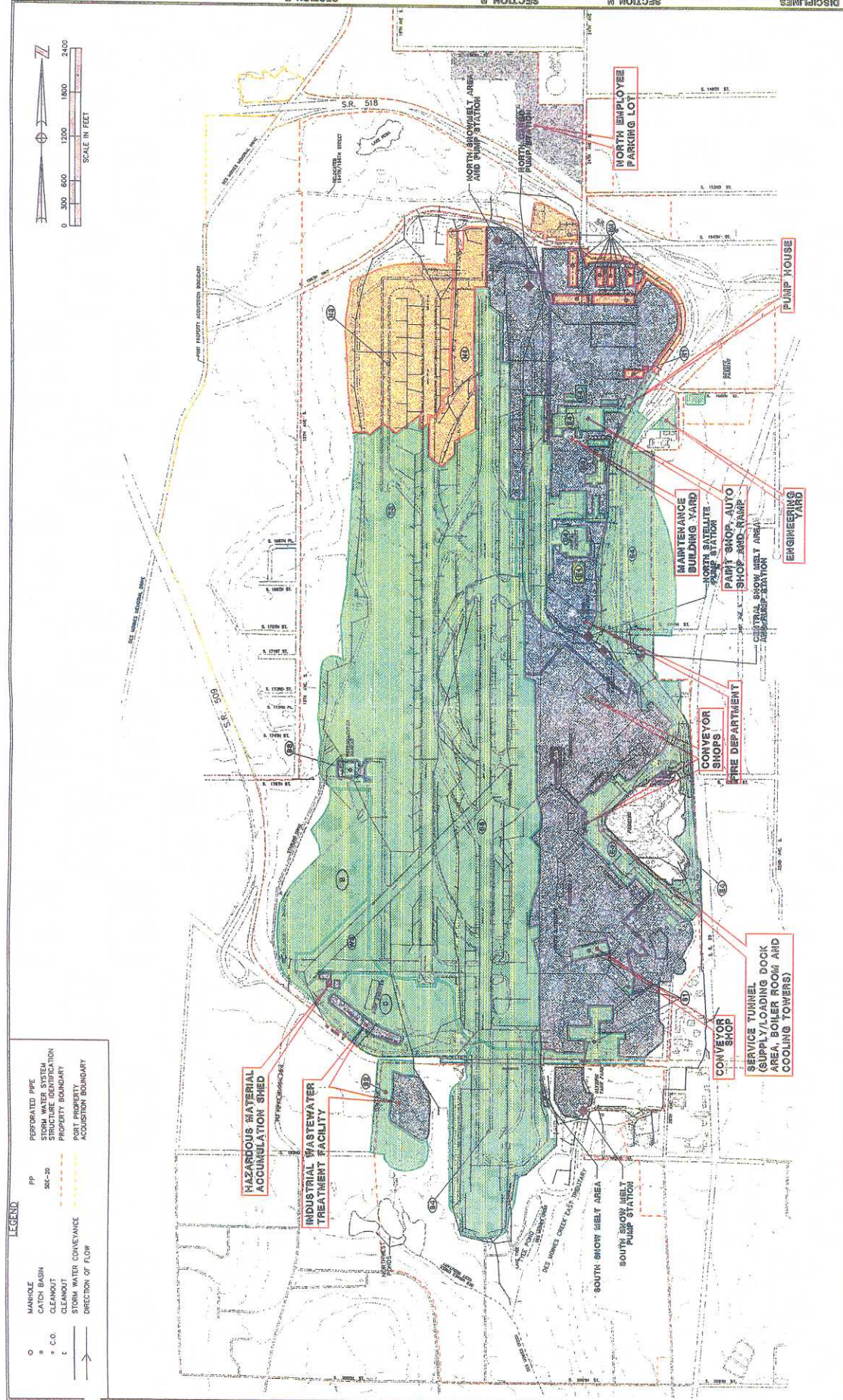
PROJECT TITLE: PORT OF SEATTLE		DATE: 1/99	
SUBJECT: SEA-TAC INTERNATIONAL AIRPORT		SCALE: AS SHOWN	
INDUSTRIAL ACTIVITY AREAS		DRAWING NO.: 976079.24	
FIGURE II.1		SHEET NO.:	

NO.	DATE	BY	DESCRIPTION
1	11/98	IK	UPDATE FIGURE
2	11/98	IK	UPDATE FIGURE
3	11/98	IK	UPDATE FIGURE
4	11/98	IK	UPDATE FIGURE
5	11/98	IK	UPDATE FIGURE
6	11/98	IK	RELOCATED AND ADDED OUTFALL LOCATIONS

AR 011243





**LEGEND**

- MANHOLE
- CATCH BASIN
- CLEANOUT
- C.O.
- C.
- PERFORATED PIPE
- STRUCTURE IDENTIFICATION
- PROPERTY BOUNDARY
- PART PROPERTY
- ACQUISITION BOUNDARY
- STORM WATER CONVEYANCE
- DIRECTION OF FLOW

**PORT OF SEATTLE**  
SEA-TAC INTERNATIONAL AIRPORT

**PORT OF SEATTLE INDUSTRIAL WASTEWATER SYSTEM SERVICE AREA AND MAJOR DRAINAGE BASINS**

PROJECT NO. 976079.2.4  
FIGURE II.2

NO.	DATE	BY	REVISIONS
1	1/28/14	EA	UPDATE FIGURE
2	1/28/14	EA	UPDATE FIGURE
3	10/24/13	EA	UPDATE FIGURE
4	7/29/13	EA	UPDATE FIGURE
5	7/29/13	EA	UPDATE FIGURE
6	1/28/14	EA	RECORDED AND ADDED OUTFALL LOCATIONS

**LEGEND**

- IWS DRAINAGE BASIN
- MILLER CREEK DRAINAGE BASIN
- DES MOINES CREEK DRAINAGE BASIN
- ◆ PUMP-STATION LOCATION

PROJECT: 976079.2.4  
DESIGNER: [blank]  
DATE: 1/29  
FILE NUMBER: P815004

AR 011244





C

AR 011246



# **Low Flow Analysis Flow Impact Offset Facility Proposal**

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July 2001

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**Parametrix, Inc.**

**AR 011247**



July 23, 2001

Ann E. Kenny  
Department of Ecology  
Northwest Regional Office  
3190 160<sup>th</sup> Avenue SE  
Bellevue, WA 98008-5452

Dear Ms. Kenny:

**SUBJECT: Low Streamflow Analysis, Summer Low Flow Impact Offset Facility Proposal, Seattle-Tacoma International Airport**

This letter summarizes the Port of Seattle's evaluation of summer low streamflow impacts in Des Moines, Miller, and Walker Creeks calculated to result from proposed airport Master Plan Update projects. This letter also summarizes the Port's proposal to offset these impacts to maintain existing summer low streamflow conditions in these creeks post project. The methodology used to determine the effects and the plan to offset the impacts was developed and discussed in a series of meetings between the Port of Seattle (Port), Department of Ecology (Ecology), and King County, with staff from Floyd Snider McCarthy, Inc., acting as facilitators.

The evaluation and low streamflow impact offset proposal is final, subject to potential conditions associated with your review during 401 permit deliberations. The Port plans to submit final documentation of the low streamflow evaluation and operational plan for mitigation facilities in the form of a detailed Low Streamflow Analysis Report and Summer Low Flow Impact Offset Facility Operational Plan, outlines of which are included as an attachment to this letter.

#### **Summary**

The Port's proposal is to detain stormwater in underground vaults and release the detained water continuously into each creek during the summer low streamflow period at a rate equal to the calculated summer low streamflow impact to that creek from planned Port projects. The summer low streamflow impacts in each creek were determined through detailed modeling analysis. The summer low streamflow periods were determined through statistical analyses of modeled streamflow from the calibrated HSPF models and discussions with biologists on the effects of low streamflow periods on stream biology. Details of the analysis used to arrive at the proposed summer low streamflow offset periods, low streamflow magnitudes, impacts to summer low streamflows from Port projects, and sizing of the vaults are included as attachments to this letter. A summary of the calculated summer low streamflow impacts and flow impact-offset proposal is listed below:

**Seattle-Tacoma  
International Airport**  
P.O. Box 68727  
Seattle, WA 98168 U.S.A.  
TELEX 703433  
FAX (206) 431-5912



**AR 011248**

	<u>Des Moines Creek</u>	<u>Miller Creek</u>	<u>Walker Creek</u>
Summer low flow period	July 24-October 24	August 1-October 31	August 1-October 31
2-year 7-day low flow (1994)	0.35 cfs	0.74 cfs	0.79 cfs
Port impact on streamflow <sup>1</sup>	0.10 cfs	0.13 cfs	0.09 cfs
Vault size needed	12.2 acre-feet	18.8 acre-feet	15.0 acre-feet
Maximum vault fill time	32 days <sup>2</sup>	58 <sup>3</sup> days	282 days <sup>3</sup>

1 Difference between 1994 and 2006 2-year, 7-day low flow, including non-hydrologic impacts.

2 Vault filling starts January 1.

3 Vault starts filling November 30.

Vaults to detain stormwater for release during summer low streamflow periods were sized based on the duration within which summer low streamflows have historically occurred (generally +/- 90 days from late July through late October), the modeled impact to streamflow in each creek, and an allowance for precipitation events during the summer low streamflow periods that will partially refill the vaults. The resulting storage volumes (12.2 acre-feet in Des Moines Creek, 18.8 acre-feet in Miller Creek, 15.0 acre-feet in Walker Creek) will provide enough water every year to offset the impacts to streamflow throughout the historic summer low streamflow period. For two years within the period of record (1977 and 1979), the Walker Creek vault does not fill up entirely by the start of the summer low streamflow period. However, in these two years, rainfall that occurs during the summer low streamflow period provides enough water so that offset flows are provided throughout the entire summer low streamflow period. The vaults will include features (both structural and operational) for managing water quality to ensure there are no adverse impacts from discharges from the flow impact offset facility. Additional details on all these issues are presented in this letter.

### **Determination of the Duration of Summer Low Streamflow Periods**

Determination of the summer low streamflow period for each creek was done by analyzing modeled streamflow from the calibrated HSPF model for each creek, which used 1994 (existing) land use conditions. The HSPF models for each watershed were calibrated by comparing model output with streamgauge data and adjusting model parameters until a satisfactory match was obtained. Additional low streamflow calibration information is provided in the attachments. Assumptions of model parameters (land use, basin delineations, impervious areas, etc.) are the same as detailed in the Comprehensive Stormwater Management Plan. The seven-day low flow period for each year (using 1994 flow conditions) in the 47-year period (1949-1995) for each creek was determined at points of compliance near the airport (200<sup>th</sup> Street in Des Moines Creek, SR 509 in Miller Creek, and at the outlet of the wetland near Des Moines Memorial Drive in Walker Creek). The seven-day low flow was selected as an indicator of persistent dry season flow. For example, a longer low streamflow would have the same or higher flow, since flows tend to have a downward trend (flows become gradually lower) before a storm increases flow. In addition, summer low streamflows tend to decrease gradually, therefore, a shorter low streamflow period is unlikely to result in significantly lower average flows or target flows. Finally, consultation with biologists concludes that summer low flows with durations of less than two weeks do not affect the carrying capacity of the creeks or cause behavioral changes to salmonids (see attachment).

The occurrences of the annual seven-day low flow periods were plotted and a histogram showing the distribution of the summer low flow periods was developed for each creek. The summer low streamflow period for each creek was selected to include all the historical seven-day low flow occurrences, with the exception of three specific occurrences during the forty-seven year record that occurred during November and December – periods typically associated with two of the three wettest months of the rainy season. The summer low streamflow periods for each creek are:

Des Moines Creek	July 24 through October 24
Miller Creek	August 1 through October 31
Walker Creek	August 1 through October 31

The Port’s proposal is to provide water to offset the impacts to summer low streamflows throughout these time periods in each creek.

**Determination of Streamflow Magnitudes (Target Streamflows)**

The magnitude of existing summer low streamflow (target streamflow) in each creek was determined through analysis of the seven-day low flow periods under existing (1994) conditions described above. The annual seven-day low flows for each creek were ranked and recurrence intervals were determined based on this ranking. The seven-day low flow with a two-year (50 percent) recurrence interval was selected as the streamflow target in each creek. The two-year seven-day low flow was selected because the magnitude of the estimated impact to seven-day low flows decreases with greater recurrence interval; i.e., the estimated reduction in the seven-day two-year frequency low flow rate is greater than that for the seven-day, ten-year frequency low flow rate. Therefore, providing mitigation equivalent to the seven-day, two-year frequency impact will provide mitigation sufficient to mitigate all of the more extreme summer low streamflow events. Based on this analysis, the existing summer low streamflows (two-year, seven-day pre-project conditions) are determined to be:

Des Moines Creek	0.35 cfs
Miller Creek	0.74 cfs
Walker Creek	0.79 cfs

**Determination of Impacts to Streamflow**

The effects to flow during the summer low streamflow periods were determined by comparing modeled streamflows before project construction to modeled streamflows after project construction. Each creek has different post-development conditions that potentially affect low streamflow; therefore each has a different approach for determining impacts. In Des Moines Creek, 2006 land use conditions (“post-project”) were modeled for the full 1949-1995 period of record. The seven-day low flow for each year was selected, ranked, and the streamflow with a two-year recurrence interval was determined. In Des Moines Creek, the two-year post project summer low streamflow is 0.25 cfs. The impact to streamflow from proposed Port projects is the difference between this flow and the existing pre-project summer low streamflow described above, as determined from the modeled 1994 (“existing”) land use conditions (0.35 cfs). Therefore, the impact to summer low streamflows in Des Moines Creek from proposed Port

projects is the difference between the post project and existing condition flows, or 0.10 cfs. The flow rate is the magnitude of offset that will be provided during the summer low streamflow period for Des Moines Creek described above (July 24 through October 24).

In Miller Creek, a different approach was applied because of the need to model the effect of the proposed runway embankment on streamflows. In areas where the embankment is proposed, recharge entering the embankment was calculated using the post-project HSPF model. The recharge was then input to the Hydrus-2D model, which simulated the spreading of recharge fronts through the unsaturated zone of the embankment fill. Output from the Hydrus-2D model were input to the "slice" model, which is a finite-difference groundwater model used to simulate flow through the proposed embankment underdrain layer. Output from the "slice" model was then input back into the HSPF model to determine the quantity and timing of discharge from the embankment, and the groundwater effects on Miller Creek. This approach was selected to accurately simulate the flow of groundwater through the proposed embankment. The analysis was a more discrete application of the Hydrus-2D and "slice" modeling approaches used in the Runway Fill Hydrologic Studies Report (Pacific Groundwater Group, June 19, 2000), prepared for Ecology.

For the post-project conditions in Miller Creek, the four-year period from 1991 through 1994 was modeled. This period was chosen as a representative dry period in the precipitation record. Output from the HSPF model was analyzed to determine the annual seven-day low streamflows for each of the four years. To determine the impact between 1994 (existing) low streamflow and 2006 (post-project) flows, the impact during 1991 was used. This year was selected because it was the only year in the four years of detailed embankment flow analysis that low streamflows were greater than the two-year flow. In Miller Creek, the estimated summer low streamflow impact due to the project is 0.11 cfs.

In addition to hydrologic impacts in Miller Creek, additional impacts, both positive and negative, will result from removal of septic tank flows (negative impact) and cessation of water uses for residential and agricultural uses (positive impact). The impact of these "non-hydrologic" changes in Miller Creek is an additional net -0.02 cfs (-0.08 cfs for septic tanks which is then adjusted by 0.7 for loss to DEEPFR; water use withdrawals are +0.04 cfs). The total Miller Creek impact (both hydrologic and non-hydrologic) is 0.13 cfs.

For the post-project conditions in Walker Creek, the entire record from 1949 through 1995 was used. To determine hydrologic impacts, it was assumed in the post-project (2006) model that new impervious areas and new fill area is simply removed from the model and can no longer contribute to low streamflow. This is a conservative assumption, since some precipitation will undoubtedly contribute to groundwater flow. This approach was chosen to allow for the largest impervious area possible to refill the Walker Creek low streamflow vault. The Port proposes to line the filter strips with impermeable material to collect infiltrated stormwater that will be directed to the low streamflow vault. The lined area (approximately six acres) does not exceed the effective impervious area used in the Comprehensive Stormwater Management Plan.

In Walker Creek, much of the groundwater that supports summer low streamflows comes from areas where surface water drains to Des Moines Creek or Miller Creek. Under existing (1994)

conditions, approximately 630 acres of pervious land is included in the Walker Creek groundwater basin, which contributes to Walker Creek summer streamflows. Thirty-eight acres of new impervious area is proposed in the approximate area of the 630 acres pervious acre area. The thirty-eight acre area is adjusted (multiplied) by 0.86 to reduce the area to effective impervious area. The result (32.7 acres) is deducted from the 630-acre pervious area in the existing conditions model to determine the post project (2006) contribution to the Walker Creek groundwater basin.

To determine the magnitude of the hydrologic impact in Walker Creek, the seven-day low flow for each year was selected, ranked, and the streamflow with a two-year recurrence interval was determined for existing (1994) and post-project (2006) conditions. The two-year 1994 seven-day low streamflow is 0.79 cfs; the 2006 summer low streamflow is 0.71 cfs. Therefore, the impact to summer low streamflow in Walker creek from proposed Port projects is the difference between the post-project and existing conditions flow, or 0.08 cfs.

In addition to hydrologic impacts in Walker creek, additional impacts will result from the removal of septic tanks. The impact of this change in Walker Creek is an additional 0.01 cfs (0.014 cfs for septic tanks which is then adjusted by 0.7 for loss to DEEPR). The total Walker Creek impact (both hydrologic and non-hydrologic) is 0.09cfs.

Based on the analyses described above, total net summer low streamflow impacts that the Port proposes to offset throughout the summer low streamflow periods in each creek are:

Des Moines Creek	0.10 cfs
Miller Creek	0.13 cfs
Walker Creek	0.09 cfs

#### **Sizing/Filling of Vaults and Vault Release**

Several of the stormwater vaults proposed in the Comprehensive Stormwater Management Plan will have storage areas sized and designed to detain the volume of water needed to continuously release a flow equivalent to the calculated summer low streamflow impacts throughout the summer low streamflow duration in each creek. The vault sizes were calculated in the following manner: the offset flow rate was multiplied by the duration of the summer low streamflow period. Analysis of precipitation records show that some amount of rainfall always occurs during the summer low streamflow period. Rainfall amounts during the summer low streamflow period from the worst (driest) year on record were converted into a volume based on the amount of impervious area that drains to each vault. This water was subtracted from the total volume to arrive at the final volume. These calculations were done on a daily basis to account for the dynamics of filling and draining the vaults throughout the summer low streamflow period in each creek. This conservative approach assures that the volume of water needed to offset the impacts to summer low streamflow will be available in a range of extreme conditions such as those found in the 47-year period of record. The net storage volumes of water needed for each creek are:

Des Moines Creek	12.2 acre-feet
Miller Creek	18.8 acre-feet
Walker Creek	15.0 acre-feet

The vaults will be filled each year during the winter, by closing the flow offset discharge outlet no later than January 1 each year, allowing stormwater to accumulate in the vaults. Analysis of historical rainfall records and the amount of impervious area that drains to each vault were used to determine the length of time required to fill the vaults. This length of time was applied to the beginning of the summer low streamflow period in each creek (the date the flow impact offset would start each year) to determine when to begin accumulating water. Based on this analysis, the maximum time needed to fill the vaults during the period of record are:

<u>Creek</u>	<u>Closure Date</u>	<u>Longest Fill Time in Record</u>
Des Moines Creek	January 1	32 days
Miller Creek	January 1	58 days
Walker Creek	December 1	282 days

The impervious area in each basin used to fill the vaults is as follows:

Des Moines Creek	234 acres impervious area
Miller Creek	82 acres impervious area
Walker Creek	3.5 acres impervious area
	6 acres lined pervious area
	2 acres Pond F cover

It is important to note that using the period of record (except for the worst (driest) year on record), the vaults will always have water remaining in them at the end of the summer low streamflow period. The Port proposes to continue releasing this water at the determined flow rate for as long as possible before the vault outlets are actually closed (after the end of the summer low streamflow periods). The operational plan will call for the vaults' outlets to be closed no later than January 1 each year to allow the filling of the vaults to take place when precipitation is generally most abundant.

### **Water Quality Considerations**

The Summer Low Streamflow Impact Offset Facilities will be designed and operated to avoid in-stream water quality violations. Class AA water quality standards are used as the applicable in-stream standards. Water quality parameters of concern include dissolved oxygen, temperature, turbidity, copper, lead, and zinc. A variety of best management practices (BMPs), facility designs, and monitoring programs are proposed (or already in place) to ensure that in-stream water quality violations will not result from operation of the flow offset facility.

Structural features of the vaults include sediment traps, settling areas, and special placement of inflow and outflow pipes to reduce turbidity, vents to allow air circulation to enhance aeration, placement of inlets to facilitate periodic flushing with "fresh" stormwater, and discharge pipes configured to enhance passive aeration. The underground configuration of the vaults will facilitate temperature management. Provisions are included to allow for additional filtration and aeration of discharges, as needed. BMPs in place in the drainage areas on the airfield will minimize the amount of sediment that will enter the vaults. An extensive water quality monitoring program is proposed to characterize the water discharged from the offset facility, and

to ensure that the facilities can be managed and operated without causing in-stream water quality violations.

**Operational Plan**

A detailed operation, maintenance, and monitoring plan will be completed for the summer low streamflow impact offset facilities. The operational plan proposes an annual schedule of activities to ensure that the facilities are meeting performance goals. An adaptive management system is proposed to allow the operation of the facilities to be refined as experience is gained. The vaults will be monitored as they are filled and as water is released. Periodic monitoring of water quality will be completed, both of the discharge water and in the creeks, to ensure that water quality criteria are not violated in the creeks. Biological monitoring is proposed as part of the Natural Resources Mitigation Plan. This monitoring will evaluate changes in the Benthic Index of Biotic Integrity (BIBI) over a ten-year period.

Information provided for review attached to this letter include:

- Outline of Summer Low Streamflow Flow Analysis and Summer Low Streamflow Impact Offset Facility Operations Plan
- Selected Draft sections of Summer Low Streamflow Impact Offset Facility Operations Plan
- Technical backup material for low flow impact evaluation and impact offset proposal including the following
  - Des Moines Creek
  - Miller Creek
  - Walker Creek
  - Stream Biology Information
  - Embankment Modeling Information
  - Non-Hydrologic Analysis
    - Methodology
    - Data (electronic only)
  - Daily Average Creek Flows (electronic only)

In addition, selected model data files have been electronically sent to Kelly Whiting for his review.

Please contact me at 206/988-5528 if you have any questions.

Sincerely,



Keith R. Smith  
Water Resources Manager

C: Kelly Whiting, KCDNR



## Draft Low Flow Analysis/Flow Effect Offset Facility Report

### • **Executive Summary**

- Report summarizes effects to streamflow in Miller, Walker, and Des Moines Creeks caused by proposed Port projects and associated "non-hydrologic" effects (buyouts, displaced water use/irrigation/septics/etc.) and provides a flow offset facility/plan
- Impacts of embankment seepage were modeled using a combination of tools (HSPF, Hydrus, Slice) for an agreed upon dry period (1991-1994).
- Non-hydrologic effects were combined with modeled effect to determine net effects to streamflow
- Result was the effect on streamflow from the project (EFS)
- That impact was used to calculate the amount of stormwater needed to be detained for release during summer low flow periods
- This calculation was based on release rates, refilling (precip), and vaults not going dry
- Seven vaults will provide reserved storage and release at appropriate rates to offset impacts
- Vaults will include design considerations for water quality treatment
- Operational plan will include test releases and monitoring to ensure water quality meets standards
- Regular maintenance will be performed to ensure proper function
- Performance standards will be (have been) developed
  - Water always available to discharge at needed Q throughout summer low streamflow period
  - In-stream WQ standards met throughout operation
- Refinements can be made based on observed performance

### • **Introduction**

- Purpose of Report
- Organization of Report
- Relationship to other documents (SMP)
- Project description
  - Master Plan, Third Runway Embankment
  - Impacts to Streamflow
  - Low Flow Analysis
  - Mitigation Plan
  - Objectives of flow offset plan

### • **Determination of impacts to streamflow**

- Brief description of overall approach
  - Compare modeling of existing conditions to modeling of post project conditions to determine effect to streamflow
  - Existing condition determined by HSPF modeling in SMP
  - Post project condition (embankment) determined by slice, hydrus, and HSPF modeling to determine flow and timing impacts of embankment

- Post project condition modeling uses 1991-1994 precipitation (dry period)
- Non-hydrologic effects conservatively estimated, converted into daily time series, and input to model of built condition
- Result of post project condition modeling gives (total) impact to streamflow of Port's projects
- This results is applied to a spreadsheet analysis and used to size storage to provide offset water throughout summer
- Modeling of Existing (Pre-embankment) Conditions -HSPF approach – pull language from SMP and frame for low flow
- Modeling of Proposed (Built embankment) Conditions
  - Summary of HSPF/Hydrus/Slice Modeling Approach
  - Results of Embankment Modeling
- Non-hydrologic effects on flows
  - Changes/revisions based on SMP
  - Conversion to time series for inclusion in HSPF modeling
- Results of all impacts to streamflow/net effect to streamflow
- **Mitigation of Impacts (Reserved Stormwater Release)**
  - Methodology used to size/design stormwater vaults
    - Criteria
    - Spreadsheet analysis based on discharge rates and periodic refilling from rainfall
    - Refilling based on rainfall in driest year in period of record (1949-1995)
  - Performance standards
    - Always able to meet flow offset requirements (vaults never go dry during summer low streamflow periods)
    - Instream water quality standards met
  - Water Quality Design Aspects
    - Sediments/Turbidity
      - Discussion of water quality standards for turbidity
      - Airfield BMPs minimize sediments/turbidity getting into vaults (typical)
      - Compartmentalized dead storage (typical)
      - Floor sloped away from outlet (typical)
      - Allow for installation/operation of filters if test releases show turbidity of reserved releases won't meet standards (reserved outlets only)
    - Temperature
      - Discussion of water quality standards for temperature
      - No special design considerations for temperature

- Nature of underground vaults ensures that water will be relatively cool compared to typical summer stream temperatures
- Port collecting temperature data from existing vaults starting summer 2001 to provide baseline data
- Dissolved Oxygen
  - Discussion of water quality standards for DO
  - No oxygen consumption in vaults (low BOD, no solar radiation driving biological activity that would consume oxygen)
  - Inlet to vaults positioned to allow flushing of reserved area to prevent accumulation of stagnant (potential low DO) water
  - Passive aeration provided by turbulent flow in steep discharge pipes and energy dissipaters at/near outlets
  - Air vents provided to allow supply of fresh air for passive aeration
  - Provide for (portable) aerators if DO still too low or if slopes not sufficient to aerate water
    - List possible/applicable technologies (microbubble diffusers, gas injection, air injection, aeration hose)
    - Discuss each reserved stormwater outlet
    - Selection will be made as part of final design
- Nutrients
  - No water quality standards to meet, but possibility of algae blooms in summer has been raised as a concern
  - Usually a concern in lakes and ponds, none in creeks downstream of airport, residence time of water in creek only a few hours
  - No special design considerations proposed for nutrients
  - No fertilizers used on airfield
  - Bioswales provide treatment
  - No solar radiation in vaults to stimulate algae growth
- Metals
  - Discussion of water quality standards for metals
  - Airport water quality data show that (total recoverable) metals in airfield runoff lower than typical urban stormwater
  - Higher metals in airport runoff associated with non-airfield basins, which won't be providing water to reserved stormwater vaults
  - Most metals associated with particulates, data shows low dissolved metals in discharges
  - Vault design and BMPs for turbidity will be effective in reducing total metals (list again)
- Other
  - Reserved stormwater discharge points will be the same as normal stormwater discharge points – no additional HPAs needed

- Discussion of valve/outlet location for each proposed reserved stormwater vault relative to creeks
  - Security – facilities and controls located inside airport fence to minimize vandalism potential
  - Performance – POS will continue to operate/inspect/monitor/report/maintain system throughout its life; adjustments/refinements can be made based on observed performance
- **Operational Plan/Annual schedule of activities**
  - Before initial operation port will submit to Ecology a operations/maintenance/monitoring plan containing
    - Monitoring/sampling protocols (follow NPDES permit requirements) and schedule
    - Specific monitoring locations (discharge and in-stream)
    - Recording/reporting plan
    - Detailed operation plan based on proposal below
    - Inspection/maintenance plan
    - Summary of characterization of discharge and creeks' water quality during summer seasons, based on existing data or data collected prior to implementing facilities
  - Calendar Year Operating Schedule
    - Jan – June (varies by vault)– operate as normal stormwater vaults, flow offset outlet closed to accumulate water
    - June/July – small test release from each vault
      - Confirm discharge rate (gage/measure flow)
      - Sample specified in monitoring plan
      - Compare samples to existing or collected data from creeks from summer (when reserved stormwater discharges would be occurring)
      - Review existing data to see if summer low flow water quality is adequately characterized by data. If not, collect during next few summers (prior to facilities going on-line) to characterize. This work will be included/discussed in final monitoring plan submitted to Ecology prior to implementation.
      - If potential water quality violations are indicated, take appropriate actions prior to initiating operational discharge, such as:
        - Install/maintain filters for sediments/turbidity/metals
        - Install portable aerators for DO
        - No actions proposed for temperature and nutrients
    - July/August - Open each flow offset outlet on date specified in analysis
    - July – October – inspect/monitor operation of each discharge
      - Sample per monitoring plan
      - Any water quality violations will be immediately investigated and corrected



## **Executive Summary**

This report presents the analyses performed to estimate the timing and volume of discharges to local receiving streams and wetlands during low flow periods from Seattle-Tacoma International Airport (STIA) considering improvements defined in the Port of Seattle's Master Plan Update. This report also presents a Flow Impact Offset Facility Plan, which is the Port's proposal to offset impacts to flows in the receiving waters during annual low streamflow periods, typically experienced in late summer/early fall. The plan is based on a detailed evaluation of the hydrologic impacts of the proposed third runway embankment and associated non-hydrologic impacts on streamflow in Miller, Walker, and Des Moines Creeks. This report, an update of the Low Flow Analysis prepared by Earth Tech in December 2000, is submitted for consideration by the Washington State Department of Ecology in reviewing permit applications related to the proposed Third Runway project. This report builds upon previous reports by Earth Tech and Pacific Groundwater Group. Analyses presented in this report were prepared by Earth Tech, Pacific Groundwater Group, AquaTerra, and Parametrix. The Washington Department of Ecology was consulted throughout the development of the plan to ensure that agency concerns are addressed in this report.

Impacts to streamflow in the three creeks were evaluated using a suite of modeling tools. The Hydrologic Simulation Program – FORTRAN (HSPF) was used to develop overall stormwater models of STIA (existing conditions and proposed conditions), as described in the Comprehensive Stormwater Management Plan (CSMP) (Parametrix 2000). These models were also used to evaluate stormwater flows and volumes in the low flow analysis. The hydrologic properties of the proposed third runway embankment were modeled using a combination of

Hydrus-2D and a finite-difference "slice" model. Hydrus-2D was used to simulate the movement of water between the root zone and water table in the proposed embankment, and the slice model was used to simulate the movement of water through the saturated portion of the proposed embankment. Results of the Hydrus and Slice modeling, along with the associated non-hydrologic impacts, were incorporated back into the HSPF model to estimate the post construction flows. By comparing these model results to the pre-project conditions model, the impacts of the proposed embankment on streamflows were determined. Statistical analyses of model output, precipitation and streamflow data for the available period of record predicted a net low-flow impact to Miller Creek of 0.19 cubic feet per second (cfs) 0.08 cfs in Walker Creek, and 0.10 cfs in Des Moines Creek..

The Port's proposal to offset effects to low streamflow is to detain stormwater runoff and release it to the impacted creeks during the predicted annual low streamflow period. The volume of water required to offset the predicted impacts was determined by multiplying the predicted impact for each creek by the duration of the summer low streamflow period (+/- 90 days from late July through late October). The resulting volumes of stormwater (\_\_\_ acre-feet for Miller Creek, \_\_\_ acre-feet for \_\_\_ Creek) were incorporated into selected proposed stormwater vaults in each watershed. Several considerations are proposed to be included in the design of these vaults to allow the management of stormwater discharges to offset the predicted low-flow impacts. Additional considerations in the design and operation of the proposed stormwater vaults to improve the water quality of discharges will also be included. In addition, an analysis of the availability of stormwater required to fill the vaults showed that even during the driest years in

the period of record, enough water can be collected and stored to offset the impacts to streamflow during the annual low streamflow period.

Key goals and objectives (performance standards) of the proposed Flow Impact Offset Facility include:

- Always being able to provide flow at the rate required to offset the predicted impacts of the proposed embankment for the entire 92-day annual low streamflow period each year.
- Operate the facility to prevent in-stream water quality violations during the annual low streamflow periods.
- To design the facility and its operation, monitoring, and maintenance plan so that an adaptive management strategy can be applied.

As stated in Ecology's draft Stormwater Management Manual for Western Washington, the objective of stormwater management is to "control the quantity and quality of stormwater produced by new development and redevelopment such that they comply with water quality standards and protect beneficial uses of the receiving water." The Department of Ecology has determined that stormwater management activities in Washington state do not require a water right. Since the Port's proposal to offset flow impacts to the receiving waters consists of stormwater management activities, a water right is not required for the flow Impact Offset Facility.



## **Introduction**

The purpose of this report is to evaluate the impacts to streamflows in Miller, Walker, and Des Moines Creeks resulting from construction projects included in the Master Plan Update for Seattle-Tacoma International Airport, and to propose a Flow Impact Offset Facility to mitigate the impacts during low streamflow periods. The principal projected expected to impact streamflows in the creeks is the third runway embankment.

## **Section not complete**

### Water Quality Design Aspects

The Washington State Department of Ecology (Ecology) has defined standards for water quality related to stormwater release, including periods of low flow. Ecology has jurisdiction to monitor and enforce these standards through their National Pollution Discharge Elimination System (NPDES) Permit. These standards include turbidity, dissolved oxygen, temperature, and dissolved metals. The Port's current stormwater design plans for the third runway expansion include a stormwater system and operational procedures to provide the storage and managed release of stormwater during low flow periods. These stormwater storage facilities employ biofiltration strips, catchbasins, detention ponds and vaults to meet current King County water quality requirements. In addition, the facilities are designed to be retrofit according to the Ecology water quality measures if specific water quality concerns are identified during post construction monitoring. The Port's monitoring and reporting program (see Section XX) is proposed to assess the performance of the facilities, allowing adaptive management to be used in the implementation of additional water quality measures to ensure that standards will continue to be met.

Des Moines, Miller, and Walker Creeks are all assumed to be Class AA (extraordinary) waters (WAC 173-201A-030). As such, the water quality standards discussed in this report are those listed for Class AA water bodies, which are the most stringent standards. Water quality standards for metals are based on toxicity, are independent of the receiving water classification, and are listed in WAC 173-201A-040 (Toxic substances). Although Ecology's proposed revisions to water quality standards from a class-based system to a use-based system is not expected to impact the design and operation of the proposed facility, the Port will further

evaluate the proposed changes as part of the final design process and make any needed changes to the facility.

The state water quality standards applicable to the managed release of stormwater to offset flow impacts are discussed below. Specific design features, assumptions and other information considered in the design of the facility are included. Operational and monitoring proposals are presented in section xxxx. References to stormwater vaults refer only to those vaults proposed to detain stormwater to offset impacts to streamflows. Likewise, references to stormwater and stormwater discharges refer only to the managed release of stormwater to offset flow impacts.

#### Turbidity

The state water quality standard for turbidity in class AA waters is a two tier standard. For receiving water with turbidity less than or equal to 50 NTU (background flow), discharged water may not increase the receiving waters more than 5 NTU over background. For receiving water with turbidity greater than 50 NTU, discharged water may not increase turbidity of the receiving waters more than 10 percent. Turbidity levels in the creeks vary between less than 5 NTU to over 1,000 NTU. The lowest turbidity levels in the creeks generally occur during low streamflow (baseflow) conditions, which correspond to the majority of periods when the stormwater would be released to the creeks to offset flow impacts. It is assumed that the releases of stormwater to offset flow impacts would have to meet the 5 NTU standard most, if not all of the time. To minimize the need to provide constant background level monitoring of the creek above and below the release locations, releases can be limited to 5 NTU or less in order to be in compliance at all times.

There are several operational considerations and water quality Best Management Practices (BMPs) on the airfield to reduce the sediment and turbidity levels in runoff water going into stormwater storage. The Port uses catchbasins, the Industrial Wastewater System (IWS) system, and biofiltration strips as BMPs on the existing airfield, and the SMP proposes to retrofit the existing airfield with additional sediment trap BMPs in the bottom of each new detention vault facility. The new airfield surface will incorporate similar BMP's to minimize the amount of sediment and suspended solids that get into the stormwater vaults. The primary BMP consists of the construction of biofiltration strips in the new and existing airfield areas that treat stormwater as it drains directly from impervious areas of runways and taxiways. The Port will also maintain catch basins to ensure they continue to trap sediments. Filter strips are already in place in the existing Taxiway "C" airfield area that drains to the stormwater vault (SDS3) located in the Des Moines Creek watershed (see Section 7 in the Comprehensive Stormwater Management Plan). In addition, the airfield is a controlled area subject to very low levels of travel by ground vehicles and frequent cleaning and inspection for debris that could be harmful to aircraft. Consequently, the airfield is generally much cleaner than most urban areas that generate stormwater runoff.

There are also operational procedures outlined in the airport's Stormwater Pollution Prevention Plan (SWPPP) that will minimize opportunities for sediment and suspended solids to enter the stormwater vaults. These include:

- Sweeping ramp areas several times per week.

- Annual inspection of catch basins and cleaning if the depth of sediment equals or exceeds one-third the depth from bottom of the basin to the invert of the lowest pipe.
- Proper storage and disposal of sediment removed from catch basins.
- Hydroblasting of runway skid-mark rubber. Water and removed rubber is vacuumed by the same machine, drained, and deposited at the decant station until disposed as solid waste.

All of these BMPs will limit the amount of sediments and suspended solids that enter the stormwater vaults, and therefore will reduce the turbidity of the water stored in the vaults and discharged to the creeks.

All of the proposed stormwater vaults, including those associated with the Flow Impact Offset Facility, employ features designed to provide treatment (settling and removal) of suspended solids and turbidity. These features include:

- Dividing the dead storage area (similar to the areas in the vaults where the stormwater detained to offset flow impacts will be held) into several compartments by constructing short walls within the dead storage area of each vault. The compartments allow areas for suspended solids to settle out and be contained. Each compartment's outlet will be configured so that the suspended solids are captured in the compartments during low flow release periods. Design considerations of this type are typically included in stormwater vaults. Details will be provided at final design of the stormwater vaults.

- The vaults will include an extra 6-inch depth for the first third of the bottom (minimum) to facilitate trapping sediment that reaches the vault.
- Maintenance of the vaults will remove and properly dispose of collected sediments prior to the anticipated low flow release periods.
- The vaults will be designed to allow installation of additional water quality measures, if needed. Additional water quality features may include filtration of the discharges.

The design of the stormwater vaults, in combination with the operational and monitoring considerations discussed below, will assure that release of stormwater will not cause violations in the turbidity standards. The Port is currently investigating filtration of stormwater associated with discharges from a landside drainage basin. This research includes determining the effectiveness of several filtration media in treating the stormwater. The results of this study will be completed before final design of the flow offset facilities, and the data will be used to select the filtration method most appropriate to treat the stormwater discharge, if needed.

Operational and monitoring considerations for the Flow Impact Offset Facility for turbidity are discussed in Section xxxx.

## Temperature

The state water quality standard for temperature in class AA waters is not to raise the temperature of the receiving water to over 16 degrees Celsius. If the baseline temperature of the receiving water is greater or equal to 16°C discharges cannot raise the temperature more than 0.3 ° C. To date, Ecology has not applied these requirements to stormwater discharges, although they have required temperature monitoring of certain stormwater discharges. Ecology could apply the temperature standard to future stormwater discharges.

The highest annual temperatures in the creeks are usually reached during the summer months, which is the period when the Flow Impact Offset Facility is expected to be in operation. Solar radiation is the primary mechanism by which stormwater temperatures increase in detention ponds. Since the stormwater vaults are typically underground structures (although some be partially exposed), there will be no direct solar warming. Underground storage provides a constant temperature that will be lower than open storage facilities, more closely matching a native seep temperature. Water release from the Flow Impact Offset Facility is not expected to increase the in-stream water temperature at all. Since the proposed underground stormwater vaults will result in relatively cool water being discharged, no special design considerations are proposed to manage water temperatures in the vaults associated with the Flow Impact Offset Facility.

The Port is collecting water temperature data from existing stormwater vaults in order to characterize the expected temperatures of the reserved stormwater discharges. Commencing in the summer of 2001, average daily water temperature data is being collected from the North

Employee Parking Lot vault and the SDS3 vault located near the south end of the airfield. Data will be collected from June through October of each year from the dead storage area of each vault. These existing vaults were selected because they are similar in size to the proposed stormwater volumes associated with the Flow Impact Offset Facility. The NEPL vault is partially exposed to sunlight (on its west side and top) while the SDS3 vault is completely underground. By collecting temperature data from both vaults, a range of expected temperatures can be established. Temperature data will be collected from the dead storage zone in each vault in order to approximate the vaults associated with the Flow Impact Offset Facility. This data will be compared to stream temperature data also being collected by the Port to characterize any cooling effects of stormwater releases on water temperatures in the creeks.

Operational and monitoring procedures for the Flow Impact Offset Facility for temperature are discussed in Section xxxx.

#### Dissolved Oxygen

The state water quality standard for dissolved oxygen (DO) in Class AA waters is 9.5 milligrams per liter (mg/l). Low DO levels in creeks during summer low flow periods is a potential water quality concern. The Flow Impact Offset Facility will be designed and operated in a manner that will not decrease the DO levels in the creeks, and under typical conditions, may actually increase DO levels.

Dissolved oxygen levels in the stormwater vaults will not be significantly reduced while the water is stored. There will be little, if any, biological activity in the vaults that could consume



oxygen as a result of the lack of sunlight and the low biological oxygen demand (BOD) typically seen in stormwater runoff from the airfield (Annual Stormwater Monitoring Report, September 2000). The infrequent and short-lived episodes of elevated BOD due to runway deicing activities are not expected to impact the DO concentrations of the stormwater detained in the Flow Impact Offset Facility because the stormwater associated with these events moves through the stormwater management system in a matter of hours, is replaced with runoff with the low BOD concentrations more typical of airport runoff (Deicing/DO Study Report, November 2000), and typically happens during the early winter months when reserved stormwater releases from the Flow Impact Offset Facility would not take place. In addition, the Port operates BMPs to remove snow containing deicing chemicals (a potential source of BOD) to snowmelt areas that drain to the Industrial Wastewater System, further reducing the BOD in water that drains to stormwater vaults.

Vents will be included in the stormwater vaults associated with the Flow Impact Offset Facility to allow for the circulation of fresh air to occur. This will help maintain the dissolved oxygen concentration of the stormwater.

An additional design consideration is the positioning of the inlet(s) to the stormwater vaults associated with the Flow Impact Offset Facility. The inlet(s) will be placed lower in the vault in order to facilitate flushing of the vault each time there is sufficient rainfall to generate stormwater runoff. Typically, stormwater inlets in vaults are placed at higher elevations within the vault. As a result, water in the lower or dead storage areas may not be circulated and may stagnate. By placing the inlet at a lower elevation, water already in the lower portions of the

vault will be displaced by the incoming water and will not have the opportunity to stagnate. By continually replacing the water in the stormwater vaults, the dissolved oxygen levels in the stormwater in the vaults will benefit. Each stormwater vault associated with the Flow Impact Offset Facility will have its inlet position carefully considered during the final design phase, and placed to enhance this circulating effect.

Passive aeration of stormwater can be achieved through natural turbulence or agitation of the discharges. Steeply sloped pipes with periodic drop structures will be required to move the water from the vault outlets to the creek elevation. An energy dissipating structure will be required near the release point at creek level to slow the velocity adequately for entering the creek safely, without causing scour or erosion. Both the steeply sloped discharge pipes and the energy dissipating structures will provide the turbulence or agitation needed to provide passive aeration. The Pond G storage vault is located within the runway embankment and has adequate fall from the vault outlet to the creek discharge location to provide aeration.

Where insufficient fall is available for this natural aeration process, the installation and operation of aeration devices may be necessary. Other vaults are located near the level of the creek discharge elevation such that active aeration measures may be required through the installation of some type of aeration device. Active aeration systems that could be utilized include microbubble diffusers, gas injection, air injection, mechanical aerators, or aeration hoses. Microbubble diffusers consist of a porous ceramic plate (similar to aquarium aeration stones) and a pump to inject air through the plate. Gas and air injection systems inject a controlled amount of gas or air under pressure into the discharge water pipe. Mechanical aerators physically agitate

water and allow air to become mixed with the water. Aeration hoses are flexible porous rubber hose, and have air pumped through them similar to the microbubble diffusers. Information on each of these devices is included in Appendix xx. Although the selection of the device(s) to be installed will be made during the final design of the Flow Impact Offset Facility, it is likely that the microbubble diffuser will be selected and installed because of their simplicity, effectiveness, cost, and ability to be installed in the discharge pipes. Other attractive features of the microbubble diffuser include low maintenance requirements, their use of a small compressor or pump to provide air instead of the use of compressed gas tanks, and their ability to be automated to function anytime the reserved stormwater discharge valve is open. Currently, the Flow Impact Offset Facility vaults associated with SDN3X, SDN2X/4X, SDN1, Cargo, NEPL, SDS4, SDW3A, and SDW2 may require aeration of their discharges through the installation of aeration devices. *(need to update this section based on final selection of vaults in facility)*

Operational and monitoring considerations for the Flow Impact Offset Facility related to managing dissolved oxygen are discussed in Section xxxx.

### Nutrients

There are no water quality standards for nutrients in the current water quality standards. However, nutrients typically found in stormwater could be of potential concern. If nutrient-rich stormwater is stored for long periods of time, exposure to solar radiation can cause algae blooms. However, it is expected that there will be no adverse water quality impacts associated with nutrients in the release of stormwater for the following reasons:

- There is no significant source of nutrients associated with the airfield areas (i.e., sources of water for the Flow Impact Offset Facility). Primary sources for nutrients in urban stormwater are fertilizers applied to lawns and landscaped areas. However, the grass infield areas of the airfield are not fertilized or irrigated because lush growth could become a wildlife attractant concern. Any landscaped areas to which fertilizers are applied are located near the terminal, and drain to stormwater basins that do not contribute flow to the Flow Impact Offset Facility. The Port's use of fertilizers includes applying the BMPs listed in the airport's Stormwater Pollution Prevention Plan, which further reduces the amount of fertilizers and nutrients that enter stormwater. With careful management of fertilizer use at the airport, there is no major source of nutrients for the drainage areas that contribute stormwater to the Flow Impact Offset Facility.
- The operation of BMPs on the airfield (biofiltration swales) would reduce the opportunity and concentrations of any nutrients that exist prior to the stormwater entering the vaults.
- Since the vaults are underground facilities, there is no sunlight that would stimulate the growth of algae often associated with elevated nutrient levels.
- In-stream residence time for the stormwater discharged from the Flow Impact Offset Facility is only a matter of hours (the time it takes water to flow from the discharge points in the airport vicinity to the creeks' discharge points in Puget Sound). Therefore, there will be minimal opportunity for biological activity (algae blooms) in the creeks. Such water quality impacts from nutrients are typically associated with lakes and ponds,

where long residence time would provide the opportunity for excess algae growth to occur. Since no lakes or ponds occur in the creeks between the airport and Puget Sound, this is not an issue.

Given the above, the Port does not propose any monitoring for nutrients in the discharges from the Flow Impact Offset Facility. Through continued implementation of the SWPPP, the BMPs currently in place that manage the use of fertilizers will continue to minimize the opportunities for nutrients to enter stormwater runoff.

#### Metals

Metals of concern include copper, lead, and zinc. Water quality standards for metals are based on the dissolved fraction, are dependant of the hardness of the water, and are applicable to the receiving waters. Chemistry data from existing airfield stormwater discharges (which are typical of the stormwater that would be reserved for release during low flow periods) have been reported in the Annual Stormwater Monitoring Reports. Metal concentrations in these discharges are reported as total recoverable metals, which are not directly comparable to the dissolved fraction listed in the water quality standards. However, this data does serve as an indication of metal concentrations to be expected in the discharges of stormwater from the Flow Impact Offset Facility. Median metals concentrations from airfield stormwater range from 0.014 – 0.031 mg/l copper, 0.001 – 0.002 mg/l lead, and 0.020 – 0.052 mg/l zinc. Note that these values are for end-of-pipe or within-pipe discharges, not the receiving waters. These metal concentrations are also less than typical urban runoff, as discussed in the Annual Stormwater Monitoring Reports. In addition, the Port has conducted Whole Effluent Toxicity testing of stormwater discharges, as

required by its NPDES permit, as discussed in the Annual Monitoring Reports. Stormwater associated with airfield sub-basins met the performance standards for whole effluent toxicity according to Ecology guidelines. All this information indicates that the Flow Impact Offset Facility can be managed to meet the water quality standards for metals in the receiving waters

The following items should be considered in the management of the Flow Impact Offset Facility for compliance with state water quality standards:

- A large portion of metals in urban stormwater is attributed to motor vehicle activity. This is illustrated in the Annual Stormwater Monitoring Reports that show higher metal concentrations are associated with the landside basins where motor vehicle activity is concentrated. Since access to the airfield is strictly controlled, motor vehicle activity is kept to a minimum, therefore, metal concentrations in stormwater runoff is minimized. The airfield basins are the areas that will be providing stormwater to the Flow Impact Offset Facility, and these areas typically have the lowest lead and zinc concentrations of all airport stormwater discharges (copper concentrations are more consistent in all airport stormwater discharges, but are still relatively low in airfield stormwater).
- Data collected by the Port show that a large fraction of the metal concentrations are associated with particulates, i.e., the metal ions are bound to particulate matter. Therefore, the design and management practices proposed to minimize or reduce particulates and turbidity will also reduce total metal concentrations in the stormwater discharges. Biofiltration swales, settling in vaults, and (additional) filtration are all

effective in reducing particulates, and therefore total metal concentrations will be reduced as well. Although these BMPs are not effective in removing dissolved metals, the majority of the metals are bound to particulates and will be removed. The design features proposed for the reserved stormwater vaults (compartmentalized storage, sloping the vault floor away from the stormwater outlets, careful placement of the stormwater outlets, and the provision for installation of filters) will ensure that the discharge of sediments and metals bound to particles will be minimized.

- The Port is currently investigating filtration of stormwater associated with discharges from a landside basin. This research includes determining the effectiveness of several filtration media in treating the stormwater. The results of this study will be completed before final design of the flow offset facilities, and the data will be used to select the filtration method most appropriate to treat the discharge from the Flow Impact Offset Facility, if needed.

Operational and monitoring considerations for the Flow Impact Offset Facility related to managing metals are discussed in Section xxxx.

#### Other Considerations

There are several other considerations relating to the design and operation of the Flow Impact Offset Facility. They include:

- The discharge points for Flow Impact Offset Facility will be the same as the typical (“live”) discharge point for each vault or pond they are associated with. This eliminates the need to permit and construct additional discharge points to the creeks. The proposed location of each stormwater discharge point for the Flow Impact Offset Facility are illustrated in the drawings in Appendix xx.
- All stormwater management facilities, including those associated with the Flow impact Offset facility, will be located within the airport’s perimeter fencing, thereby controlling access to the facilities and reducing the potential for damage to the facilities from vandalism.
- The Port will operate, inspect, monitor, and maintain the Flow impact Offset Facility as long as there is an airport at the site. In addition, the Port will provide annual monitoring reports to ensure that the Flow Impact Offset Facility is meeting its performance goals. An adaptive management method will be used to allow for needed adjustments in the operation of the facilities, and to allow for the installation of new management/monitoring technology, if needed.



### **Operation and Monitoring Plan/Annual Schedule of Activities**

This section discusses details of the Port's proposed Operation and Monitoring Plan for the Flow Impact Offset Facility. The Operation and Monitoring Plan will be finalized and submitted to Ecology after final design of the facility is completed and before operation commences. The final plan will be based on the proposal discussed in this section and will include:

- A detailed annual schedule of operation for each stormwater vault associated with the Flow Impact Offset Facility
- A monitoring plan, including specific monitoring locations, sampling protocols, etc.
- A reporting plan
- An inspection/maintenance plan
- Characterization of the stormwater quality associated with the Flow Impact Offset Facility, based on existing data and data currently being collected.

The proposal discussed in this section includes information on all the points listed above. However, slight changes in configurations of vaults and conveyance may take place during final design of the facilities, and data currently being collected by the Port may suggest other modifications to the proposed facilities. The Final Plan will include the details and specificity that is not available at the present time.

### **Annual Operating Schedule**

This section contains the proposed annual operating schedule for the Flow Impact Offset Facility. The schedule is based on a calendar year for ease of presentation. The proposed

schedule also combines the four proposed stormwater vaults associated with the Flow Impact Offset Facility into a common schedule. It is envisioned that each vault may have a specific operating schedule once final design details are available. The specific operating schedule for an individual vault may be determined based on the final size of the vault, the contributing drainage area to the vault, the time required to fill the vault, and the hydrologic regime of the stream system to which it discharges. Specific operating schedules for each vault will be submitted to Ecology in the Final plan.

Following is the proposed general operating schedule for the Flow impact Offset Facility:

- January through May
  - Operate as “normal” stormwater detention vaults
    - Flow Impact Offset outlet closed to accumulate water
    - Placement of inlet will allow stormwater to be flushed with “fresh” water
    - Monitor filling of stormwater vault (see monitoring plan for details)
  
- June/July
  - Continue to accumulate stormwater
  - Conduct small test release from each vault associated with the Flow Impact Offset Facility
    - Confirm/adjust discharge rates for Flow Impact Offset Facility (gauge/measure flow)

- Collect water quality samples from each discharge (see monitoring section for details)
  - Compare test discharge water quality samples to existing water quality collected data from creeks during summer periods (corresponding to the periods when the Flow Impact Offset Facility would be discharging)
  - If potential water quality violations are indicated, take appropriate actions prior to initiating operational discharge, such as:
    - Install/maintain filters for sediments/turbidity/metals
    - Install portable aerators for DO
    - No actions proposed for temperature and nutrients
  - Continue to monitor filling of vaults associated with the Flow Impact Offset Facility
- July/August
    - Open each outlets associated with the Flow Impact Offset Facility
      - July 24<sup>th</sup> for Des Moines Creek facility
      - August 1 for Miller and Walker Creek Facilities
    - Conduct sampling as described in monitoring plan
  - August through October
    - Operation of Flow Impact Offset Facility
      - Inspection/monitoring of each discharge

- Inspect/monitor (sample) weekly throughout duration of operation
  - Any water quality violations will be immediately investigated and corrected
  - Continue to monitor water levels in reserved stormwater storage areas
- November 1 (or later, upon commencement of seasonal rains)
    - Close outlets associated with the Flow Impact Offset Facility
      - If water remains in vaults and seasonal rains have not commenced, allow discharge to continue until water is exhausted or rains begin
    - Inspect vaults/complete annual maintenance
    - Compile collected data
    - Begin accumulating water in vaults associated with the Flow Impact Offset Facility
      - Close outlets associated with Flow Impact Offset Facility upon completion of inspection/maintenance, but no later than January 1
    - Continue monitoring water levels in vaults associated with the Flow Impact Offset Facility
- December 31
    - Submit annual data report to Ecology by December 31

## **Monitoring Plan**

The Port is proposing a comprehensive monitoring plan for the Flow Impact Offset Facility to ensure that the water quality performance standards are met and no violations of state water quality standards occur in the receiving waters. Monitoring consists of three elements: characterization of existing/expected water quality, monitoring of annual test releases from the Flow Impact Offset Facility, and monitoring of the discharges and receiving waters during operation of the facility. Each element is discussed below.

### **Characterization of existing/expected water quality**

A great deal of water quality data already exists on the Port's stormwater discharges and on the creeks. This data has been collected for a variety of purposes including satisfying the Port's NPDES permit requirements, basin planning activities, and other studies done in the area by the Port and others. The data set includes water quality measurements within the stream systems during the summer periods when the Flow Impact Offset Facility will be scheduled to discharge to the streams. In addition, the Port has started to collect data to characterize the discharges from the Flow impact Offset Facility. Temperature data is being collected starting in 2001 from the NEPL vault and the SDS3 vault in order to characterize the expected temperatures of the Flow Impact Offset Facility. The Port has collected temperature data in the creeks since September 2000, so comparisons can be made. The NEPL vault is partially exposed to sunlight (on its west side and top), while the SDS3 vault is completely underground. By collecting temperature data from both vaults, a range of expected temperatures can be established for each type of vault (buried and partially exposed). Temperature data will be collected from the dead storage zone in each vault in order to approximate the Flow impact Offset Facility. Other data may be collected,

as needed, prior to the operation of the Flow Impact Offset Facility, that can be used to characterize the proposed discharges and expected water quality within the creeks during the summer months, when the facility will be discharging. All of this data will be analyzed and presented in the Final Operation and Monitoring Plan, which will be submitted to Ecology prior to the initial operation of the Flow Impact Offset Facility.

#### Monitoring of annual test releases from the Flow Impact Offset Facility

Each year, in May or June, prior to the operation of the Flow Impact Offset Facility, the Port proposes to conduct small test discharges from each outlet. The test discharges are intended to confirm the operation of each discharge, and to detect and respond to potential problems prior to the annual operation of the Flow Impact Offset Facility. For example, because of the small orifices needed to control discharges to the required rate, small amount of debris in the orifice could significantly impact the discharge rates. Debris can be removed at this time to prevent impacts to the annual operation of the facility. Any other problems that may occur within the facility can be detected and corrected at this time.

Water quality sampling of small-volume test discharges is proposed. By conducting this sampling, potential water quality problems can be detected and corrective measures taken prior to scheduled annual releases to the stream systems. Water quality data obtained from the test discharges will be compared to the characterization data in the creeks to determine the potential for water quality violations. If any are indicated, the Port will take corrective action prior to the annual operation of the facility, such as installing portable aerators or additional filtration in the discharges prior to their entry into the creeks.

Water quality sampling of the test discharges will include:

- Flow (measured/gauged in the field)
- Turbidity (field measurement)
- Dissolved Oxygen (field measurement)
- Temperature (field measurement)
- Metals (copper, lead, and zinc, (grab samples))

### Operational Monitoring

The Port is proposing to monitor the operation of the Flow Impact Offset Facility to provide assurance that the facility is achieving its performance goals and not causing any water quality violations in the receiving waters. This will be accomplished by periodic monitoring of both the discharge and receiving waters during the annual operation of the facility. The specific monitoring proposal for the Flow Impact Offset Facility includes:

- Water levels within the stormwater vaults
  - Installation of a pressure transducer and datalogger in each vault will allow daily water levels to be collected
  - This data can be applied to the vault geometry to calculate the volume of water in the stormwater vaults
  - Vault filling and emptying (average daily water levels) will be monitored throughout the year

- Flow
  - Measured/gauged in the field
  - Taken upon opening of the Flow Impact Offset Facility outlets
  - Taken weekly throughout annual operation of facility
  
- Turbidity
  - Field Measurements
  - Measured in discharges, upstream in receiving waters, downstream in receiving waters after thorough mixing
  - Taken upon opening of Flow impact Offset Facility outlets
  - Taken weekly throughout operation of facility
  
- Dissolved Oxygen
  - Field Measurements
  - Measured in discharges and downstream in receiving waters after thorough mixing
  - Taken upon opening of Flow Impact Offset Facility outlets
  - Taken weekly throughout operation of facility
  
- Temperature
  - Field Measurements



- Measured in discharges, upstream in receiving waters, downstream in receiving waters after thorough mixing
  - Taken upon opening of Flow Impact Offset Facility outlets
  - Taken weekly throughout operation of facility
- Metals
    - Grab samples analyzed for copper, lead, and zinc
    - Measured in discharges and receiving waters downstream after thorough mixing
    - Measured upon opening of Flow impact Offset Facility outlets
    - Measured monthly throughout operation of facility

Weekly monitoring of the discharges for the quality parameters (except metals) is sufficient because the facility will be discharging from a stored volume of water, i.e., the water quality of the discharges is not expected to change. In the event of a significant rainfall event during the operation of the facility (greater than 0.5 inches in a 24-hour period), the Port will conduct additional sampling to ensure that the rainfall did not substantially change the character of the water within the Flow Impact Offset Facility that could potentially cause a violation of in-stream water quality standards. Monthly sampling for metals is sufficient because existing data shows that the metals concentrations in stormwater runoff from the airfield is relatively consistent and low compared to stormwater discharges from other urban areas.

Specific monitoring locations, both of the discharges and in-stream, will be included in the Final Operation and Monitoring Plan to be submitted to Ecology prior to the initial operation of the

Flow impact Offset Facility. All water quality data will be recorded and reported in an annual monitoring report to be submitted to Ecology by December 31 of each year. If the monitoring data show that the discharges from the Flow Impact Offset Facility consistently meet water quality standards within the receiving waters, the Port may propose a modified monitoring plan for subsequent operation of the facility. If any water quality problems were encountered during operation of the facilities, the Annual Report will include a discussion of the immediate actions taken to address the problem, and actions taken or proposed to prevent a reoccurrence of the problem in the future. All sampling and analytical methods used to monitor the Flow Impact Offset Facility will conform to the latest revision of the *Guidelines Establishing Test Procedures for the Analysis of Pollutants* contained in 40 CFR Part 136 or to the latest revision of *Standard Methods for the Examination of Water and Wastewater* (APHA). This will make the monitoring methods for the Flow Impact Offset Facility consistent with other water quality monitoring done under the NPDES permit for the airport.

#### Biological Monitoring

Biological monitoring is proposed as part of the Natural Resources Mitigation Plan. This monitoring will evaluate changes in the Benthic Index of Biotic Integrity (BIBI) over a ten-year period. This monitoring will be able to be used in assessing any biological effects of the flow offset facility in the receiving waters.

## **Maintenance Plan**

The Port develops operation and maintenance manuals for all of its stormwater facilities. While the manuals follow the same generalized schedule for inspection and maintenance, each facility has its own specific manual to address the unique features of each facility. Following is an outline of what is contained in existing operation and maintenance plans for facilities including the Tye Pond Detention Facility, the Miller Creek Detention Facility, the North Employee Parking Lot, the North Cargo Area Pump Station, the North End Snow Melt Facility, the North Satellite Snow Melt Facility, and the South End Snow Melt Facility. Typical inspection/maintenance activities occur on monthly, quarterly, semi-annual, or annual intervals, depending on the facility and activity. Operation and maintenance manuals developed for the stormwater vaults associated with the Flow Impact Offset Facility will contain similar elements and at least the same frequency of activities.

### **General Operation and Maintenance Manual Outline**

- Purpose of Facility/Purpose of Manual
- Description of Facility and Operation
  - General Description
  - Hydraulic Properties
  - Water Quality Properties
  - Monitoring Systems/Alarms
- Historical Maintenance Operations (as applicable)
- Site/Facility Access
- Personnel and Emergency Contacts

- **Inspection and Maintenance Procedures (items included as applicable to each facility)**

- Culverts
- Ponds
- Vegetation
- Inlet Structure(s)
- Outlet Structure(s)
- Sensors and Alarms
- Pumps and Valves
- Filters/other water quality facilities
- Access Roads/Hatches
- Berms/Dikes/Levees
- Spillways/Slopes
- Creek/Stream Protection
- Photographs

- **Inspection and Maintenance Schedule**
- **Sediment/Waste Disposal Requirements**
- **Inspection Forms and Maintenance Checklist**
- **Maintenance Work Order Request Form**
- **Maintenance Crew/Shop Crew Responsibilities**
- **Health and Safety Plan**
- **Combined Space Entry Program**
- **Drawings/As Built Diagrams**
- **Process Diagrams**

- **References/Equipment Manuals**

A sample operation and maintenance manual is included in Appendix XX. Each stormwater vault associated with the Flow Impact Offset Facility will have its own operation and maintenance manual developed as part of the final design process. The operation and maintenance manual for these stormwater vaults will be included in the overall operation and maintenance manual for the Flow Impact Offset Facility to be submitted to Ecology prior to initial operation of the facility.

## **DES MOINES CREEK**

**7-DAY LOW FLOW OCCURRENCES IN DES MOINES CREEK (1994)**

**HISTOGRAM LOW FLOW OCCURRENCES IN DES MOINES CREEK (1994)**

**7-DAY LOW FLOW OCCURRENCES IN DES MOINES CREEK (2006)**

**HISTOGRAM LOW FLOW OCCURRENCES IN DES MOINES CREEK (2006)**

**7-DAY LOW FLOW OCCURRENCES IN DES MOINES CREEK  
(WITH MITIGATION)**

**HISTOGRAM LOW FLOW OCCURRENCES IN DES MOINES CREEK  
(WITH MITIGATION)**

**COMPARISON OF 7-DAY LOW FLOW BY YEAR**

**COMPARISON OF 7-DAY LOW FLOW BY RANK**

**PLOTTED 7-DAY LOW FLOWS BY PERCENT RETURN FREQUENCY**

**SUMMARY OF LOW STREAM FLOW MITIGATION VAULT STORAGE AND  
FILLING**

**LOW FLOW HYDROGRAPHS WITH MITIGATION (1991-1994)**

**DES MOINES CREEK**  
**7-DAY LOW FLOW OCCURRENCES IN DES MOINES CREEK**  
**(1994)**

**AR 011293**

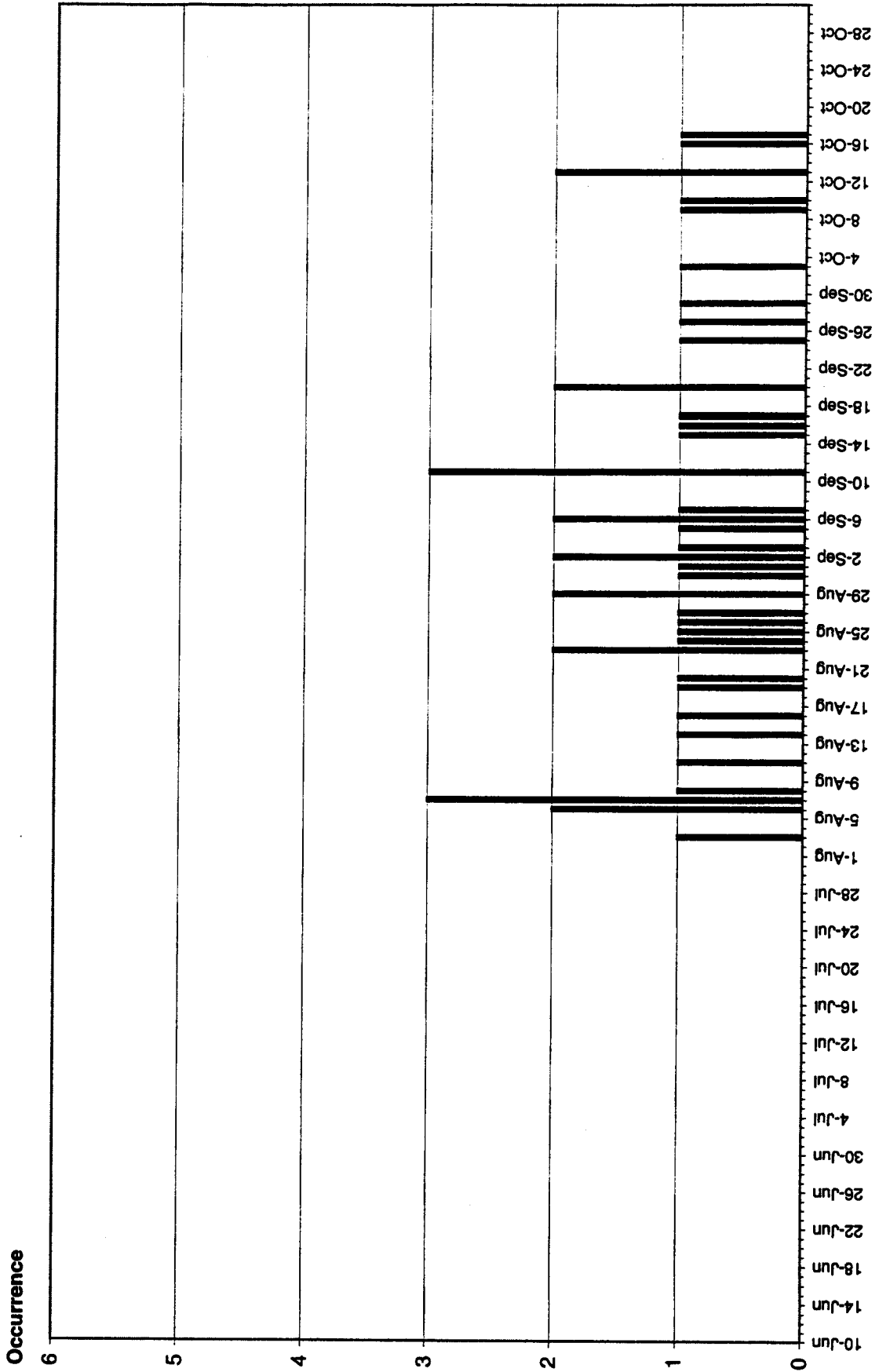




**DES MOINES CREEK**  
**HISTOGRAM LOW FLOW OCCURRENCES**  
**IN DES MOINES CREEK (1994)**

**AR 011295**

**7-Day Low Flow Occurrences in Des Moines Creek, 1949-1995 (1994 HSPF)**



**DES MOINES CREEK**  
**7-DAY LOW FLOW OCCURRENCES IN DES MOINES CREEK**  
**(2006)**

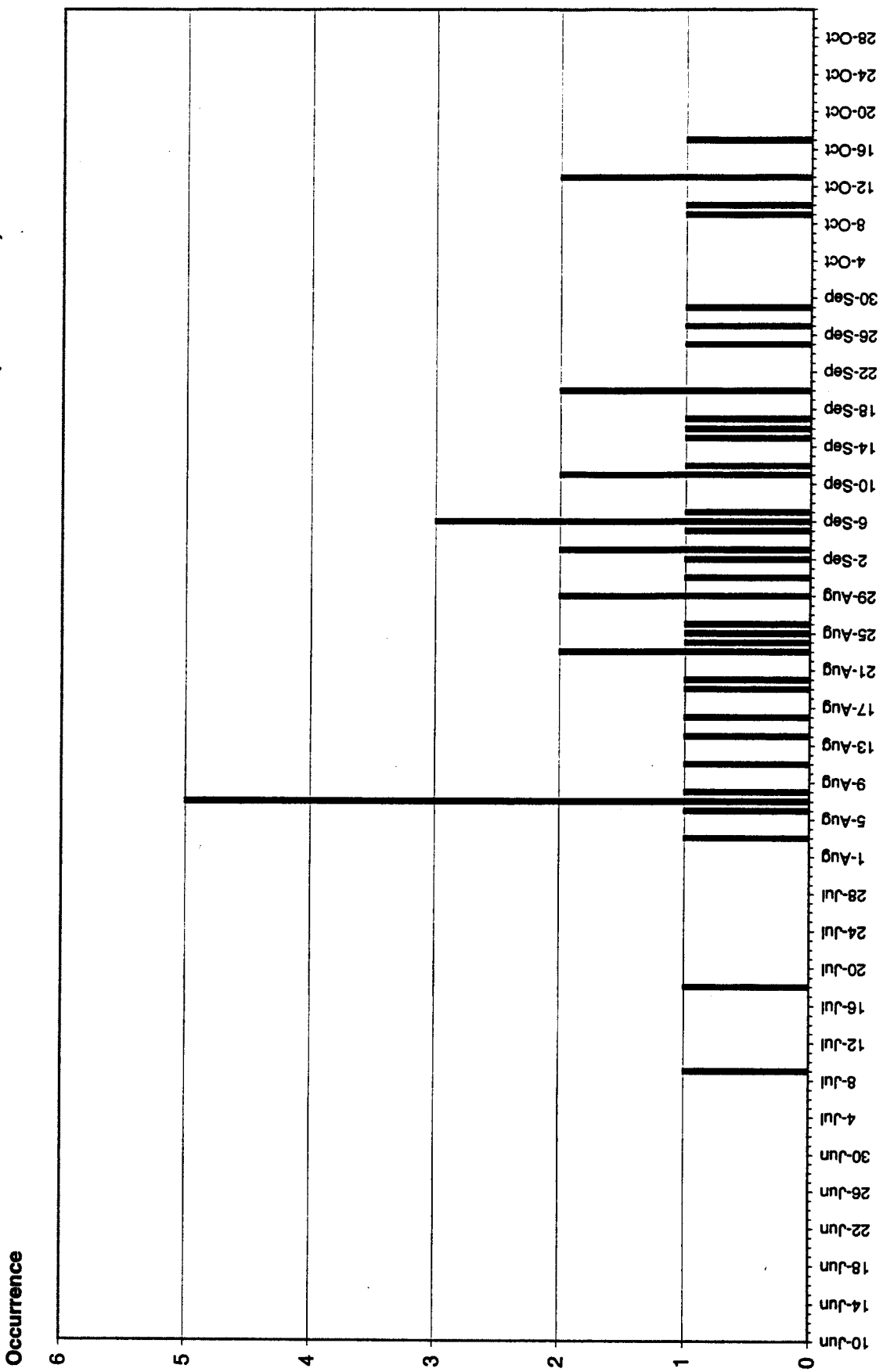
**AR 011297**

Start of 7-Day Low Flows with Average Flow Rates				Statistical Ranking of Average 7-Day Low Flows Period of Record: 1949-1995				
2006 HSPF Des Moines Creek at 200th St. / Flow cfs Average 7-Day Low Flow								
Date				Average 7-Day Lows			Return Frequency	
Date				Date	Ordered	Rank	Rank/47+1	Return Frequency
1949	SEP	7	0.14	1977	0.12	1	0.02	2.1
1950	SEP	17	0.32	1949	0.14	2	0.04	4.2
1951	AUG	20	0.25	1952	0.15	3	0.06	6.3
1952	OCT	13	0.15	1994	0.16	4	0.08	8.3
1953	SEP	15	0.28	1979	0.17	5	0.10	10.4
1954	AUG	7	0.37	1988	0.18	6	0.13	12.5
1955	SEP	6	0.25	1958	0.19	7	0.15	14.6
1956	SEP	3	0.35	1985	0.19	8	0.17	16.7
1957	SEP	20	0.25	1986	0.19	9	0.19	18.8
1958	SEP	2	0.19	1993	0.19	10	0.21	20.8
1959	AUG	24	0.28	1973	0.20	11	0.23	22.9
1960	AUG	7	0.28	1981	0.21	12	0.25	25.0
1961	AUG	23	0.30	1974	0.22	13	0.27	27.1
1962	SEP	3	0.27	1987	0.22	14	0.29	29.2
1963	SEP	27	0.31	1990	0.23	15	0.31	31.3
1964	AUG	31	0.39	1966	0.24	16	0.33	33.3
1965	AUG	3	0.30	1967	0.24	17	0.35	35.4
1966	AUG	19	0.24	1975	0.24	18	0.38	37.5
1967	AUG	25	0.24	1992	0.24	19	0.40	39.6
1968	AUG	7	0.38	1951	0.25	20	0.42	41.7
1969	SEP	6	0.29	1955	0.25	21	0.44	43.8
1970	JUL	18	0.25	1957	0.25	22	0.46	45.8
1971	AUG	14	0.31	1970	0.25	23	0.48	47.9
1972	AUG	8	0.42	1980	0.25	24	0.50	50.0
1973	SEP	11	0.20	1991	0.25	25	0.52	52.1
1974	OCT	13	0.22	1962	0.27	26	0.54	54.2
1975	AUG	11	0.24	1976	0.27	27	0.56	56.3
1976	OCT	17	0.27	1984	0.27	28	0.58	58.3
1977	AUG	16	0.12	1953	0.28	29	0.60	60.4
1978	JUL	9	0.33	1959	0.28	30	0.63	62.5
1979	AUG	7	0.17	1960	0.28	31	0.65	64.6
1980	AUG	23	0.25	1969	0.29	32	0.67	66.7
1981	SEP	12	0.21	1995	0.29	33	0.69	68.8
1982	AUG	6	0.32	1961	0.30	34	0.71	70.8
1983	OCT	10	0.40	1965	0.30	35	0.73	72.9
1984	AUG	29	0.27	1989	0.30	36	0.75	75.0
1985	AUG	29	0.19	1963	0.31	37	0.77	77.1
1986	SEP	5	0.19	1971	0.31	38	0.79	79.2
1987	SEP	6	0.22	1950	0.32	39	0.81	81.3
1988	SEP	11	0.18	1982	0.32	40	0.83	83.3
1989	AUG	7	0.30	1978	0.33	41	0.85	85.4
1990	SEP	25	0.23	1956	0.35	42	0.88	87.5
1991	OCT	9	0.25	1954	0.37	43	0.90	89.6
1992	SEP	16	0.24	1988	0.38	44	0.92	91.7
1993	SEP	29	0.19	1964	0.39	45	0.94	93.8
1994	AUG	26	0.16	1983	0.40	46	0.96	95.8
1995	SEP	20	0.29	1972	0.42	47	0.98	97.9

Rank = Numerical position of ordered average 7-day low flow values with the driest year equal to one.  
N = 47

**DES MOINES CREEK  
HISTOGRAM LOW FLOW OCCURRENCES  
IN DES MOINES CREEK (2006)**

**7-Day Low Flow Occurrences in Des Moines Creek, 1949-1995 (2006 HSPF)**



**DES MOINES CREEK**  
**7-DAY LOW FLOW OCCURRENCES IN DES MOINES CREEK**  
**(WITH MITIGATION)**

**AR 011301**

Start of 7-Day Low Flows with Average Flow Rates				Statistical Ranking of Average 7-Day Low Flows Period of Record: 1949-1995				
2006 HSPF Des Moines Creek at 200th St. / Flow cfs				Average 7-Day Lows				Return
Average 7-Day Low Flow Jul 24 Release 0.10 cfs				Date	Ordered	Rank	Rank/47+1	Frequency
1949	JUL	16	0.22	1977	0.22	1	0.02	2.1
1950	SEP	17	0.42	1949	0.22	2	0.04	4.2
1951	AUG	20	0.35	1979	0.23	3	0.06	6.3
1952	OCT	13	0.25	1994	0.23	4	0.08	8.3
1953	SEP	15	0.38	1985	0.24	5	0.10	10.4
1954	AUG	7	0.47	1952	0.25	6	0.13	12.5
1955	SEP	6	0.35	1970	0.25	7	0.15	14.6
1956	SEP	3	0.45	1987	0.26	8	0.17	16.7
1957	SEP	20	0.35	1988	0.28	9	0.19	18.8
1958	SEP	2	0.29	1986	0.29	10	0.21	20.8
1959	AUG	24	0.38	1958	0.29	11	0.23	22.9
1960	AUG	7	0.38	1993	0.29	12	0.25	25.0
1961	AUG	23	0.40	1973	0.30	13	0.27	27.1
1962	SEP	3	0.37	1981	0.31	14	0.29	29.2
1963	SEP	27	0.41	1974	0.32	15	0.31	31.3
1964	AUG	31	0.49	1978	0.33	16	0.33	33.3
1965	JUL	13	0.35	1990	0.33	17	0.35	35.4
1966	AUG	19	0.34	1976	0.33	18	0.38	37.5
1967	AUG	25	0.34	1966	0.34	19	0.40	39.6
1968	AUG	7	0.48	1992	0.34	20	0.42	41.7
1969	SEP	6	0.39	1967	0.34	21	0.44	43.8
1970	JUL	17	0.25	1975	0.34	22	0.46	45.8
1971	AUG	14	0.41	1957	0.35	23	0.48	47.9
1972	AUG	8	0.52	1951	0.35	24	0.50	50.0
1973	SEP	11	0.30	1991	0.35	25	0.52	52.1
1974	OCT	13	0.32	1955	0.35	26	0.54	54.2
1975	JUL	17	0.34	1965	0.35	27	0.56	56.3
1976	DEC	10	0.33	1980	0.35	28	0.58	58.3
1977	AUG	16	0.22	1982	0.36	29	0.60	60.4
1978	JUL	9	0.33	1984	0.37	30	0.63	62.5
1979	JUN	12	0.23	1962	0.37	31	0.65	64.6
1980	AUG	23	0.35	1959	0.38	32	0.67	66.7
1981	SEP	12	0.31	1953	0.38	33	0.69	68.8
1982	JUN	19	0.36	1960	0.38	34	0.71	70.8
1983	OCT	10	0.50	1969	0.39	35	0.73	72.9
1984	AUG	29	0.37	1995	0.39	36	0.75	75.0
1985	JUL	17	0.24	1961	0.40	37	0.77	77.1
1986	SEP	5	0.29	1989	0.40	38	0.79	79.2
1987	OCT	24	0.26	1971	0.41	39	0.81	81.3
1988	SEP	11	0.28	1963	0.41	40	0.83	83.3
1989	AUG	7	0.40	1950	0.42	41	0.85	85.4
1990	SEP	25	0.33	1956	0.45	42	0.88	87.5
1991	OCT	9	0.35	1954	0.47	43	0.90	89.6
1992	JUN	22	0.34	1968	0.48	44	0.92	91.7
1993	SEP	29	0.29	1964	0.49	45	0.94	93.8
1994	JUL	17	0.23	1983	0.50	46	0.96	95.8
1995	SEP	20	0.39	1972	0.52	47	0.98	97.9

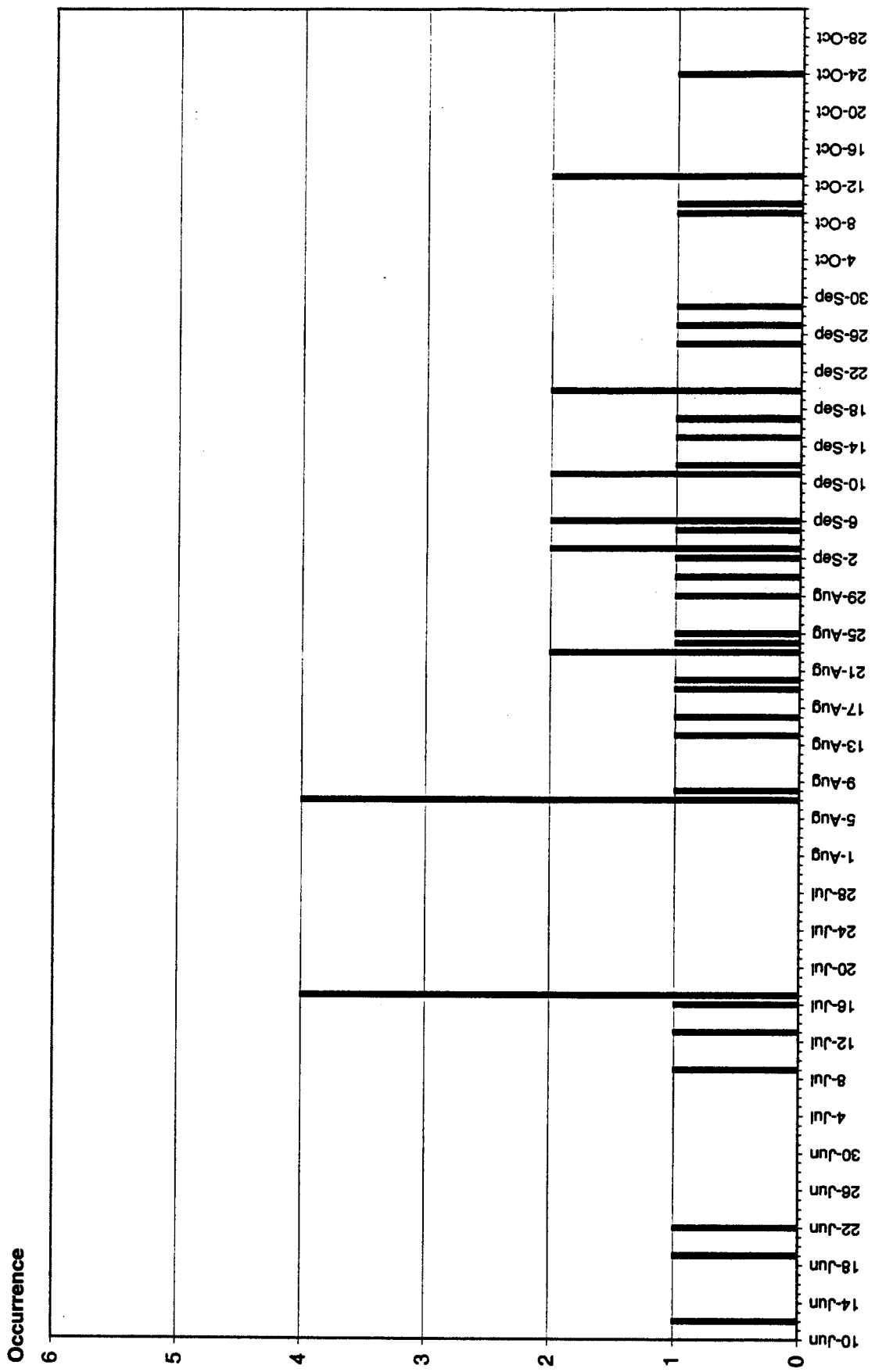
Rank = Numerical position of ordered average 7-day low flow values with the driest year equal to one.

N = 47



**DES MOINES CREEK**  
**HISTOGRAM LOW FLOW OCCURRENCES IN**  
**DES MOINES CREEK**  
**(WITH MITIGATION)**

**7-Day Low Flow Occurrences in Des Moines Creek 1949-1995 ( 24 Jul 2006 HSPF + 0.10 cfs)**



**DES MOINES CREEK  
COMPARISON OF 7-DAY LOW FLOW BY YEAR**

Comparison of July Low Flow By Year  
for Des Moines Creek

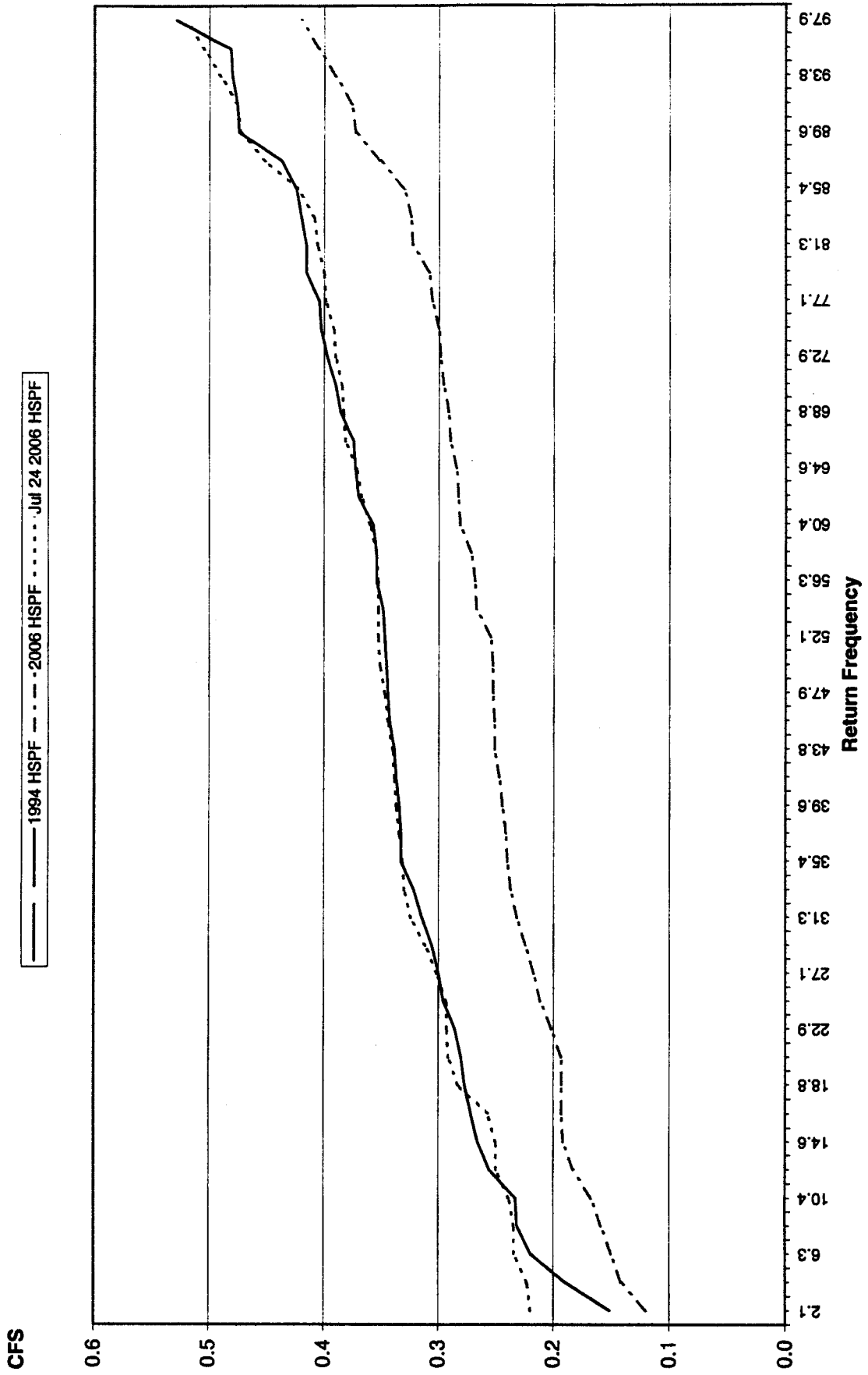
1994 HSPF		2006 HSPF		-1994		2006 HSPF		-1994			
Des Moines Creek at 200th St. / Flow cfs Average 7-Day Low Flow		Des Moines Creek at 200th St. / Flow cfs Average 7-Day Low Flow		Des Moines Creek at 200th St. / Flow cfs Average 7-Day Low Flow		Des Moines Creek at 200th St. / Flow cfs Average 7-Day Low Flow		Des Moines Creek at 200th St. / Flow cfs Average 7-Day Low Flow			
Date		Date		Date		Date		Date			
1949	SEP 7	0.19		1949	SEP 7	0.14		1949	JUL 16	0.03	
1950	SEP 17	0.40		1950	SEP 17	0.32		1950	SEP 17	0.42	
1951	AUG 20	0.36		1951	AUG 20	0.25		1951	AUG 20	0.35	
1952	OCT 13	0.22		1952	OCT 13	0.15		1952	OCT 13	0.25	
1953	SEP 15	0.35		1953	SEP 15	0.28		1953	SEP 15	0.38	
1954	AUG 7	0.48		1954	AUG 7	0.37		1954	AUG 7	0.47	
1955	SEP 6	0.35		1955	SEP 6	0.25		1955	SEP 6	0.35	
1956	SEP 3	0.44		1956	SEP 3	0.35		1956	SEP 3	0.45	
1957	SEP 20	0.33		1957	SEP 20	0.25		1957	SEP 20	0.35	
1958	SEP 2	0.29		1958	SEP 2	0.19		1958	SEP 2	0.29	
1959	AUG 24	0.40		1959	AUG 24	0.28		1959	AUG 24	0.38	
1960	AUG 7	0.40		1960	AUG 7	0.28		1960	AUG 7	0.38	
1961	AUG 23	0.42		1961	AUG 23	0.30		1961	AUG 23	0.40	
1962	SEP 2	0.34		1962	SEP 2	0.27		1962	SEP 2	0.37	
1963	SEP 27	0.37		1963	SEP 27	0.31		1963	SEP 27	0.41	
1964	AUG 31	0.48		1964	AUG 31	0.39		1964	AUG 31	0.49	
1965	AUG 3	0.39		1965	AUG 3	0.30		1965	JUL 13	0.35	
1966	AUG 19	0.33		1966	AUG 19	0.24		1966	AUG 19	0.34	
1967	AUG 25	0.35		1967	AUG 25	0.24		1967	AUG 25	0.34	
1968	AUG 6	0.47		1968	AUG 6	0.38		1968	AUG 6	0.48	
1969	SEP 6	0.39		1969	SEP 6	0.29		1969	SEP 6	0.39	
1970	AUG 27	0.33		1970	JUL 18	0.25		1970	JUL 17	0.25	
1971	AUG 14	0.42		1971	AUG 14	0.31		1971	AUG 14	0.41	
1972	AUG 8	0.53		1972	AUG 8	0.42		1972	AUG 8	0.52	
1973	SEP 11	0.28		1973	SEP 11	0.20		1973	SEP 11	0.30	
1974	OCT 13	0.32		1974	OCT 13	0.22		1974	OCT 13	0.32	
1975	AUG 11	0.34		1975	AUG 11	0.24		1975	JUL 17	0.34	
1976	OCT 17	0.34		1976	OCT 17	0.27		1976	DEC 10	0.33	
1977	AUG 16	0.15		1977	AUG 16	0.12		1977	AUG 16	0.22	
1978	OCT 16	0.42		1978	JUL 9	0.33		1978	JUL 9	0.33	
1979	AUG 7	0.23		1979	AUG 7	0.17		1979	JUN 12	0.23	
1980	AUG 23	0.35		1980	AUG 23	0.25		1980	AUG 23	0.35	
1981	SEP 11	0.30		1981	SEP 12	0.21		1981	SEP 12	0.31	
1982	AUG 6	0.42		1982	AUG 6	0.32		1982	JUN 19	0.36	
1983	OCT 10	0.48		1983	OCT 10	0.40		1983	OCT 10	0.50	
1984	AUG 29	0.37		1984	AUG 29	0.27		1984	AUG 29	0.37	
1985	AUG 29	0.27		1985	AUG 29	0.19		1985	JUL 17	0.24	
1986	SEP 5	0.27		1986	SEP 5	0.19		1986	SEP 5	0.29	
1987	SEP 7	0.30		1987	SEP 6	0.22		1987	OCT 24	0.26	
1988	SEP 11	0.26		1988	SEP 11	0.18		1988	SEP 11	0.28	
1989	OCT 3	0.37		1989	AUG 7	0.30		1989	AUG 7	0.40	
1990	SEP 25	0.31		1990	SEP 25	0.23		1990	SEP 25	0.33	
1991	OCT 9	0.34		1991	OCT 9	0.25		1991	OCT 9	0.35	
1992	SEP 16	0.31		1992	SEP 16	0.24		1992	JUN 22	0.34	
1993	SEP 29	0.28		1993	SEP 29	0.19		1993	SEP 29	0.29	
1994	AUG 28	0.23		1994	AUG 28	0.16		1994	JUL 17	0.23	
1995	SEP 20	0.35		1995	SEP 20	0.29		1995	SEP 20	0.39	
		16.32				12.29				16.45	

**DES MOINES CREEK**  
**COMPARISON OF 7-DAY LOW FLOW BY RANK**

Comparison of 7-Day Low Flow by Rank for Des Moines Creek						
Rank	1994	2006	-1994	2006 Release Jul 24	-1994	
1	0.15	0.12	-0.03	0.22	0.07	
2	0.19	0.14	-0.05	0.22	0.03	
3	0.22	0.15	-0.07	0.23	0.01	
4	0.23	0.16	-0.07	0.23	0.00	
5	0.23	0.17	-0.07	0.24	0.01	
6	0.26	0.18	-0.07	0.25	-0.01	
7	0.27	0.19	-0.07	0.25	-0.02	
8	0.27	0.19	-0.08	0.26	-0.01	
9	0.28	0.19	-0.08	0.28	0.01	
10	0.28	0.19	-0.09	0.29	0.01	
11	0.29	0.20	-0.08	0.29	0.01	
12	0.30	0.21	-0.08	0.29	0.00	
13	0.30	0.22	-0.08	0.30	0.00	
14	0.31	0.22	-0.08	0.31	0.01	
15	0.31	0.23	-0.08	0.32	0.01	
16	0.32	0.24	-0.08	0.33	0.01	
17	0.33	0.24	-0.09	0.33	0.00	
18	0.33	0.24	-0.09	0.33	0.00	
19	0.33	0.24	-0.09	0.34	0.00	
20	0.34	0.25	-0.09	0.34	0.00	
21	0.34	0.25	-0.09	0.34	0.00	
22	0.34	0.25	-0.09	0.34	0.00	
23	0.34	0.25	-0.09	0.35	0.00	
24	0.35	0.25	-0.09	0.35	0.01	
25	0.35	0.25	-0.09	0.35	0.01	
26	0.35	0.27	-0.08	0.35	0.00	
27	0.35	0.27	-0.09	0.35	0.00	
28	0.35	0.27	-0.08	0.35	0.00	
29	0.36	0.28	-0.08	0.36	0.00	
30	0.37	0.28	-0.09	0.37	0.00	
31	0.37	0.28	-0.09	0.37	0.00	
32	0.37	0.29	-0.08	0.38	0.01	
33	0.39	0.29	-0.09	0.38	0.00	
34	0.39	0.30	-0.09	0.38	-0.01	
35	0.40	0.30	-0.10	0.39	-0.01	
36	0.40	0.30	-0.10	0.39	-0.01	
37	0.40	0.31	-0.10	0.40	-0.01	
38	0.42	0.31	-0.11	0.40	-0.02	
39	0.42	0.32	-0.09	0.41	-0.01	
40	0.42	0.32	-0.10	0.41	-0.01	
41	0.42	0.33	-0.09	0.42	0.00	
42	0.44	0.35	-0.09	0.45	0.01	
43	0.47	0.37	-0.10	0.47	0.00	
44	0.48	0.38	-0.10	0.48	0.00	
45	0.48	0.39	-0.09	0.49	0.01	
46	0.48	0.40	-0.08	0.50	0.02	
47	0.53	0.42	-0.11	0.52	-0.01	

**DES MOINES CREEK**  
**PLOTTED 7-DAY LOW FLOWS BY**  
**PERCENT RETURN FREQUENCY**

# Des Moines Creek 7-Day Low Flow Values





**DES MOINES CREEK**  
**SUMMARY OF LOW STREAM FLOW MITIGATION**  
**VAULT STORAGE AND FILLING**

**Reserve Storage Vaults for Des Moines Creek.**

**Vault sizes (dead storage in acre ft) required for 92-day release of 0.13 cfs (1949 – 1995).**

	<u>SDS3</u>	<u>SDS4</u>
Mean	4.31	0.75
Median	3.9	0.67
Max	10.4	1.8
Min	1.87	0.32

**Contributing Drainage Areas**

<u>Subbasin</u>	<u>Vault</u>	<u>Area</u>	<u>% Contribution</u>
SDS3	SDS3	199.22	85.25
SDS4	SDS4	34.47	14.75
Total		233.69	

**Fill time for 12.2 acre foot volume / Days to fill after closing vault on January 1.**

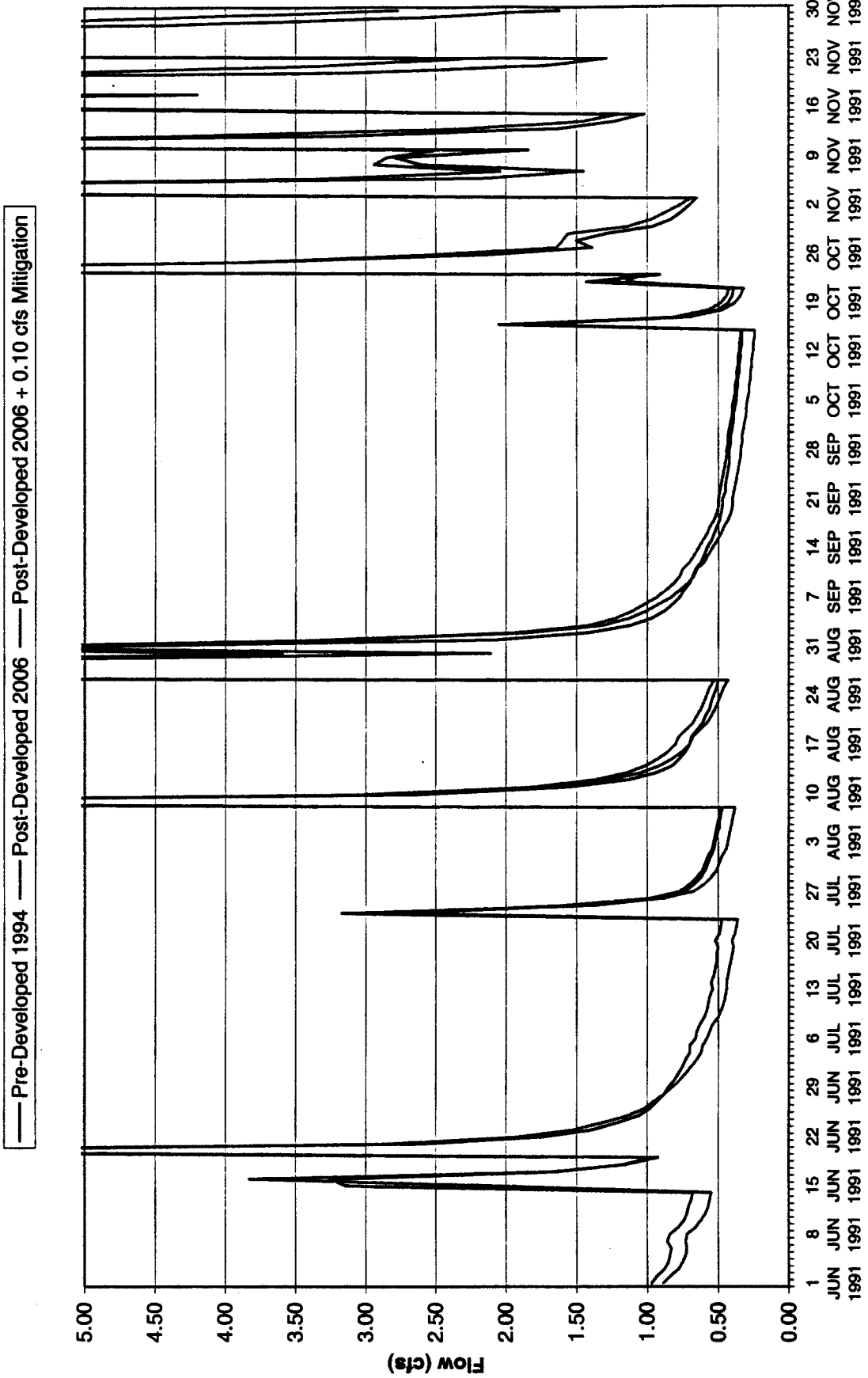
	<u></u>
Mean	8.0
Media	6.0
Max	32.0
Min	2.0

**Remaining volume in vaults on October 23**

	<u>Volume ac ft</u>	<u>Remaining Days</u>
Mean	11.40	57
Median	12.00	60
Max	12.20	61
Min	7.23	36

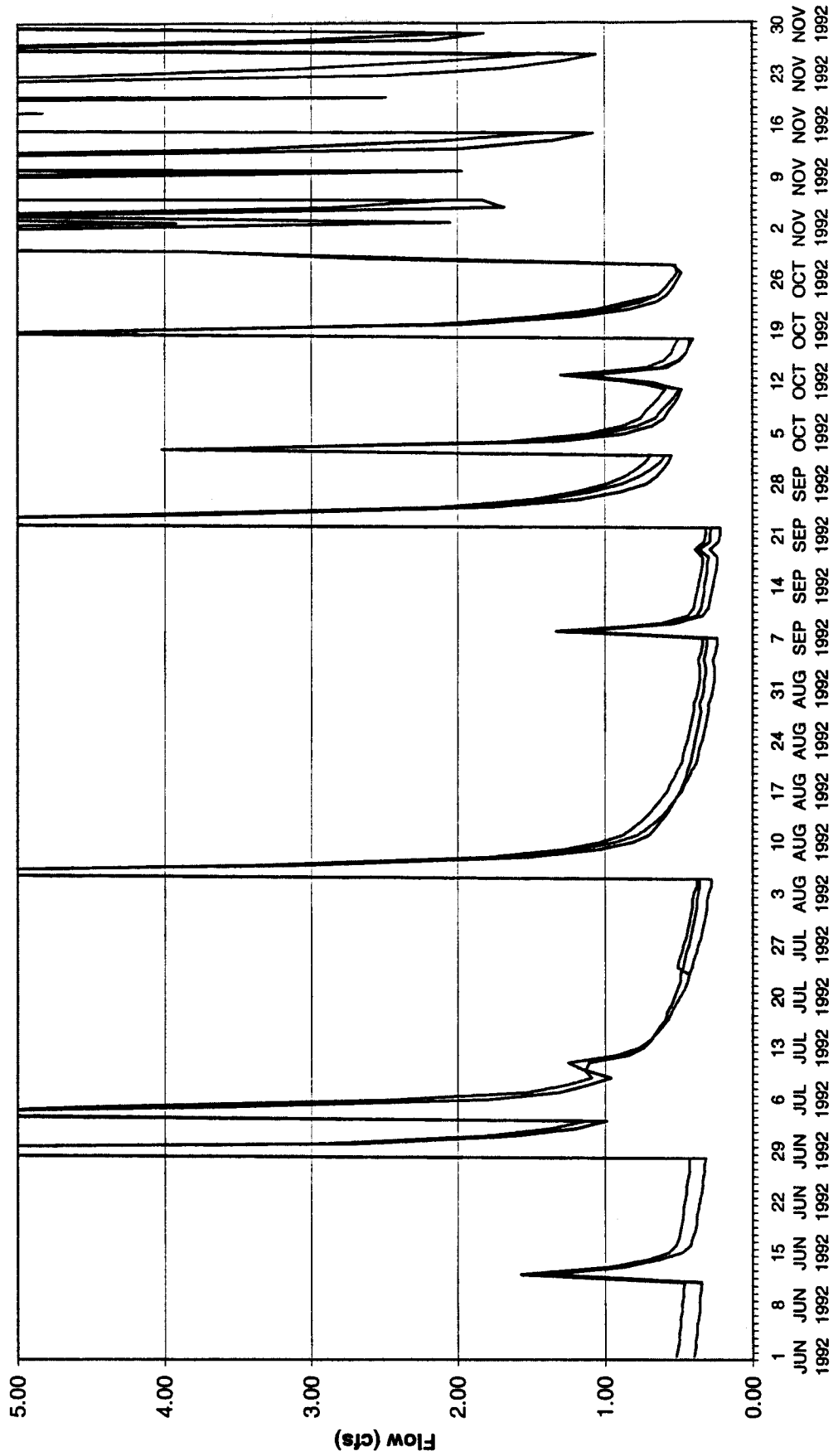
**DES MOINES CREEK**  
**LOW FLOW HYDROGRAPHS WITH MITIGATION (1991-1994)**

# Des Moines Creek Low Flow 1991 + Mitigation



# Des Moines Creek Low Flow 1992 + Mitigation

Pre-Developed 1994   
  Post-Developed 2006   
  Post-Developed 2006 + 0.01 cfs Mitigation



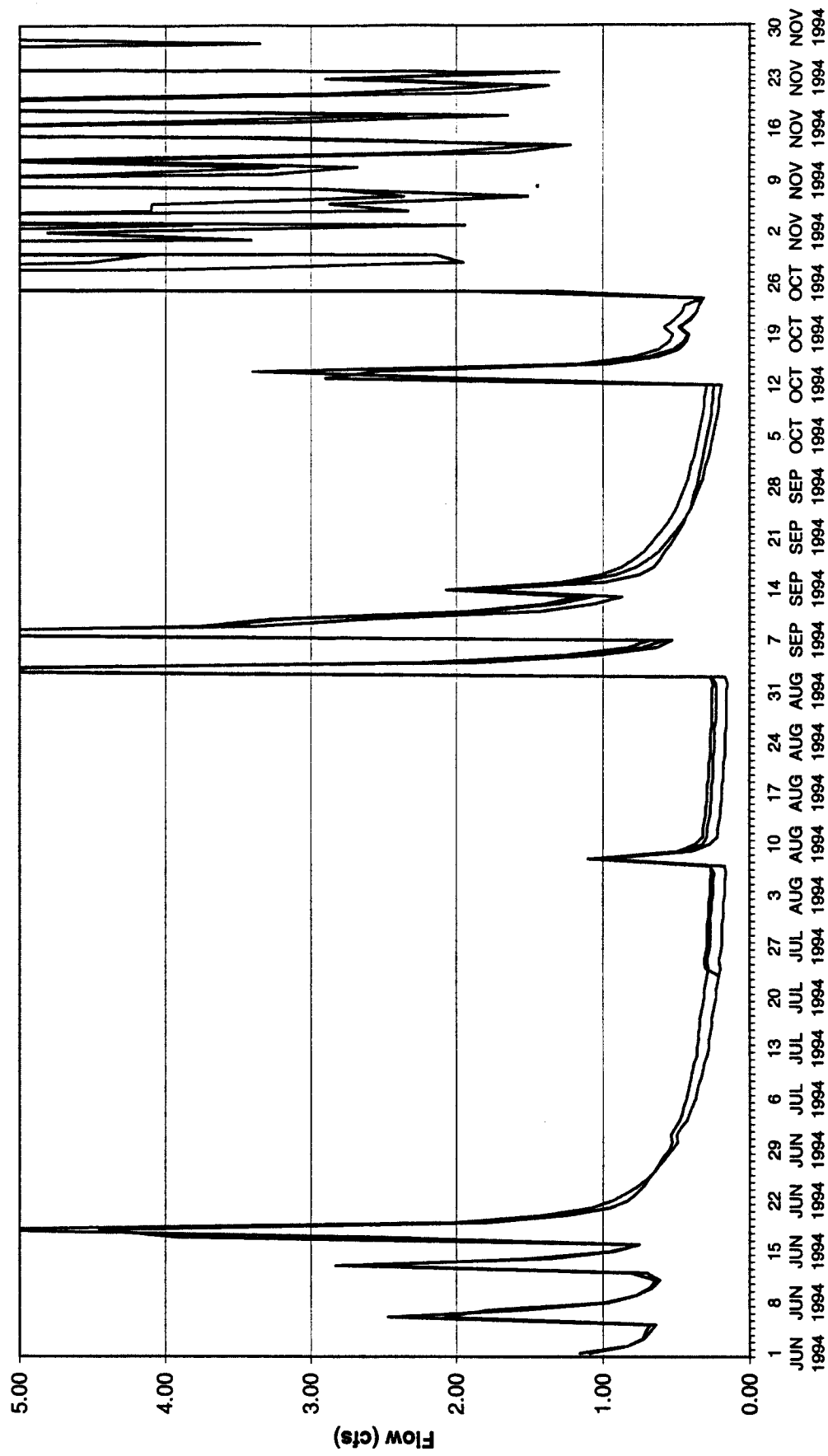
# Des Moines Creek Low Flow 1993 + Mitigation

— Pre-Developed 1994 — Post-Developed 2006 — Post-Developed 2006 + Mitigation



# Des Moines Creek Low Flow 1994 + Mitigation

— Pre-Developed 1994 — Post-Developed 2006 — Post-Developed 2006 + Mitigation



## **MILLER CREEK**

**7-DAY LOW FLOW OCCURRENCES IN MILLER CREEK (1994 HSPF)**

**HISTOGRAM LOW FLOW OCCURRENCES IN MILLER CREEK (1994)**

**7-DAY LOW FLOW OCCURRENCES IN MILLER CREEK 1991 – 1994,  
POST-PROJECT CONDITIONS (2006)**

**HISTOGRAM LOW FLOW OCCURRENCES IN MILLER CREEK 1991 – 1994,  
POST-PROJECT CONDITIONS (2006)**

**7-DAY LOW FLOW OCCURRENCES IN MILLER CREEK, 1991 – 1994, POST-  
PROJECT CONDITIONS (2006+ MITIGATION)**

**HISTOGRAM LOW FLOW OCCURRENCES IN MILLER CREEK 1991 – 1994,  
POST-PROJECT CONDITIONS (2006+ MITIGATION)**

**COMPARISON OF 7-DAY LOW FLOW BY YEAR, 1991-1994**

**COMPARISON OF 7-DAY LOW FLOW BY RANK, 1991-1994**

**PLOTTED 7-DAY LOW FLOWS BY PERCENT RETURN FREQUENCY, 1991-1994**

**SUMMARY OF LOW STREAM FLOW MITIGATION VAULT  
STORAGE AND FILLING**

**LOW FLOW HYDROGRAPHS WITH MITIGATION (1991-1994)**

**7-DAY LOW FLOW OCCURRENCES IN MILLER CREEK (1991-1994)**



**MILLER CREEK**  
**7-DAY LOW FLOW OCCURRENCES IN MILLER CREEK**  
**(1994 HSPF)**

**AR 011319**

Start of 7-Day Low Flows with Average Flow Rates				Statistical Ranking of Average 7-Day Low Flows				
				Period of Record: 1949-1995				
1994				Average				
Miller Creek at				7-Day Lows			Return	
	Date		Hwy 509 Flow / cfs	Date	Ordered	Rank	Rank/N+1	Frequency
1949	SEP	7	0.42	1949	0.42	1	0.02	2.1
1950	SEP	17	0.85	1994	0.49	2	0.04	4.2
1951	SEP	17	0.78	1952	0.52	3	0.06	6.3
1952	NOV	23	0.52	1977	0.52	4	0.08	8.3
1953	SEP	15	0.74	1979	0.54	5	0.10	10.4
1954	OCT	3	0.86	1987	0.57	6	0.13	12.5
1955	SEP	6	0.82	1993	0.57	7	0.15	14.6
1956	SEP	3	0.86	1986	0.58	8	0.17	16.7
1957	SEP	20	0.77	1988	0.59	9	0.19	18.8
1958	SEP	30	0.62	1985	0.61	10	0.21	20.8
1959	AUG	24	0.87	1958	0.62	11	0.23	22.9
1960	SEP	29	0.83	1973	0.64	12	0.25	25.0
1961	SEP	21	0.82	1992	0.64	13	0.27	27.1
1962	SEP	3	0.72	1980	0.66	14	0.29	29.2
1963	SEP	27	0.70	1989	0.66	15	0.31	31.3
1964	OCT	25	0.87	1981	0.68	16	0.33	33.3
1965	SEP	27	0.77	1976	0.68	17	0.35	35.4
1966	SEP	4	0.77	1963	0.70	18	0.38	37.5
1967	SEP	22	0.73	1990	0.70	19	0.40	39.6
1968	AUG	7	0.96	1974	0.71	20	0.42	41.7
1969	SEP	6	0.81	1970	0.71	21	0.44	43.8
1970	AUG	27	0.71	1962	0.72	22	0.46	45.8
1971	AUG	14	0.91	1967	0.73	23	0.48	47.9
1972	SEP	11	1.00	1953	0.74	24	0.50	50.0
1973	SEP	12	0.64	1995	0.75	25	0.52	52.1
1974	OCT	13	0.71	1957	0.77	26	0.54	54.2
1975	AUG	11	0.77	1966	0.77	27	0.56	56.3
1976	DEC	10	0.68	1975	0.77	28	0.58	58.3
1977	AUG	16	0.52	1965	0.77	29	0.60	60.4
1978	AUG	17	0.79	1984	0.77	30	0.63	62.5
1979	OCT	7	0.54	1951	0.78	31	0.65	64.6
1980	OCT	17	0.66	1991	0.79	32	0.67	66.7
1981	SEP	12	0.68	1978	0.79	33	0.69	68.8
1982	SEP	17	0.80	1982	0.80	34	0.71	70.8
1983	OCT	10	0.89	1969	0.81	35	0.73	72.9
1984	OCT	1	0.77	1961	0.82	36	0.75	75.0
1985	SEP	29	0.61	1955	0.82	37	0.77	77.1
1986	OCT	18	0.58	1960	0.83	38	0.79	79.2
1987	OCT	24	0.57	1950	0.85	39	0.81	81.3
1988	SEP	11	0.59	1956	0.86	40	0.83	83.3
1989	OCT	3	0.66	1954	0.86	41	0.85	85.4
1990	SEP	26	0.70	1964	0.87	42	0.88	87.5
1991	OCT	9	0.79	1959	0.87	43	0.90	89.6
1992	SEP	16	0.64	1983	0.89	44	0.92	91.7
1993	NOV	9	0.57	1971	0.91	45	0.94	93.8
1994	OCT	6	0.49	1968	0.96	46	0.96	95.8
1995	SEP	20	0.75	1972	1.00	47	0.98	97.9

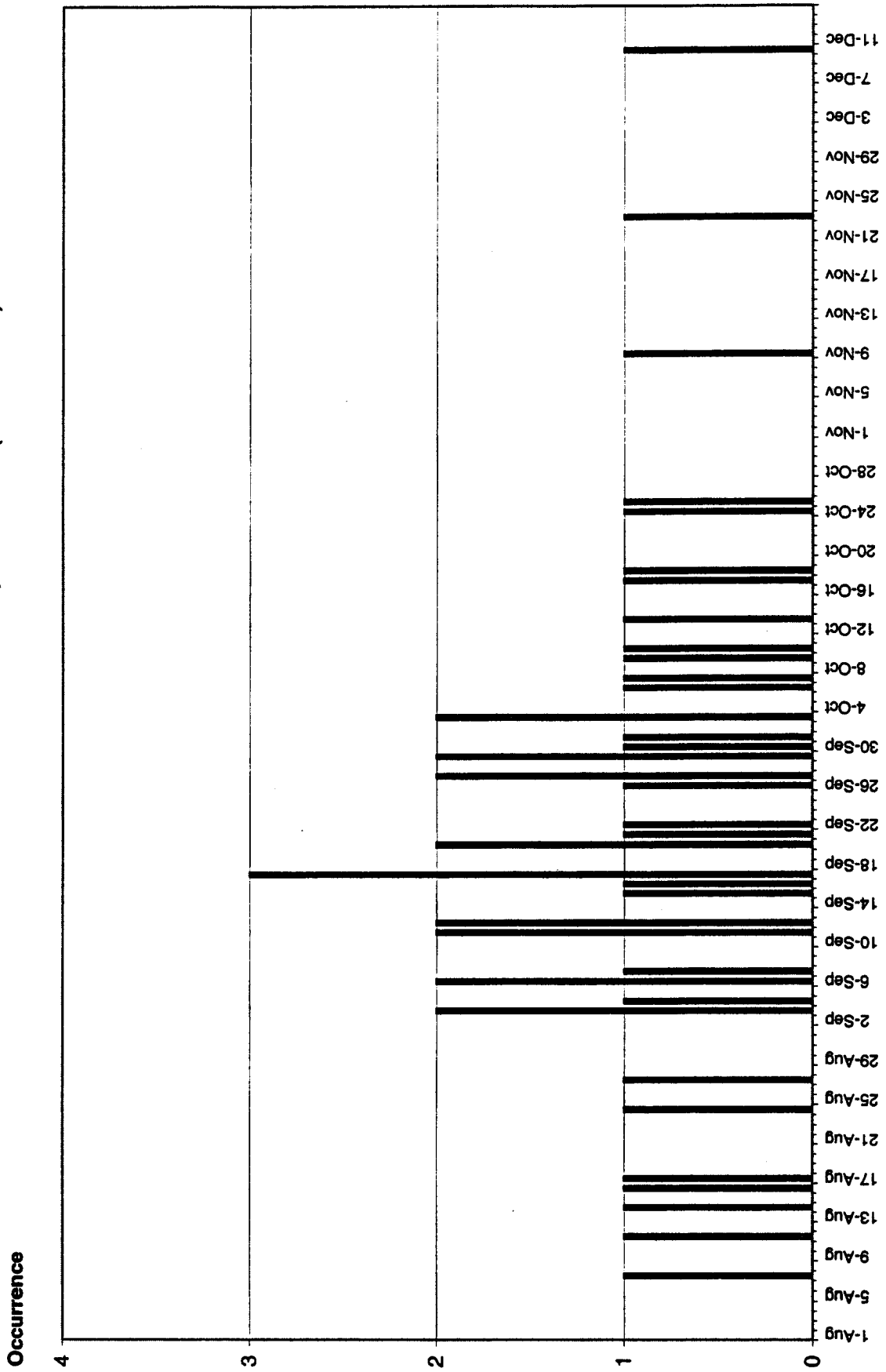
Rank = Numerical position or ordered low flow data with driest year equal to one.

N = 47

**MILLER CREEK**  
**HISTOGRAM LOW FLOW OCCURRENCES**  
**IN MILLER CREEK (1994)**

**AR 011321**

**Low Flow Occurrences in Miller Creek, 1949-1995 (1994 HSPF)**



**MILLER CREEK**  
**7-DAY LOW FLOW OCCURRENCES**  
**IN MILLER CREEK (1991-1994)**

**AR 011323**



**MILLER CREEK**

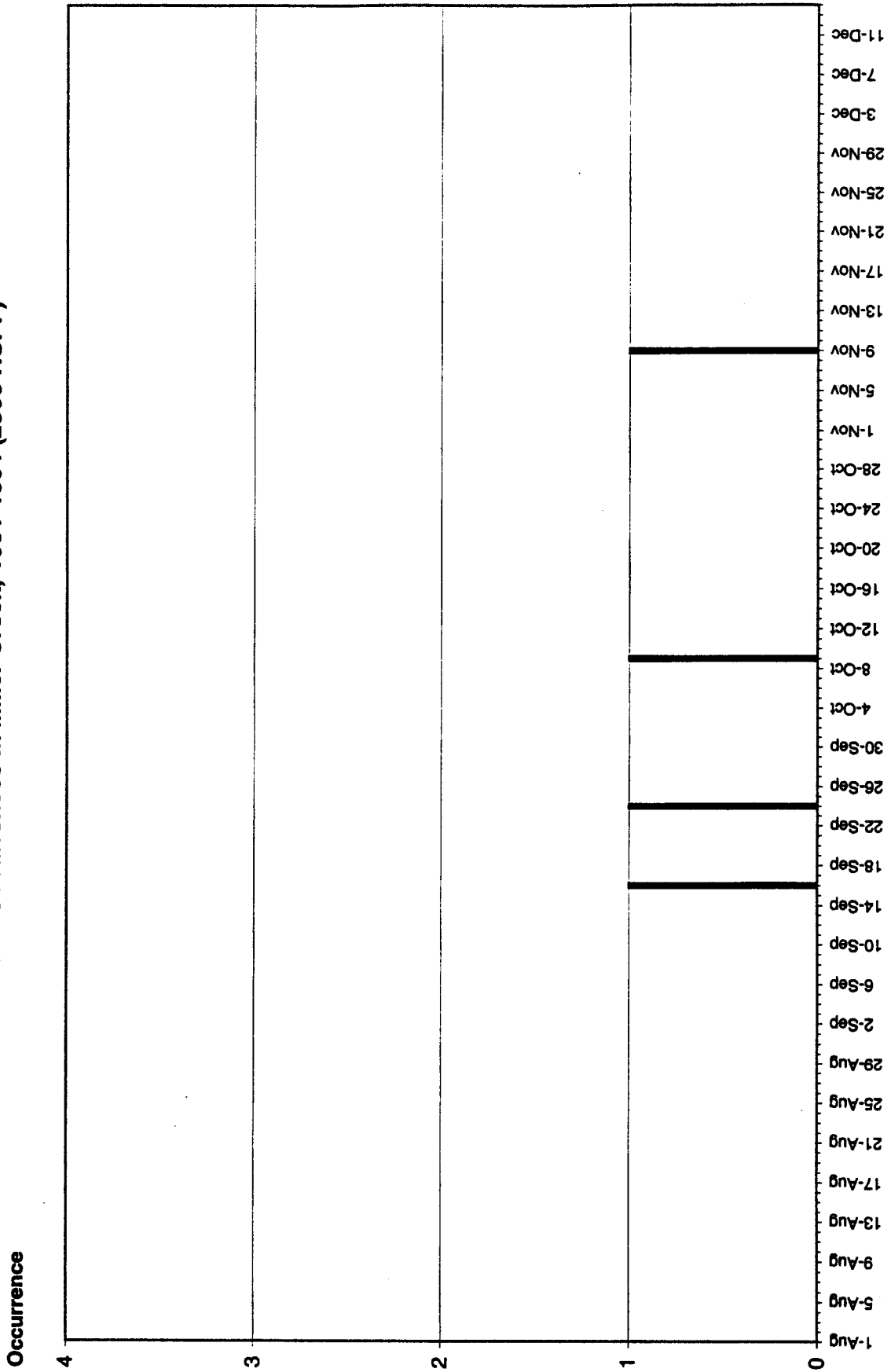
**7-DAY LOW FLOW OCCURRENCES IN MILLER CREEK 1991 – 1994,  
POST-PROJECT CONDITIONS (2006)**





**MILLER CREEK**  
**HISTOGRAM LOW FLOW OCCURRENCES**  
**IN MILLER CREEK 1991 – 1994,**  
**POST-PROJECT CONDITIONS (2006)**

**Low Flow Occurrences in Miller Creek, 1991-1994 (2006 HSPF)**



**MILLER CREEK**

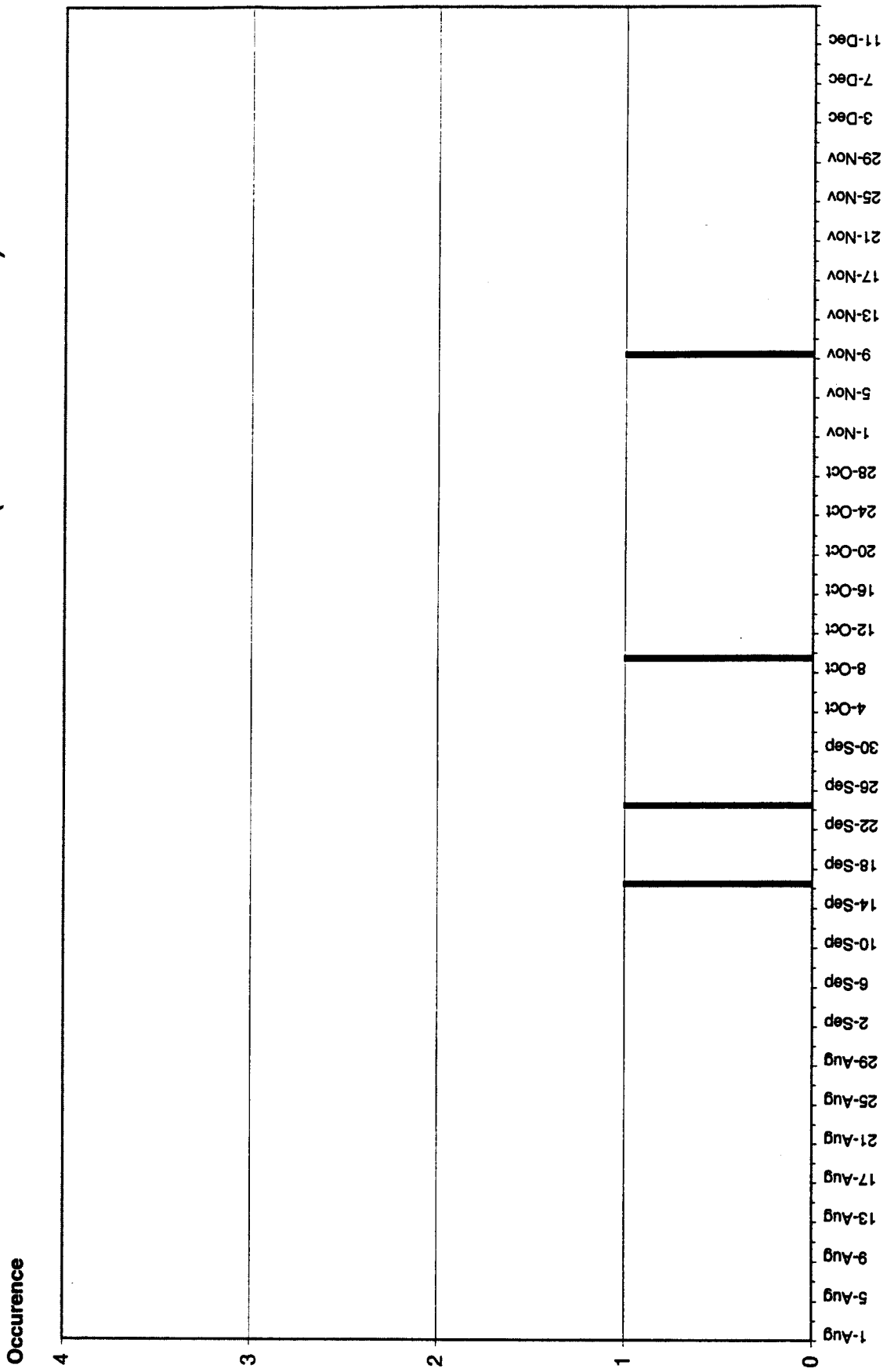
**7-DAY LOW FLOW OCCURRENCES IN MILLER CREEK,  
1991 – 1994, POST-PROJECT CONDITIONS  
(2006+ MITIGATION)**



**MILLER CREEK**

**HISTOGRAM LOW FLOW OCCURRENCES IN MILLER CREEK  
1991 – 1994, POST-PROJECT CONDITIONS  
(2006+ MITIGATION)**

**Low Flow Occurrences in Miller Creek 1991-1994 (2006 HSPF + 0.13 cfs)**



**MILLER CREEK**  
**COMPARISON OF 7-DAY LOW FLOW BY YEAR, 1991-1994**





**MILLER CREEK**  
**COMPARISON OF 7-DAY LOW FLOW BY RANK, 1991-1994**

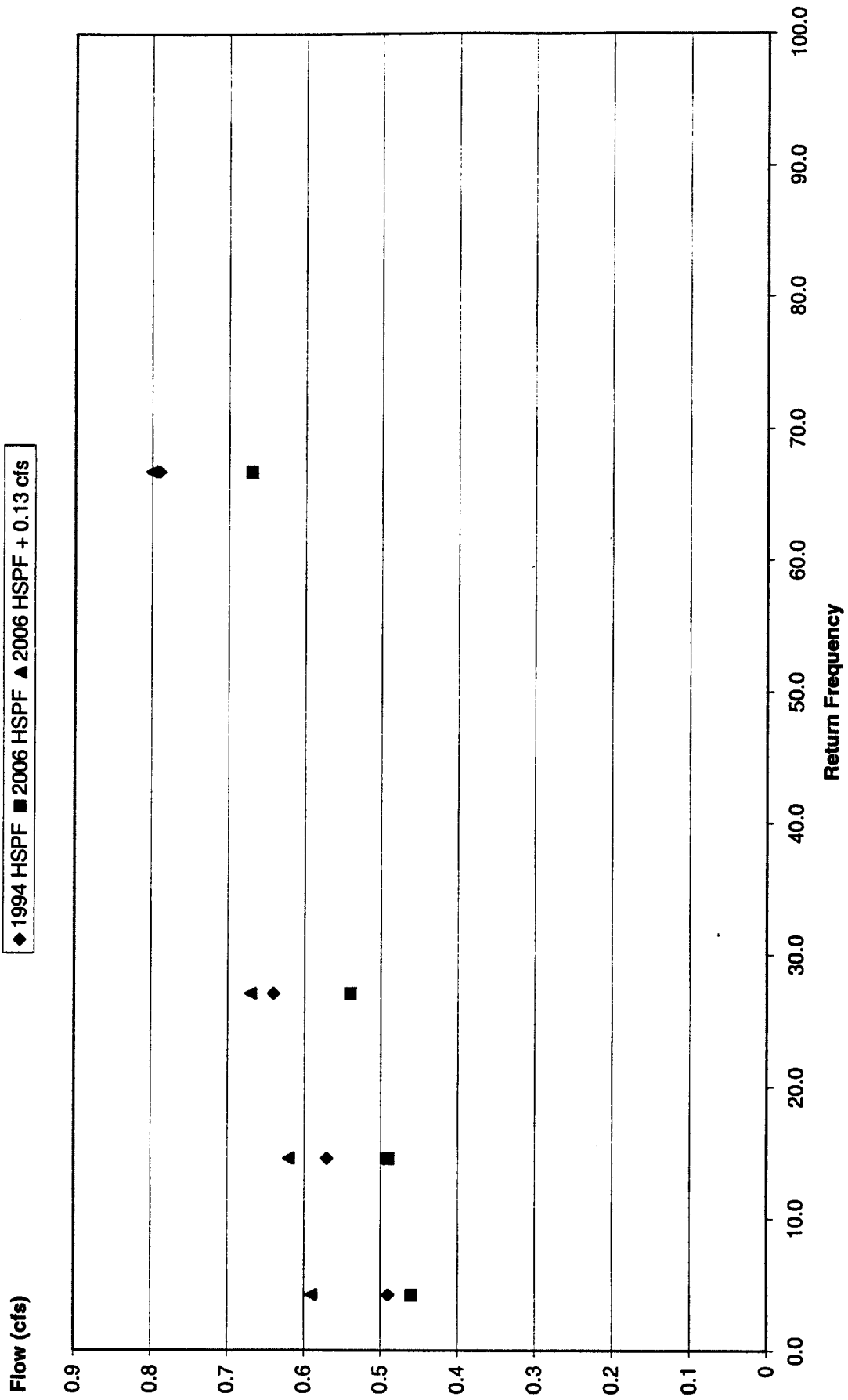
**AR 011335**



**MILLER CREEK**  
**PLOTTED 7-DAY LOW FLOWS BY**  
**PERCENT RETURN FREQUENCY, 1991-1994**

**AR 011337**

### Miller Creek 7-Day Low Flow Values



**MILLER CREEK**  
**SUMMARY OF LOW STREAM FLOW MITIGATION**  
**VAULT STORAGE AND FILLING**

**AR 011339**

**Reserve Storage Vaults for Miller Creek.**

**Vault sizes (dead storage in acre ft) required for 92-day release of 0.13 cfs (1949 – 1995).**

Mean	8.25
Median	7.71
Max	18.79
Min	3.47

**Contributing Drainage Areas**

<u>Subbasin</u>	<u>Vault</u>	<u>Area</u>	<u>% Contribution</u>
NEPL	NEPL	32.31	39.62
Cargo	Cargo	8.12	9.95
SDN2X	SDN2X/4X	0.36	20.64
SDN4		12.26	
SDN4X		4.21	
SDN3	SDN3X	24.30	29.79
SDN3X		0.00	
	<b>Total</b>	<b>81.56</b>	

**Fill time for 18.8 acre foot volume / Days to fill after closing vault on January 1.**

Mean	20.56
Media	16.00
Max	58.00
Min	4.0

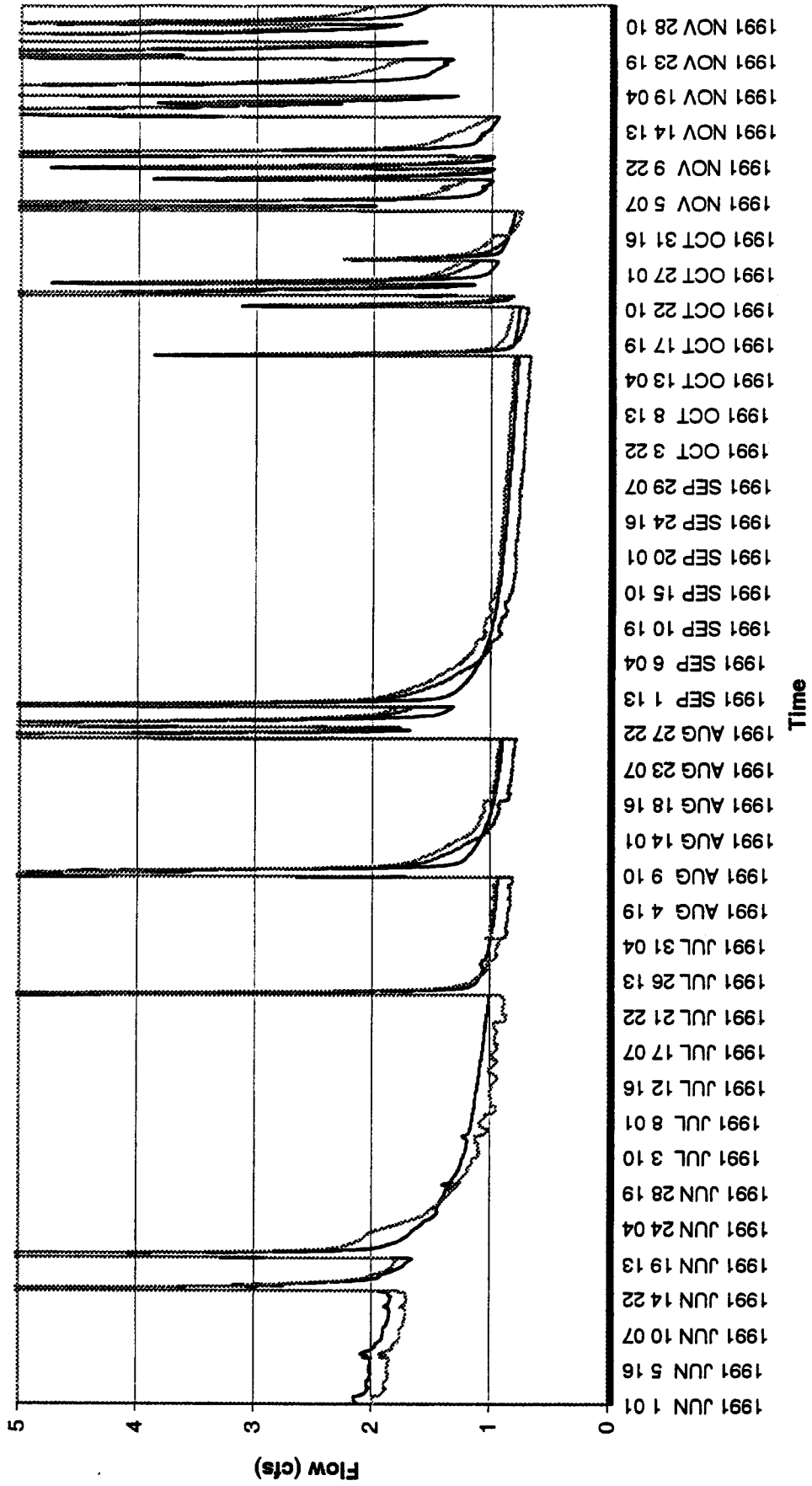
**Remaining volume in vaults on Oct 31**

	<u>Volume ac ft</u>	<u>Remaining Days</u>
Mean	16.99	65
Median	18.24	70
Max	18.80	72
Min	5.34	20

**MILLER CREEK**  
**LOW FLOW HYDROGRAPHS WITH MITIGATION (1991-1994)**

# Miller Creek Low Flow 1991 + Mitigation

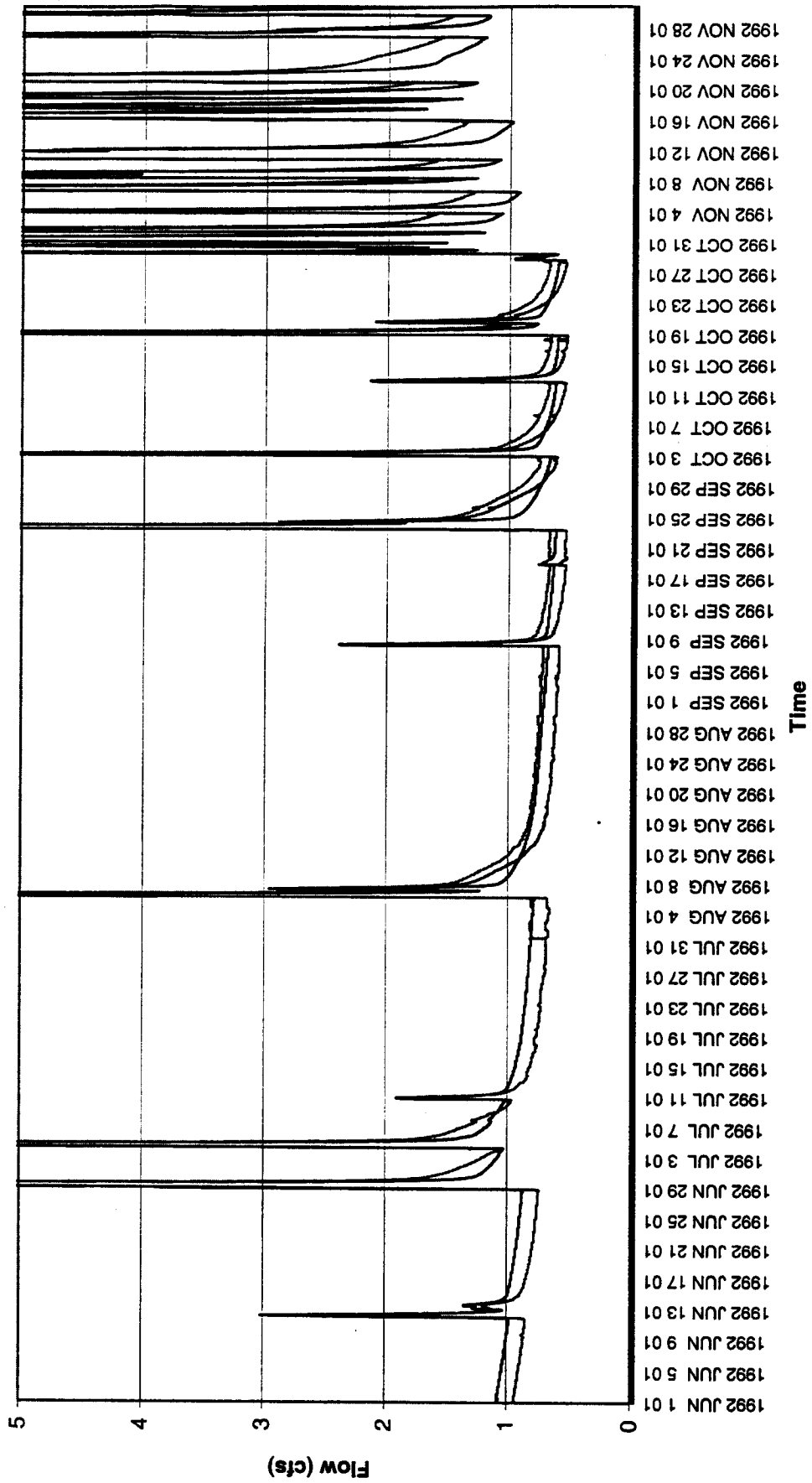
— Pre-Developed 1994      ..... Post-Developed 2006      ..... Post-Developed 2006 + Mitigation





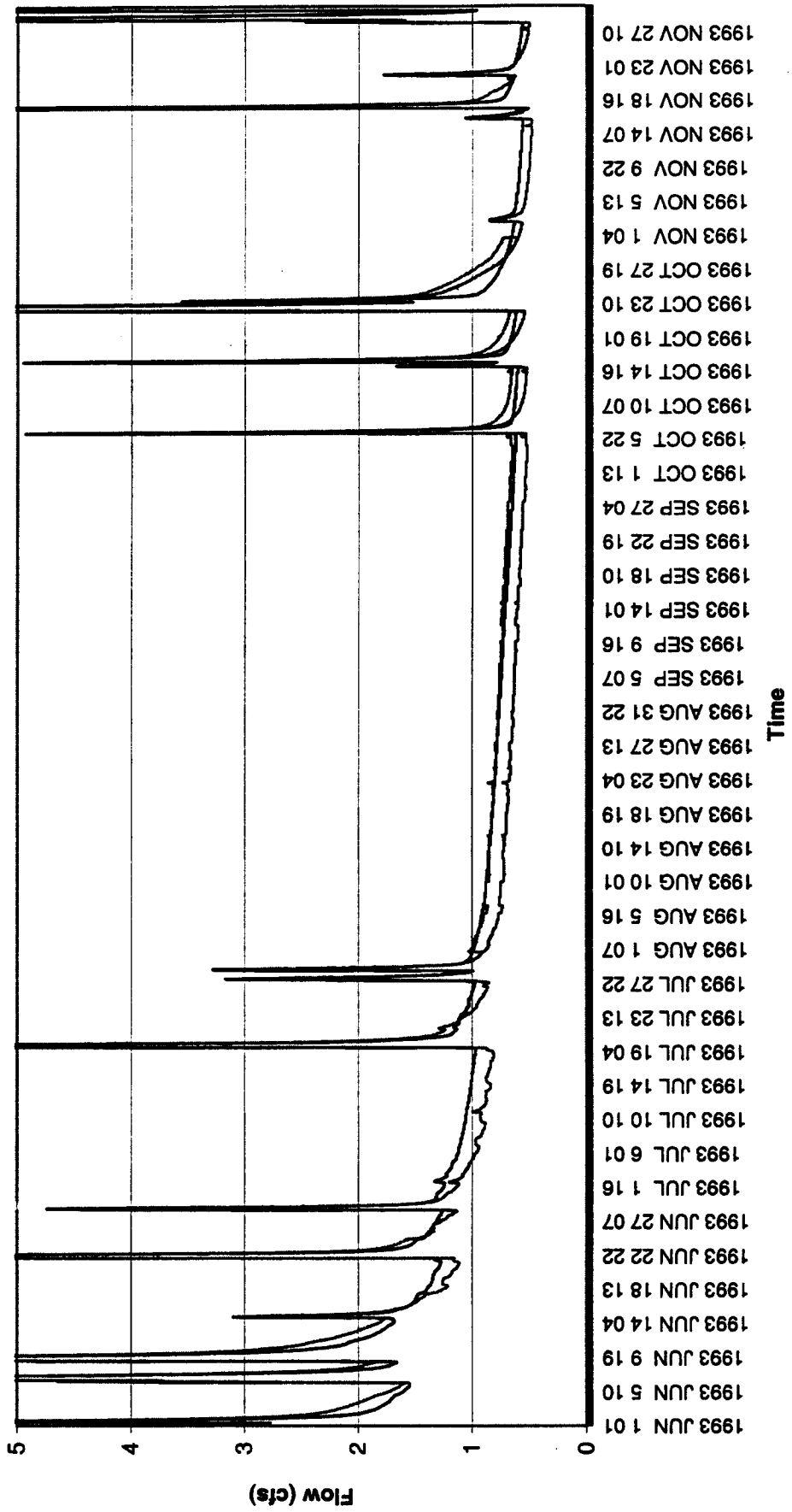
# Miller Creek Low Flow 1992 + Mitigation

— Pre-Developed 1994    — Post-Developed 2006    — Post-Developed 2006 + Mitigation



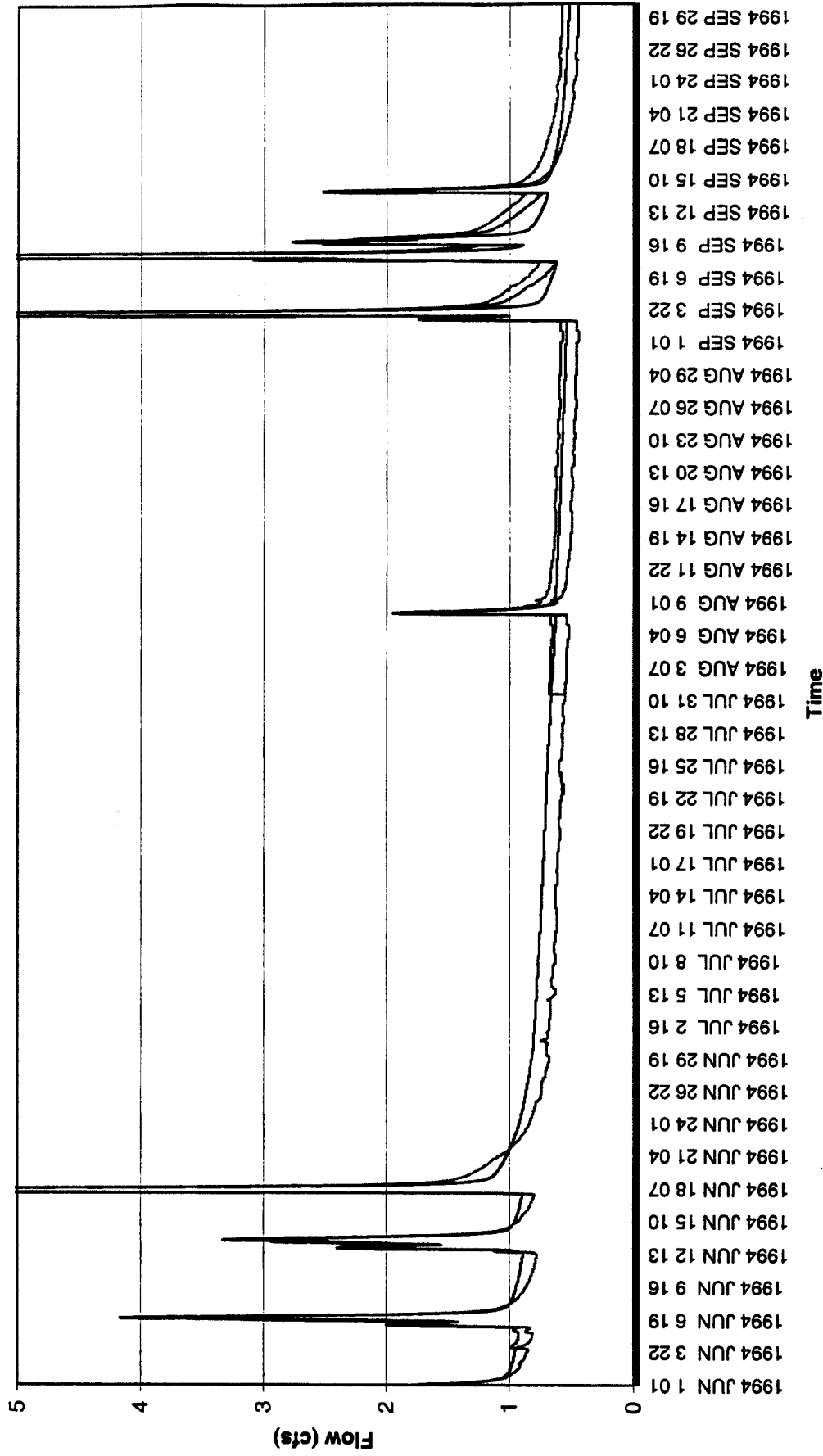
# Miller Creek Low Flow 1993 + Mitigation

Pre-Developed 1994   
  Post-Developed 2006   
  Post-Developed 2006 + Mitigation



# Miller Creek Low Flow 1994 + Mitigation

— Pre-Developed 1994    — Post-Developed 2006    — Post-Developed 2006 + Mitigation



## **WALKER CREEK**

**7-DAY LOW FLOW OCCURRENCES IN WALKER CREEK (1994)**

**HISTOGRAM LOW FLOW OCCURRENCES IN WALKER CREEK (1994)**

**7-DAY LOW FLOW OCCURRENCES IN WALKER CREEK (2006)**

**HISTOGRAM LOW FLOW OCCURRENCES IN WALKER CREEK (2006)**

**7-DAY LOW FLOW OCCURRENCES IN WALKER CREEK  
(WITH MITIGATION)**

**HISTOGRAM LOW FLOW OCCURRENCES IN WALKER CREEK  
(WITH MITIGATION)**

**COMPARISON OF 7-DAY LOW FLOW BY YEAR**

**COMPARISON OF 7-DAY LOW FLOW BY RANK**

**PLOTTED 7-DAY LOW FLOWS BY PERCENT RETURN FREQUENCY**

**SUMMARY OF LOW STREAM FLOW MITIGATION VAULT  
STORAGE AND FILLING**

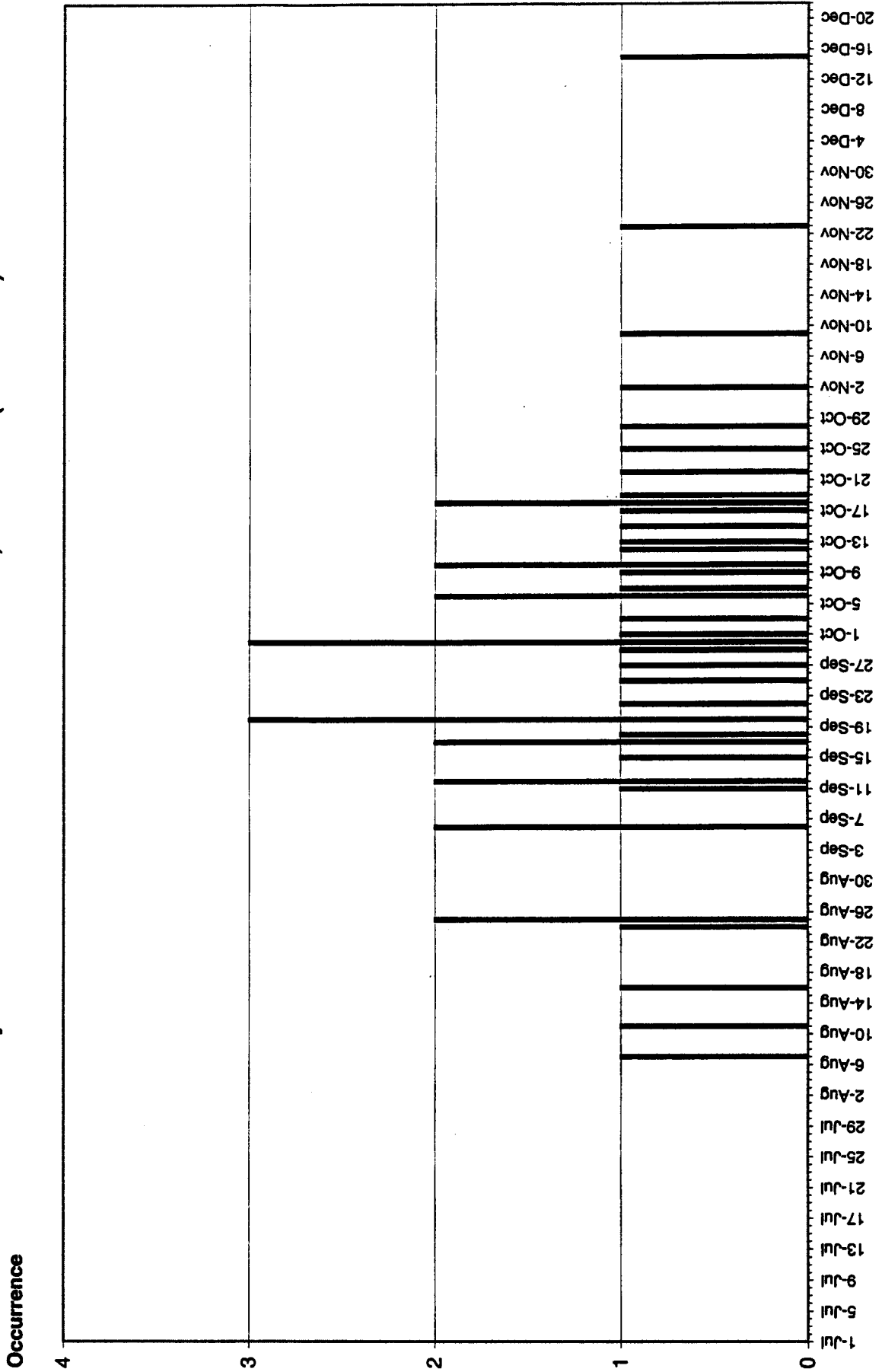
**LOW FLOW HYDROGRAPHS WITH MITIGATION (1991-1994)**

**WALKER CREEK**  
**7-DAY LOW FLOW OCCURRENCES IN WALKER CREEK (1994)**



**WALKER CREEK**  
**HISTOGRAM LOW FLOW OCCURRENCES**  
**IN WALKER CREEK (1994)**

7-Day Low Flow Occurrences in Walker Creek, 1949-1995 (1994 HSPF)





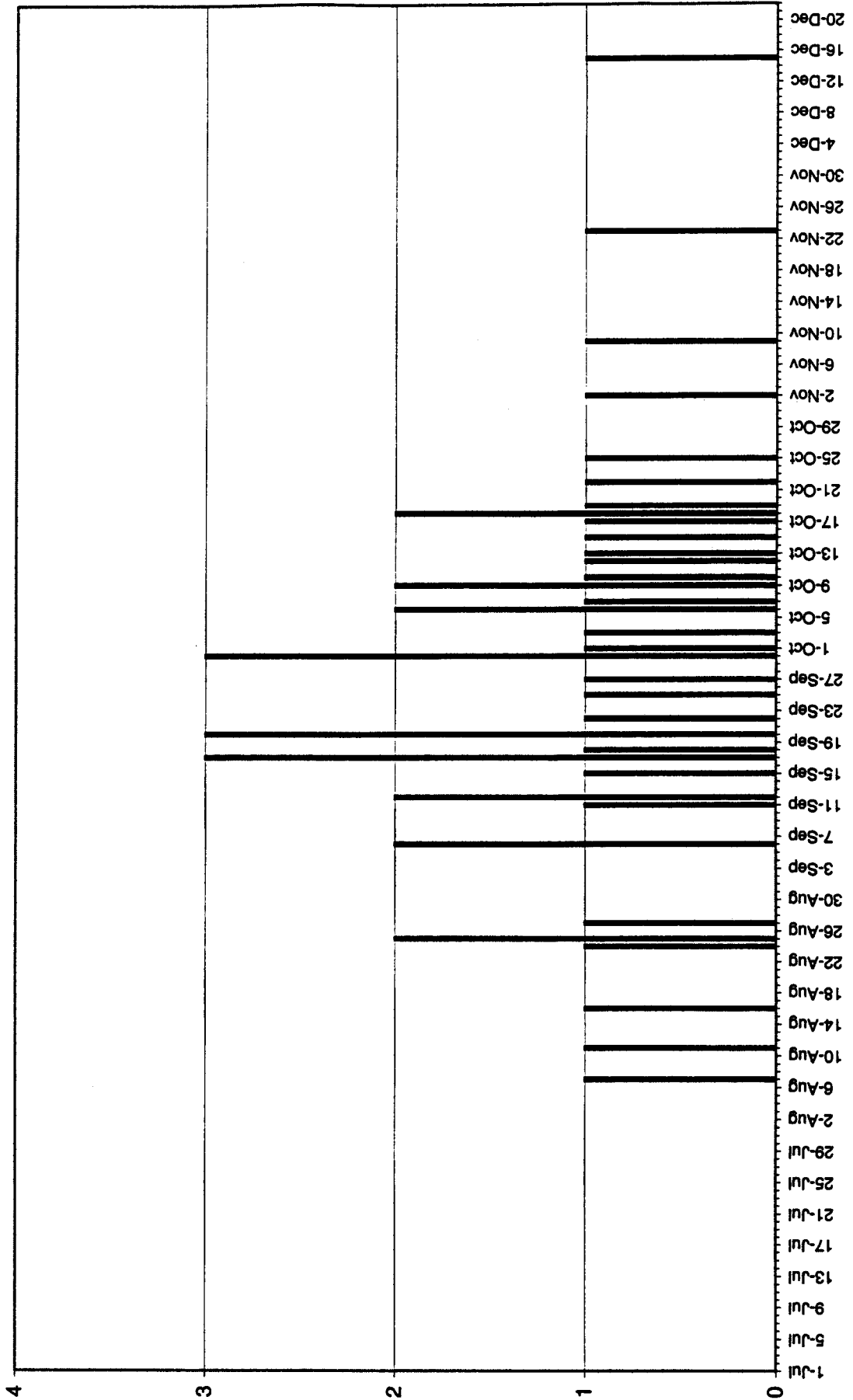
**WALKER CREEK**  
**7-DAY LOW FLOW OCCURRENCES IN WALKER CREEK (2006)**



**WALKER CREEK**  
**HISTOGRAM LOW FLOW OCCURRENCES**  
**IN WALKER CREEK (2006)**

# 7-Day Low Flow Occurrences in Walker Creek, 1949-1995 (2006 HSPF)

Occurrence

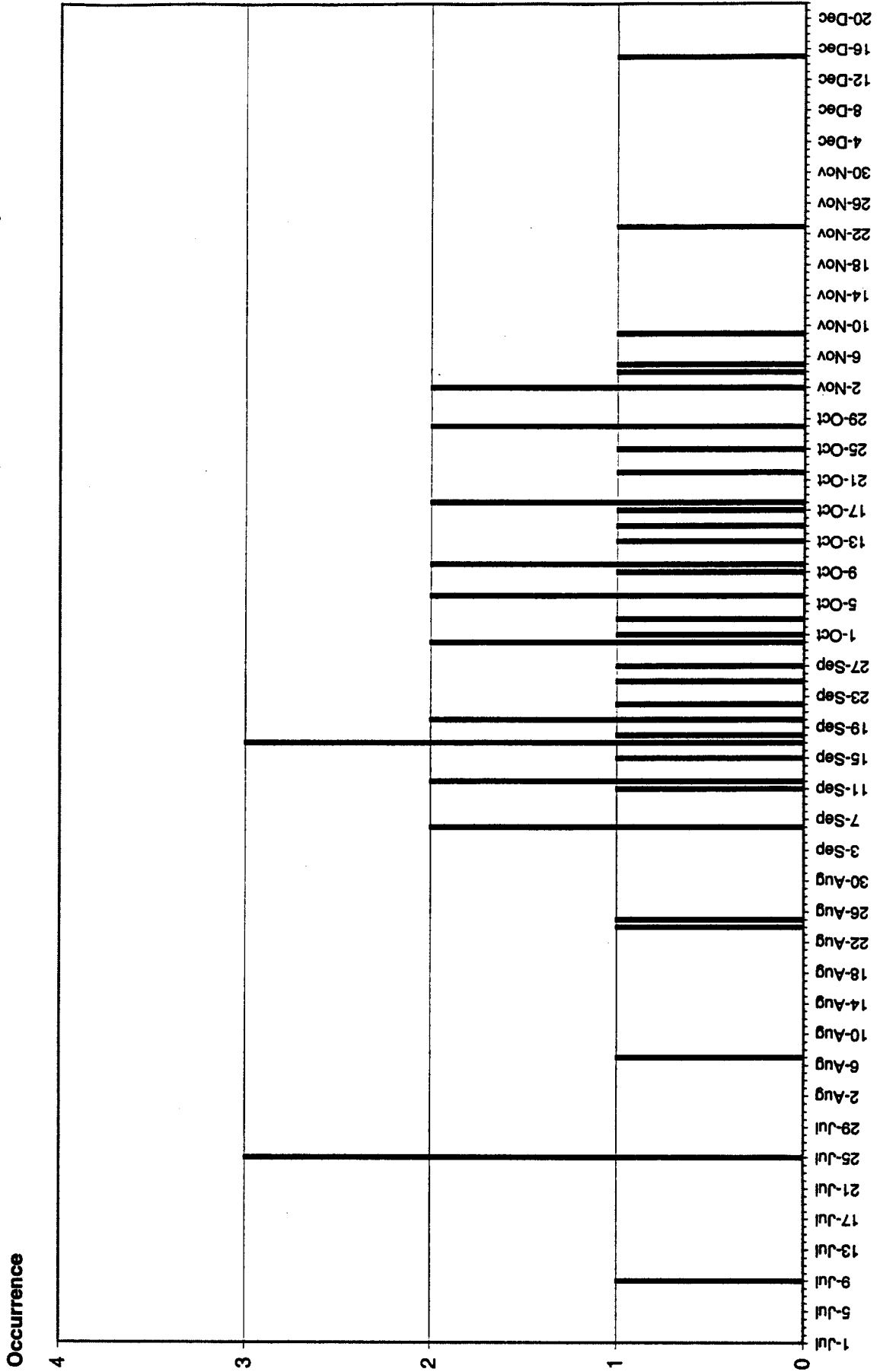


**WALKER CREEK**  
**7-DAY LOW FLOW OCCURRENCES IN WALKER CREEK**  
**(WITH MITIGATION)**



**WALKER CREEK**  
**HISTOGRAM LOW FLOW OCCURRENCES IN WALKER CREEK**  
**(WITH MITIGATION)**

7-Day Low Flow Occurrences in Walker Creek, 1949-1995 (2006 HSPF + 0.09 cfs)





**WALKER CREEK**  
**COMPARISON OF 7-DAY LOW FLOW BY YEAR**

Comparison of Low Flow by Year for Walker Creek

1994 HSPF Walker Creek at POC		2006 HSPF Walker Creek at POC		Walker Creek at POC 2006 + Mitigation Aug 1 - Oct 31	
Date	POC	Date	POC	Date	0.09 cfs
19-Oct-49	0.63	19-Oct-49	0.57	2-Nov-49	0.64
17-Sep-50	0.83	17-Sep-50	0.75	17-Sep-50	0.84
17-Sep-51	0.79	17-Sep-51	0.71	17-Sep-51	0.90
23-Nov-52	0.66	23-Nov-52	0.59	23-Nov-52	0.59
16-Sep-53	0.79	16-Sep-53	0.71	16-Sep-53	0.80
12-Oct-54	0.89	12-Oct-54	0.81	28-Oct-54	0.87
6-Sep-55	0.86	6-Sep-55	0.77	6-Sep-55	0.86
18-Sep-56	0.88	18-Sep-56	0.79	18-Sep-56	0.88
20-Sep-57	0.83	20-Sep-57	0.75	5-Nov-57	0.83
30-Sep-58	0.72	30-Sep-58	0.65	30-Sep-58	0.74
24-Aug-59	0.86	24-Aug-59	0.79	24-Aug-59	0.86
15-Oct-60	0.88	15-Oct-60	0.77	15-Oct-60	0.86
30-Sep-61	0.82	30-Sep-61	0.74	30-Sep-61	0.83
20-Sep-62	0.81	20-Sep-62	0.73	20-Sep-62	0.82
6-Oct-63	0.76	6-Oct-63	0.68	6-Oct-63	0.78
25-Oct-64	0.93	25-Oct-64	0.84	25-Oct-64	0.93
27-Sep-65	0.80	27-Sep-65	0.72	27-Sep-65	0.81
30-Sep-66	0.79	30-Sep-66	0.71	4-Nov-66	0.79
22-Sep-67	0.78	22-Sep-67	0.70	22-Sep-67	0.79
7-Aug-68	0.97	7-Aug-68	0.87	7-Aug-68	0.96
6-Sep-69	0.84	6-Sep-69	0.76	6-Sep-69	0.85
10-Oct-70	0.78	10-Oct-70	0.71	10-Oct-70	0.80
25-Aug-71	0.91	25-Aug-71	0.82	25-Aug-71	0.91
11-Sep-72	0.96	11-Sep-72	0.87	11-Sep-72	0.96
12-Sep-73	0.76	12-Sep-73	0.69	12-Sep-73	0.78
13-Oct-74	0.79	13-Oct-74	0.71	13-Oct-74	0.80
11-Aug-75	0.82	11-Aug-75	0.74	25-Jul-75	0.80
15-Dec-76	0.75	15-Dec-76	0.68	15-Dec-76	0.68
16-Aug-77	0.68	16-Aug-77	0.62	25-Jul-77	0.68
25-Aug-78	0.81	25-Aug-78	0.73	9-Jul-78	0.80
7-Oct-79	0.64	7-Oct-79	0.58	25-Jul-79	0.65
17-Oct-80	0.70	17-Oct-80	0.63	17-Oct-80	0.72
12-Sep-81	0.73	12-Sep-81	0.65	12-Sep-81	0.74
28-Sep-82	0.77	17-Sep-82	0.70	17-Sep-82	0.79
10-Oct-83	0.93	10-Oct-83	0.84	10-Oct-83	0.93
1-Oct-84	0.80	1-Oct-84	0.72	1-Oct-84	0.81
9-Oct-85	0.70	9-Oct-85	0.63	9-Oct-85	0.72
18-Oct-86	0.68	18-Oct-86	0.61	18-Oct-86	0.70
2-Nov-87	0.66	2-Nov-87	0.60	2-Nov-87	0.60
6-Oct-88	0.67	6-Oct-88	0.60	6-Oct-88	0.68
3-Oct-89	0.71	3-Oct-89	0.64	3-Oct-89	0.73
25-Sep-90	0.74	25-Sep-90	0.66	25-Sep-90	0.75
28-Oct-91	0.79	28-Oct-91	0.71	28-Oct-91	0.72
22-Oct-92	0.68	22-Oct-92	0.62	22-Oct-92	0.71
9-Nov-93	0.67	9-Nov-93	0.60	9-Nov-93	0.64
18-Oct-94	0.61	18-Oct-94	0.55	18-Oct-94	0.64
20-Sep-95	0.75	20-Sep-95	0.67	20-Sep-95	0.76
	36.59		33.02		36.65

**WALKER CREEK**  
**COMPARISON OF 7-DAY LOW FLOW BY RANK**

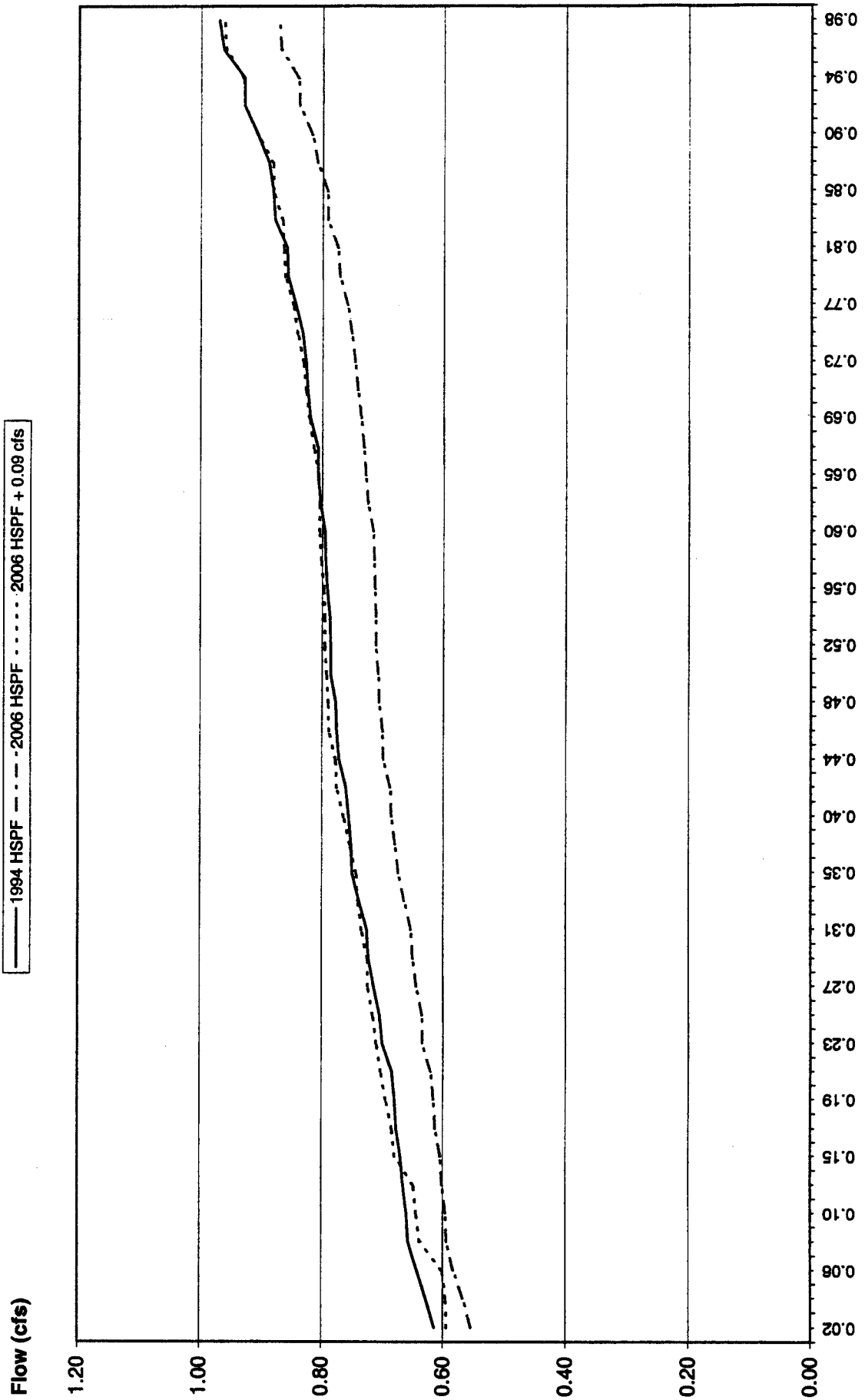
Comparison of 7-Day Low Flows by Rank for Walker Creek						
Date	Rank	1994 HSPF	2006 HSPF		Walker Creek at POC	
		Walker Creek at POC	Walker Creek at POC	-1994	2006 + Mitigation Aug 1 - Oct 31 0.09 cfs	-1994
19-Oct-49	1	0.63	0.57	-0.06	0.59	-0.03
17-Sep-50	2	0.83	0.75	-0.08	0.60	-0.24
17-Sep-51	3	0.79	0.71	-0.08	0.60	-0.19
23-Nov-52	4	0.66	0.59	-0.07	0.64	-0.02
15-Sep-53	5	0.79	0.71	-0.08	0.64	-0.14
12-Oct-54	6	0.89	0.81	-0.08	0.65	-0.24
6-Sep-55	7	0.86	0.77	-0.09	0.68	-0.18
18-Sep-56	8	0.88	0.79	-0.09	0.68	-0.19
20-Sep-57	9	0.83	0.75	-0.08	0.69	-0.13
30-Sep-58	10	0.72	0.65	-0.07	0.70	-0.02
24-Aug-59	11	0.88	0.79	-0.09	0.71	-0.17
15-Oct-60	12	0.86	0.77	-0.08	0.72	-0.14
30-Sep-61	13	0.82	0.74	-0.08	0.72	-0.10
20-Sep-62	14	0.81	0.73	-0.08	0.72	-0.08
6-Oct-63	15	0.76	0.69	-0.07	0.73	-0.02
25-Oct-64	16	0.93	0.84	-0.09	0.74	-0.19
27-Sep-65	17	0.80	0.72	-0.08	0.74	-0.06
30-Sep-66	18	0.79	0.71	-0.07	0.75	-0.03
22-Sep-67	19	0.78	0.70	-0.08	0.76	-0.01
7-Aug-68	20	0.97	0.87	-0.10	0.78	-0.19
6-Sep-69	21	0.84	0.76	-0.09	0.78	-0.07
10-Oct-70	22	0.78	0.71	-0.07	0.79	0.01
25-Aug-71	23	0.91	0.82	-0.09	0.79	-0.12
11-Sep-72	24	0.96	0.87	-0.09	0.79	-0.17
12-Sep-73	25	0.76	0.69	-0.07	0.80	0.04
13-Oct-74	26	0.79	0.71	-0.08	0.80	0.00
11-Aug-75	27	0.82	0.74	-0.08	0.80	-0.02
15-Dec-76	28	0.75	0.68	-0.07	0.80	0.05
16-Aug-77	29	0.68	0.62	-0.06	0.80	0.12
25-Aug-78	30	0.81	0.73	-0.08	0.80	0.00
7-Oct-79	31	0.64	0.58	-0.06	0.81	0.16
17-Oct-80	32	0.70	0.63	-0.07	0.81	0.11
12-Sep-81	33	0.73	0.65	-0.07	0.82	0.10
29-Sep-82	34	0.77	0.70	-0.07	0.83	0.06
10-Oct-83	35	0.93	0.84	-0.09	0.83	-0.10
1-Oct-84	36	0.80	0.72	-0.08	0.84	0.05
9-Oct-85	37	0.70	0.63	-0.07	0.85	0.15
18-Oct-86	38	0.68	0.61	-0.06	0.86	0.18
2-Nov-87	39	0.66	0.60	-0.06	0.86	0.21
6-Oct-88	40	0.67	0.60	-0.07	0.87	0.20
3-Oct-89	41	0.71	0.64	-0.07	0.88	0.17
25-Sep-90	42	0.74	0.66	-0.07	0.88	0.14
28-Oct-91	43	0.79	0.71	-0.08	0.91	0.12
22-Oct-92	44	0.68	0.62	-0.06	0.93	0.24
9-Nov-93	45	0.67	0.60	-0.06	0.93	0.26
18-Oct-94	46	0.61	0.55	-0.06	0.96	0.34
20-Sep-95	47	0.75	0.67	-0.08	0.96	0.21
		36.59	33.02		36.65	

AR 011362

**WALKER CREEK**  
**PLOTTED 7-DAY LOW FLOWS BY**  
**PERCENT RETURN FREQUENCY**

**AR 011363**

# Walker Creek 7-Day Low Flow Values



**WALKER CREEK**  
**SUMMARY OF LOW STREAM FLOW MITIGATION**  
**VAULT STORAGE AND FILLING**

**AR 011365**

**Reserve Storage Vaults for Walker Creek.**

**Vault sizes (dead storage in acre ft) required for 92-day release of 0.09 cfs (1949 - 1995).**

	<u>Vault F</u>
Mean	11.02
Median	11.22
Max	15.02
Min	6.04

**Contributing Drainage Areas**

<u>Subbasin</u>	<u>Vault</u>	<u>Area</u>	<u>% Contribution</u>
SDW2	F	3.5	30
SDW2 Lined Area	F	6.0	52
SDW2 Pond Cover	F	2.0	18
	Total	11.5	

**Fill time for 15.0 acre foot volume / Days to fill after closing vault on November 30**

	<u>Vault F</u>
Mean	102.33
Media	81.50
Max *	282.00
Min	47.00

**Remaining volume in vaults on October 31**

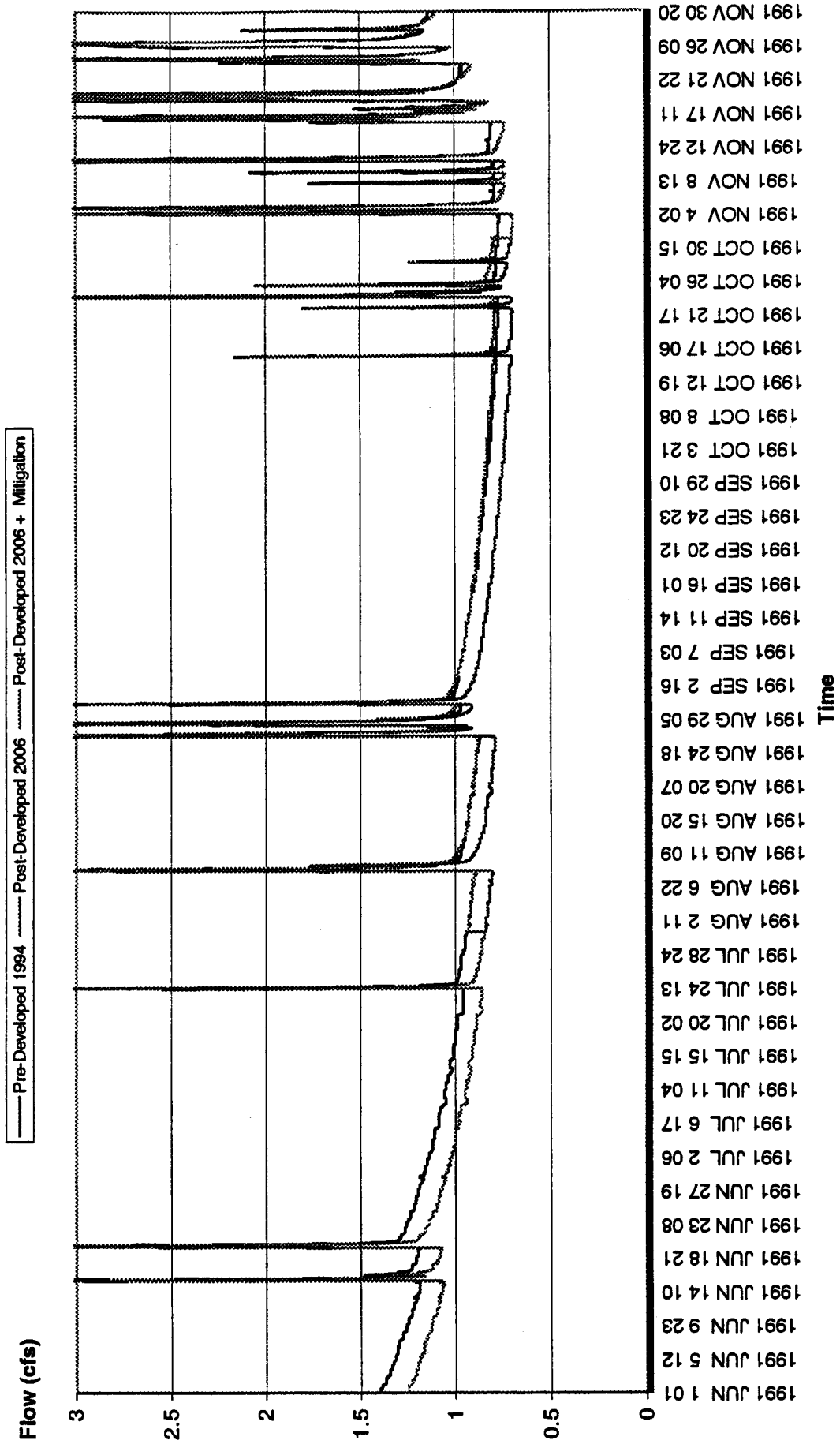
	<u>Volume ac ft</u>	<u>Remaining Days</u>
Mean	4.62	25.8
Median	4.52	25.3
Max	10.42	58.4
Min	0.04	0.2

\* Vault fills all years except 1977 and 1979



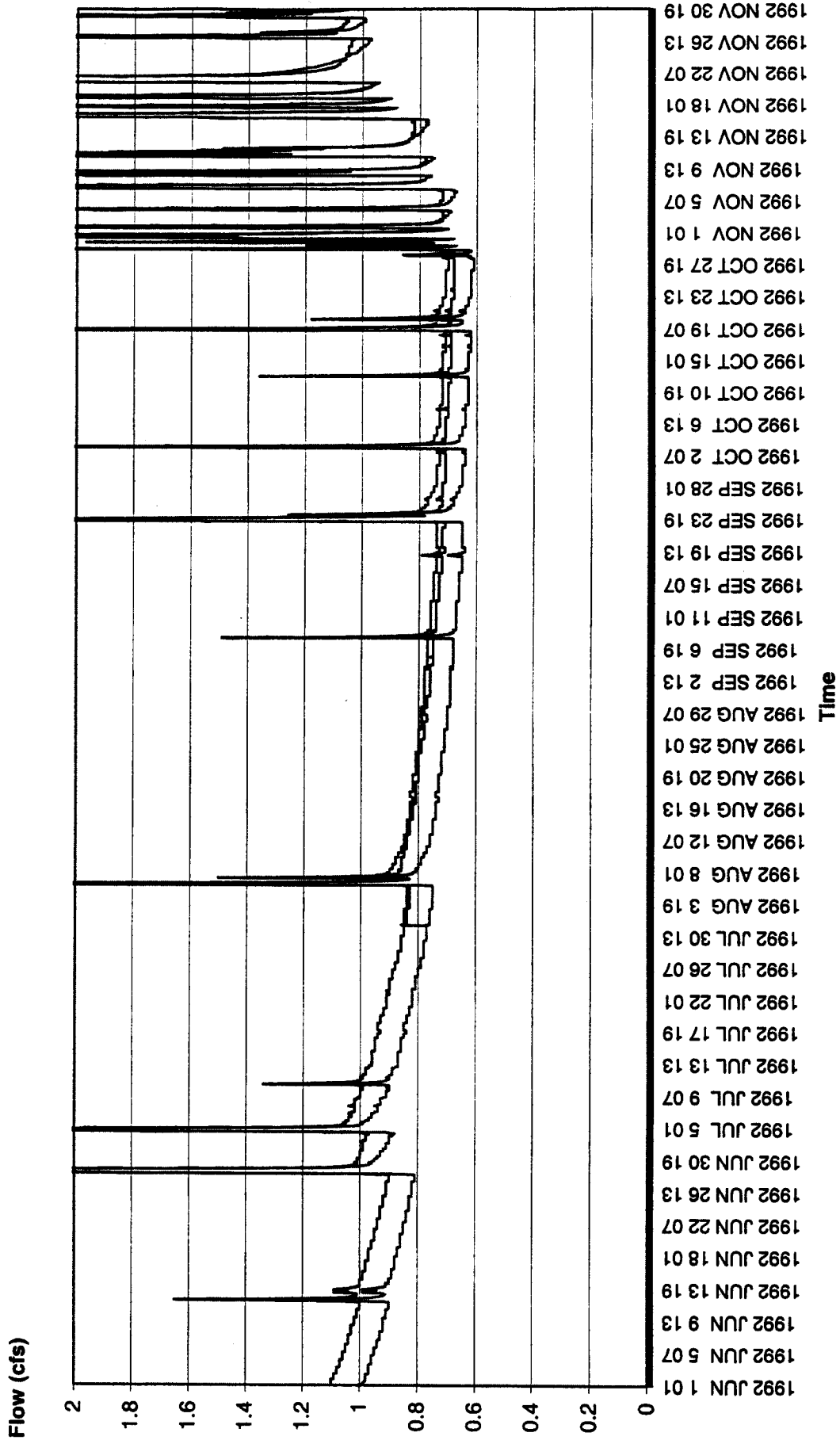
**WALKER CREEK**  
**LOW FLOW HYDROGRAPHS WITH MITIGATION (1991-1994)**

# Walker Creek Low Flow 1991 + Mitigation

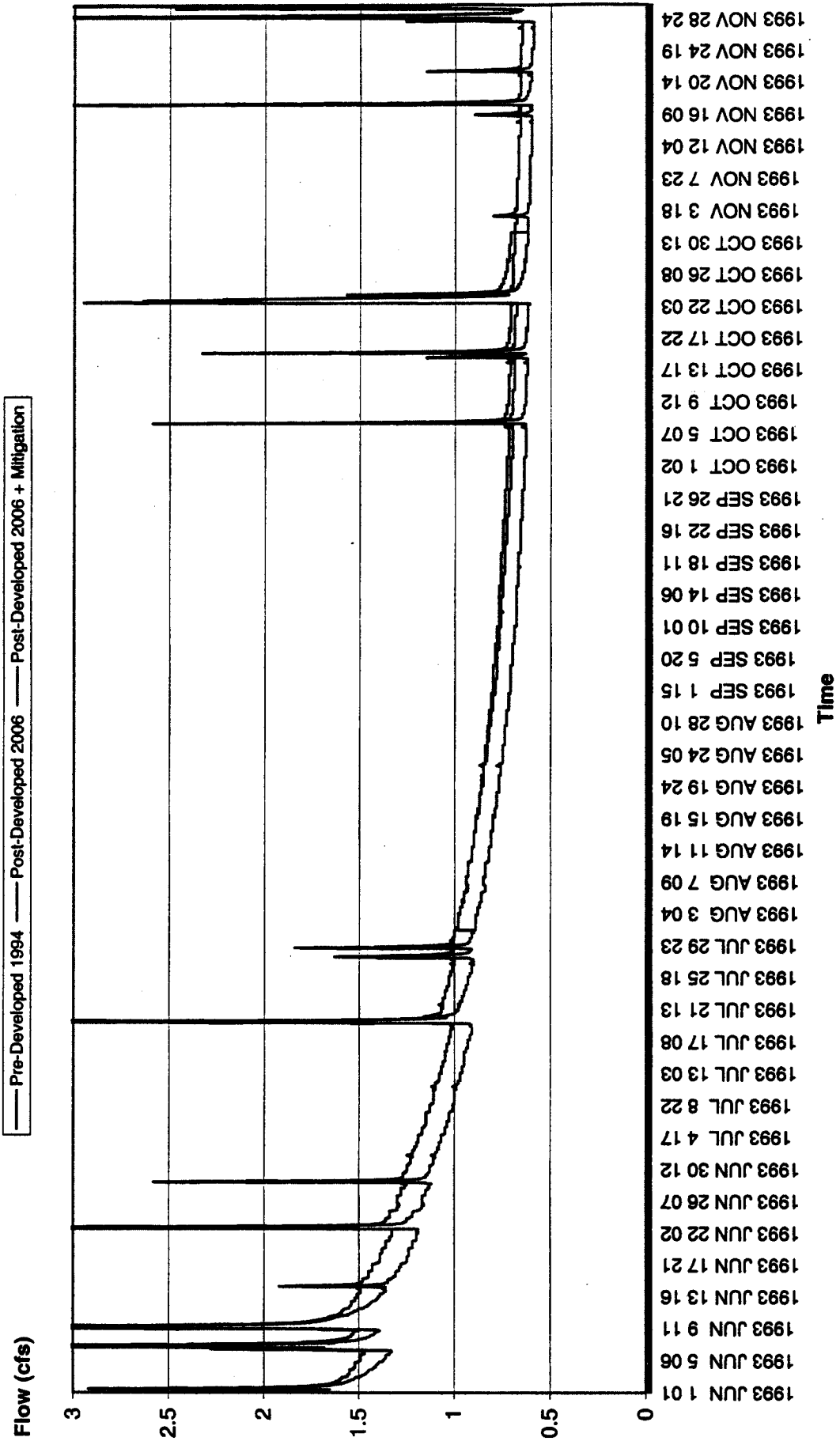


# Walker Creek Low Flow 1992 + Mitigation

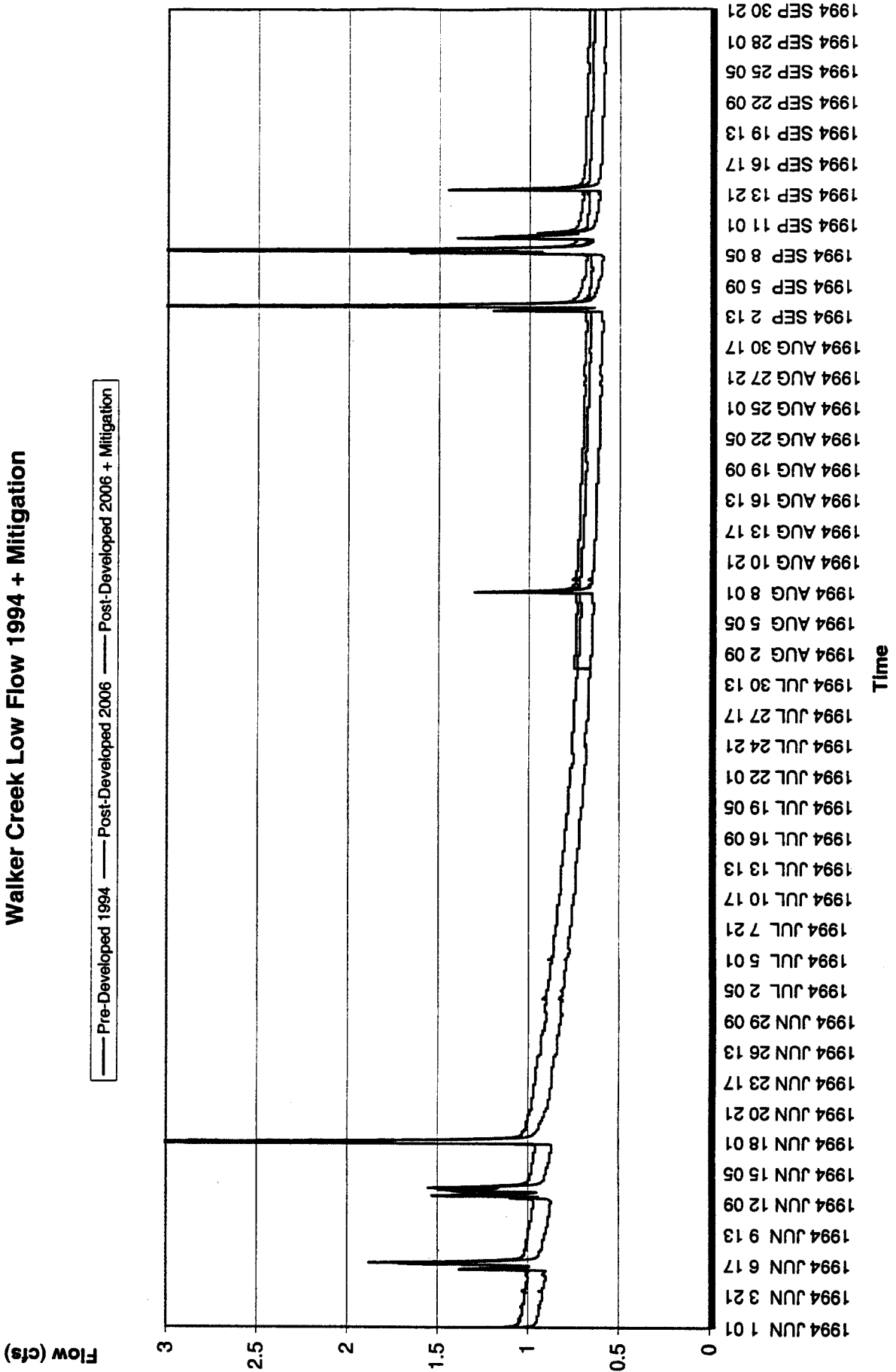
Pre-Developed 1984   
  Post-Developed 2006   
  Post-Developed 2006 + Mitigation



# Walker Creek Low Flow 1993 + Mitigation



# Walker Creek Low Flow 1994 + Mitigation



## MEMORANDUM

to: Paul Fendt July 20, 2001  
from: Don Weitkamp 556 2912 001 1 28  
re: Low Stream Flow Fish Behavior and Stream Characteristics

The following are some thoughts on the issues raised in the e-mail message from Kelly Whiting to Ann Kenny of July 2, 2001 which we discussed at our meeting with the agencies.

1. Migrations of juvenile and adult salmonids appear to be stimulated/regulated by a variety of biological and physical factors of which flow is one. Young fish generally require growth to a certain size range before migration will occur. The physiological processes (smoltification) that ready them for migration also depend of a sequence of events that appears to be triggered by a combination of temperature, photoperiod, and phase of the moon that stimulate hormonal mediated physiological changes. Once these fish are prepared to migrate a substantial change in flow will often trigger the initiation of migration. However, flow increases alone are not likely to initiate a process that depends on many factors.

Likewise the migrations of adult salmonids are commonly the result of a variety of factors. Adults commonly do not approach streams or prepare to move into headwaters until specific times apparently regulated by photoperiod, temperature, potentially food supply, etc. When ready to migrate the adults are commonly stimulated by freshet conditions that involve both a flow increase and runoff from riparian areas. The chemical cues in the runoff appear to play some role in triggering the migrations. The relationship to flow is far from certain. Sometimes minor increases in flow will stimulate migrations, sometimes minor increases are ignored, and sometimes migrations occur in the absence of flow increases. A minor flow supplementation during normal low flow periods is not likely to provide a trigger for biological processes that would not normally occur during that time period.

2. Severe drought conditions are generally a habitat-limiting factor that can severely limit the carry capacity of a stream. In most Puget Sound lowland streams the low flow conditions of late July to mid-September determine the minimum habitat available to support the resident fish populations including juvenile salmon and trout. Prolonged low flows of several weeks to months during this period are commonly the factor that establishes the carrying capacity of a stream.

Protection or restoration of stream flows during the late July through September period will maintain the habitat that sustains and controls fish populations remaining within the stream through out the year. Stream flow supplementation during this period can have a substantial

influence on the abundance of resident and anadromous salmonids. Providing flow outside this late summer low flow period is unlikely to provide an increase in the fish population that can be sustained within the stream. Additional habitat provided by flow increases preceding the limiting low flow period is unlikely to provide an increase in the fish population following the low flow period. The potential effects of providing additional habitat outside the limiting flow period would likely be negated by the subsequent low flows. Maintaining existing flow conditions during the common low flow period will ensure that habitat does not become more of a limiting factor than it is under existing conditions.

3. Water quality can be a critical factor in determining salmonid habitat. However, sudden changes of a moderate degree are naturally common in small streams. Freshet conditions (storm events) can naturally change temperature and chemical parameters within hours, depending on the nature of the storm event, the drainage basin, stream characteristics and ambient conditions. Generally decreasing temperatures have little immediate effect on salmonids other than reduced activity for a brief period. Decreasing temperatures rapidly within the range of a few degrees can be a benefit if temperatures are near the upper end of the range of acceptable temperatures or greater. Decreases in DO appear to have little effect on behavior when they remain within a range acceptable for survival and growth. The relatively small amount of water provided during low flow mitigation together with the proposed oxygenation techniques is likely to maintain adequate DO conditions.



## Memorandum

**To:** Keith Smith – Port of Seattle  
**From:** Crispin Prael, Charles Ellingson – Pacific Groundwater Group  
**Date:** 05/25/01  
**Re:** Selection of Cross Sections for Third Runway Slice Modeling

---

This memorandum documents basin fill geometries and the selection of hydrogeologic cross sections required as input to low flow analyses for the SeaTac third runway fill. This memorandum and the attached figures serve as the Task 1 deliverable of Pacific Groundwater Group's Hydrus+Slice (H-S) modeling scope dated May 22, 2001. We welcome your comments on the cross section locations presented herein.

PGG used existing GIS coverages of existing topography, "built" topography, and third runway pavement distribution to calculate areas for H-S modeling. The areas to be removed from HSPF and modeled by Hydrus and Slice are shown on Figure 1. The area includes all contiguous fill areas in Miller and Walker Creek basins planned for the third runway, minus the steep apron slopes at the perimeter of the fill. The areas are as follows:

	Miller Creek Basin	Walker Creek Basin
Pervious Fill Area	2,836,250 sq. ft. (65.2 ac)	645,000 sq. ft. (14.8 ac)
Runway and Taxiway Impervious Area	1,722,328 sq. ft. (39.6 ac)	372,343 sq. ft. (8.6 ac)
Total Area in Basin to be modeled by H-S	4,558,578 sq. ft. (104.8 ac)	1,017,343 sq. ft. (23.4 ac)

PGG reviewed hydrogeologic conditions and embankment geometries along the extent of the embankment fill. Three representative hydrogeologic cross sections were selected using available subsurface data and interpretations. Cross sections 1 and 2 are representative of conditions within the Miller Creek basin fill while cross section 3 is representative of conditions in the Walker Creek basin fill. Hydrogeologic cross sections will be used to create slice models under Task 4 of our work scope.



Cross sections are based on subsurface data described in available geotechnical and hydrogeologic reports and from the existing and "built" topography of the third runway area. PGG compiled and reviewed consulting reports completed since the Ecology Study in order to select cross section alignments. Structural surface maps (AESI, 1999) for glacial till will be used to refine hydrogeologic conditions in the final cross sections and related slice models.

The following is a brief summary of each cross section and the extent of embankment fill that each cross section represents.

### **MILLER CREEK SECTIONS**

For purposes of developing the slice models and low flow estimates, two cross sections represent subsurface conditions within the Miller Creek basin fill. Both cross sections are oriented parallel to horizontal groundwater flow directions expected in the fill. **Figure 1** shows the approximate locations of cross sections proposed through the Miller Creek fill.

**Cross Section 1 (Slice 1):** This cross section is located through the thickest portion of the fill embankment with a fill thickness of up to 160 feet. Cross section 1 is located at the same location as the original slice model used by PGG in the Ecology study. **Figure 2** presents a working draft of the Slice 1 model, with implied hydrogeologic conditions. Fill located behind the West MSE wall will be modeled using Slice 1. This represents approximately 1,640 linear feet (30%) of the fill embankment within the Miller Creek basin area.

**Cross Section 2 (Slice 2):** This cross section is located through the northern portion of the fill embankment near the northern end of the third runway. Cross section 2 will be developed from a generalized hydrogeologic cross section originally created by Hart Crowser through the northern toe of the fill embankment (Hart Crowser, 1999). The slice location is based on availability of suitable subsurface data. **Figure 3** presents Hart Crowser's Section A-A' which will be used as the basis for the Slice 2 model. Working drafts of soilunit designations to be used in the slice are also shown. Modifications that will be made to the final cross section through this area include the use of existing topography (thereby more accurately defining the drain slope) and "built" fill topography (thereby defining fill thickness), with consideration of till surface configurations defined by AESI (1999). For purposes of the low flow analysis, cross section 2 represents subsurface conditions in the bulk of Miller Creek embankment fill. It will be used to model subsurface conditions for all fill in the Miller Creek basin not modeled using Slice 1 as described above. Approximately 3,760 linear feet (70%) of fill will be represented by fill geometry and hydrogeologic conditions described by cross section 2.

### **WALKER CREEK SECTION**

For purposes of developing the slice models, one cross section will be used to represent subsurface conditions within the Walker Creek basin fill. The section was chosen through fill of intermediate thickness. (Although this single section will not accurately represent the variety of fill thicknesses in Walker Creek basin fill, the thick portion of the fill is of small areal extent.) **Figure 1** shows the approximate cross section location proposed through the Walker Creek fill.

**Cross Section 3 (Slice 3):** This cross section is located immediately north of the South MSE wall. A fill thickness of up to 40 feet occurs in this slice. Cross section 3 will be based on a generalized hydrogeologic cross section originally created by Hart Crowser through the northern end of the MSE wall study area (Hart Crowser, 2000b). It will be updated base on supplemental geotechnical data (Hart Crowser, 2000c), existing and "built" topography, and available till mapping data. **Figure 4** presents Hart Crowser's Section E-E' which will be used as the basis for cross section 3. Working drafts of soil-unit designations to be used in the slice are also shown.

#### REFERENCES CITED

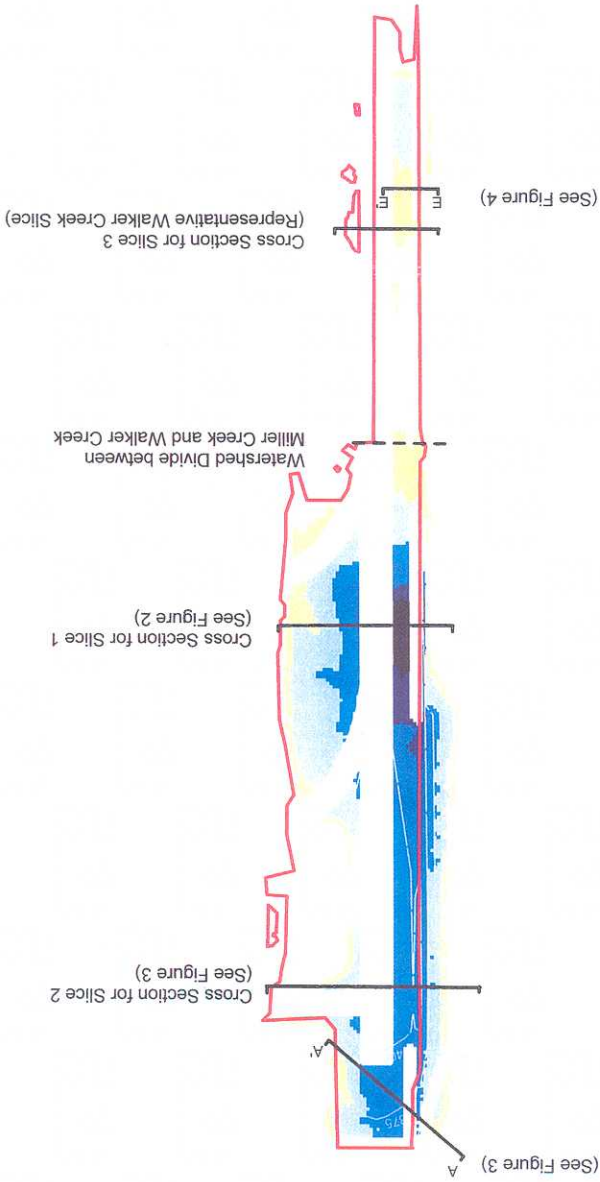
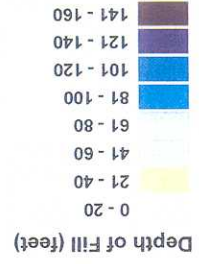
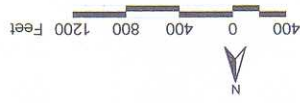
- Associated Earth Sciences, Inc. (AESI), 1999. *Seattle-Tacoma International Airport Ground Water Study – Model Boundary Presentation*. Unpublished figures and cross sections prepared for The Port of Seattle dated October 15, 1999.
- Hart Crowser, Inc., 1999a. *Subsurface Conditions Data Report 404 Support Third Runway Embankment*. Unpublished consulting report prepared for HNTB and The Port of Seattle dated July 1999.
- Hart Crowser, Inc., 2000a. *Draft Subsurface Conditions Data Report Additional Field Explorations and Advanced Testing, Third Runway Project Sea-Tac International Airport*. Unpublished consulting report prepared for HNTB dated September 5, 2000.
- Hart Crowser, Inc., 2000b. *Draft Subsurface Conditions Data Report South MSE Wall and Adjacent Embankment Third Runway Project Sea-Tac International Airport*. Unpublished consulting report prepared for Port of Seattle and HNTB dated April 7, 2000.
- Hart Crowser, Inc., 2000c. *Subsurface Conditions Data Report Phase 4 Fill, Third Runway Project Sea-Tac International Airport*. Unpublished consulting report prepared for Port of Seattle and HNTB dated November 29, 2000.

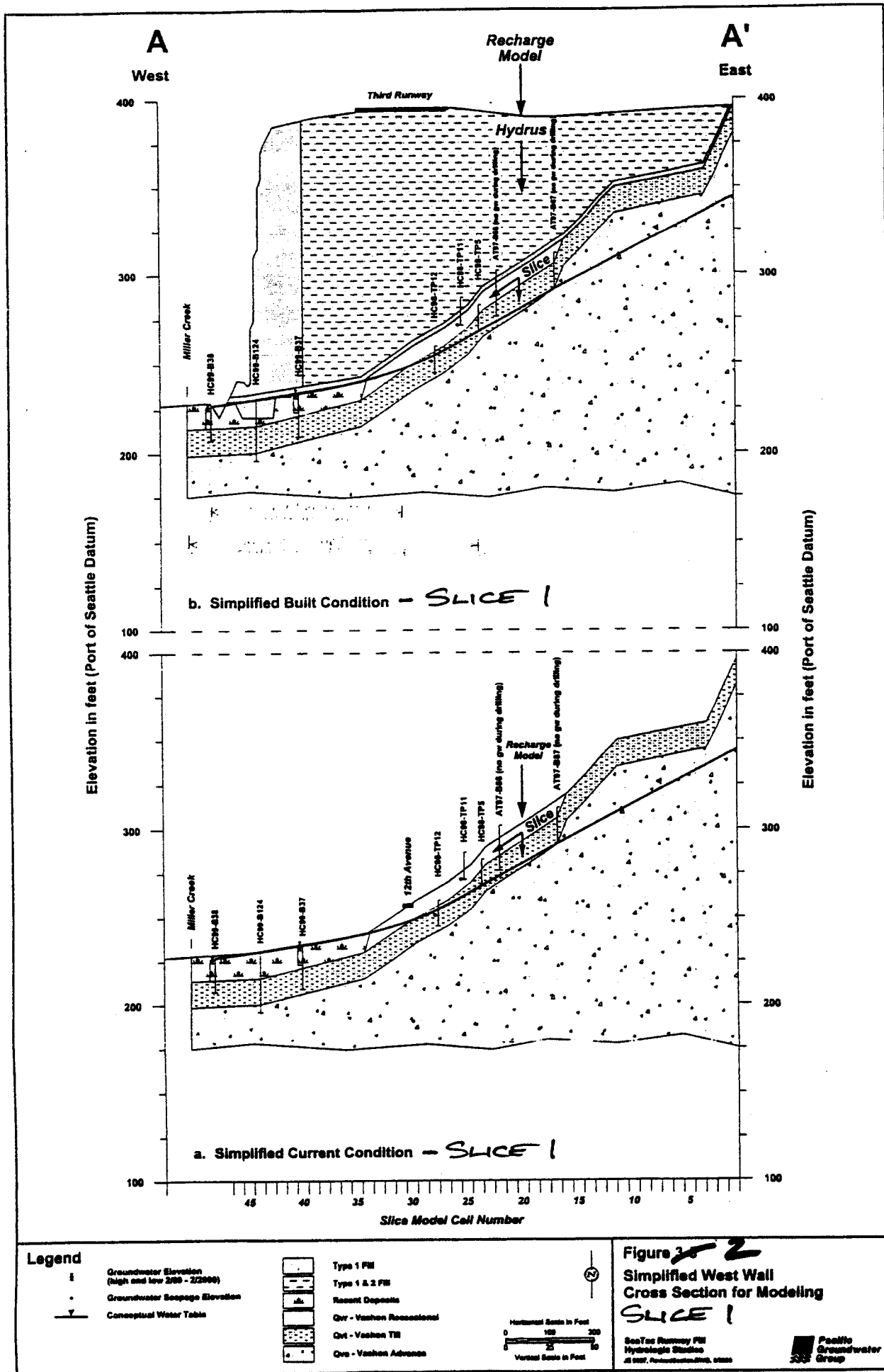
#### ATTACHMENTS

- Figure 1 – Cross section locations, fill thickness intervals, and model area designation
- Figure 2 – Draft cross section for Slice 1 (Figure 3-5 of Ecology Study)
- Figure 3 - Draft cross section for Slice 2 (H-C generalized geologic cross section A-A' with markups)
- Figure 4 - Draft cross section for Slice 3 (H-C generalized subsurface cross section E-E' with markups)

Figure 1

Area to be Modeled by Hydrus and Slice  
 ImperVIOUS Area



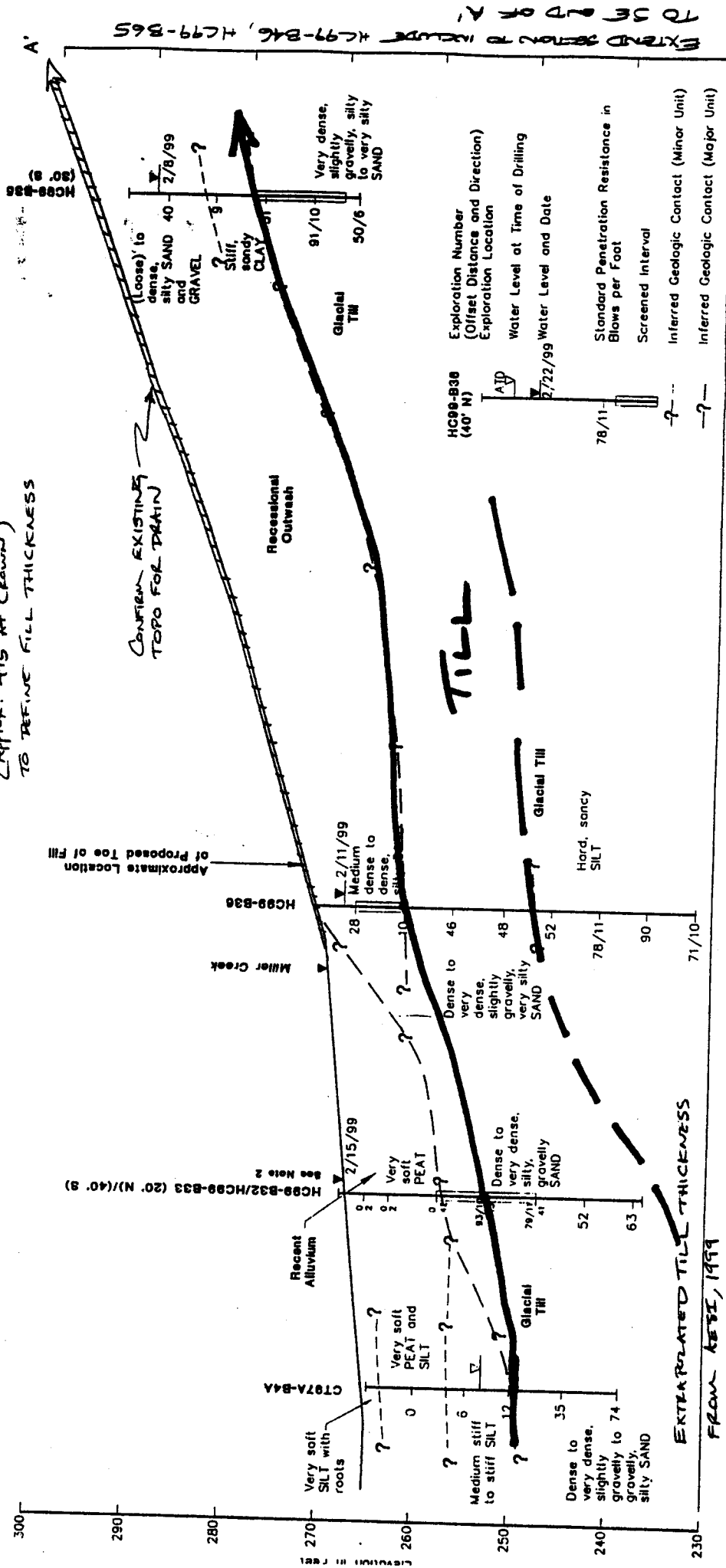


AR 011378

# SLICE 2

## Generalized Geologic Cross Section A-A' Looking Northeast

ADD "BUILT" TOPO (Approx. 415' Mt Crown) TO DEFINE FILL THICKNESS



HARTCROWSER  
J-4978-00 7/99

FLORIDA DEPT. OF TRANSPORTATION  
SECTION FOR SLICE 2

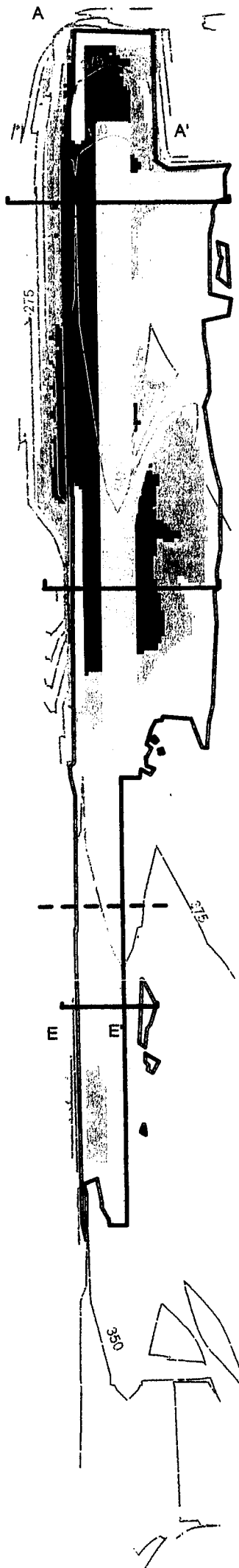
### FIGURE 3

Note: 1) Contacts between soil units are based upon interpretation between borings and represent our interpretation of subsurface conditions based on currently available data.

2) Screened interval for groundwater level shown is for HC99-832, however, HC99-833 is similar.

SOURCE: HMC CROWSER, 1999





Cross Section 2 for Slice 2  
(See Figure 4)

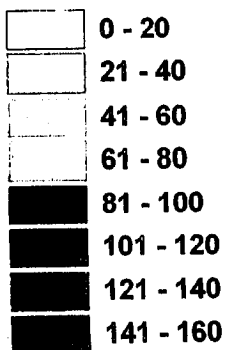
Cross Section 1 for Slice 1  
(See Figure 3)




Watershed Divide between  
Miller Creek and Walker Creek

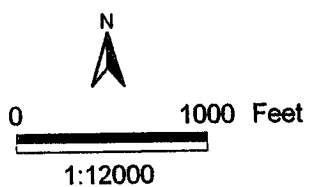
Cross Section 3 for Slice 3  
(See Figure 5)

DRAFT

**Depth of Fill (feet)**



-  Area to be Modeled by Hydrus and Slice
-  Impervious Area
-  As-Built Elevation Contours (25 ft Interval)



**Figure 1**

SeaTac Third Runway

Site Features for  
Hydrus-Slice Modeling





**Pacific Groundwater Group**  
2377 Eastlake Ave. E.  
Seattle, Washington 98102

206.329.0141 FAX 329.6968

## Memorandum

---

To: Keith Smith, Port of Seattle  
From: Charles Ellingson, Pacific Groundwater Group  
Re: Modeled Area and Hydrus Model Results Draft Interim Deliverables  
Date: June 25, 2001

---

This memo presents the following interim draft deliverables related to Hydrus/Slice modeling of third runway fill:

- Definition of the area to be modeled by Hydrus/Slice instead of HSPF
- Effective Recharge
- Selection of Cross Sections (locations for Slice models)
- Fill thickness
- Definition of soil as modeled in Hydrus
- Draft Interim Hydrus Modeling Results

### ***Definition of the Area to be Modeled by Hydrus/Slice Instead of HSPF***

PGG used existing GIS coverages of existing topography, “built” topography, and third runway pavement distribution to calculate areas for Hydrus/Slice modeling. The areas to be modeled by Hydrus and Slice (and therefore removed from HSPF) are shown on **Figure 1** and **Table 1**. The areas include proposed additional runway fill in the Miller and Walker Creek basins minus the steep slopes along the western edge of the constructed fill.

The north-south extent of the fill within the Miller Creek and Walker Creek basins will be used along with other data to integrate Slice model results along the respective basin lengths. A dashed line is drawn on **Figure 1** between the Miller and Walker Creek Basins. The location of the line is approximately the same as the co-incident surface water and groundwater basin boundaries used within the HSPF models (Parametrix SMP **Figure B2-2**). The areas indicated in **Table 1** below are consistent with the line drawn on **Figure 1**. The basin areas to be modeled by Hydrus/Slice are as follows:

AR 011382



**Table 1**  
**Summary of Areas to be Modeled by Hydrus/Slice**

	Miller Creek Basin	Walker Creek Basin
Pervious Fill Area	3,030,620 sq ft	450,630 sq ft
Runway and Taxiway Impervious Area	1,833,928 sq ft	260,743 sq ft
Total Area in Basin to be modeled by H-S	4,864,548 sq ft	711,373 sq ft

Runoff from impervious area (IA) is assumed to infiltrate in pervious areas (PA). Therefore the impervious and pervious areas in **Table 1** above are used to calculate effective recharge on pervious areas. IA in Walker creek consists of only the western half of the runway because runoff from the eastern half will not flow onto new third runway fill.

**Effective Recharge**

Effective recharge was calculated using the following algorithm which is called “recharge 1”:

- *AquaTerra applied daily precipitation between 1984 and 1994 to grass on flat outwash in HSPF (regional parameters)*
- *the resulting daily recharge (R) was increased to account for secondary infiltration of runoff from impervious surfaces using the following formula for effective recharge (ER):*

$$ER=R+(R*(IA/PA))$$

While this method accounts for runoff from the impervious areas, we acknowledge that it employs a lower-end estimate of impervious runoff. Impervious runoff is underestimated because it is assumed equal to the recharge rate below grass on outwash soils. In actuality, the impervious areas will lose less water to evapotranspiration than grass would, and would therefore have more water available for runoff to the pervious area. The simplifying assumption that runoff rates equal calculated recharge rates was adopted to facilitate the timeline of the modeling exercise. However, we performed additional analysis on whether an upper-end estimation of runoff from impervious areas is not likely to cause significant overland flow in the pervious areas. Based on conversations with team members, the following algorithm (called “recharge 2”) was used to assess the upper-end estimation of runoff:

**AR 011383**

- *AquaTerra increased precipitation (P) to account for runoff from impervious surfaces to pervious surfaces using the following formula for effective precipitation:*

$$EP=P+(P*IA/PA)$$

- *AquaTerra applied EP between water years 1984 to 1994 for grass on flat outwash in HSPF (regional parameters)*

The sum of daily “recharge 1” over the 11-year period was 18.7% less than the sum of daily “recharge 2” over the same period. This result suggests that the difference in runoff from the two methods is relatively small, and that recharge 1 may underestimate actual recharge. **Figure 2** is a plot of Miller Creek ER as calculated by “recharge 1” and “recharge 2”. Walker Creek has a very similar IA/PA ratio and therefore similar ER.

### ***Selection of Cross Sections (Locations for Slice Models)***

PGG reviewed hydrogeologic conditions and embankment geometries along the extent of the embankment fill. Three representative hydrogeologic cross sections were selected using available subsurface data and interpretations. Cross sections 1 and 2 are representative of conditions within the thicker Miller Creek basin fill while cross section 3 is representative of conditions in the Walker Creek basin fill and the southern, thinner portion of the Miller Creek fill. Hydrogeologic cross sections will be used to create Slice models.

Cross sections are based on subsurface data described in available geotechnical and hydrogeologic reports and from the existing and proposed topography of the third runway area. PGG compiled and reviewed consulting reports completed since the Ecology Study in order to select cross section alignments. The following is a brief summary of each cross section. **Figure 1** shows the approximate locations of cross sections.

***Cross Section 1 (Slice 1):*** This cross section is located through the thickest portion of the fill embankment with a fill thickness of up to 160 feet. Cross section 1 is located at the same location as the original slice model used by PGG in the Ecology study. **Figure 3** presents the schematic cross section upon which the Slice 1 model will be based.

***Cross Section 2 (Slice 2):*** This cross section is located through the northern portion of the fill embankment near the northern end of the proposed third runway. Cross section 2 was developed from a generalized hydrogeologic cross section originally created by Hart Crowser through the northern toe of the fill embankment (Hart Crowser, 1999) and from supplemental test pit data in this area. **Figure 4** presents the schematic cross section which will be used as the basis for the Slice 2 model. The third runway fill in Cross Section 2 is thinner than Cross Section 1. Soil unit designations to be used in the slice model are also shown.

***Cross Section 3 (Slice 3):*** This cross section is located immediately north of the South MSE wall. A fill thickness of up to 23 feet occurs in this slice. Cross section 3 was based on a generalized hydrogeologic cross section originally created by Hart Crowser through

the northern end of the MSE wall study area (Hart Crowser, 2000b). It was updated based on supplemental geotechnical data (Hart Crowser, 2000c), existing and proposed topography, and available till mapping data. **Figure 5** presents the schematic cross section which will be used as the basis for the Slice 3 model. Soil-unit designations to be used in the slice model are also shown.

Only Cross Section 3 will be used to represent subsurface conditions within the Walker Creek basin fill. The section was chosen through fill of intermediate thickness. Although this single section will not accurately represent the variety of fill thicknesses in Walker Creek basin fill, the thick portion of the fill is of small areal extent.

### ***Fill Thickness***

The thickness of fill is mapped on **Figure 1** based on pre- and post- construction topography as provided in existing GIS coverages. A series of Hydrus models was used to represent discrete values of fill thickness over the observed range.

### ***Definition of Soil as Modeled in Hydrus***

Soils are defined in Hydrus using variables that relate hydraulic conductivity to moisture content and soil tension. For this modeling exercise, soils were defined the same way as they were for the modeling work done for the Ecology project (Sea-Tac Runway Fill Hydrologic Studies Report, PGG, 2000). Appendix C of PGG (2000) provides a detailed description of the fill soils and modeled soil parameters.

### ***Draft Interim Hydrus Modeling Results***

Separate one-dimensional Hydrus models were run for fill thicknesses of 10, 30, 50, 70, 90, 110, 130, and 150 feet in both the Miller and Walker Creek basins (as dictated by the thicknesses observed on the cross sections). In addition, a 20-foot model was developed for Walker Creek only. **Figure 6** shows daily effective recharge (input to the top boundary of all Hydrus models) and eight daily outflow graphs (flow out of the bottom boundary of the Hydrus model) for the Miller Creek basin over the "test period" of October 1, 1990 through September 30, 1994. Conservation of mass was confirmed for each model by comparing the total effective recharge and total outflow. Time series outflow from Hydrus runs of varying thicknesses will be used as input to the three Slice models.

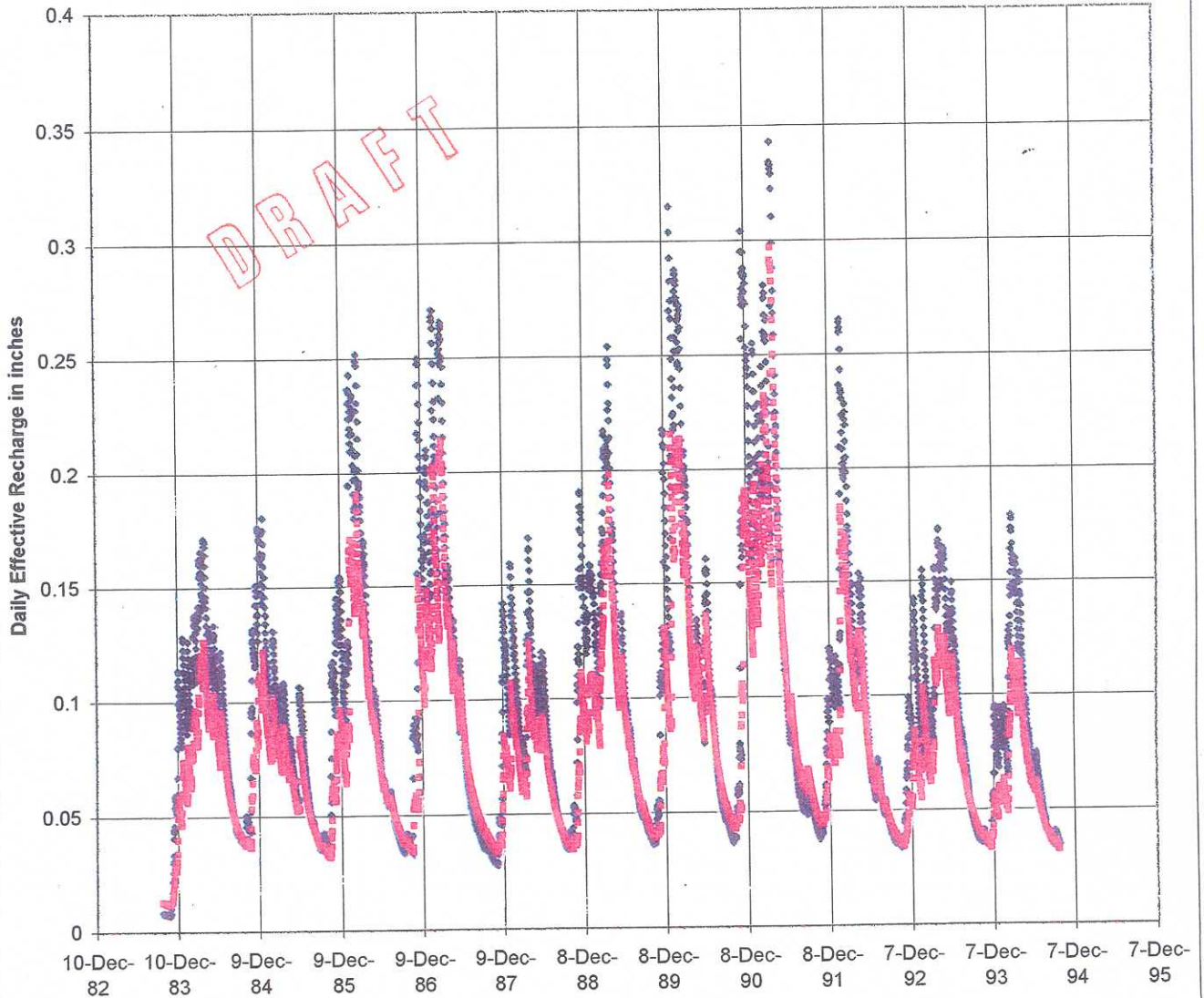
**Figure 6** shows that the seasonal recharge pulse introduced at the land surface (ER) is predicted to be lagged and dampened as a function of the thickness of the fill. Lagging causes the arrival of the recharge pulse to be delayed from its introduction at the land surface to its arrival at the bottom of the fill. Dampening causes a reduction in the overall range of recharge values (high minus low) due to uptake and subsequent release of

**AR 011385**

recharge inflow into the soil's pore water. Lagging and dampening both increase with increasing thicknesses of fill. These effects on the timing of recharge will impact the arrival of flow to the top of the slice model, and ultimately the arrival of baseflow to the streams bordering the study area.

**AR 011386**

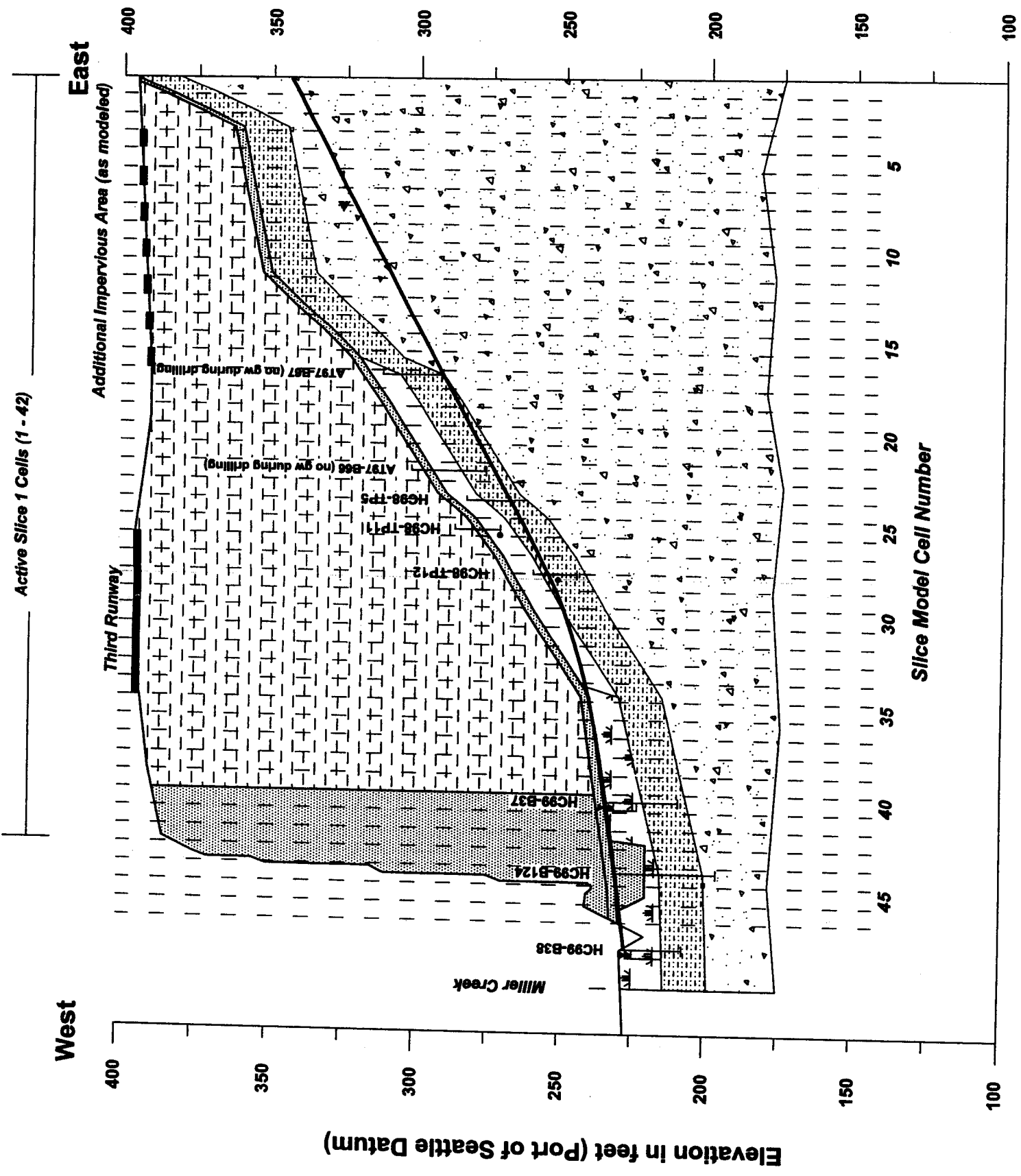
**Figure 2**  
**Comparison of Effective Recharge Estimates**  
**For Miller Creek**



Recharge 1 total inches over 10 years = 331.44  
 Recharge 2 total inches over 10 years = 407.48  
 Difference = 18.7%  $((R2-R1)/R2)$

- Recharge 2 - ER from scaled EP->HSPF
- Recharge 1 - ER from P->HSPF then scaled

DRAFT



**LEGEND**

- Groundwater Elevation
- Groundwater Seepage Elevation
- Conceptual Water Table

**FILL**

- Cvr - Vashon Recessional
- Cvt - Vashon Till
- Cva - Vashon Advance

Horizontal Scale in Feet  
 0 100 200  
 Vertical Scale in Feet  
 0 25 50

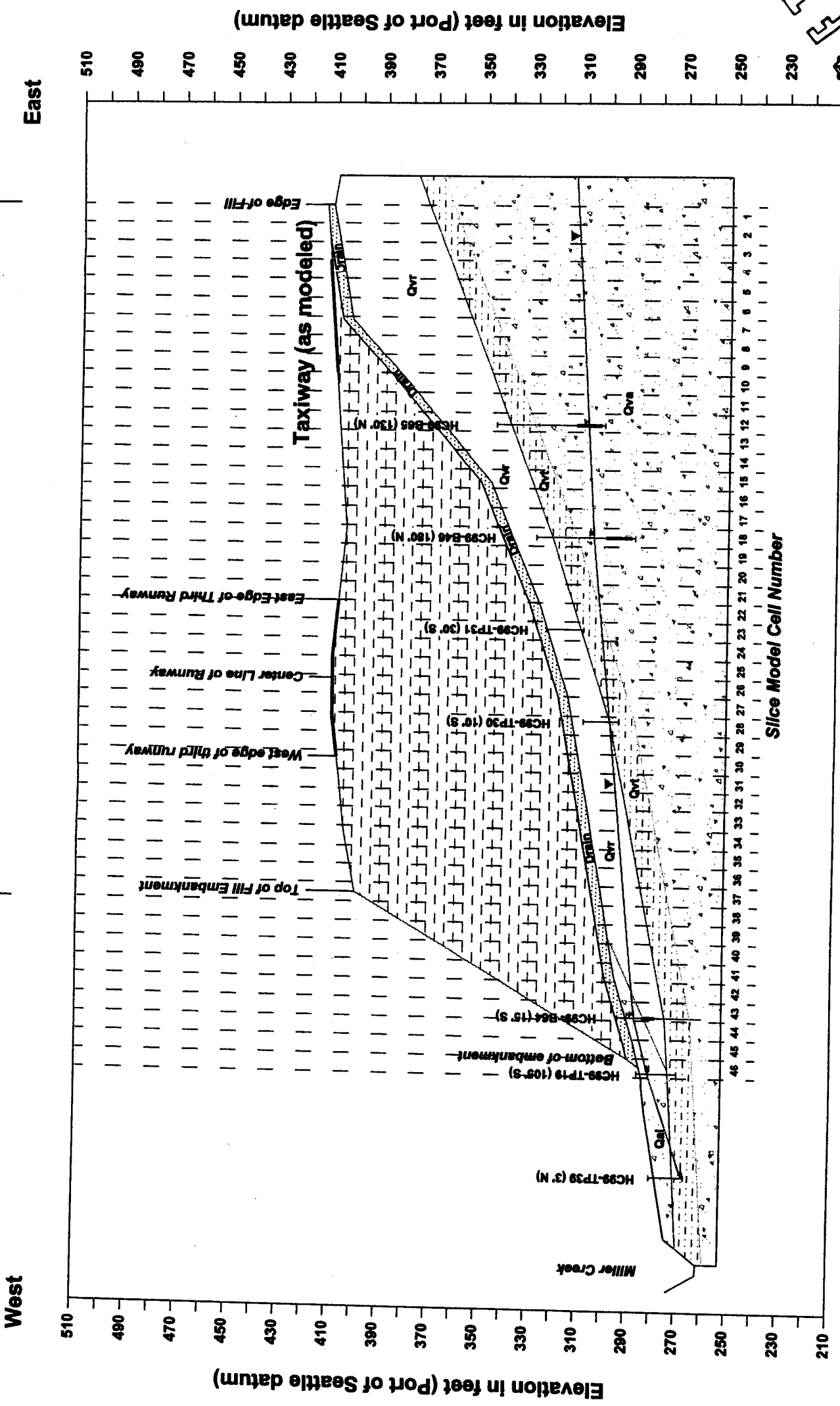
**FIGURE 3**  
 Simplified Cross Section for Slice 1

SeaTec Third Runway  
 Embankment Fill Modeling  
 J20105, Runway-Seattle, Aug. 04/01



AR 011388

Slice 2 Active Cells (1 - 37)



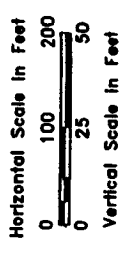
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AR 011389

**LEGEND**

- x Groundwater Elevation
- Groundwater Seepage Elevation
- Conceptual Water Table

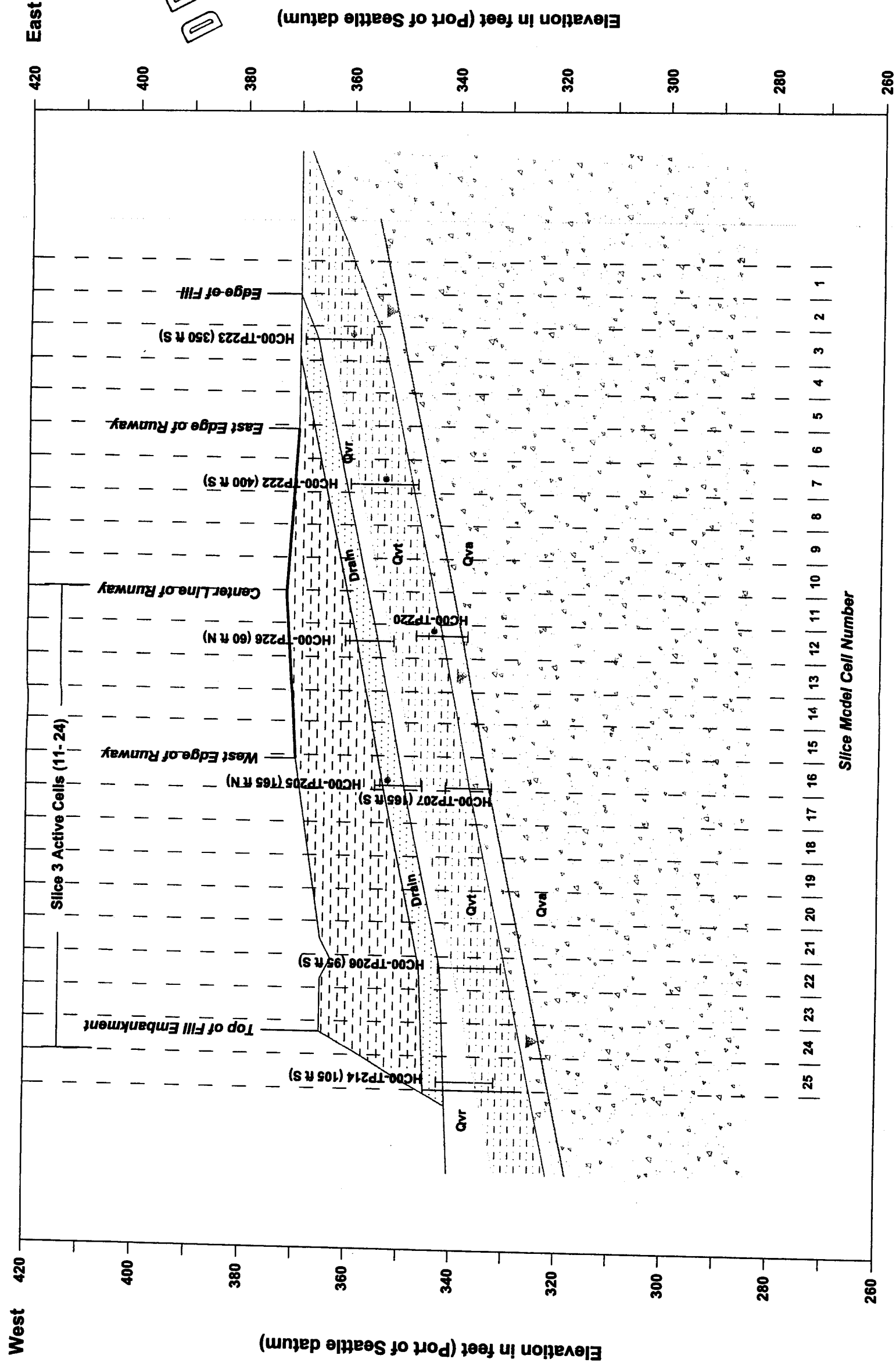
- Fill
- Qvr - Vashon Recessional
- Qvt - Vashon Till
- Qva - Vashon Advance



**FIGURE 4**  
Simplified Cross Section for Slice 2

SeaTac Third Runway  
Embankment Fill Modeling  
20105, Runway-Embankment, 01/01



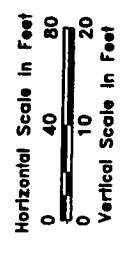


AR 011390

**LEGEND**

- Groundwater Elevation
- Groundwater Seepage Elevation
- Conceptual Water Table

- █ Fill
- ▨ Qvr - Vashon Recessional
- ▩ Qvt - Vashon Till
- ▧ Qva - Vashon Advance



**FIGURE 6**  
Simplified Cross Section for Slice 3

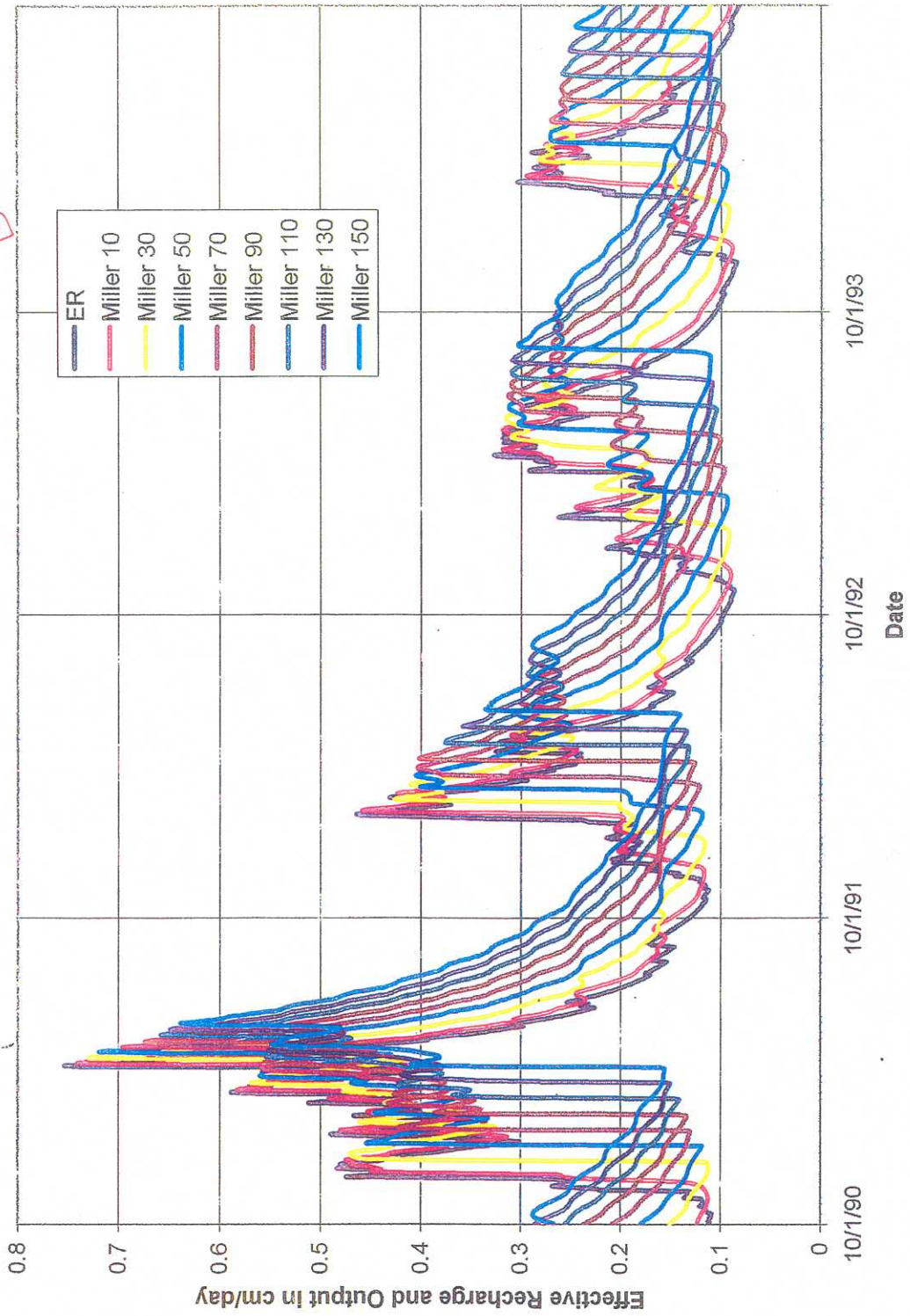
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Embankment FEM Modeling  
201105, Runway-Seattle2.dwg, 09/01



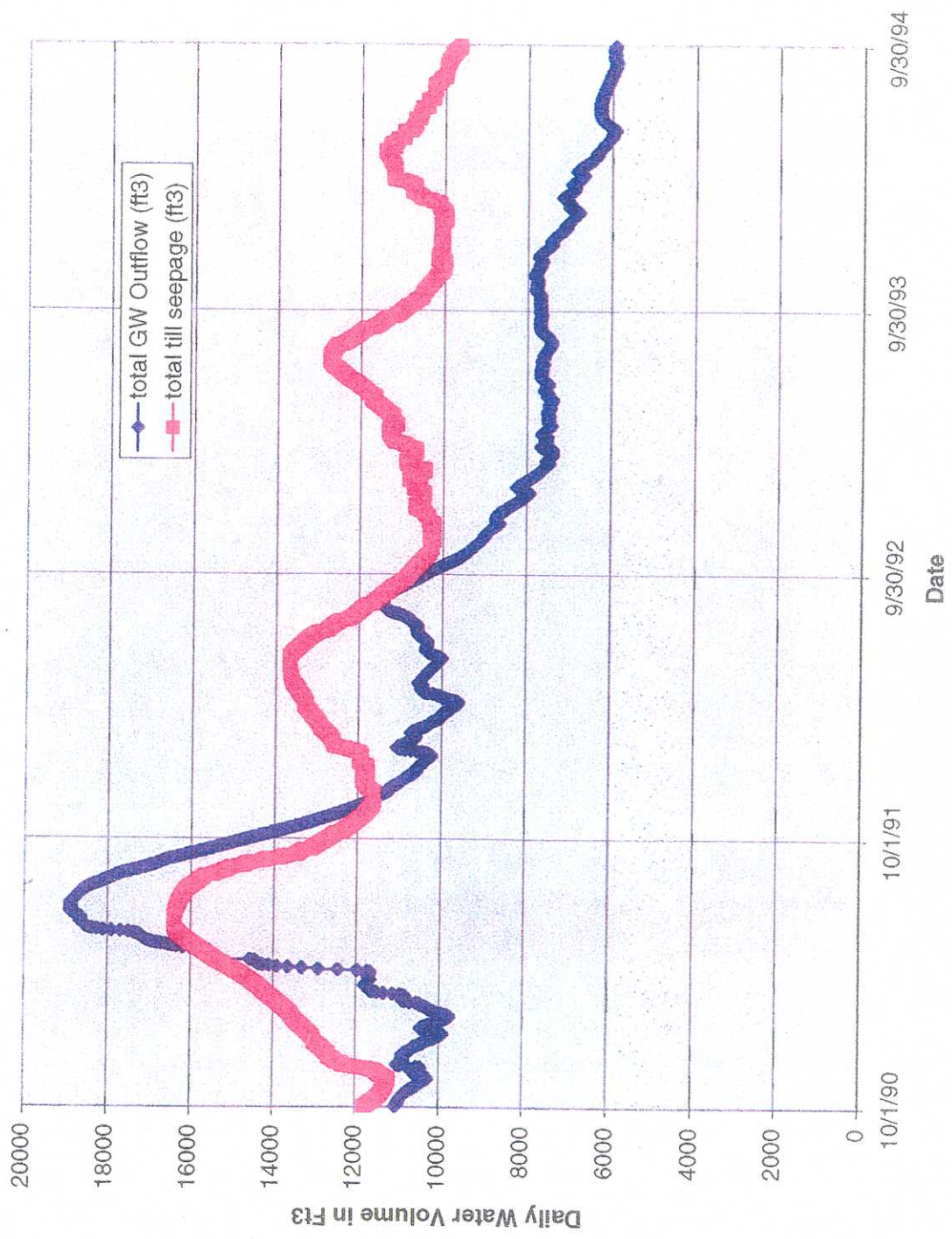


DRAFT

Figure 6  
Miller Creek Hydrus Input and Output  
(Oct 1, 1990 - Sep 30, 1994)



# Integrated Flows for Miller Creek Embankment



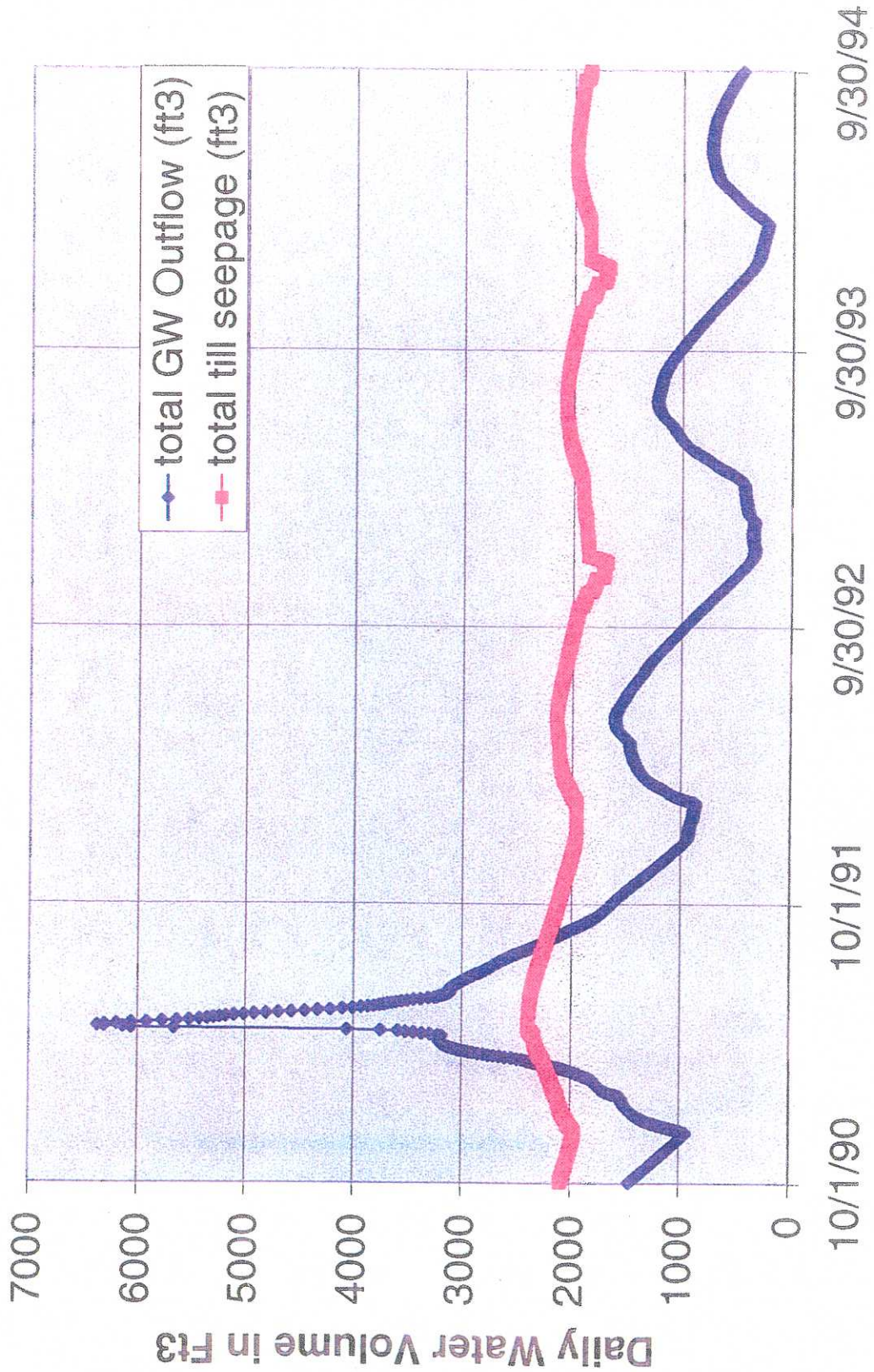
AR 011392

6/28/01

test period integrated results jun27.xlsMiller Creek chart



# Integrated Flows for Walker Creek Embankment



**Summary of non-hydrologic impacts.**

Creek	Number of Septic Tanks	Flows (cfs)			Total Impact
		Septic Tank Estimated	Septic Tank Adjusted <sup>a</sup>	Withdrawals <sup>b</sup>	
Miller	236	-0.082	-0.057	+0.042	-0.02
Walker	41	-0.014	-0.009	N/A	-0.01

<sup>a</sup> Septic tank flow adjusted by percentage of seepage to DEEPFR (flow x 0.7)

<sup>b</sup> Assumed average withdrawal rates form June 1 through September 30.

## Water Recharge from Imported Water

### Documentation

This spreadsheet was created for the purpose of developing time series representing water recharge from water imported into the buyout area via water distribution systems. Recharge takes two potential forms: 1. recharge from septic systems; 2. recharge from infiltration of irrigation water. These time series is intended to represent conditions prior to the acquisition and removal of homes in the buyout area.

The time series on this worksheet (Recharge from Septic) represents water recharge from septic systems in the buyout area. The recharge to each watershed from septic systems was based on estimated water consumption. Recharge from septic systems is based on winter water use. Additional water use during summer months (above and beyond winter water use) is provided in a separate time series.

The estimated annual water recharge from septic systems in the buyout area to each of the three watersheds is provided below in Columns A - D. The time series is constructed with an hourly time step. However, the recharge rates reflect estimated daily recharge.

The estimated annual water recharge from infiltration of imported irrigation water in the buyout area to each of the three watersheds is provided on the "infiltration" worksheet in Columns A - D. The time series is constructed with an hourly time step. However, the recharge rates reflect estimated daily recharge. It should be noted that this time series contains the quantity of imported water available for infiltration. It is recommended that losses due to evaporation be applied to the time series before it is added as recharge flow.

**Estimated Recharge Quantity**

A total of 291 homes were served by septic in Walker and Miller Creek Basins prior to the buy-out. Some of these homes (14) were also served by sewer, and the septic systems were inactive. Based on an inventory of septic tanks removed, there were 41 active tanks in the Walker Creek basin buy-out area and 236 in the Miller Creek basin buy-out area. (277 total active tanks, 14 inactive tanks)

Summer water use was not used to estimate recharge from septic systems. It was used to estimate the quantity of imported water potentially available for infiltration, with the assumption that the additional water use is for irrigation. Typical values for summer water use were provided by Water District #20 and Water District #125.

Summer water use exceeding winter water use may be available for infiltration. The time series constructed on the "Infiltration" worksheet is based on this daily flow.

Winter water use was used to estimate recharge from septic systems. Typical values for winter water use were provided by Water District #20 and Water District #125.

Estimated recharge to each watershed expressed in gallons per day.

Number of Active Septic Tanks	236	41	0
Summer Water Use (gallons per home per day)	349	349	349
Summer Recharge via Infiltration (gallons per day)	23,539	4,089	-
Winter Water Use (gallons per home per day)	249	249	249
Recharge from Septic Systems (gallons per day)	52,962	9,201	-

## Documentation

This spreadsheet was created for the purpose of developing a time series representing water withdrawal from Miller Creek due to pumping. This time series is intended to represent conditions prior to the acquisition and removal of homes in the buyout area.

The worksheet "Annual Time Series" contains the estimated annual water withdrawal from Miller Creek. The total water withdrawal is shaded in yellow in Column B. It is the sum at each hour of the estimated water withdrawal from each parcel contained in Columns C - Y

The time series was constructed using the values and notes presented in Table G-2 that was provided on 6/11/01. A reproduction of the values included in Table G-2 is provided below.

Key notes from Table G-2 used to construct the time series are listed below:

The quantity of water withdrawn was estimated as follows:

Available Pumping Rate x Estimated Months of Water Use Per Year x 0.5 (daily pumping duration)

The time series was constructed using the available pumping rate for 12 hours per day (6 am to 6 pm each day).

Estimated months of water use per year was assumed as follows:

6 Months = April 1 - Sep 30

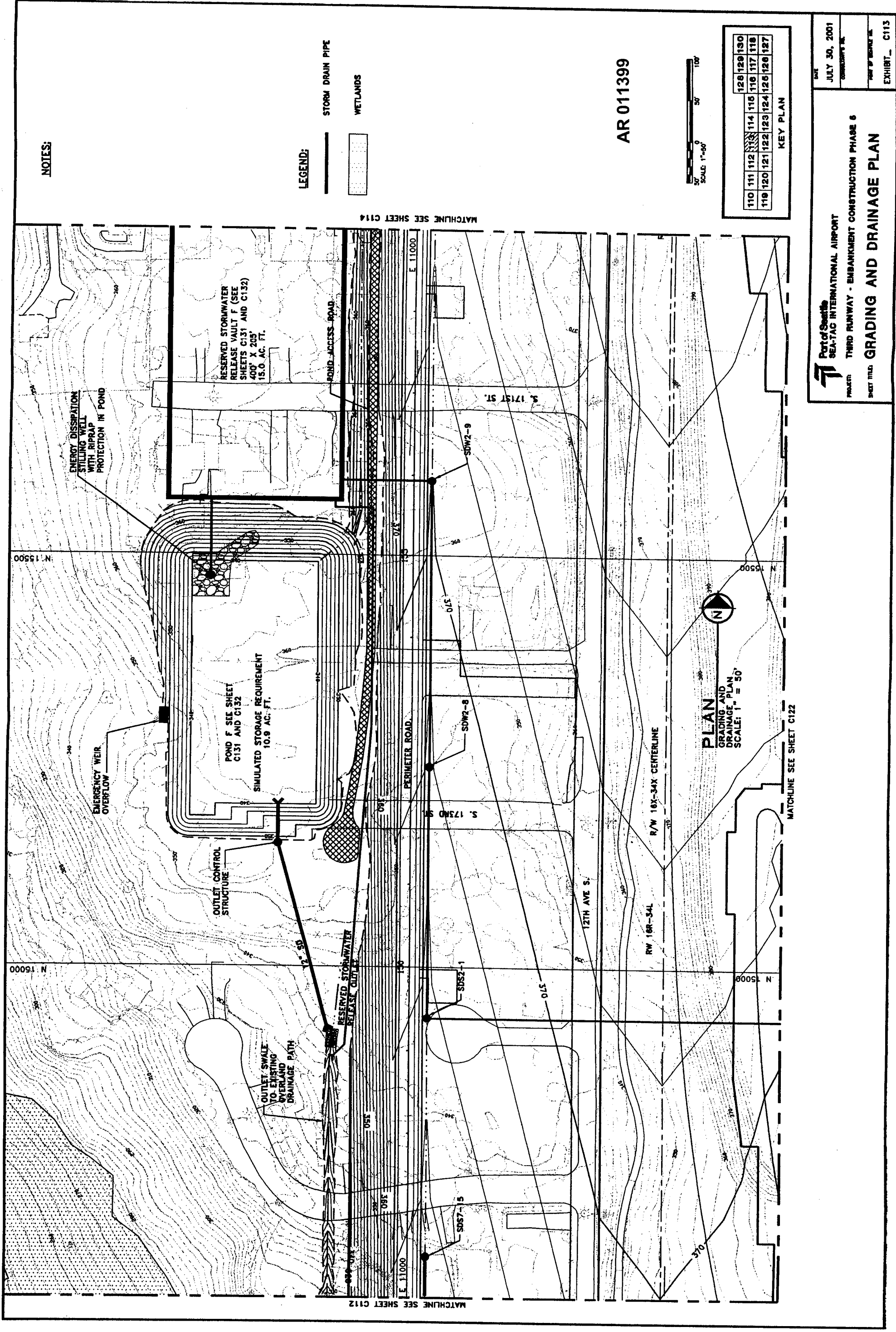
4 Months = June 1 - Sep 30

AR 011397

**RESERVE STORMWATER  
RELEASE STORAGE DRAWINGS**

**AR 011398**





MATCHLINE SEE SHEET C112

MATCHLINE SEE SHEET C114

**NOTES:**

**LEGEND:**

- STORM DRAIN PIPE
- ▨ WETLANDS

AR 011399



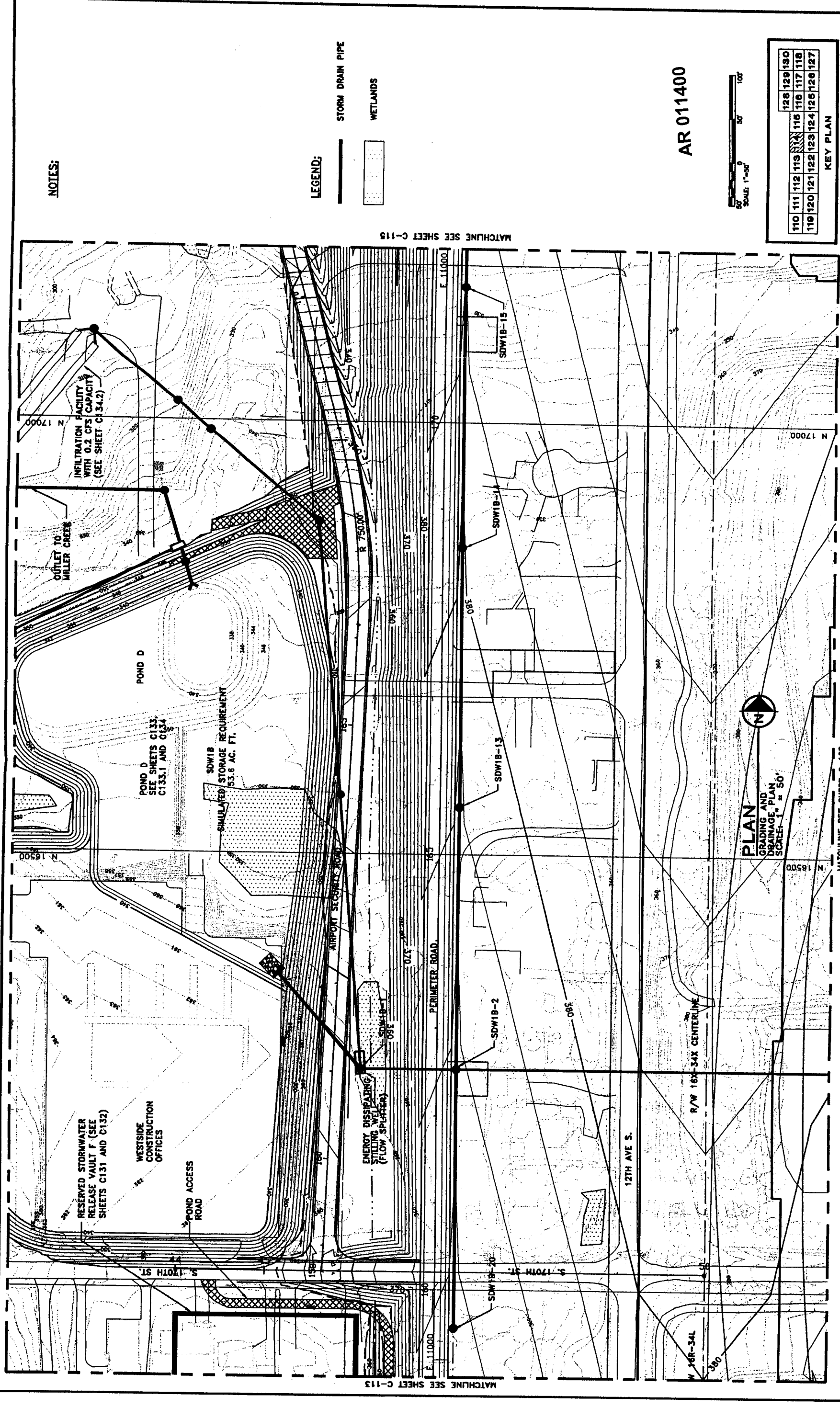
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128	129	130						

KEY PLAN

**PLAN**  
GRADING AND  
DRAINAGE PLAN  
SCALE: 1" = 50'

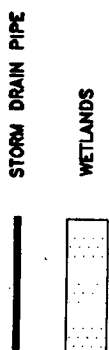
**Port of Seattle**  
SEA-TAC INTERNATIONAL AIRPORT  
PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6  
SHEET TITLE: GRADING AND DRAINAGE PLAN

DATE: JULY 30, 2001  
CONTRACT NO.:  
SHEET NO.:  
EXHIBIT: C113



NOTES:

LEGEND:

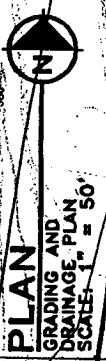


AR 011400



KEY PLAN

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128	129	130						



MATCHLINE SEE SHEET C-123

MATCHLINE SEE SHEET C-115

MATCHLINE SEE SHEET C-113

Part of Seattle  
SEA-TAC INTERNATIONAL AIRPORT  
PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6  
SHEET TITLE: **GRADING AND DRAINAGE PLAN**

DATE: JULY 30, 2001  
CONTRACT NO.:  
SHEET NO.:  
EXHIBIT: C114

NOTES:

LEGEND:

STORM DRAIN PIPE

WETLANDS

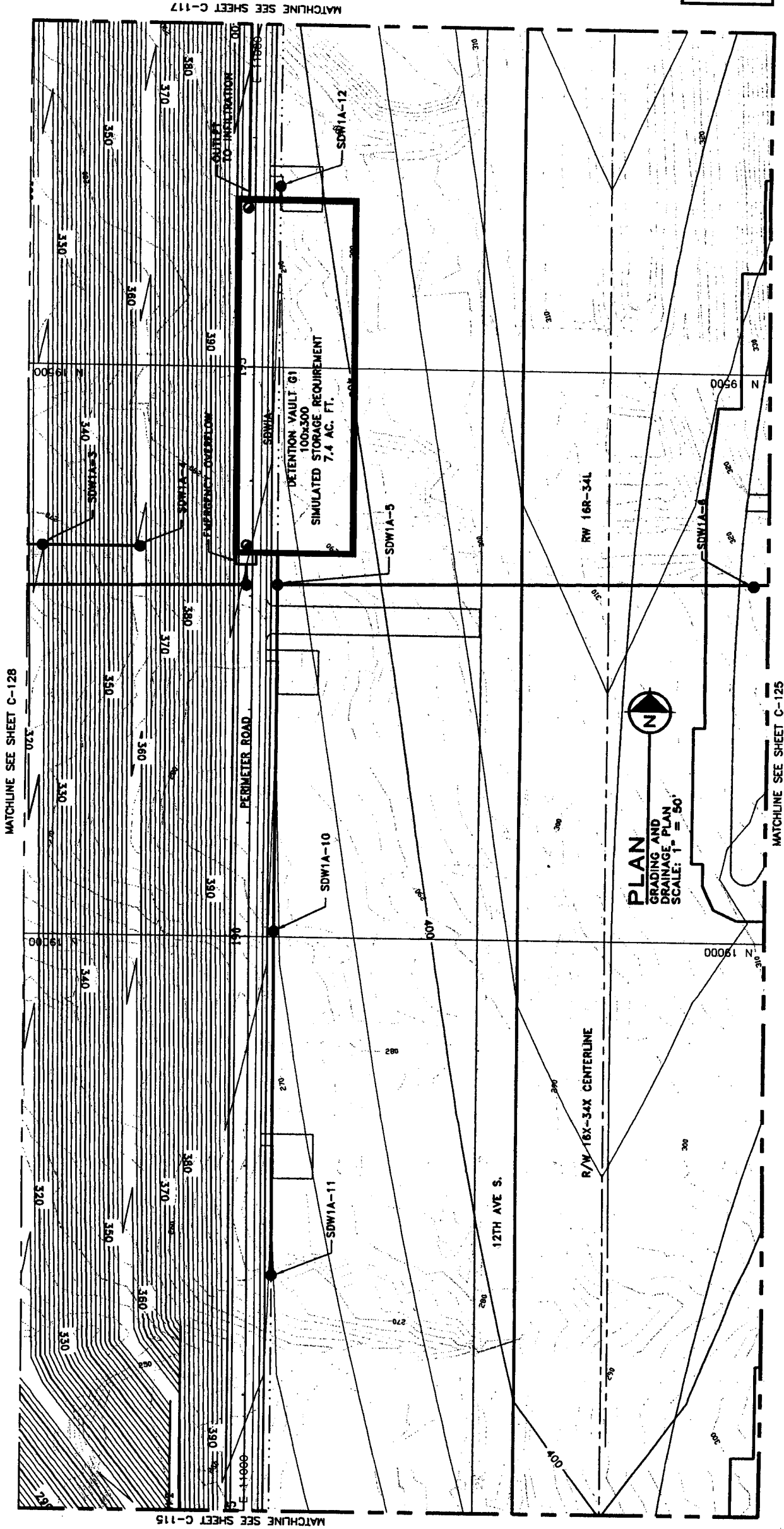


AR 011401



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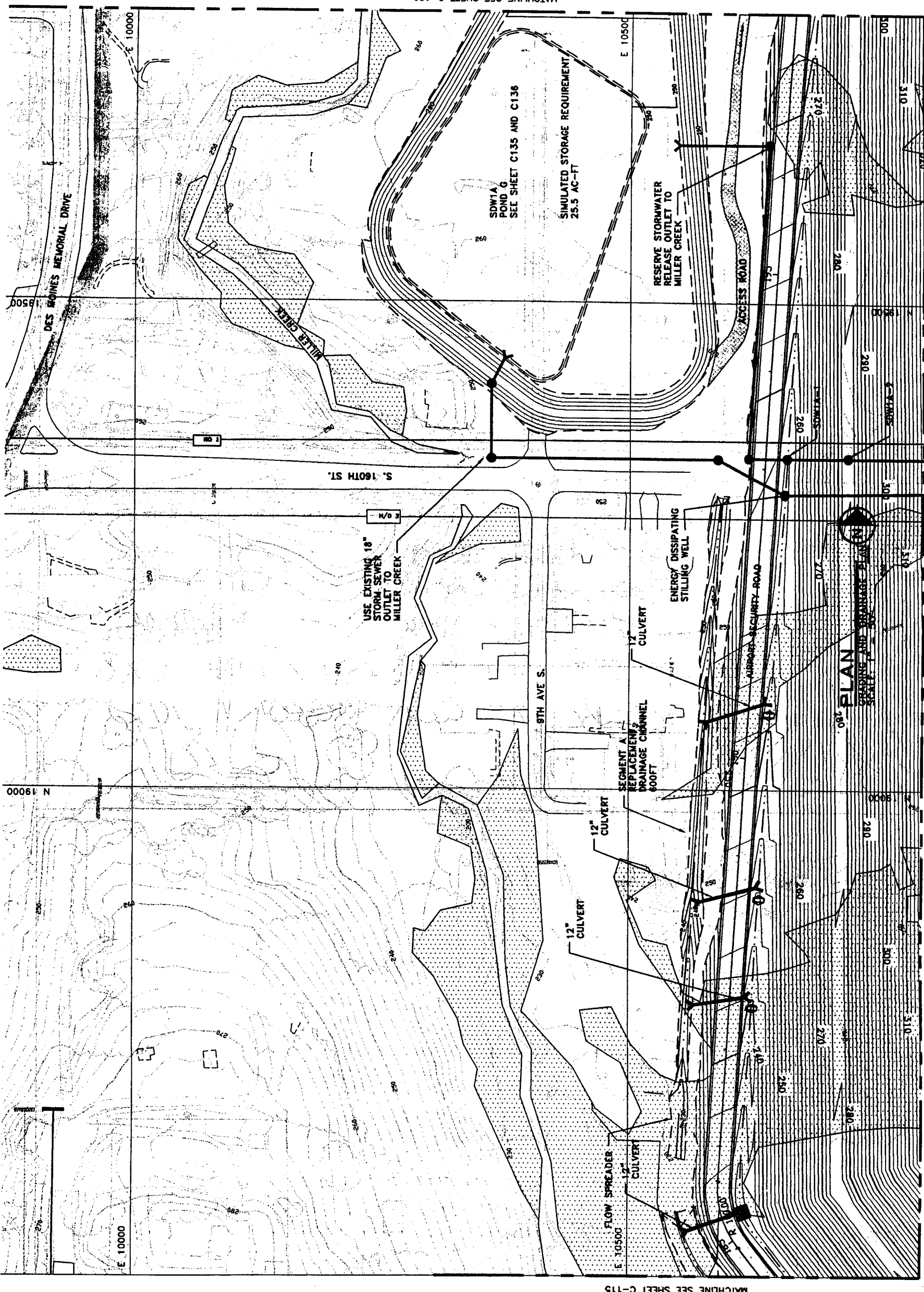
KEY PLAN



**PLAN**  
 GRADING AND  
 DRAINAGE PLAN  
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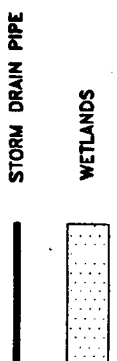
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DATE: JULY 30, 2001  
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 EXHIBIT: C116



NOTES:

LEGEND:



AR 011402



128	129	130
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KEY PLAN

MATCHLINE SEE SHEET C-116

MATCHLINE SEE SHEET C-129



**Port of Seattle**  
 SEA-TAC INTERNATIONAL AIRPORT  
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NOTES:

LEGEND:

STORM DRAIN PIPE

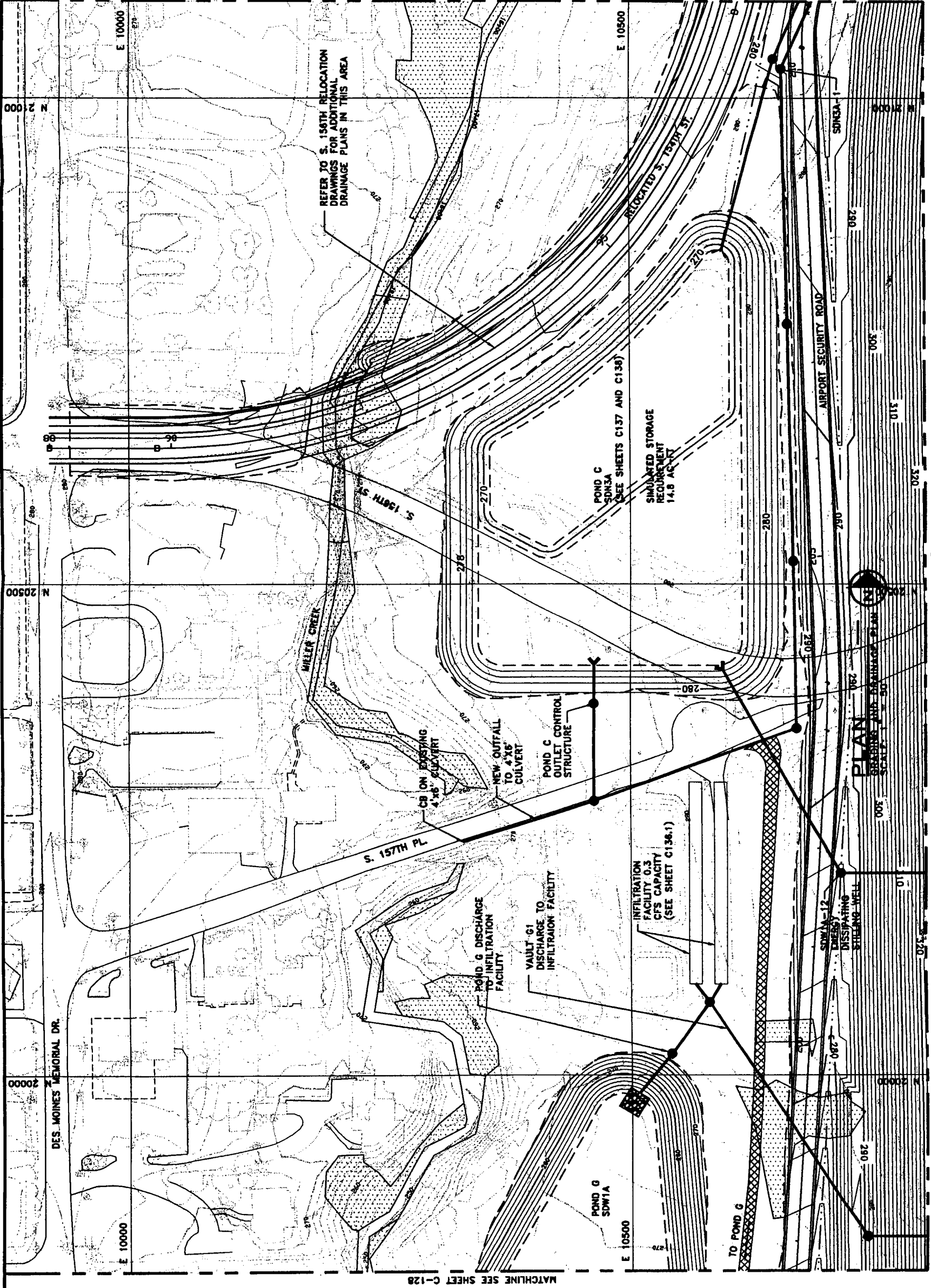
WETLANDS



AR 011403

DATE  
JULY 30, 2001  
DRAWN BY  
TMR/BJ/SHR/ML  
EXHIBIT  
C129

Port of Seattle  
SEA-TAC INTERNATIONAL AIRPORT  
PROJECT THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6  
SHEET TITLE GRADING AND DRAINAGE PLAN



REFER TO S. 156TH RELOCATION DRAWINGS FOR ADDITIONAL DRAINAGE PLANS IN THIS AREA

POND C SINKS (SEE SHEETS C137 AND C138)

SIMULATED STORAGE REQUIREMENT 14.8 AC-FIT

CB ON EXISTING 4\"/>

NEW OUTFALL TO 4\"/>

POND C OUTLET CONTROL STRUCTURE

POND G DISCHARGE TO INFILTRATION FACILITY

VAULT G1 DISCHARGE TO INFILTRATION FACILITY

INFILTRATION FACILITY G.3 CFS CAPACITY (SEE SHEET C136.1)

PLAN

SCALE: 1\"/>

MATCHLINE SEE SHEET C-128

MATCHLINE SEE SHEET C-117

MATCHLINE SEE SHEET C-130

DES. MOINES MEMORIAL DR.

N 21000

N 20500

E 10000

E 10000

E 10500

E 10500

E 10000

**NOTE:**

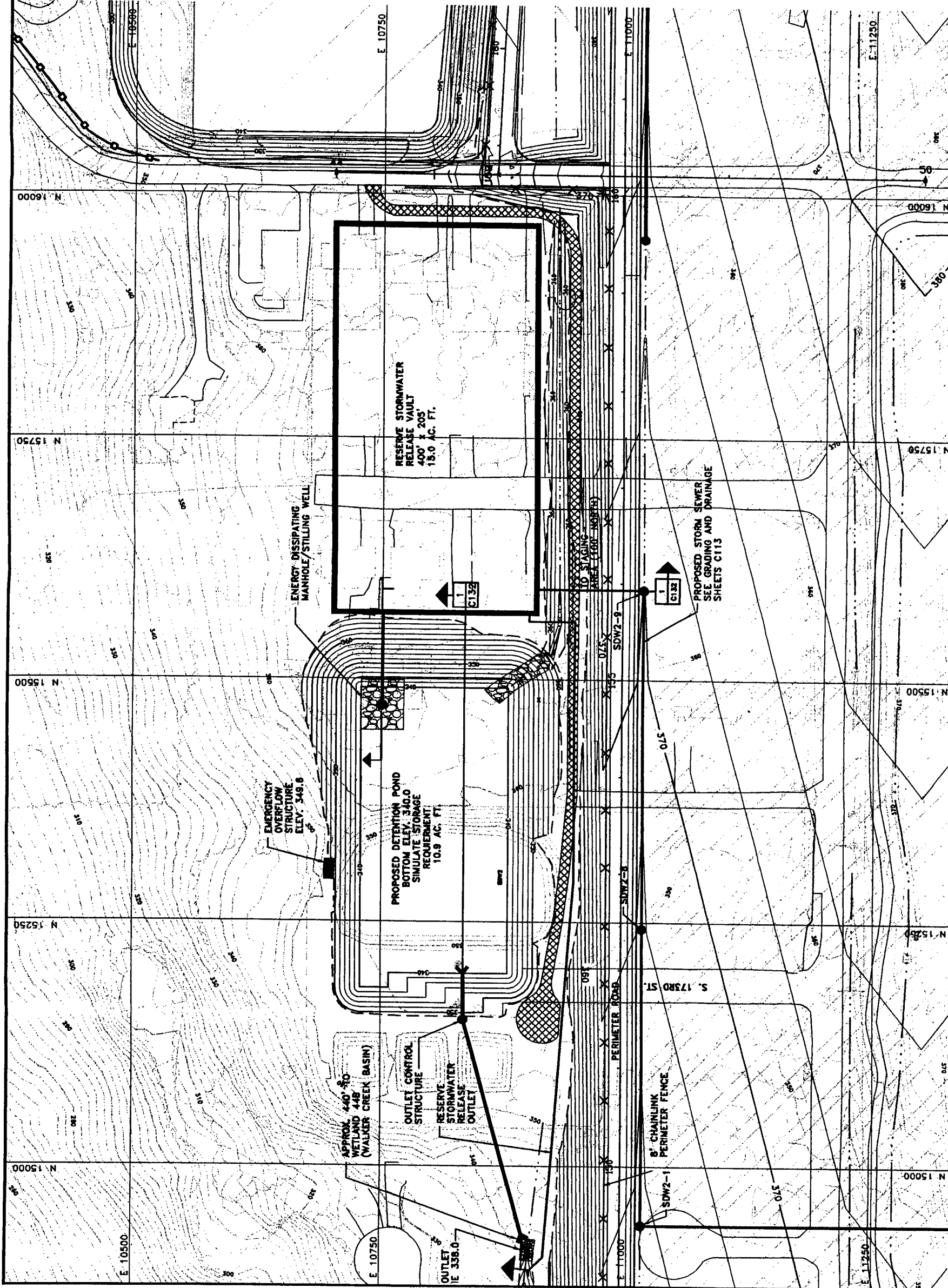
THE STORM SEWER WILL CONNECT TO AN INTERNAL PIPING AND DIVIDING WALL SYSTEM TO PROMOTE CIRCULATION AND FLUSHING OF THE RESERVE STORMWATER RELEASE WATERS WITHIN THE DEAD STORAGE AREA.

**LEGEND:**

- CHAINLINK FENCE
- PROPOSED STORM DRAIN
- ACCESS ROAD
- SUBJECT TO CHANGE PENDING FINAL AIRFIELD DESIGN



**AR 011404**



**PLAN**  
POND F PLAN  
SDW2 BASIN POND  
SCALE: 1" = 50'

**Port of Seattle**  
SEA-TAC INTERNATIONAL AIRPORT

PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6

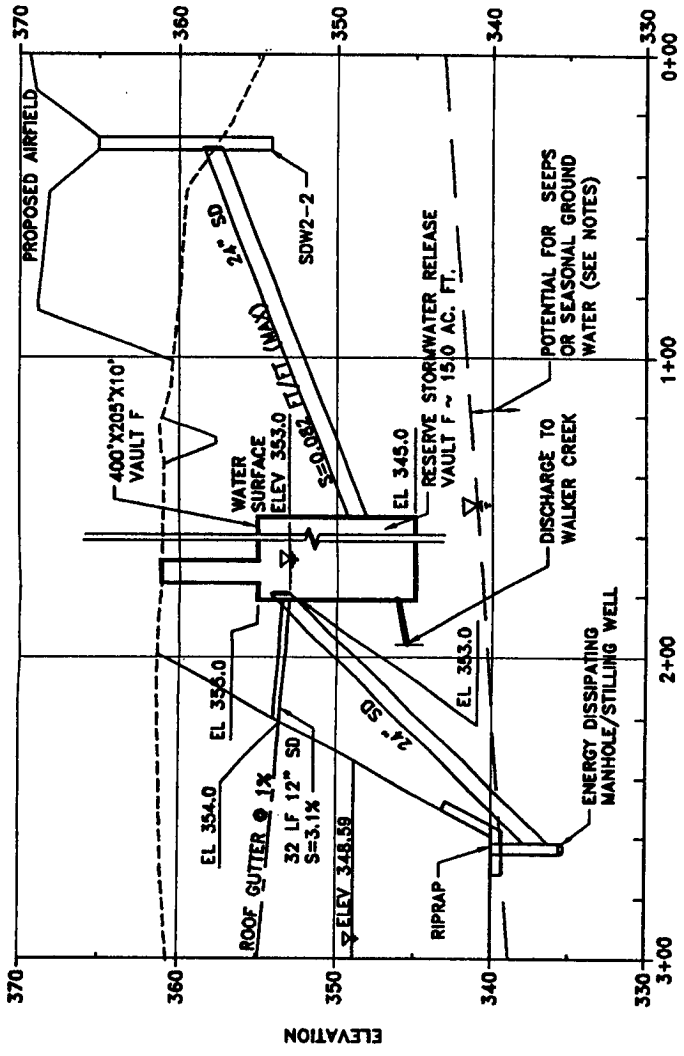
SHEET TITLE: **POND F PLAN**  
**SDW2 BASIN POND**

DATE: JULY 30, 2001

DESIGNER'S NO.

DATE OF REVISION NO.

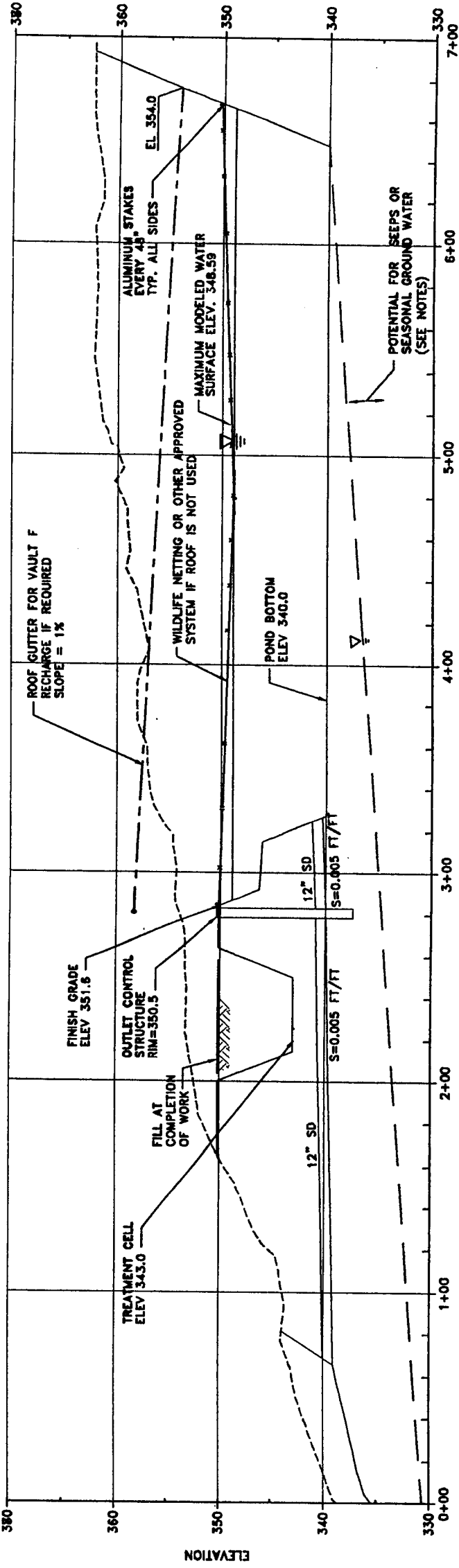
EXHIBIT - C131



**PROFILE 2**  
 C131  
 STORM DRAIN AND RESERVE  
 STORMWATER RELEASE VAULT PROFILE  
 SCALE: HORIZ 1" = 30'  
 VERT 1" = 6'

**NOTES:**

- GROUND WATER ELEVATIONS ARE APPROXIMATE BASED ON AVAILABLE GEOTECHNICAL INVESTIGATIONS AND HYDROGEOLOGIC ELEVATION WILL BE INCLUDED AS PART OF THE FINAL DESIGN ANALYSIS.
- FINAL POND CONFIGURATION MAY VARY TO MAINTAIN STORM WATER STORAGE ABOVE THE OBSERVED GROUND WATER.



**PROFILE 1**  
 C131  
 POND F PROFILE  
 SDW-2 BASIN POND  
 SCALE: HORIZ 1" = 30'  
 VERT 1" = 6'

GROUND WATER AT ELEV. 260±

AR 011405



Port of Seattle  
 SEA-TAC INTERNATIONAL AIRPORT

PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6

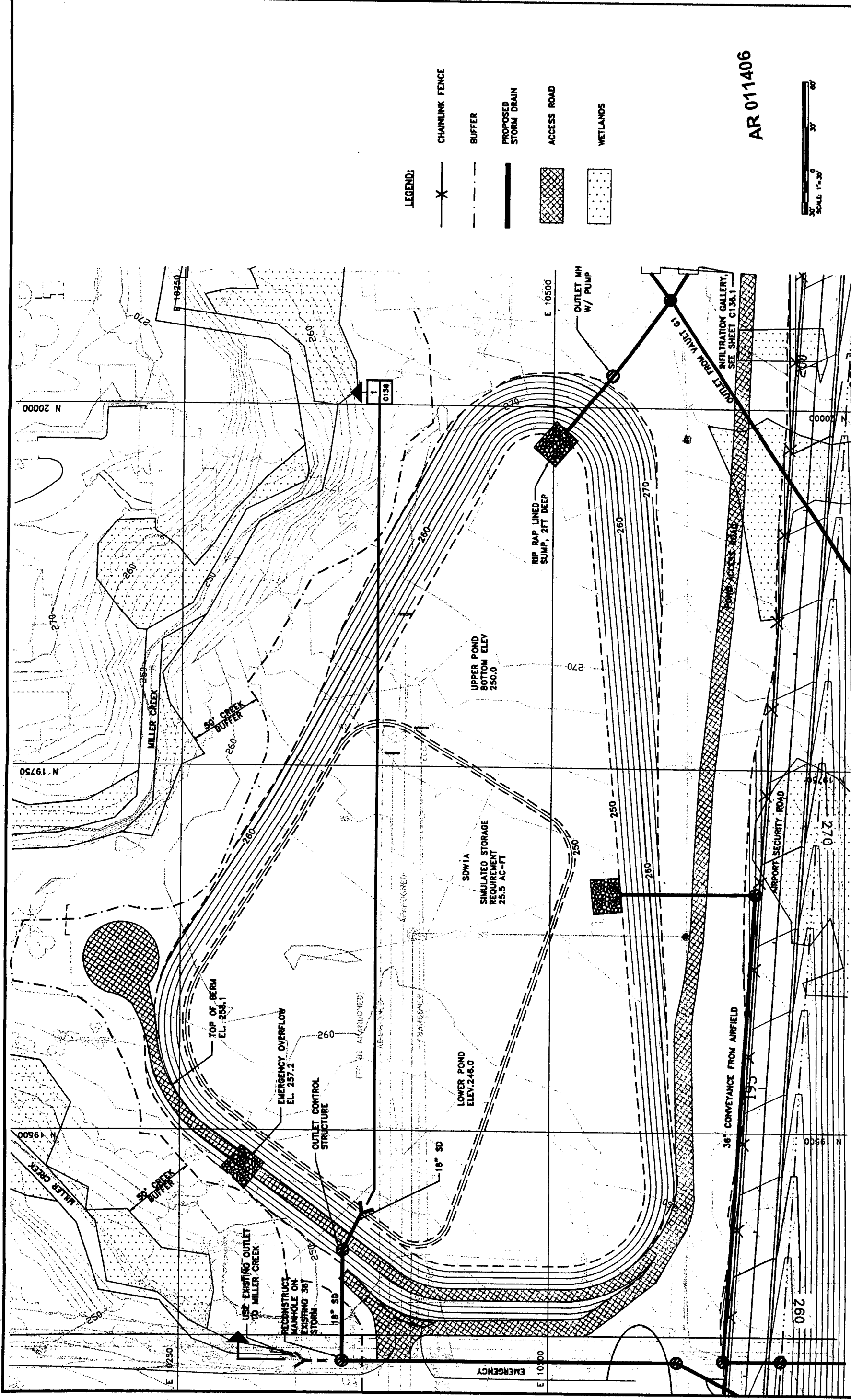
SHEET TITLE: POND F PROFILE  
 SDW2 BASIN POND

DATE: JULY 30, 2001

DESIGNER: [unreadable]

DATE OF REVISION: [unreadable]

EXHIBIT: C132



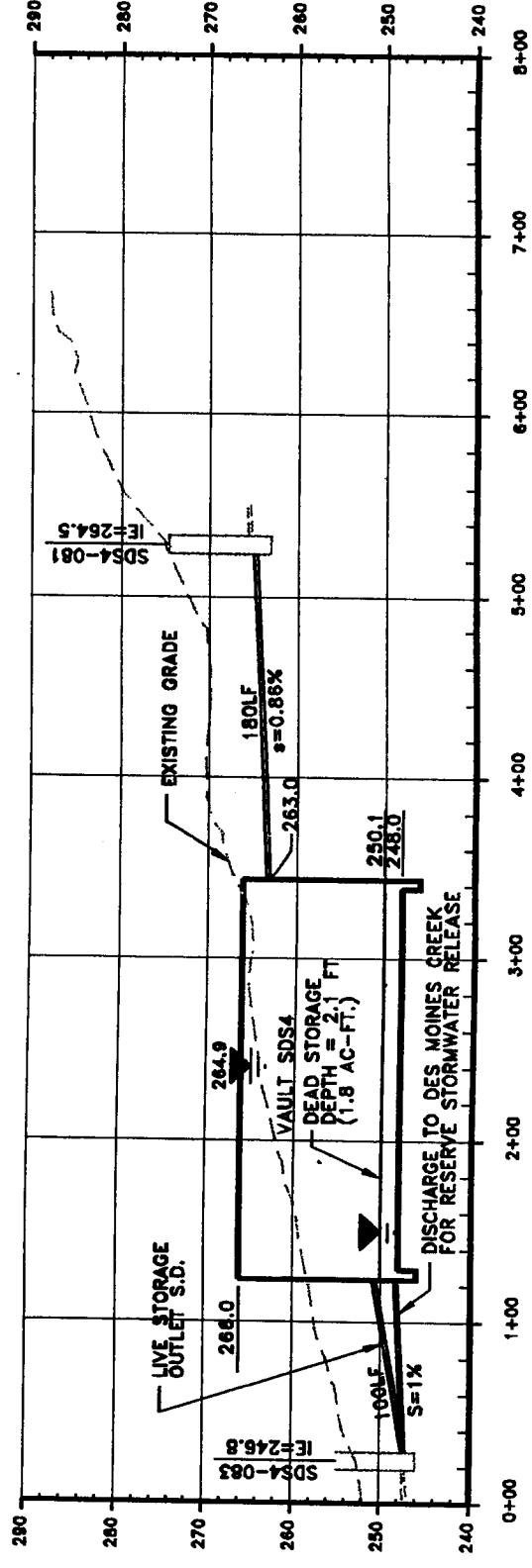
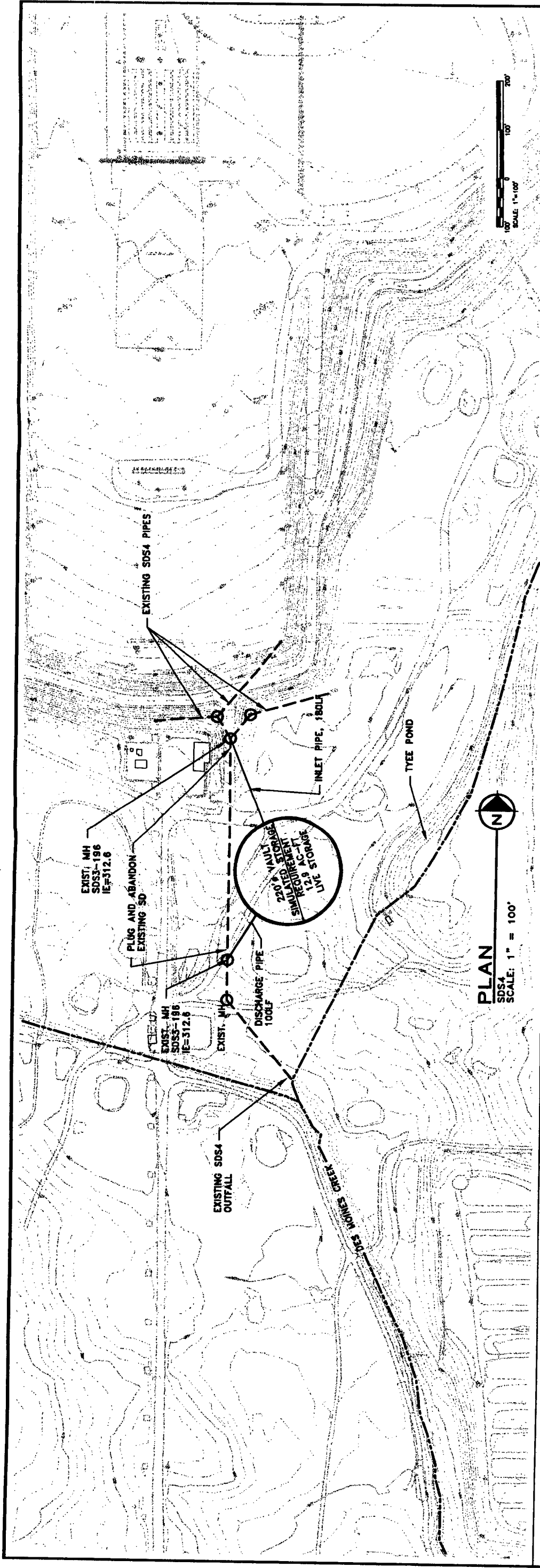
**PLAN**  
 POND G PLAN  
 SDW1A BASIN POND  
 SCALE: 1" = 30'

**AR 011406**


**Port of Seattle**  
 SEA-TAC INTERNATIONAL AIRPORT  
 THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6  
 SHEET TITLE: **POND G PLAN**  
 SHEET NO.: **SDW1A BASIN POND**

**DATE:** JULY 30, 2001  
**DESIGNED BY:**  
**CHECKED BY:**  
**APP'D BY:**

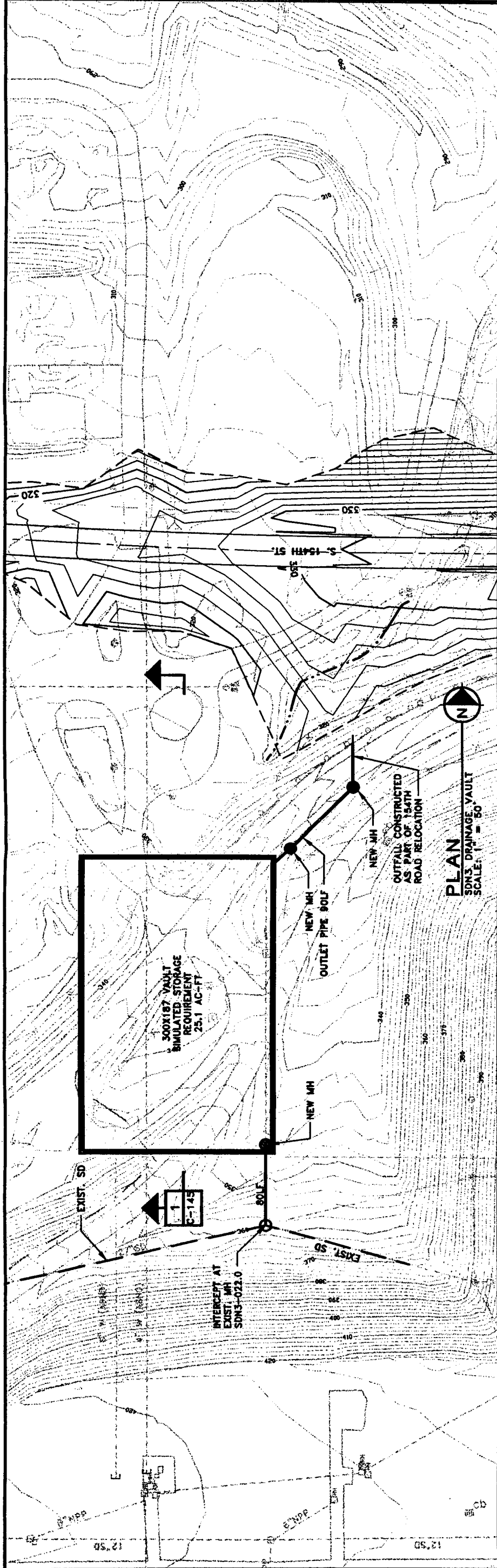




AR 011407


**Port of Seattle**  
 SEA-TAC INTERNATIONAL AIRPORT  
 PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE B  
 SHEET TITLE: SDS4 BASIN VAULT  
 PLAN AND PROFILE  
 DATE: July 2001  
 556-2912-001 (28)  
 CONSULTOR'S SEAL  
 PART OF EXHIBIT NO.  
 EXHIBIT: C139

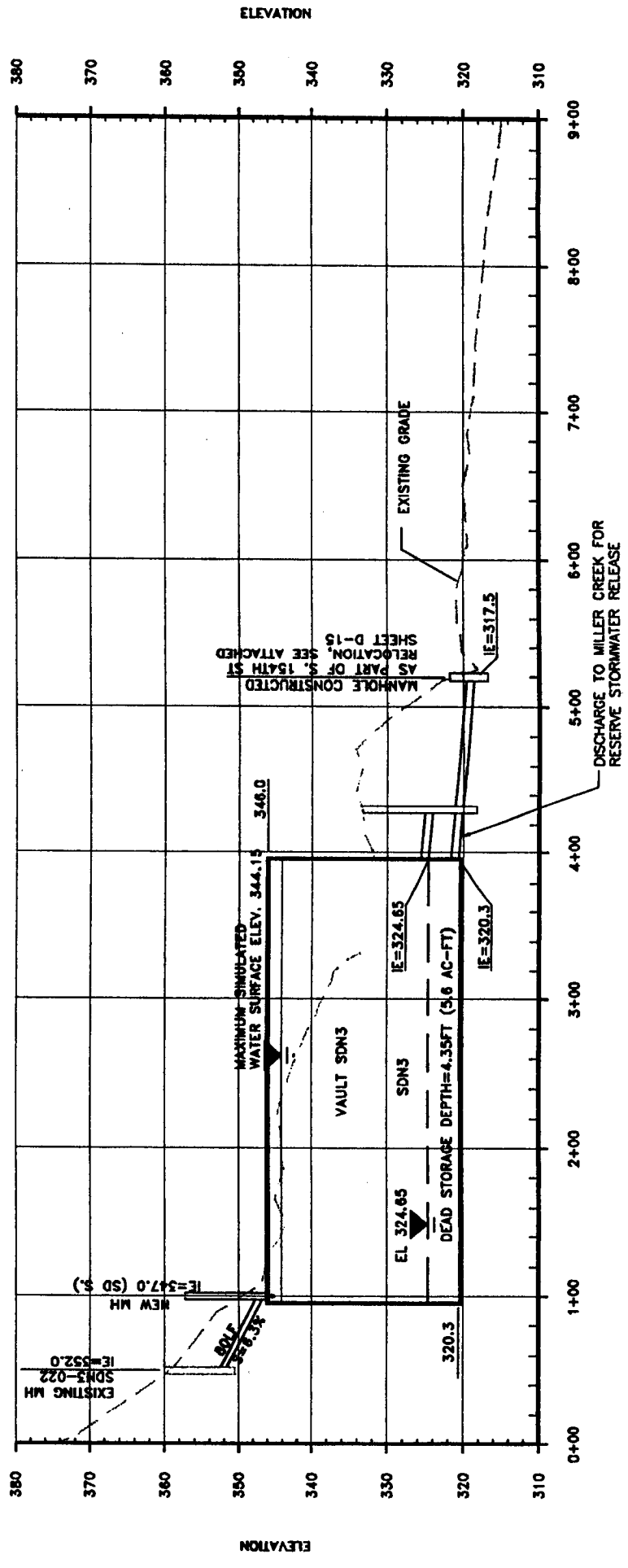




SCALE: 1" = 50'

**PLAN**  
SDN3 DRAINAGE VAULT  
SCALE: 1" = 50'

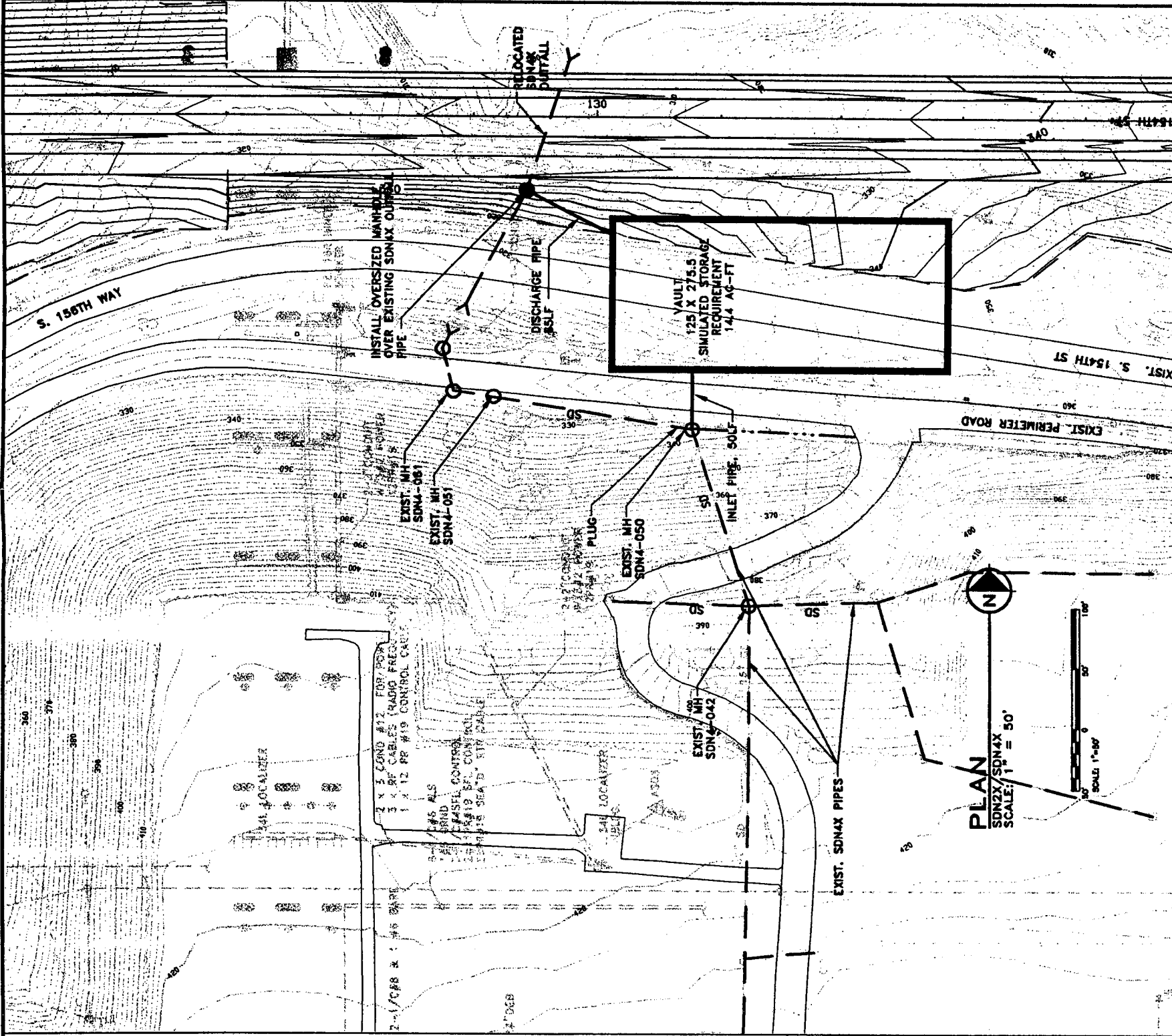
**NOTE:**  
THE STORM SEWER WILL CONNECT TO AN INTERNAL PIPING AND DIVIDING WALL SYSTEM TO PROMOTE CIRCULATION AND FLUSHING OF THE RESERVE STORMWATER RELEASE WATERS WITHIN THE DEAD STORAGE AREA.



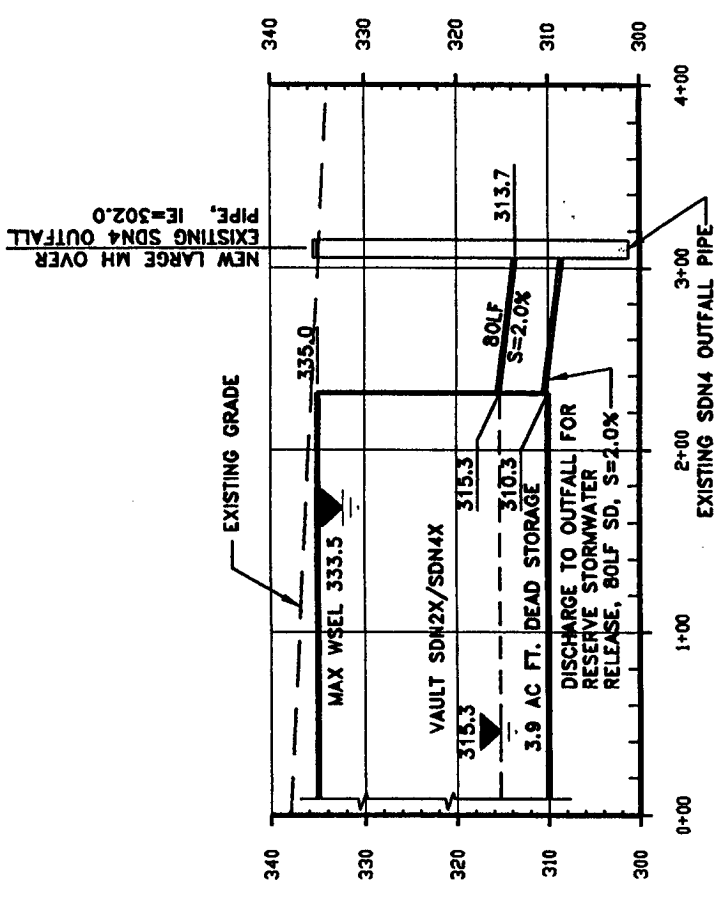
**SECTION A**  
SDN3 DRAINAGE VAULT  
SCALE: HORIZ 1" = 50'  
VERT 1" = 10'

Part of Seattle  
SEA-TAC INTERNATIONAL AIRPORT  
PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE B  
SHEET TITLE: SDN3 BASIN VAULT  
PLAN AND PROFILE  
DATE: JULY 30, 2001  
DRAWN BY: [blank]  
CHECKED BY: [blank]  
EXHIBIT: C145

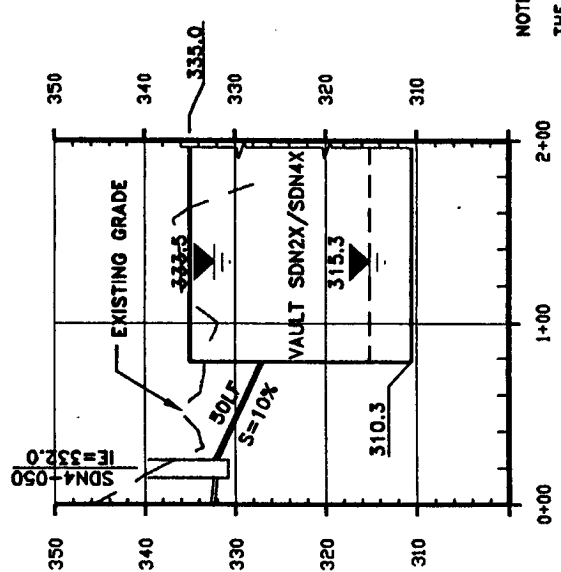
AR 011409



Part of Seattle  
 SEA-TAC INTERNATIONAL AIRPORT  
 PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6  
 SHEET TITLE: SDN2X/SDN4X BASIN VAULT  
 PLAN AND PROFILE  
 DATE: JULY 30, 2001  
 DRAWN BY: [blank]  
 CHECKED BY: [blank]  
 EXHIBIT: C146

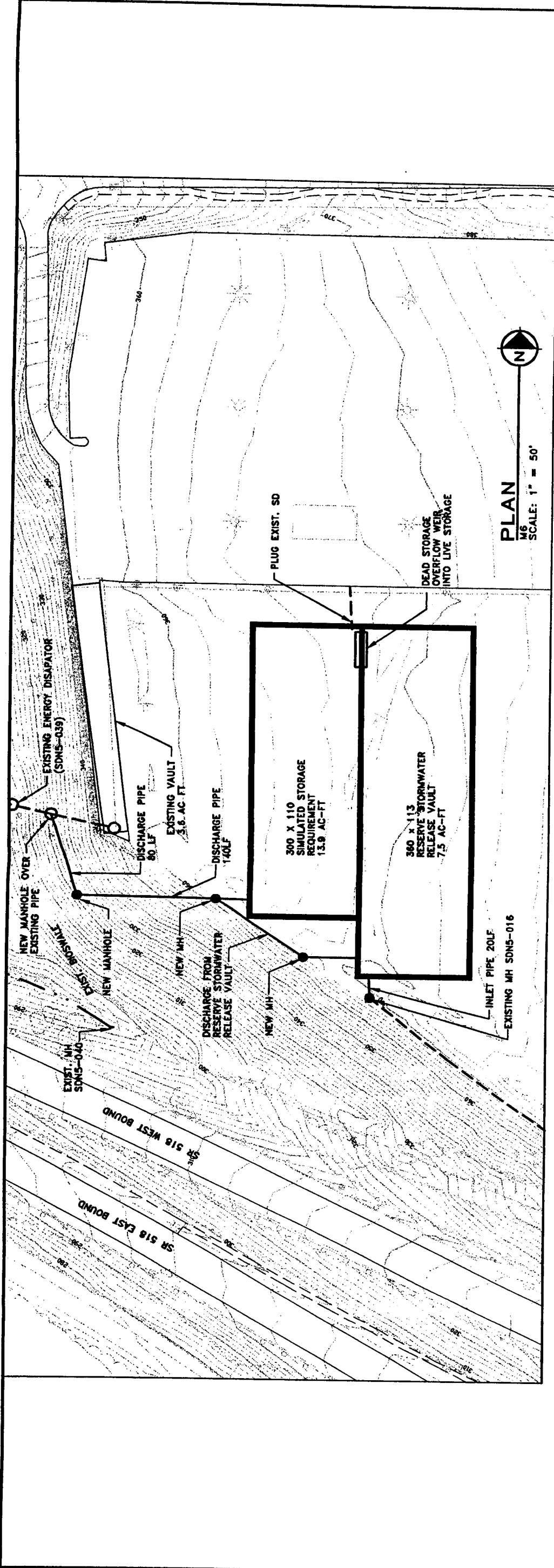


**PROFILE**  
 SDN2X/SDN4X  
 SCALE: HORIZ 1" = 50'  
 VERT 1" = 10'

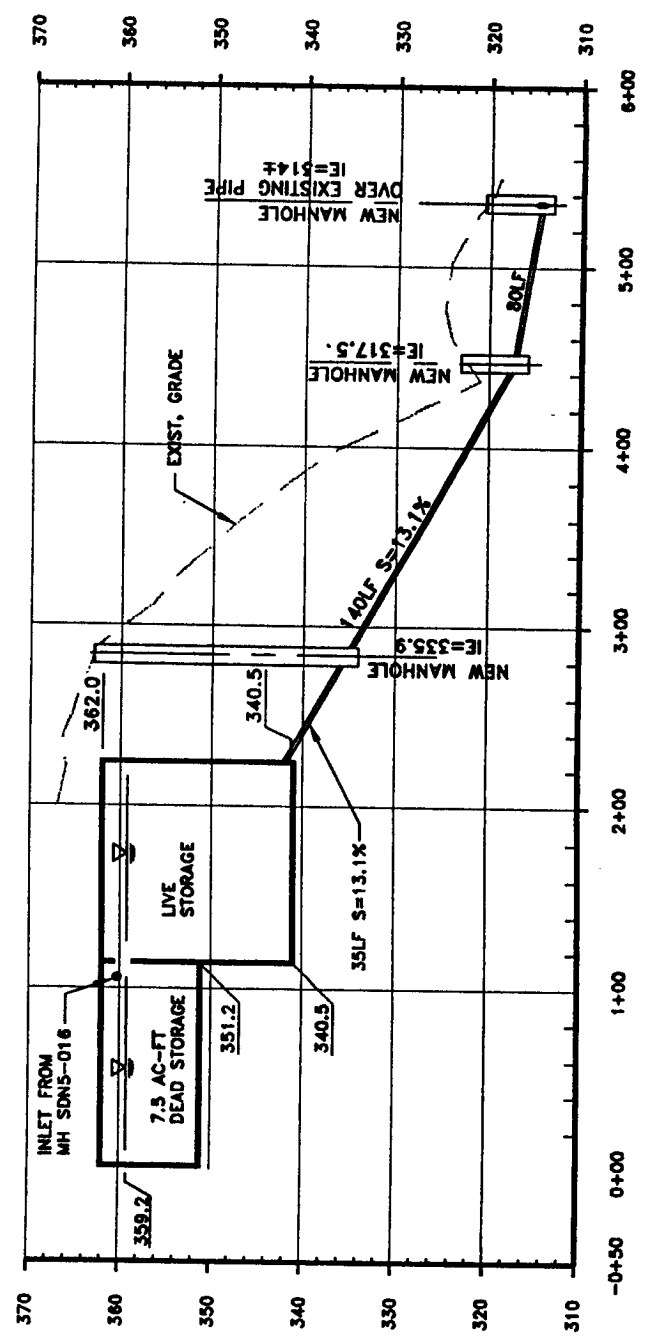


**PROFILE**  
 SDN2X/SDN4X  
 SCALE: HORIZ 1" = 50'  
 VERT 1" = 10'

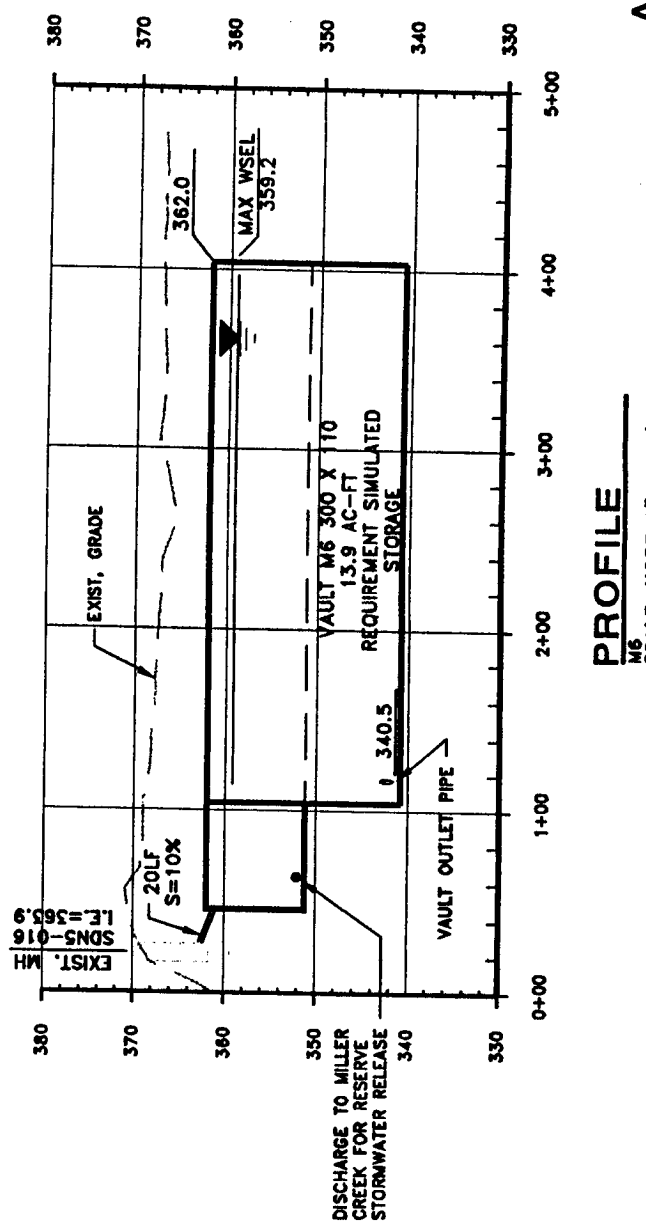
NOTE:  
 THE STORM SEWER WILL CONNECT TO AN INTERNAL PIPING AND DIVIDING WALL SYSTEM TO PROMOTE CIRCULATION AND FLUSHING OF THE RESERVE STORMWATER RELEASE WATERS WITHIN THE DEAD STORAGE AREA.



**PLAN**  
M6  
SCALE: 1" = 50'




**PROFILE**  
M6  
SCALE: HORIZ 1" = 50'  
VERT 1" = 10'



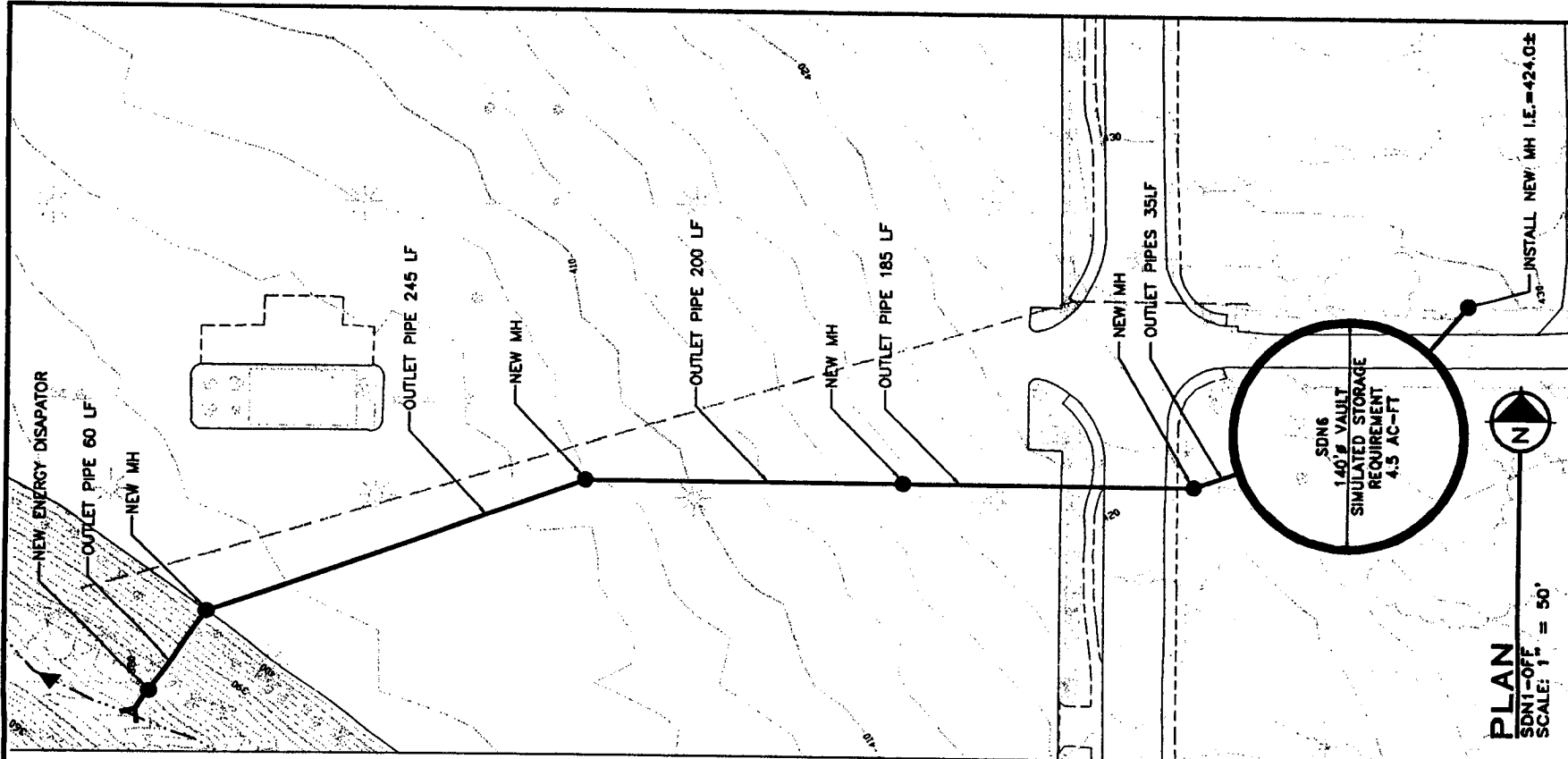
**PROFILE**  
M6  
SCALE: HORIZ 1" = 50'  
VERT 1" = 10'

NOTE:  
THE STORM SEWER WILL CONNECT TO AN INTERNAL PIPING AND DIVIDING WALL SYSTEM TO PROMOTE CIRCULATION AND FLUSHING OF THE RESERVE STORMWATER RELEASE WATERS WITHIN THE DEAD STORAGE AREA.

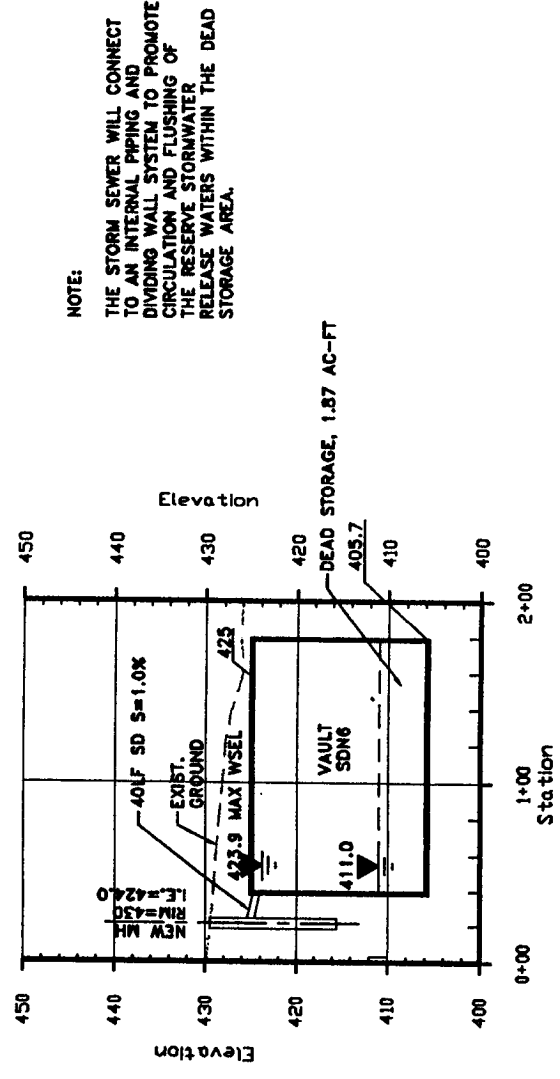
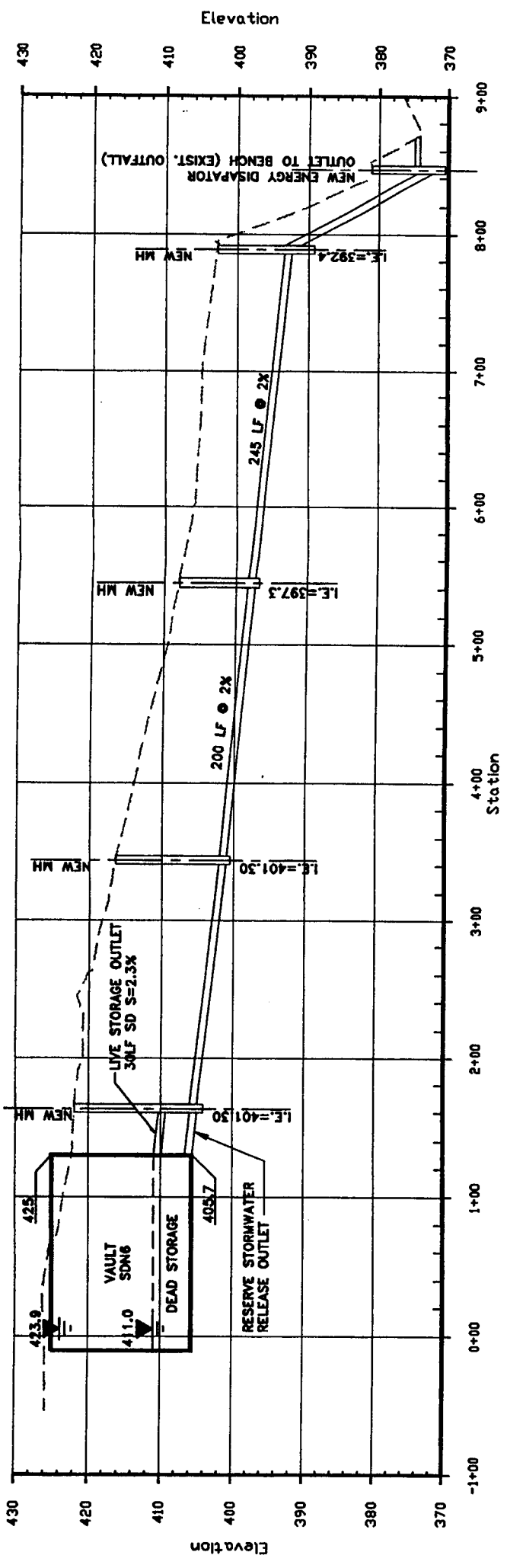

**Port of Seattle**  
 SEA-TAC INTERNATIONAL AIRPORT  
 PROJECT: THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE B  
 SHEET TAG: M6 BASIN VAULT (NEPL)  
 PLAN AND PROFILE  
 DATE: JULY 30, 2001  
 CHECKED BY: [Blank]  
 PREP BY: [Blank]  
 EXHIBIT: C147

AR 011411





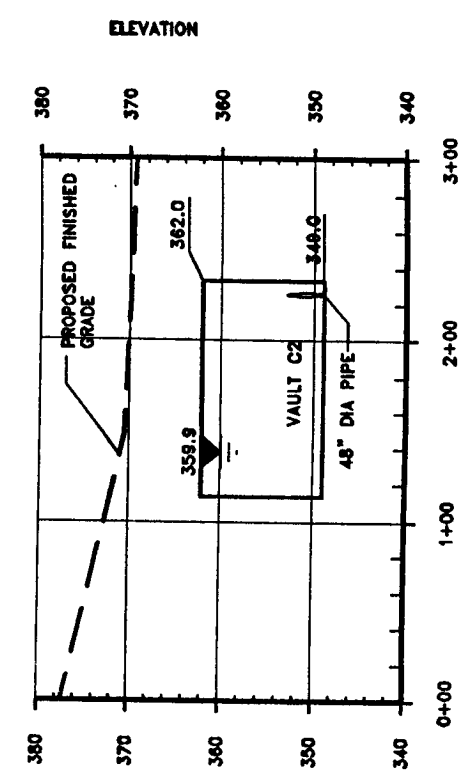
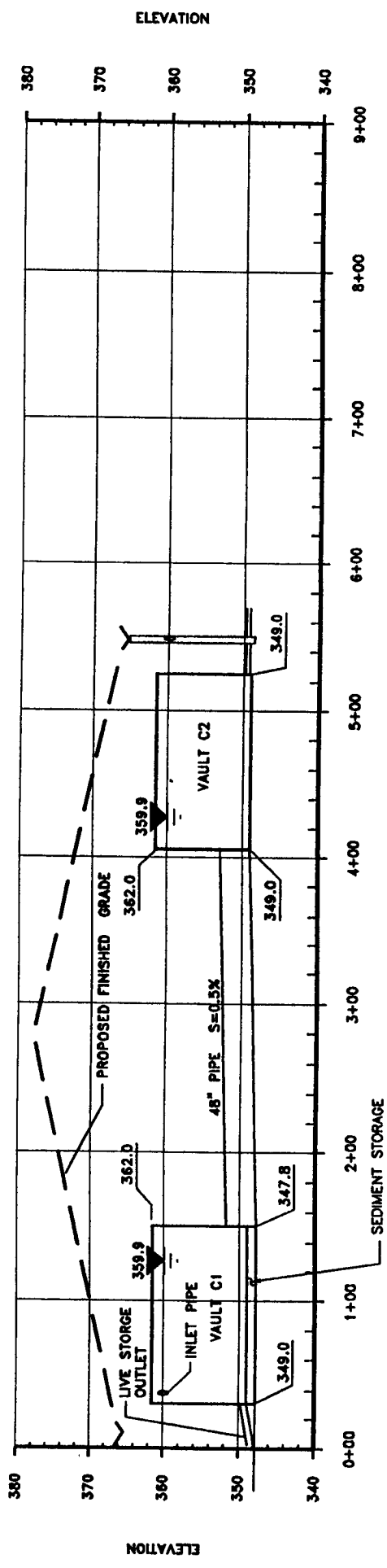
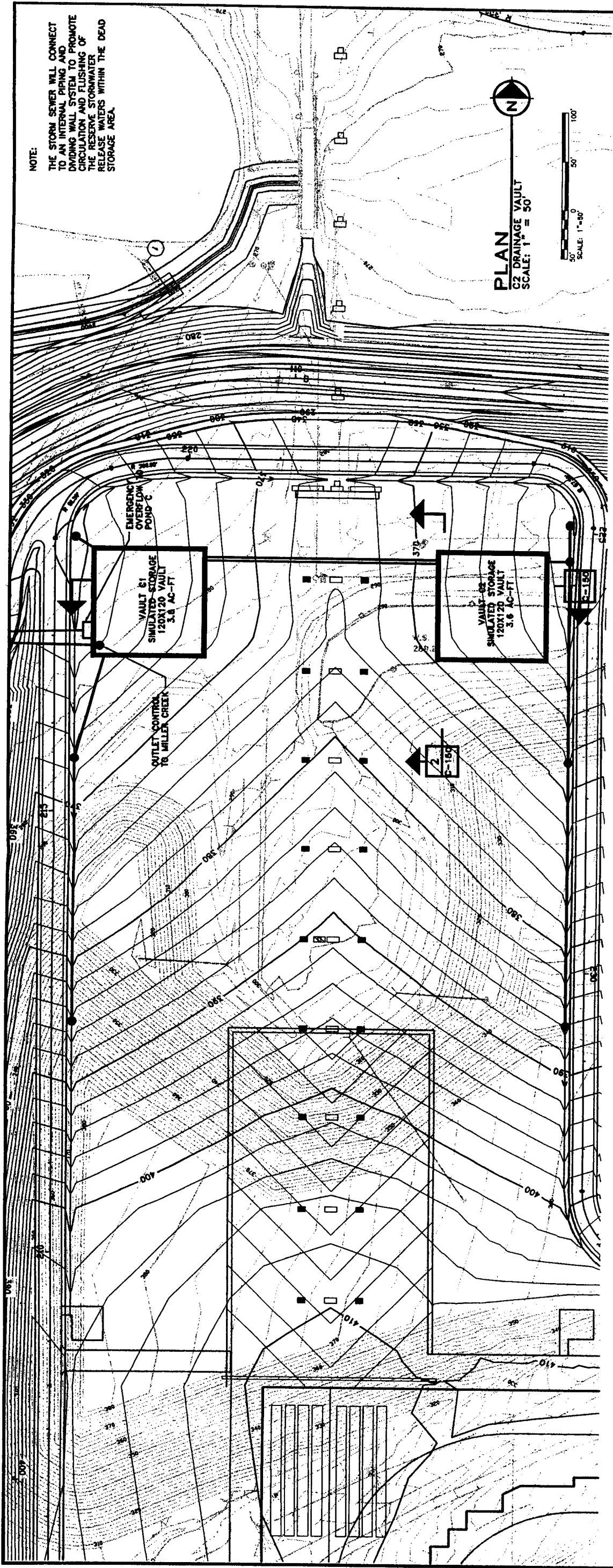
DATE: JULY 30, 2001  
 CHECKED BY: [ ]  
 PROJECT: SEA-TAC INTERNATIONAL AIRPORT  
 THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE B  
 SHEET TITLE: SDN6 (CARGO) BASIN VAULT  
 PLAN AND PROFILE  
 SHEET NO.: [ ]  
 EXHIBIT: C149



AR 011412

NOTE:

THE STORM SEWER WILL CONNECT TO AN INTERNAL PIPING AND DIVIDING WALL SYSTEM TO PROMOTE CIRCULATION AND FLUSHING OF THE RESERVE STORMWATER RELEASE WATERS WITHIN THE DEAD STORAGE AREA.

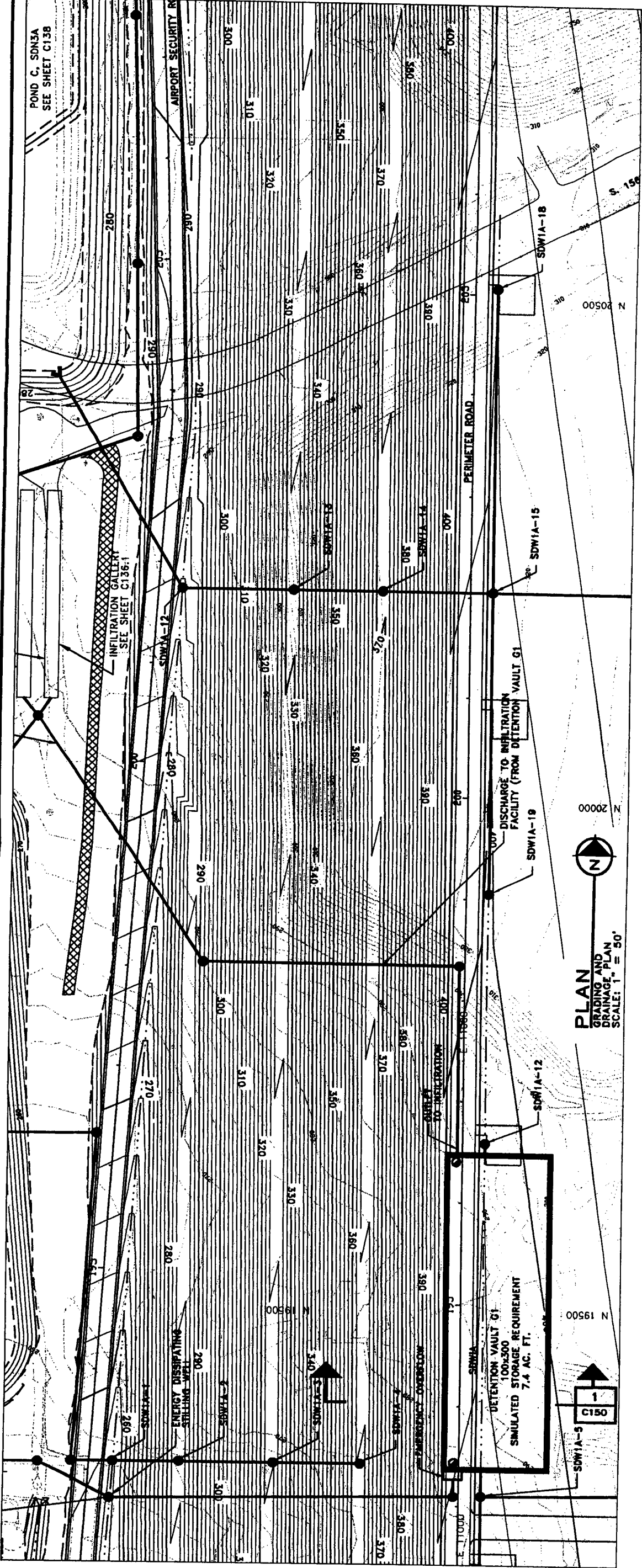


**SECTION A**  
C2 DRAINAGE VAULT  
SCALE: HORIZ 1" = 50'  
VERT 1" = 10'

**SECTION B**  
C2 DRAINAGE VAULT  
SCALE: HORIZ 1" = 50'  
VERT 1" = 10'

AR 011413

	DATE	JULY 30, 2001
	CONTRACT NO.	
PROJECT	THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 5	
SHEET TITLE	SDN3A BASIN VAULT C1/C2	
	EXHIBIT	C150



AR 011414

**LEGEND:**

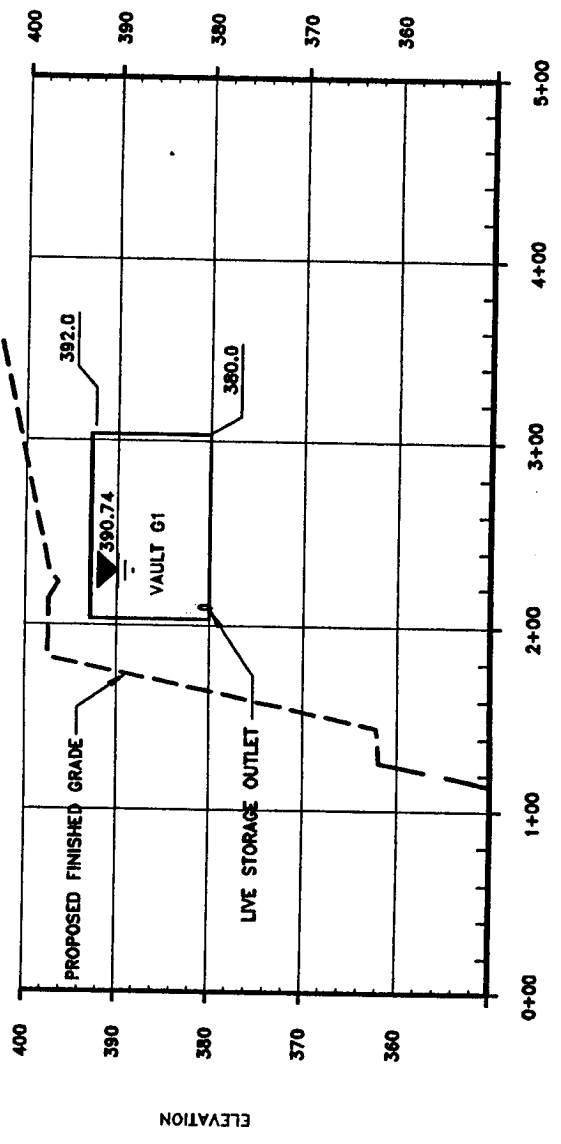
- STORM DRAIN PIPE
- WETLANDS

**NOTES:**



**KEY PLAN**

108	109	110	111	112	113	114	115	116	117	118	119	120	121	122	123	124	125	126	127	128	129	130
-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----



**Part of Seattle**  
**SEA-TAC INTERNATIONAL AIRPORT**

PROJECT: **THIRD RUNWAY - EMBANKMENT CONSTRUCTION PHASE 6**

SHEET TITLE: **SDW1A BASIN VAULT G1  
PLAN AND SECTION**

DATE: **JULY 30, 2001**

EXHIBIT: **C151**



RUN

GLOBAL

\*\*\* SEATAC AIRPORT HSPF BASIN MODEL OF MILLER CREEK  
\*\*\* FILE: MLOWFLOW2.INP - 2006 future condition  
\*\*\* LOW FLOW ANALYSIS  
\*\*\* BASED ON MILL65.INP FILE FROM AQUA TERRA  
\*\*\* REMOVED FILL AREAS  
\*\*\* PERLND 80 is the groundwater PERLND for the Fill area.  
\*\*\* FOLLOWING STATEMENTS MAY NOT APPLY  
\*\*\* ADDED PERLND 47,57,  
\*\*\* ADDED GROUND WATER INFILTRATION TO WDM FOR USE WITH MCAGWO.INP  
\*\*\* FK revised SDW1A and SDW1B with flow splitters, storages at SDN3/3X,  
SDN2X/4X;  
\*\*\* FK revised MC-1 and SDN-2X land uses, added POC at Lake Reba, removed  
run-of-river tables

\*\*\* FOUR YEAR RUN USING LONG TERM 1990 INITIAL CONDITIONS

MILLER CREEK BASIN HSPF MODEL

\*\*\* START 1994 1 1 0 0 END 1996 8 30 24 0  
START 1990 10 1 0 0 END 1994 9 30 24 0  
RUN INTERP OUTPUT LEVEL 3  
RESUME 0 RUN 1

END GLOBAL

FILES

<type> <fun>\*\*\*<-----fname----->  
MESSU 24 D:\PARA\SEATAC\MILLER\LOWFLOW\MLOWFLOW3.MES  
WDM 25 D:\PARA\SEATAC\MILLER\LOWFLOW\MLOWFLOW.WDM  
61 D:\PARA\SEATAC\MILLER\LOWFLOW\PER.L61  
62 D:\PARA\SEATAC\MILLER\LOWFLOW\RCH.L62

END FILES

OPN SEQUENCE

INGRP INDELT 01:00

PERLND 16  
PERLND 26  
PERLND 34  
PERLND 44  
PERLND 45  
\*\*\* special PERLND for infiltration SDW1A  
PERLND 47  
PERLND 54  
\*\*\* special PERLND for infiltration SDW1B  
PERLND 57  
\*\*\* PERLND FOR INFLOW OF LOW FLOW FROM FILL PGG  
PERLND 80  
IMPLND 14  
RCHRES 1  
RCHRES 23  
RCHRES 24  
RCHRES 2  
RCHRES 3  
RCHRES 33  
RCHRES 4  
RCHRES 5  
RCHRES 50  
RCHRES 242  
RCHRES 240

COPY 61  
 COPY 44  
 RCHRES 51  
 RCHRES 43  
 RCHRES 451  
 RCHRES 452  
 COPY 45  
 COPY 645  
 RCHRES 46  
 RCHRES 552  
 RCHRES 52  
 RCHRES 53  
 COPY 53  
 RCHRES 54  
 RCHRES 37  
 RCHRES 237  
 COPY 37  
 RCHRES 147  
 RCHRES 247  
 COPY 66  
 COPY 69  
 RCHRES 47  
 COPY 62  
 COPY 63  
 COPY 67  
 COPY 68

\*\*\* output special PERLND outflow to check

COPY 47  
 COPY 70  
 RCHRES 34  
 RCHRES 135  
 RCHRES 570  
 RCHRES 57  
 RCHRES 257  
 COPY 64  
 COPY 65  
 COPY 357  
 COPY 56

\*\*\* output special PERLND outflow to check

COPY 57  
 COPY 71  
 RCHRES 35  
 COPY 55  
 RCHRES 10  
 RCHRES 16  
 RCHRES 11  
 RCHRES 13  
 RCHRES 12  
 RCHRES 15  
 RCHRES 14  
 RCHRES 17

END INGRP

END OPN SEQUENCE

\*\*\*

PERLND

GEN-INFO

<PLS >

Name

NBLKS

Unit-systems

Printer

\*\*\*

AR 011416

```

# - # User t-series Engl Metr
in out
16 TFM- TILL FOR MOD 1 1 1 1 61 0
26 TGM- TILL GR MOD 1 1 1 1 61 0
34 OF - OUTWASH FOR 1 1 1 1 61 0
44 OG - OUTWASH GR 1 1 1 1 61 0
***PERLND FOR NEW AIRPORT FILL; NONE IN CALIBRATION
45 AIRPORT FILL 1 1 1 1 61 0
47 OG - INFILTRATION 1 1 1 1 61 0
54 SA - WETLANDS 1 1 1 1 61 0
57 OG - INFILTRATION 3 1 1 1 1 61 0
80 LOW FLOW 1 1 1 1 61 0
END GEN-INFO
ACTIVITY
<PLS > ***** Active Sections *****
# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC
14 200 0 0 1 0 0 0 0 0 0 0 0 0 0
END ACTIVITY
PRINT-INFO
<PLS > ***** Print-flags ***** PIVL PYR
# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC
14 200 0 0 5 0 0 0 0 0 0 0 0 0 1 9
END PRINT-INFO
PWAT-PARM1
<PLS > ***** Flags *****
# - # CSNO RTOP UZFG VCS VUZ VNN VIFW VIRC VLE
14 200 0 0 0 0 0 0 0 0 0
END PWAT-PARM1
PWAT-PARM2
<PLS > ***
# - # ***FOREST LZSN INFILT LSUR SLSUR KVARY AGWRC
16 9.0000 0.3200 400.00 0.1000 0.5000 0.9960
26 9.0000 0.1200 400.00 0.1000 0.5000 0.9960
34 10.0000 2.0000 400.00 0.0500 0.3000 0.9960
44 10.0000 0.8000 400.00 0.0500 0.3000 0.9960
45 7.5000 0.0200 300.00 0.0700 0.0000 0.9960
47 10.0000 0.8000 400.00 0.0500 0.3000 0.9960
54 8.0000 2.0000 100.00 0.0010 0.5000 0.9960
57 10.0000 0.8000 400.00 0.0500 0.3000 0.9960
80 9.0000 0.1200 400.00 0.1000 0.5000 0.9960
END PWAT-PARM2
PWAT-PARM3
<PLS >***
# - #*** PETMAX PETMIN INFEXP INFILD DEEPPR BASETP AGWETP
16 2.0000 2.0000 0.33 0.00 0.0
26 2.0000 2.0000 0.33 0. 0.
34 2.0000 2.0000 0.33 0.00 0.0
44 2.0000 2.0000 0.33 0. 0.
47 2.0000 2.0000 0.33 0. 0.
45 2.0000 2.0000 0.33 0. 0.
54 10.000 2.0000 0.33 0. 0.7
57 2.0000 2.0000 0.33 0. 0.
80 2.0000 2.0000 0.33 0. 0.
END PWAT-PARM3
PWAT-PARM4
<PLS >
# - # CEpsc UZSN NSUR INTFW IRC LZETP***

```

16	0.2000	0.7500	0.3500	9.000	0.7000	0.7000
26	0.1000	0.3750	0.2500	9.000	0.7000	0.2500
34	0.2000	0.7500	0.3500	0.000	0.7000	0.7000
44	0.1000	0.7500	0.2500	0.000	0.7000	0.2500
47	0.1000	0.7500	0.2500	0.000	0.7000	0.2500
45	0.1000	0.2800	0.2500	6.000	0.1500	0.6000
54	0.1000	2.2500	0.5000	1.000	0.7000	0.8000
57	0.1000	0.7500	0.2500	0.000	0.7000	0.2500
80	0.1000	0.3750	0.2500	9.000	0.7000	0.2500

END PWAT-PARM4

PWAT-STATE1

<PLS > PWATER state variables\*\*\*

#	CEPS	SURS	UZS	IFWS	LZS	AGWS	GWVS
16	0.000	0.	0.0010	0.00	0.941	3.108	0.048
26	0.000	0.	0.0010	0.00	7.672	3.341	0.071
34	0.000	0.	0.0010	0.00	1.187	3.776	0.052
44	0.000	0.	0.0040	0.00	9.402	4.905	0.104
45	0.000	0.	0.0000	0.00	2.000	2.000	0.000
54	0.000	0.	0.0960	0.00	3.211	0.000	0.000
47	0.000	0.	0.0000	0.00	2.000	2.000	0.000
57	0.000	0.	0.0000	0.00	2.000	2.000	0.000
80	0.000	0.	0.0000	0.00	7.672	3.341	0.071

END PWAT-STATE1

END PERLND

IMPLND

GEN-INFO

<ILS >	Name	Unit-systems	Printer	***
# - #		User t-series	Engl Metr	***
		in	out	***
14	IMPERVIOUS	1	1 1	60 0

END GEN-INFO

ACTIVITY

<ILS >	*****	Active	Sections	****			
# - #	ATMP	SNOW	IWAT	SLD	IWG	IQAL	***
14	0	0	1	0	0	0	

END ACTIVITY

PRINT-INFO

<ILS >	*****	Print-flags	*****	PIVL	PYR		
# - #	ATMP	SNOW	IWAT	SLD	IWG	IQAL	*****
14	0	0	6	0	0	0	1 9

END PRINT-INFO

IWAT-PARM1

<ILS >	Flags	***				
# - #	CSNO	RTOP	VRS	VNN	RTLI	***
14	0	0	0	0	0	

END IWAT-PARM1

IWAT-PARM2

<ILS >	***				
# - #	LSUR	SLSUR	NSUR	RETSC	***
14	100.00	0.0100	0.1000	0.1000	

END IWAT-PARM2

IWAT-PARM3

<ILS >	***		
# - #	PETMAX	PETMIN	***
14			

```

END IWAT-PARM3
IWAT-STATE1
  <ILS > IWATER state variables
  # - #      RETS      SURS
  14      1.0000E-3  1.0000E-3
END IWAT-STATE1
END IMPLND

EXT SOURCES

*** NOTE: The only RCHRES that precip and PET are applied to are lakes and ponds
*** FOLLOWING RCHRES ARE PONDS: 57, 247, 237

<-Volume-> <Member> SsysSgap<--Mult-->Tran <-Target vols> <-Grp> <-Member->
<Name> # <Name> # tem strg<--factor-->strg <Name> # # <Name> # #
*** PRECIP/EVAP TO PERVIOUS/IMPERV SURFACES
WDM 1002 PREC ENGLZERO 1.00 PERLND 14 65 EXTNL PREC
WDM 1002 PREC ENGLZERO 0.0 PERLND 80 EXTNL PREC
WDM 1002 PREC ENGLZERO 1.00 IMPLND 14 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 PERLND 14 65 EXTNL PETINP
WDM 1 EVAP ENGLZERO 0.0 PERLND 80 EXTNL PETINP
WDM 1 EVAP ENGLZERO 0.8 IMPLND 14 EXTNL PETINP
*** --> lateral inflow from reinfiltration chamber for SDW1A
*** WDM 5 FLOW ENGLZERO 1.0 PERLND 47 EXTNL AGWLI
*** --> lateral inflow from reinfiltration chamber for SDW1B
*** WDM 6 FLOW ENGLZERO 1.0 PERLND 57 EXTNL AGWLI
*** PRECIP/EVAP TO LAKES
WDM 1002 PREC ENGLZERO RCHRES 1 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 1 EXTNL POTEV
WDM 1002 PREC ENGLZERO RCHRES 4 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 4 EXTNL POTEV
WDM 1002 PREC ENGLZERO RCHRES 11 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 11 EXTNL POTEV
WDM 1002 PREC ENGLZERO RCHRES 13 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 13 EXTNL POTEV
WDM 1002 PREC ENGLZERO RCHRES 23 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 23 EXTNL POTEV
WDM 1002 PREC ENGLZERO RCHRES 34 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 34 EXTNL POTEV
WDM 1002 PREC ENGLZERO RCHRES 53 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 53 EXTNL POTEV
WDM 1002 PREC ENGLZERO RCHRES 54 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 54 EXTNL POTEV
WDM 1002 PREC ENGLZERO RCHRES 237 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 237 EXTNL POTEV
WDM 1002 PREC ENGLZERO RCHRES 247 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 247 EXTNL POTEV
WDM 1002 PREC ENGLZERO RCHRES 57 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 57 EXTNL POTEV
*** till seepage groundwater flow from Fill area. PGG time series
WDM 7001 FLOW ENGLZERO.000000099 PERLND 80 EXTNL AGWLI
*** Fill flow directly to stream
WDM 7000 FLOW ENGL .000000957 RCHRES 35 INFLOW IVOL

END EXT SOURCES

EXT TARGETS

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```

<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Volume-> <Member> Tsys Tgap Amd ***
<Name> # <Name> # #<-factor->strg <Name> # <Name> tem strg strg***
***PROJECT CONDITION FLOWS
*** RCHRES=LOCATION:
*** 54=MCDF 47=SDW1A INFILTRATION TANK 43=SDN3X 247=SDW1A POND G
*** 17=MOUTH 49=SDW2 44=SDN4X 52=SDN1 451= EXISTING NEPL
*** 61=SDN2X 57=SDW1B 51=SDN2X+SDN4X 53=Lake Reba 452=NEW NEPL
*** 45=NEPL POC 55=SR509 39=SDN3A/SDW1A POC
*** 46=CARGO 37=SDN3AI VAULT 237=SDN3AO POND
*** GAUGE POINTS (17=MOUTH, 54=MILLER RDF, 55=SR509)
RCHRES 17 HYDR RO 1 1 WDM 7017 FLOW ENGL REPL
RCHRES 35 HYDR RO 1 1 WDM 7036 FLOW ENGL REPL
***COPY 55 OUTPUT MEAN 1 1 12.1 WDM 118 FLOW ENGL REPL
RCHRES 54 HYDR RO 1 1 WDM 7054 FLOW ENGL REPL
*** DETENTION POND FLOWS
***COPY 61 OUTPUT MEAN 1 1 12.1 WDM 101 FLOW ENGL REPL
***RCHRES 552 HYDR RO 1 1 WDM 102 FLOW ENGL REPL
***RCHRES 451 HYDR RO 1 1 WDM 105 FLOW ENGL REPL
***RCHRES 452 HYDR RO 1 1 WDM 119 FLOW ENGL REPL
***RCHRES 46 HYDR RO 1 1 WDM 106 FLOW ENGL REPL
*** write RCHRES 47 (Inf. Area # 1)outlet 1 and 2 to WDM 107 and 108 like so:
***COPY 62 OUTPUT MEAN 1 1 12.1 WDM 107 FLOW ENGL REPL
***COPY 63 OUTPUT MEAN 1 1 12.1 WDM 108 FLOW ENGL REPL
***COPY 66 OUTPUT MEAN 1 1 12.1 WDM 112 FLOW ENGL REPL
***COPY 69 OUTPUT MEAN 1 1 12.1 WDM 1120 FLOW ENGL REPL
*** write SDW1a vault flows to WDM:
***COPY 67 OUTPUT MEAN 1 1 12.1 WDM 109 FLOW ENGL REPL
***COPY 68 OUTPUT MEAN 1 1 12.1 WDM 1090 FLOW ENGL REPL
*** write RCHRES 570 outlet 1 and 2 to WDM 110 and 115 like so:
***RCHRES 570 HYDR RO 1 1 WDM 210 FLOW ENGL REPL
***COPY 64 OUTPUT MEAN 1 1 12.1 WDM 110 FLOW ENGL REPL
***COPY 65 OUTPUT MEAN 1 1 12.1 WDM 115 FLOW ENGL REPL
***COPY 357 OUTPUT MEAN 1 1 12.1 WDM 211 FLOW ENGL REPL
***COPY 56 OUTPUT MEAN 1 1 12.1 WDM 121 FLOW ENGL REPL
*** write RCHRES 37 vault to WDM 111
***RCHRES 37 HYDR RO 1 1 WDM 111 FLOW ENGL REPL
***RCHRES 237 HYDR RO 1 1 WDM 122 FLOW ENGL REPL
***RCHRES 43 HYDR RO 1 1 WDM 103 FLOW ENGL REPL
***COPY 44 OUTPUT MEAN 1 1 12.1 WDM 104 FLOW ENGL REPL
***RCHRES 51 HYDR RO 1 1 WDM 139 FLOW ENGL REPL
*** DETENTION STAGES
***RCHRES 47 HYDR STAGE WDM 652 STAG ENGL REPL
***RCHRES 147 HYDR STAGE WDM 657 STAG ENGL REPL
***RCHRES 247 HYDR STAGE WDM 654 STAG ENGL REPL
***RCHRES 552 HYDR STAGE WDM 601 STAG ENGL REPL
***RCHRES 57 HYDR STAGE WDM 651 STAG ENGL REPL
***RCHRES 257 HYDR STAGE WDM 655 STAG ENGL REPL
***RCHRES 237 HYDR STAGE WDM 656 STAG ENGL REPL
***RCHRES 37 HYDR STAGE WDM 650 STAG ENGL REPL
***RCHRES 54 HYDR STAGE WDM 61 STAG ENGL REPL
***RCHRES 451 HYDR STAGE WDM 662 STAG ENGL REPL
***RCHRES 452 HYDR STAGE WDM 667 STAG ENGL REPL
***RCHRES 46 HYDR STAGE WDM 663 STAG ENGL REPL
***RCHRES 43 HYDR STAGE WDM 664 STAG ENGL REPL
***RCHRES 44 HYDR STAGE WDM 665 STAG ENGL REPL
***RCHRES 51 HYDR STAGE WDM 666 STAG ENGL REPL
*** DETENTION VOLUMES

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***RCHRES 47 HYDR VOL WDM 752 VOL ENGL REPL
***RCHRES 147 HYDR VOL WDM 757 VOL ENGL REPL
***RCHRES 247 HYDR VOL WDM 754 VOL ENGL REPL
***RCHRES 552 HYDR VOL WDM 602 VOL ENGL REPL
***RCHRES 57 HYDR VOL WDM 751 VOL ENGL REPL
***RCHRES 257 HYDR VOL WDM 755 VOL ENGL REPL
***RCHRES 237 HYDR VOL WDM 756 VOL ENGL REPL
***RCHRES 37 HYDR VOL WDM 750 VOL ENGL REPL
***RCHRES 54 HYDR VOL WDM 62 VOL ENGL REPL
***RCHRES 451 HYDR VOL WDM 762 VOL ENGL REPL
***RCHRES 452 HYDR VOL WDM 767 VOL ENGL REPL
***RCHRES 46 HYDR VOL WDM 763 VOL ENGL REPL
***RCHRES 43 HYDR VOL WDM 764 VOL ENGL REPL
***RCHRES 44 HYDR VOL WDM 765 VOL ENGL REPL
***RCHRES 51 HYDR VOL WDM 766 VOL ENGL REPL

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\*\*\* POINT OF COMPLIANCE (POC) FLOWS

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***COPY 37 OUTPUT MEAN 1 1 12.1 WDM 125 FLOW ENGL REPL
***COPY 45 OUTPUT MEAN 1 1 12.1 WDM 199 FLOW ENGL REPL
***COPY 53 OUTPUT MEAN 1 1 12.1 WDM 399 FLOW ENGL REPL
***COPY 70 OUTPUT MEAN 1 1 12.1 WDM 7000 FLOW ENGL REPL
***COPY 71 OUTPUT MEAN 1 1 12.1 WDM 7001 FLOW ENGL REPL

```

\*\*\* SPECIAL PERLND REINFILTRATION RESULTS

```

*** --> output special PERLND parameters to check operations:
*** --> PERLND 47 active ground water storage depth (in)
*** PERLND 47 PWATER AGWS WDM 471 AGWS ENGL
REPL
*** --> PERLND 47 active ground water outflow (acft/2ac -> in/acre)
***COPY 47 OUTPUT MEAN 1 1 12 WDM 472 FLOW ENGL REPL
*** --> PERLND 57 active ground water storage depth (in)
***PERLND 57 PWATER AGWS WDM 571 AGWS ENGL REPL
*** --> PERLND 57 active ground water outflow (acft/2ac -> in/acre)
***COPY 57 OUTPUT MEAN 1 1 12 WDM 572 FLOW ENGL REPL

```

END EXT TARGETS

SCHEMATIC

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<-Source->          <--Area-->          <-Target->          MBLK   ***
<Name>   #          <-factor->          <Name>   #          Tbl#   ***
*** SUB-CATCHMENT 1 all agwo goes to sound
PERLND 16          3.41          RCHRES 1           6
PERLND 26          232.36         RCHRES 1           6
PERLND 34          3.07          RCHRES 1           6
PERLND 44          38.03         RCHRES 1           6
PERLND 54          3.87          RCHRES 1           6
IMPLND 14          56.14         RCHRES 1           2
*** SUB-CATCHMENT 2 10% of area GW goes to vaca 90% goes to sound
PERLND 16          5.56          RCHRES 2           6
PERLND 26          200.05        RCHRES 2           6
PERLND 34          0.46          RCHRES 2           6
PERLND 44          38.71         RCHRES 2           6
PERLND 16          0.56          RCHRES 135         7
PERLND 26          20.00        RCHRES 135         7
PERLND 34          0.05          RCHRES 135         7
PERLND 44          3.87          RCHRES 135         7
IMPLND 14          42.22         RCHRES 2           2
*** SUB-CATCHMENT 23 New subbasin 15 % OF GW GOES TO VACCA 85% TO SOUND
PERLND 16          3.09          RCHRES 23          6

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PERLND	26	156.15	RCHRES	23	6
PERLND	34	2.25	RCHRES	23	6
PERLND	44	45.84	RCHRES	23	6
PERLND	16	0.46	RCHRES	135	7
PERLND	26	23.42	RCHRES	135	7
PERLND	34	0.34	RCHRES	135	7
PERLND	44	6.88	RCHRES	135	7
IMPLND	14	58.44	RCHRES	23	2
*** SUB-CATCHMENT 24 New subbasin 60 % OF GW GOES TO 11 40% TO SOUND					
PERLND	26	135.43	RCHRES	24	6
PERLND	34	2.02	RCHRES	24	6
PERLND	44	69.29	RCHRES	24	6
PERLND	26	81.26	RCHRES	11	7
PERLND	34	1.21	RCHRES	11	7
PERLND	44	41.57	RCHRES	11	7
IMPLND	14	79.98	RCHRES	24	2
*** SUB-CATCHMENT 3 agwo goes to vaca(135)					
PERLND	16	8.26	RCHRES	3	6
PERLND	26	108.38	RCHRES	3	6
PERLND	34	16.02	RCHRES	3	6
PERLND	44	102.89	RCHRES	3	6
PERLND	54	0.04	RCHRES	3	6
PERLND	16	8.26	RCHRES	135	7
PERLND	26	108.38	RCHRES	135	7
PERLND	34	16.02	RCHRES	135	7
PERLND	44	102.89	RCHRES	135	7
PERLND	54	0.04	RCHRES	135	7
IMPLND	14	27.30	RCHRES	3	2
*** SUB-CATCHMENT 4 10% of agwo goes to rchres 90% goes to sound					
PERLND	16	2.95	RCHRES	4	6
PERLND	26	85.95	RCHRES	4	6
PERLND	34	3.75	RCHRES	4	6
PERLND	44	92.06	RCHRES	4	6
PERLND	16	0.30	RCHRES	4	7
PERLND	26	8.59	RCHRES	4	7
PERLND	34	0.38	RCHRES	4	7
PERLND	44	9.21	RCHRES	4	7
IMPLND	14	18.43	RCHRES	4	2
*** SUB-CATCHMENT 4a 70% of agwo goes to rchres 30% goes to sound					
PERLND	16	8.66	RCHRES	4	6
PERLND	26	61.64	RCHRES	4	6
PERLND	34	22.06	RCHRES	4	6
PERLND	44	78.09	RCHRES	4	6
PERLND	54	12.50	RCHRES	4	6
PERLND	16	6.06	RCHRES	4	7
PERLND	26	43.15	RCHRES	4	7
PERLND	34	15.44	RCHRES	4	7
PERLND	44	54.66	RCHRES	4	7
PERLND	54	8.75	RCHRES	4	7
IMPLND	14	29.14	RCHRES	4	2
*** SUB-CATCHMENT 5					
PERLND	26	10.29	RCHRES	5	1
PERLND	44	50.05	RCHRES	5	1
PERLND	54	10.74	RCHRES	5	1
IMPLND	14	16.31	RCHRES	5	2
*** SUB-CATCHMENT 6					
PERLND	16	1.42	COPY	645	26



PERLND	26	20.38	COPY	645	26
PERLND	34	13.44	COPY	645	26
PERLND	44	11.79	COPY	645	26
PERLND	54	0.82	COPY	645	26
PERLND	16	1.42	RCHRES	53	7
PERLND	26	20.38	RCHRES	53	7
PERLND	34	13.44	RCHRES	53	7
PERLND	44	11.79	RCHRES	53	7
PERLND	54	0.82	RCHRES	53	7
IMPLND	14	6.23	COPY	645	22
*** SUB-CATCHMENT 8					
PERLND	44	22.21	RCHRES	35	1
IMPLND	14	6.60	RCHRES	35	2
*** SUB-CATCHMENT 9					
PERLND	16	4.98	RCHRES	34	1
PERLND	26	14.38	RCHRES	34	1
PERLND	34	0.05	RCHRES	34	1
PERLND	44	56.71	RCHRES	34	1
PERLND	54	0.01	RCHRES	34	1
IMPLND	14	22.47	RCHRES	34	2
*** SUB-CATCHMENT 10					
PERLND	16	4.15	RCHRES	10	1
PERLND	26	31.94	RCHRES	10	1
PERLND	44	95.22	RCHRES	10	1
IMPLND	14	71.98	RCHRES	10	2
*** SUB-CATCHMENT 11 25% OF AGWO GOES TO 15					
PERLND	16	0.89	RCHRES	11	6
PERLND	26	217.92	RCHRES	11	6
PERLND	34	1.32	RCHRES	11	6
PERLND	44	65.65	RCHRES	11	6
PERLND	16	0.67	RCHRES	11	7
PERLND	26	163.44	RCHRES	11	7
PERLND	34	0.99	RCHRES	11	7
PERLND	44	49.24	RCHRES	11	7
PERLND	16	0.22	RCHRES	15	7
PERLND	26	54.48	RCHRES	15	7
PERLND	34	0.33	RCHRES	15	7
PERLND	44	16.41	RCHRES	15	7
IMPLND	14	230.80	RCHRES	11	2
*** SUB-CATCHMENT 12					
PERLND	16	0.39	RCHRES	12	1
PERLND	26	101.18	RCHRES	12	1
PERLND	34	5.64	RCHRES	12	1
PERLND	44	54.98	RCHRES	12	1
PERLND	54	0.64	RCHRES	12	1
IMPLND	14	79.83	RCHRES	12	2
*** SUB-CATCHMENT 13					
PERLND	16	0.79	RCHRES	13	1
PERLND	26	197.68	RCHRES	13	1
IMPLND	14	27.66	RCHRES	13	2
*** SUB-CATCHMENT 14 50% OF AGWO GOES TO SOUND					
PERLND	16	0.24	RCHRES	14	6
PERLND	26	118.67	RCHRES	14	6
PERLND	34	13.46	RCHRES	14	6
PERLND	44	41.91	RCHRES	14	6
PERLND	16	0.12	RCHRES	14	7
PERLND	26	59.34	RCHRES	14	7

PERLND	34	6.73	RCHRES	14	7
PERLND	44	20.95	RCHRES	14	7
IMPLND	14	20.66	RCHRES	14	2
*** SUB-CATCHMENT 15					
PERLND	16	6.59	RCHRES	15	1
PERLND	26	49.55	RCHRES	15	1
PERLND	34	50.09	RCHRES	15	1
PERLND	44	86.52	RCHRES	15	1
IMPLND	14	19.47	RCHRES	15	2
*** SUB-CATCHMENT 16					
PERLND	16	10.93	RCHRES	16	1
PERLND	26	29.93	RCHRES	16	1
PERLND	34	20.03	RCHRES	16	1
PERLND	44	31.83	RCHRES	16	1
IMPLND	14	15.58	RCHRES	16	2
*** SUB-CATCHMENT 17 AGWO GOES TO SOUND					
PERLND	16	0.90	RCHRES	17	6
PERLND	26	16.31	RCHRES	17	6
PERLND	34	34.82	RCHRES	17	6
PERLND	44	82.11	RCHRES	17	6
PERLND	54	2.19	RCHRES	17	6
IMPLND	14	10.49	RCHRES	17	2
*** SUB-CATCHMENT MC-1					
PERLND	26	0.14	RCHRES	52	1
PERLND	44	9.44	RCHRES	52	1
PERLND	45	0.14	RCHRES	52	1
PERLND	54	0.27	RCHRES	52	1
IMPLND	14	1.98	RCHRES	52	2
*** SUB-CATCHMENT MC-2					
PERLND	16	0.08	RCHRES	53	1
PERLND	26	0.53	RCHRES	53	1
PERLND	34	3.60	RCHRES	53	1
PERLND	44	9.20	RCHRES	53	1
PERLND	45	2.22	RCHRES	53	1
PERLND	54	15.14	RCHRES	53	1
IMPLND	14	2.54	RCHRES	53	2
*** SUB-CATCHMENT MC-3					
PERLND	34	3.70	RCHRES	54	1
PERLND	44	4.91	RCHRES	54	1
PERLND	45	1.07	RCHRES	54	1
PERLND	54	1.84	RCHRES	54	1
IMPLND	14	1.42	RCHRES	54	2
*** SUB-CATCHMENT MC-4					
PERLND	34	0.27	RCHRES	135	1
PERLND	44	16.51	RCHRES	135	1
PERLND	45	4.23	RCHRES	135	1
PERLND	54	11.98	RCHRES	135	1
IMPLND	14	3.31	RCHRES	135	2
*** SUB-CATCHMENT MC-5					
PERLND	26	13.43	RCHRES	35	1
PERLND	44	33.84	RCHRES	35	1
PERLND	54	7.44	RCHRES	35	1
IMPLND	14	0.02	RCHRES	35	2
*** SUB-CATCHMENT MC-6					
*** --> reduce by 2 acres to make special PERLND 47 for SDW1A					
***PERLND	44	14.10	RCHRES	35	1
PERLND	44	12.10	RCHRES	35	1

PERLND	45	0.09	RCHRES	35	1
PERLND	54	0.90	RCHRES	35	1
IMPLND	14	0.26	RCHRES	35	2
***	--> add 2 acres from special PERLND 47 for SDW1A				
PERLND	47	2.00	RCHRES	35	1
***	--> output outflow from special PERLND 47 (acft/ac)				
PERLND	47	1.00	COPY	47	21

\*\*\* SUB-CATCHMENT MC-7

***	--> reduce by 2 acres to make special PERLND 57 for SDW1B				
PERLND	26	11.26	COPY	55	21
***	--> reduce by 2 acres to make special PERLND 57 for SDW1B				
***PERLND	44	31.80	COPY	55	21
***	--> add 2 acres from special PERLND 57 for SDW1B				
PERLND	57	2.00	COPY	55	21
***	--> output outflow from special PERLND 57 (acft/ac)				
PERLND	57	1.00	COPY	57	21
PERLND	44	29.80	COPY	55	21
PERLND	54	3.20	COPY	55	21
IMPLND	14	0.03	COPY	55	22

\*\*\*note: SDN AGWO TO VACCA FARMS (135)NOT TO PONDS

\*\*\* SUB-CATCHMENT SDN-1

PERLND	26	1.97	RCHRES	552	6
PERLND	44	1.29	RCHRES	552	6
PERLND	54	0.20	RCHRES	552	6
PERLND	26	1.97	RCHRES	135	7
PERLND	44	1.29	RCHRES	135	7
PERLND	54	0.20	RCHRES	135	7
IMPLND	14	12.68	RCHRES	552	2

\*\*\* SUB-CATCHMENT SDN-1-LWR

PERLND	44	4.79	RCHRES	552	6
PERLND	54	0.07	RCHRES	552	6
PERLND	44	4.79	RCHRES	135	7
PERLND	54	0.07	RCHRES	135	7
IMPLND	14	0.56	RCHRES	552	2

\*\*\* SUB-CATCHMENT SDN-1-OFF

PERLND	26	23.01	RCHRES	52	6
PERLND	44	3.58	RCHRES	52	6
PERLND	54	1.67	RCHRES	52	6
PERLND	26	23.01	RCHRES	135	7
PERLND	44	3.58	RCHRES	135	7
PERLND	54	1.67	RCHRES	135	7
IMPLND	14	8.00	RCHRES	52	2

\*\*\* SUB-CATCHMENT SDN-2X (TO POND)

PERLND	26	0.63	COPY	61	26
PERLND	44	2.40	COPY	61	26
PERLND	45	0.86	COPY	61	26
PERLND	26	0.63	RCHRES	135	7
PERLND	44	2.40	RCHRES	135	7
PERLND	45	0.86	RCHRES	135	7
IMPLND	14	0.36	COPY	61	22

\*\*\* SUB-CATCHMENT SDN-3 (TO POND)

PERLND	26	23.56	RCHRES	43	6
PERLND	26	23.56	RCHRES	135	7
IMPLND	14	24.30	RCHRES	43	2

\*\*\* SUB-CATCHMENT SDN-3X (TO POND)

PERLND	26	1.61	RCHRES	43	6
--------	----	------	--------	----	---

\*\*\*original PERLND area

***PERLND	45	23.77	RCHRES	43	6
***PERLND AREA TO BE REMOVED = 0.29 AC					
PERLND	45	23.48	RCHRES	43	6
PERLND	80	0.29	RCHRES	135	7
PERLND	26	1.61	RCHRES	135	7
PERLND	45	23.48	RCHRES	135	7

\*\*\* SUB-CATCHMENT SDN-4 (TO POND)

PERLND	26	15.75	COPY	44	26
PERLND	44	1.31	COPY	44	26
PERLND	45	0.99	COPY	44	26
PERLND	26	15.75	RCHRES	135	7
PERLND	44	1.31	RCHRES	135	7
PERLND	45	0.99	RCHRES	135	7
IMPLND	14	12.26	COPY	44	22

\*\*\* SUB-CATCHMENT SDN-4X (TO POND)

PERLND	26	1.92	COPY	44	26
PERLND	44	0.75	COPY	44	26
PERLND	45	8.31	COPY	44	26
PERLND	26	1.92	RCHRES	135	7
PERLND	44	0.75	RCHRES	135	7
PERLND	45	8.31	RCHRES	135	7
IMPLND	14	4.21	COPY	44	22

\*\*\* SUB-CATCHMENT IWS-NCPS (TO POND)

PERLND	26	4.78	RCHRES	242	6
PERLND	26	4.78	RCHRES	135	7
IMPLND	14	30.93	RCHRES	242	2

\*\*\* SUB-CATCHMENT IWS-NSMPS (TO POND)

PERLND	26	2.69	RCHRES	240	6
PERLND	44	1.97	RCHRES	240	6
PERLND	45	0.01	RCHRES	240	6
PERLND	26	2.69	RCHRES	135	7
PERLND	44	1.97	RCHRES	135	7
PERLND	45	0.01	RCHRES	135	7
IMPLND	14	1.95	RCHRES	240	2

\*\*\* SUB-CATCHMENT NEPL (TO POND)

PERLND	26	10.00	RCHRES	452	6
PERLND	26	10.00	RCHRES	135	7
IMPLND	14	6.00	RCHRES	451	2
IMPLND	14	26.29	RCHRES	452	2

\*\*\* SUB-CATCHMENT CARGO (TO POND)

IMPLND	14	8.12	RCHRES	46	2
--------	----	------	--------	----	---

\*\*\* SUB-CATCHMENT SDN3AI (TO VAULT)

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***original IMPLND area
***IMPLND 14 5.87 RCHRES 37 2
***IMPLND AREA TO BE REMOVED = 5.69 AC
IMPLND 14 0.18 RCHRES 37 2
PERLND 80 5.69 RCHRES 135 7

*** SUB-CATCHMENT SDN3AO (TO POND)
PERLND 26 0.08 RCHRES 237 6
PERLND 44 0.03 RCHRES 237 6
***original PERLND area
***PERLND 45 22.12 RCHRES 237 6
***PERLND AREA TO BE REMOVED = 11.16 AC
***PERLND AREA TO BE REMOVED = 4.56 AC
PERLND 45 6.40 RCHRES 237 6
PERLND 80 11.16 RCHRES 135 7
PERLND 80 4.56 RCHRES 135 7
PERLND 26 0.08 RCHRES 135 7
PERLND 44 0.03 RCHRES 135 7
PERLND 45 6.40 RCHRES 135 7
***original IMPLND area
***IMPLND 14 2.35 RCHRES 237 2
***IMPLND AREA TO BE REMOVED = 2.19 AC
IMPLND 14 0.16 RCHRES 237 2
PERLND 80 2.19 RCHRES 135 7

*** SUB-CATCHMENT SDW1AO (TO POND)
***original PERLND area
***PERLND 26 4.28 RCHRES 247 6
***PERLND AREA TO BE REMOVED = 0.67 AC
PERLND 26 3.61 RCHRES 247 6
PERLND 80 0.67 RCHRES 135 7
PERLND 44 0.69 RCHRES 247 6
***original PERLND area
***PERLND 45 32.44 RCHRES 247 6
***PERLND AREA TO BE REMOVED = 18.06 AC
***PERLND AREA TO BE REMOVED = 0.60 AC
PERLND 45 13.78 RCHRES 247 6
PERLND 80 18.06 RCHRES 135 7
PERLND 80 0.60 RCHRES 135 7
PERLND 26 3.61 RCHRES 135 7
PERLND 44 0.69 RCHRES 135 7
PERLND 45 13.78 RCHRES 135 7
***original IMPLND area
***IMPLND 14 1.64 RCHRES 247 2
***IMPLND AREA TO BE REMOVED = 0.93 AC
IMPLND 14 0.71 RCHRES 247 2
PERLND 80 0.93 RCHRES 135 7
*** PERVIOUS AREA FOR 1AI IS IN 1AO
*** SUB-CATCHMENT SDN1AI (TO VAULT)
***original IMPLND area
***IMPLND 14 13.78 RCHRES 147 2
***IMPLND AREA TO BE REMOVED = 13.07 AC
IMPLND 14 0.71 RCHRES 147 2
PERLND 80 13.07 RCHRES 35 7
*** CONTAINS BOTH I AND O
*** SUB-CATCHMENT SDW1B (TO POND)
*** AGWO TO 35, AS 57 IS D/S OF VACCA FARMS (135)

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```

***original PERLND area
***PERLND 26                21.25      RCHRES 570      6
***PERLND AREA TO BE REMOVED = 0.54 AC
PERLND 26                20.71      RCHRES 570      6
PERLND 80                0.54      RCHRES 35       7
PERLND 44                2.39      RCHRES 570      6
***original PERLND area
***PERLND 45                46.26      RCHRES 570      6
***PERLND AREA TO BE REMOVED = 34.71 AC
***PERLND AREA TO BE REMOVED = 1.34 AC
PERLND 45                10.21      RCHRES 570      6
PERLND 80                34.71      RCHRES 35       7
PERLND 80                1.34      RCHRES 35       7
PERLND 26                20.71      RCHRES 35       7
PERLND 44                2.39      RCHRES 35       7
PERLND 45                10.21      RCHRES 35       7
***original IMPLND area
***IMPLND 14                26.95      RCHRES 570      2
***IMPLND AREA TO BE REMOVED = 20.79 AC
***IMPLND AREA TO BE REMOVED = 1.62 AC
IMPLND 14                4.54      RCHRES 570      2
PERLND 80                20.79      RCHRES 35       7
PERLND 80                1.62      RCHRES 35       7

```

\*\*\* ADD SUB-CATCHMENT IWS-PRIMARY TO PREDEVELOPEMENT ONLY

\*\*\*ROUTING FOR MILLER CREEK

```

*** M1 TO M2 TO M3 TO STORAGE 50.  M4 TO M5 TO STORAGE 50
RCHRES 1                RCHRES 2      4
RCHRES 23               RCHRES 24      4
RCHRES 24               RCHRES 3       3
RCHRES 2                RCHRES 3       3
RCHRES 3                RCHRES 33      3
RCHRES 33               RCHRES 50      3
RCHRES 4                RCHRES 5       4
RCHRES 5                RCHRES 50      3
*** PONDS TO 52, 53 & 54
RCHRES 242              RCHRES 240     5
*** OVERFLOW ONLY TO 61
RCHRES 240              RCHRES 51      5
COPY 61                 RCHRES 51     12
COPY 44                 RCHRES 51     12
RCHRES 51               RCHRES 52      3
RCHRES 43               RCHRES 54      3
*** 2 NEPL VAULTS* (FK-Changed to eliminate run-of-river tables)
RCHRES 451              COPY 45       11
RCHRES 452              COPY 45       11
COPY 45                 COPY 645     10
COPY 645                RCHRES 53     12
RCHRES 46               RCHRES 53      3
*** NEW STREAM REACH 52 TO LAKE REBA 53 TO RDF 54 (FK-changed to insert new POC
at Lake Reba)
RCHRES 552              RCHRES 52      3
RCHRES 52               RCHRES 53      3
RCHRES 53               COPY 53       11
COPY 53                 RCHRES 54     12
RCHRES 50               RCHRES 54      3

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*** RDF 54 TO 35
RCHRES 54
<-Source->          <--Area-->          <-Target->          MBLK          ***
<Name> #            <-factor->          <Name> #          Tbl#          ***
*** PONDS TO 34
RCHRES 37
RCHRES 237
COPY 37
*** SDW1A flow to bypass added (FK, June 2001)
SDW1AI VAULT FLOW TO INFILTRATION 1
RCHRES 147
SDW1AI VAULT FLOW TO BYPASS
RCHRES 147
STORMWATER Q 1ST EXIT AT POND G (Bypass)
RCHRES 247
RCHRES 247
RCHRES 247
2ND EXIT TO INFILTRATION TANK-MILLER CREEK
RCHRES 247
STORMWATER Q 1ST EXIT TO BYPASS
RCHRES 47
*** 2ND EXIT TO SOIL AND MILLER CREEK (2nd exit intr. as AGWLI)***
*** RCHRES 47
COPY BLOCK FOR OUTPUT PURPOSES
RCHRES 47
RCHRES 47
RCHRES 147
RCHRES 147
COPY 70
RCHRES 34
RCHRES 34
RCHRES 135
RCHRES 10
*** PONDS TO 35
*** Configuration changed to flow splitter to Pond D and Infiltration Basin 3
(FK, June 2001)
STORM Q - 1ST EXIT OF FLOW SPLITTER TO POND D
RCHRES 570
***INFILTRATION Q - 2ND EXIT OF FLOW SPLITTER TO SOIL
*** RCHRES 570
STORM Q EXIT OF POND D TO MILLER CREEK
RCHRES 57
COPY BLOCK FOR OUTPUT PURPOSES
RCHRES 570
RCHRES 570
RCHRES 57
RCHRES 257
RCHRES 257
RCHRES 257
COPY 71
RCHRES 35
COPY 55
RCHRES 11
RCHRES 13
RCHRES 13
RCHRES 12
RCHRES 16
RCHRES 135 3
COPY 37 11
COPY 37 11
RCHRES 135 12
RCHRES 47 4
COPY 70 15
COPY 70 14
COPY 66 14
COPY 69 15
RCHRES 47 5
COPY 70 14
COPY 70 15
COPY 62 14
COPY 63 15
COPY 67 14
COPY 68 15
RCHRES 135 12
RCHRES 135 4
RCHRES 135 5
RCHRES 35 3
RCHRES 16 3
RCHRES 57 4
RCHRES 257 5
COPY 71 11
COPY 64 14
COPY 65 15
COPY 357 11
COPY 56 14
COPY 71 14
COPY 71 15
RCHRES 35 12
COPY 55 11
RCHRES 16 12
RCHRES 15 3
RCHRES 12 4
RCHRES 12 5
RCHRES 15 3
RCHRES 15 3

```

RCHRES 14  
 RCHRES 15  
 END SCHEMATIC

RCHRES 17 3  
 RCHRES 17 3

NETWORK

\*\*\* <MEMBER> SSSYSSGAP<--MULT-->TRAN <-TARGET VOLTS> <-MEMBER->  
 <NAME> # <NAME> TEM STRG<-FACTOR->STRG <NAME> # # <-GRP> <NAME> # # \*\*\*  
 END NETWORK

\*\*\*

RCHRES

GEN-INFO

RCHRES	Name	Nexits	Unit Systems		Printer				
#	#		User	T-series	Engl	Metr	LKFG		
			in	out					
1	Arbor Lake M 1	2	1	1	1	62	0	0	
2	Arbor Ck -03710 M 2	1	1	1	1	62	0	0	
3	Arbor Ck M 3	1	1	1	1	62	0	0	
4	Tub Lake M 4	2	1	1	1	62	0	0	
5	Miller Ck SR518 M5	1	1	1	1	62	0	0	
10	Trib (0371G) M 10	1	1	1	1	62	0	0	
11	M11 Ambaum Detention	1	1	1	1	62	0	0	
12	Trib(0354) M 12	1	1	1	1	62	0	0	
13	Burien Lake M 13	2	1	1	1	62	0	0	
14	Trib (0353) M 14	1	1	1	1	62	0	0	
15	M/S U/S OF 17	1	1	1	1	62	0	0	
16	U/S OF 15 M/S	1	1	1	1	62	0	0	
17	GAGE	1	1	1	1	62	0	0	
23	BASIN M23	2	1	1	1	62	0	0	
24	BASIN M24	1	1	1	1	62	0	0	
33	detention m3	1	1	1	1	62	0	0	
34	LORA LAKE	2	1	1	1	62	0	0	
35	D/S OF VACA FARM	1	1	1	1	62	0	0	
37	sdn3ai vault	1	1	1	1	62	0	0	
38	MC basins	1	1	1	1	62	0	0	
*** 39	SDN3A/SDW1A POC	1	1	1	1	62	0	0	
43	sdn3 pond	1	1	1	1	62	0	0	
*** 44	sdn4 pond	1	1	1	1	62	0	0	
*** 45	nepl poc	1	1	1	1	62	0	0	
46	cargo pond	1	1	1	1	62	0	0	
47	sdw1a infiltration	2	1	1	1	62	0	0	
50	sr 518	1	1	1	1	62	0	0	
51	SDN2X+SDN4X	1	1	1	1	62	0	0	
52	U/S OF LAKE REBA	1	1	1	1	62	0	0	
53	Reba outflow	1	1	1	1	62	0	0	
54	Miller RDF outflow	1	1	1	1	62	0	0	
57	sdw1b pond	1	1	1	1	62	0	0	
135	VACA FARMS	1	1	1	1	62	0	0	
147	sdw1a vault	2	1	1	1	62	0	0	
237	sdn3ao-pond c	1	1	1	1	62	0	0	
240	iws-ncps	2	1	1	1	62	0	0	
242	iws-nsmpps	2	1	1	1	62	0	0	
247	sdw1a pond g	2	1	1	1	62	0	0	
257	sdw1b infiltration	2	1	1	1	62	0	0	
451	nepl VAULT	1	1	1	1	62	0	0	
452	nepl VAULT	1	1	1	1	62	0	0	
552	SDN1 POC	1	1	1	1	62	0	0	
570	SDW1B flow splitter	2	1	1	1	62	0	0	

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23	23	0.379	0.0	0.3
24	24	0.379		0.3
33	33	0.200		0.3
34	34	0.852		0.3
35	35	0.663		0.3
37	37	0.010	0.0	0.3
38	38	0.010		0.3
43	43	0.010		0.3
46	46	0.010		0.3
47	47	0.010	0.0	0.3
50	50	0.010		0.3
51	51	0.010		0.3
52	52	0.010		0.3
53	53	0.010		0.3
54	54	0.010	0.0	0.3
57	57	0.010	0.0	0.3
135	135	0.350		0.3
147	147	0.010		0.3
237	237	0.010	0.0	0.3
240	240	0.010		0.3
242	242	0.010		0.3
247	247	0.010	0.0	0.3
257	257	0.010	0.0	0.3
451	451	0.010	0.0	0.3
452	452	0.010	0.0	0.3
552	552	0.010	0.0	0.3
570	570	0.010	0.0	0.3

END HYDR-PARM2

HYDR-INIT

		RCHRES Initial conditions for each HYDR section		***
#	# ***	VOL	Initial value of COLIND	Initial value of OUTDGT
	***	ac-ft	for each possible exit	for each possible exit
<-----><----->		<---><---><---><---><--->		*** <---><---><---><---><--->
1	2.0		4.0 5.0	
2	0.0		4.0	
3	0.0		4.0	
4	2.0		4.0 5.0	
5	0.0		4.0	
10	0.0		4.0	
11	0.0		4.0	
12	0.0		4.0	
13	10.0		4.0 5.0	
14	0.0		4.0	
15	0.0		4.0	
16	0.0		4.0	
17	0.0		4.0	
23	6.0		4.0 5.0	
24	0.0		4.0	
33	0.0		4.0	
34	9.0		4.0 5.0	
35	0.1		4.0	
37	0.0		4.0	
38	0.1		4.0	
43	0.0		4.0	
46	0.0		4.0	
47	0.0		4.0 5.0	

50	0.0	4.0	
51	0.0	4.0	
52	0.0	4.0	
53	0.1	4.0	
54	2.25	4.0	
57	0.0	4.0	
237	0.00	4.0	
147	0.00	4.0	5.0
135	0.00	4.0	
240	0.0	4.0	5.0
242	0.0	4.0	5.0
247	0.0	4.0	5.0
257	0.0	4.0	5.0
451	0.0	4.0	
452	0.0	4.0	
552	0.0	4.0	
570	0.0	4.0	5.0

END HYDR-INIT  
 END RCHRES

FTABLES  
 \*\*\*UPPER BASIN  
 \*\*\*=====

FTABLE 1  
 \*\*\* REVISED 8/16/00 ADDED 2ND OUTFLOW  
 ROWS COLS \*\*\*

11	5				
DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2***	
0.00	3.00	0.00	0.00	0.00	
2.50	3.00	7.50	0.00	0.11	
3.00	3.00	9.00	1.80	0.11	
3.50	3.30	10.58	5.00	0.11	
4.00	3.60	12.30	10.90	0.11	
4.50	3.90	14.18	17.50	0.11	
5.00	4.10	16.18	26.20	0.11	
5.50	4.30	18.28	32.50	0.11	
6.00	4.50	20.48	35.90	0.11	
7.00	5.00	25.23	38.10	0.11	
8.00	5.50	30.48	46.40	0.11	

END FTABLE 1

FTABLE 2  
 ROWS COLS \*\*\*

9	4				
DEPTH	AREA	VOLUME	OUTFLOW	***	
0.000	0.0000	0.0000	0.00		
0.100	0.2571	0.0129	0.16		
0.500	0.3873	0.1417	6.53		
1.000	0.5501	0.3761	25.95		
1.500	0.7128	0.6918	59.86		
2.000	0.8756	1.0889	110.67		
3.000	1.2011	2.1273	272.24		
3.500	1.3639	2.7685	387.38		
4.000	1.5266	3.4912	528.19		

END FTABLE 2

```

FTABLE      3
ROWS COLS  ***
 12      4
  DEPTH    AREA    VOLUME    OUTFLOW  ***
  0.000    0.0000   0.0000    0.00
  0.100    0.9669   0.0483    0.13
  0.500    1.0637   0.4545    4.92
  1.000    1.1846   1.0165   17.12
  1.500    1.3055   1.6390   34.92
  2.000    1.4264   2.3220   57.95
  2.500    1.5473   3.0654   86.14
  3.000    1.6682   3.8693  119.53
  3.500    1.7891   4.7336  158.24
  4.000    1.9100   5.6584  202.41
  4.500    2.0294   6.6310  251.52
  5.000    2.1488   7.6624  306.28
END FTABLE  3

```

```

FTABLE      4
*** REVISED 8/16/00 ADDED 2ND OUTFLOW
ROWS COLS  ***
 7      5
  DEPTH    AREA    VOLUME    OUTFLOW  OUTFLOW2***
  0.00    3.00    0.00    0.00    0.00
  2.50    4.50    9.38    0.00    0.11
  3.00    6.00   12.00    6.00    0.11
  4.00   10.00   20.00   13.00    0.11
  5.00   15.00   32.50   20.00    0.11
  6.00   20.00   50.00   26.00    0.11
  7.00   25.00   72.50  168.00    0.11
END FTABLE  4

```

```

FTABLE      5
ROWS COLS  ***
 10      4
  DEPTH    AREA    VOLUME    OUTFLOW  ***
  0.000    0.0000   0.0000    0.00
  0.100    0.1010   0.0051    0.03
  0.500    0.1754   0.0603    1.46
  1.000    0.2684   0.1713    6.16
  1.500    0.3614   0.3288   14.89
  2.000    0.4544   0.5327   28.48
  2.500    0.5474   0.7832   47.70
  3.000    0.6404   1.0801   73.29
  3.500    0.7334   1.4236  105.94
  4.000    0.8264   1.8136  146.33
END FTABLE  5

```

```

FTABLE      10
ROWS COLS  ***
 9      4
  DEPTH    AREA    VOLUME    OUTFLOW  ***
  0.000    0.0000   0.0000    0.00
  0.100    0.1010   0.0051    0.06
  0.500    0.1660   0.0585    2.27
  1.000    0.2472   0.1618    9.32
  1.500    0.3285   0.3057   22.08

```

2.000	0.4097	0.4902	41.66
2.500	0.4909	0.7154	69.09
3.000	0.5722	0.9811	105.37
4.000	0.6887	1.6116	209.70

END FTABLE 10

POST AMBAUM DETENTION \*\*\*

FTABLE 11

ROWS COLS \*\*\*

12	4			
DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
1.000	0.1000	0.2300	3.90	
2.000	0.2000	0.6000	6.30	
3.000	0.3000	0.9700	8.10	
4.000	0.4000	1.3400	11.10	
5.000	0.5000	1.8200	16.00	
6.000	0.6000	2.2700	19.10	
7.000	0.7000	2.8300	21.60	
8.000	0.8000	3.3700	30.80	
9.000	0.9000	4.0000	38.10	
10.000	1.0000	4.6500	74.10	
10.500	1.1000	5.2000	133.00	
11.000	1.1500	6.0000	500.00	
11.500	1.3000	11.000	1300.00	

END FTABLE 11

FTABLE 12

ROWS COLS \*\*\*

6	4			
DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.6327	0.0316	0.15	
0.500	0.7960	0.3174	5.87	
1.000	1.0002	0.7664	21.53	
1.500	1.2043	1.3176	46.43	
2.000	1.4085	1.9708	81.20	
3.000	1.8168	3.5834	183.79	
4.000	2.2251	5.6044	336.22	
5.000	2.6335	8.0337	545.30	
6.000	3.0418	10.8713	817.51	

END FTABLE 12

FTABLE 13

\*\*\* REVISED 8/16/00 ADDED 2ND OUTFLOW

ROWS COLS \*\*\*

7	5			
DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2***
0.000	40.000	0.0000	0.00	0.00
1.000	41.400	40.000	0.00	0.11
1.500	42.000	60.000	10.00	0.11
2.000	42.700	80.000	16.00	0.11
2.500	43.300	100.00	20.00	0.11
3.000	44.000	120.00	28.00	0.11
5.000	45.000	210.00	45.00	0.11

END FTABLE 13

FTABLE 14  
 ROWS COLS \*\*\*  
 6 4  
 DEPTH AREA VOLUME OUTFLOW \*\*\*  
 0.000 0.0000 0.0000 0.00  
 0.100 0.3361 0.0168 0.24  
 0.500 0.3809 0.1602 9.04  
 1.000 0.4370 0.3647 31.61  
 1.500 0.4930 0.5972 65.00  
 2.000 0.5491 0.8577 108.85  
 2.500 0.6051 1.1462 163.33  
 3.000 0.6612 1.4628 228.78  
 END FTABLE 14

FTABLE 15  
 ROWS COLS \*\*\*  
 4 4  
 DEPTH AREA VOLUME OUTFLOW \*\*\*  
 0.00 0.10 0.00 0.00  
 1.00 1.00 0.55 91.00  
 2.00 1.10 1.60 268.00  
 3.00 1.20 2.75 493.00  
 END FTABLE 15

FTABLE 16  
 ROWS COLS \*\*\*  
 4 4  
 DEPTH AREA VOLUME OUTFLOW \*\*\*  
 0.00 0.10 0.00 0.00  
 1.00 1.00 0.55 74.00  
 2.00 1.10 1.60 219.00  
 3.00 1.20 2.75 403.00  
 END FTABLE 16

FTABLE 17  
 ROWS COLS \*\*\*  
 5 4  
 DEPTH AREA VOLUME OUTFLOW \*\*\*  
 0.00 0.10 0.00 0.00  
 1.00 1.00 0.55 59.00  
 2.00 1.10 1.60 173.00  
 3.00 1.20 2.75 318.00  
 4.00 1.30 4.00 484.00  
 END FTABLE 17

FTABLE 23  
 ROWS COLS \*\*\* HERMES  
 9 5  
 DEPTH AREA VOLUME OUTFLOW OUTFLOW \*\*\*  
 0.00 0.00 0.00 0.00 0.00 0.00  
 5.00 0.50 1.91 0.00 0.00 305.00  
 11.00 0.79 5.79 0.00 0.00 311.00  
 15.00 1.13 9.64 0.50 0.01 315.00  
 19.00 1.72 15.34 0.50 0.05 319.00  
 29.00 2.86 38.25 0.50 0.10 329.00  
 39.00 4.40 74.55 0.50 0.20 339.00  
 50.00 6.22 132.98 0.50 0.30 350.00

60.00 10.00 1212.98 0.50 0.40 360.00  
 END FTABLE 23

FTABLE 24  
 ROWS COLS \*\*\*

9	4				***
DEPTH	AREA	VOLUME	OUTFLOW		
0.000	0.0000	0.0000	0.00		
0.100	0.2571	0.0129	0.16		
0.500	0.3873	0.1417	6.53		
1.000	0.5501	0.3761	25.95		
1.500	0.7128	0.6918	59.86		
2.000	0.8756	1.0889	110.67		
3.000	1.2011	2.1273	272.24		
3.500	1.3639	2.7685	387.38		
4.000	1.5266	3.4912	528.19		

END FTABLE 24

FTABLE 33  
 ROWS COLS \*\*\*

11	4				***
DEPTH	AREA	VOLUME	OUTFLOW		
0.00	1.00	0.00	0.00		
0.50	1.20	0.55	2.00		
1.00	1.40	1.20	6.00		
1.50	1.60	1.95	9.00		
2.00	1.80	2.80	13.00		
2.50	2.00	3.75	16.50		
3.00	2.20	4.80	20.00		
3.50	2.40	5.95	23.00		
4.00	2.60	7.20	26.00		
5.00	2.80	9.90	104.00		
6.00	3.00	12.80	246.00		

END FTABLE 33

FTABLE 34  
 ROWS COLS \*\*\* REVISED 11/19/97 BASED ON HEC-RAS MODEL  
 \*\*\* REVISED 8/16/00 ADDED 2ND OUTFLOW

6	5					***
DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2		
0.00	3.00	0.00	0.00	0.00		
3.00	3.05	9.08	0.00	0.11		
4.00	3.10	12.15	0.00	0.11		
5.00	3.15	15.28	0.00	0.11		
6.00	3.20	18.45	72.0	0.11		
7.00	3.25	21.68	225.0	0.11		

END FTABLE 34

FTABLE 35  
 ROWS COLS \*\*\* REVISED 11/19/97 BASED ON HECRAS MODEL

5	4				***
DEPTH	AREA	VOLUME	OUTFLOW		
0.00	0.10	0.00	0.00		
1.00	1.10	0.60	38.00		
2.00	1.20	1.75	108.00		
3.00	1.30	3.00	194.00		
4.00	1.40	4.35	290.00		

END FTABLE 35

FTABLE 38  
ROWS COLS \*\*\*  
7 4  
DEPTH AREA VOLUME OUTFLOW \*\*\*  
0.000 0.0000 0.0000 0.00  
1.000 0.4000 0.4000 2.00  
1.500 0.5000 1.0000 4.00  
2.000 0.9000 1.3000 11.00  
2.500 1.3000 1.6000 15.00  
3.000 1.6000 2.0000 18.00  
3.500 1.9000 2.5000 20.80  
END FTABLE 38

FTABLE 45  
ROWS COLS \*\*\*  
4 4  
DEPTH AREA VOLUME OUTFLOW \*\*\*  
0.000 0.0010 0.0000 0.00  
0.000 0.0100 0.0100 10.00  
0.100 0.1000 0.1000 100.00  
1.000 1.0000 1.0000 1000.00  
10.000 10.0000 10.0000 10000.00  
END FTABLE 45

FTABLE 645  
ROWS COLS \*\*\*  
4 4  
DEPTH AREA VOLUME OUTFLOW \*\*\*  
0.000 0.0010 0.0000 0.00  
0.000 0.0100 0.0100 10.00  
0.100 0.1000 0.1000 100.00  
1.000 1.0000 1.0000 1000.00  
10.000 10.0000 10.0000 10000.00  
END FTABLE645

FTABLE 50  
ROWS COLS \*\*\*  
10 4  
DEPTH AREA VOLUME OUTFLOW \*\*\*  
0.00 1.00 0.00 0.00  
0.50 1.10 0.53 5.00  
1.00 1.20 1.10 15.00  
1.50 1.30 1.73 25.00  
2.00 1.40 2.40 35.00  
2.50 1.50 3.13 52.00  
3.00 1.60 3.90 70.00  
3.50 1.70 4.73 87.00  
4.00 1.80 5.60 105.00  
6.00 1.90 9.30 165.00  
END FTABLE 50

FTABLE 52  
ROWS COLS \*\*\*  
6 4



DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.3680	0.0184	0.25	
0.500	0.3717	0.1664	9.39	
1.000	0.3763	0.3534	31.06	
2.000	0.3819	0.7325	94.37	
3.000	0.3874	1.1171	174.33	

END FTABLE 52

FTABLE 552  
ROWS COLS \*\*\* SDN1 VAULT EFFECTIVE DEPTH=12 FT RISER=24 INCHES  
15 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.4308	0.0000	0.00	
1.290	0.4308	0.6520	0.111	
2.130	0.4308	1.0760	0.143	
3.530	0.4308	1.7830	0.184	
4.640	0.4308	2.3430	0.211	
5.200	0.4308	2.6260	0.223	
6.320	0.4308	3.1920	0.246	
7.430	0.4308	3.7530	0.267	
8.200	0.4308	4.1410	0.280	
9.220	0.4308	4.6570	0.407	
10.190	0.4308	5.1460	0.567	
11.250	0.4308	5.6820	0.954	
12.100	0.4308	6.1110	2.130	
12.300	0.4308	6.2120	4.730	
13.700	0.4308	6.9190	21.360	

END FTABLE552

FTABLE 53  
OLD LAKE REBA \*\*\*  
MAX DEPTH = 4.9 FEET \*\*\*  
30" CMP, 40 CFS DISCHARGE AT MAX DEPTH \*\*\*  
ROWS COLS \*\*\*  
7 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	2.4000	0.0000	0.00	
1.000	2.5800	2.5000	18.00	
2.000	2.9400	5.3000	26.00	
3.000	3.4100	8.4000	31.00	
4.000	3.8800	12.100	36.00	
4.900	4.3000	15.800	40.00	
6.000	4.3000	15.810	500.00	

END FTABLE 53

FTABLE 54  
EXISTING MILLER CREEK DETENTION FACILITY\*\*\* REVISED STORAGE/Q DATA  
GATE SETTING: 2.0 FEET\*\*\* BASED ON CALIBRATION FILE  
ROWS COLS \*\*\*  
12 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.00	0.00	0.00	
1.300	0.01	0.01	10.00	
2.000	0.01	0.02	20.00	
2.900	0.70	0.40	30.00	
4.000	1.50	1.50	40.00	

5.400	3.50	4.90	50.00
7.000	8.60	13.30	60.00
8.800	15.60	34.80	70.00
10.000	19.90	57.30	76.00
10.500	21.50	68.00	92.00
11.000	23.10	78.80	179.00
11.500	24.70	88.60	303.00

END FTABLE 54

FTABLE 104  
 MILLER CREEK DETENTION FACILITY\*\*\* WITH ADD'L AREA 1+AREA 2 55.5 ACFT @ 10FT  
 GATE SETTING: 2.0 FEET\*\*\* EXISTING OUTLET NO LOW FLOW CONTROL  
 ROWS COLS \*\*\*

17	4			
DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.500	0.0100	0.0100	2.50	
1.500	0.0300	0.2800	14.29	
2.500	1.1100	1.3900	24.88	
3.500	2.6100	4.0000	34.51	
4.500	4.6100	9.1400	43.20	
5.500	7.1200	19.600	50.98	
6.000	8.3600	21.180	54.53	
6.500	11.870	30.060	57.87	
7.000	15.370	38.930	61.00	
7.500	18.870	47.800	63.91	
8.000	21.860	59.160	66.62	
8.500	24.850	70.510	69.12	
9.000	27.340	84.160	71.42	
9.500	29.820	97.820	73.53	
10.000	32.050	112.83	75.44	
10.500	34.275	127.84	90.74	
11.500	38.220	161.54	320.00	

END FTABLE104

FTABLE 69  
 PRE-MILLER CREEK DETENTION FACILITY\*\*\*  
 ROWS COLS \*\*\*

12	4			
DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.1860	0.0093	0.12	
0.500	0.2552	0.0975	4.84	
1.000	0.3417	0.2467	18.49	
1.500	0.4282	0.4392	41.30	
2.000	0.5148	0.6750	74.40	
2.500	0.6013	0.9540	119.01	
3.000	0.6878	1.2763	176.30	
3.500	0.7744	1.6418	247.41	
4.000	0.8609	2.0506	333.43	
4.500	0.9470	2.4992	434.59	
5.000	1.0331	2.9905	552.33	

END FTABLE 69

\*\*\* PROJECT CONDITION PONDS/VAULTS

FTABLE 452  
 ROWS COLS \*\*\*

AR 011440

\*\*\* NEW NORTH EMPLOYEE PARKING LOT VAULT (NEPL)  
 \*\*\* PARALLEL VAULT BASED ON KCRTS EFFECTIVE DEPTH=20 FT

DEPTH	AREA	VOLUME	OUTFLOW	***
0.00	3.214	0.000	0.000	
1.11	3.214	0.826	0.129	
1.57	3.214	1.168	0.154	
3.43	3.214	2.551	0.227	
4.83	3.214	3.593	0.269	
8.08	3.214	6.010	0.348	
10.41	3.214	7.743	0.395	
12.74	3.214	9.476	0.437	
14.00	3.214	10.413	0.458	
14.65	3.214	10.897	0.557	
16.09	3.214	11.968	0.665	
16.23	3.214	12.072	0.754	
17.92	3.214	13.329	1.140	
18.22	3.214	13.552	1.310	
18.81	3.214	13.991	1.860	
19.11	3.214	14.214	2.190	
20.00	3.214	14.876	3.350	
20.20	3.214	15.025	5.110	
20.70	32.14	15.397	14.820	
21.00	32.14	15.620	18.560	

END FTABLE452

FTABLE 451  
 ROWS COLS \*\*\*

\*\*\* NORTH EMPLOYEE PARKING LOT VAULT (NEPL)  
 \*\*\* EXISTING VAULT W/MODIFIED OUTLET EFFECTIVE DEPTH= 18.0 FT

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.2240	0.0000	0.00	
2.170	0.2240	0.4860	0.031	
4.260	0.2240	0.9550	0.043	
5.930	0.2240	1.3290	0.051	
8.030	0.2240	1.8000	0.059	
10.120	0.2240	2.2680	0.066	
12.210	0.2240	2.7360	0.073	
14.040	0.2240	3.1460	0.109	
15.510	0.2240	3.4760	0.166	
16.220	0.2240	3.6350	0.295	
18.000	0.2240	4.0340	1.080	
18.400	0.2240	4.1240	5.400	
19.000	0.2240	4.2580	12.680	
19.900	0.2240	4.4600	17.080	

END FTABLE451

FTABLE 46  
 ROWS COLS \*\*\*

SDN-6: 24TH STREET CARGO VAULT \*\*\* EFFECTIVE DEPTH=14 FT RISER DIA=12 IN

DEPTH	AREA	VOLUME	OUTFLOW	***
0.00	0.35	0.000	0.000	
0.37	0.35	0.131	0.021	
1.19	0.35	0.421	0.037	

3.39	0.35	1.198	0.063
5.03	0.35	1.778	0.077
7.23	0.35	2.556	0.092
9.15	0.35	3.235	0.104
10.25	0.35	3.624	0.110
10.53	0.35	3.723	0.111
10.92	0.35	3.861	0.128
12.00	0.35	4.242	0.165
12.13	0.35	4.288	0.190
12.95	0.35	4.578	0.245
13.77	0.35	4.868	0.282
14.00	0.35	4.949	0.291
14.10	0.35	4.985	0.910
14.20	0.35	5.020	2.040
14.30	0.35	5.056	3.500
14.50	0.35	5.126	7.200
14.70	0.35	5.197	11.720

END FTABLE 46

\*\*\* SDW-1A: 3RD RUNWAY POND G TO MILLER CREEK (LEVEL 2): \*\*\*

FTABLE 47

\*\*\* PROJECT SDW1A EFFECTIVE DIAMETER=3.0 FT  
 ROWS COLS \*\*\* INFILTRATION TANK TO OBTAIN 0.3 CFS

14 5

DEPTH	AREA	VOLUME	STORMQ	INFILTRQ ***
0.000	0.000	0.000	0.000	0.000
0.250	0.002	0.002	0.000	0.027
0.500	0.004	0.004	0.000	0.054
1.000	0.012	0.012	0.000	0.109
1.500	0.020	0.020	0.000	0.164
2.000	0.029	0.029	0.000	0.218
2.500	0.036	0.036	0.000	0.272
3.000	0.041	0.0406	0.000	0.327
3.100	0.041	0.0419	0.596	0.338
3.200	0.041	0.0420	1.685	0.349
3.300	0.041	0.0421	3.096	0.360
3.400	0.041	0.0422	4.766	0.371
3.500	0.041	0.0423	6.661	0.382
3.750	0.041	0.0424	12.237	0.409

END FTABLE 47

\*\*\* SDW-1A: 3RD RUNWAY NORTH POND G TO MILLER CREEK (LEVEL 2): \*\*\*

FTABLE 147

\*\*\* PROJECT SDW1A EFFECTIVE DEPTH=14.0 FT RISER DIA 24 INCHES  
 ROWS COLS \*\*\* VAULT BASED ON INFILTRATION=0.15CFS

17 5

DEPTH	AREA	VOLUME	INFILTRQ	BYPASS Q***
0.000	0.689	0.000	0.0000	0.0000
0.010	0.689	0.007	0.1400	0.0000
1.000	0.689	0.689	0.1408	0.0000
2.000	0.689	1.377	0.1417	0.0000
4.000	0.689	2.755	0.1432	0.0000
6.000	0.689	4.132	0.1446	0.0000
8.000	0.689	5.510	0.1461	0.0000
10.000	0.689	6.887	0.1475	0.0000
12.000	0.689	8.264	0.1489	0.0000

14.000	0.689	9.642	0.1503	0.0000
16.000	0.689	11.019	0.1517	0.0000
16.750	0.689	11.536	0.1517	10.7600
16.900	0.689	11.639	0.1517	13.9600
17.000	0.689	11.708	0.1517	16.1000
17.100	0.689	11.777	0.1517	18.5700
17.300	0.689	11.915	0.1517	23.8600
18.000	0.689	12.397	0.1517	45.5400

END FTABLE147

\*\*\* SDW-1A: 3RD RUNWAY NORTH POND G TO MILLER CREEK (LEVEL 2): \*\*\*

FTABLE 247

\*\*\* PROJECT SDW1A EFFECTIVE DEPTH=12.0 FT RISER DIA 12 INCHES  
 ROWS COLS \*\*\* POND BASED ON INFILTRATION=0.15CFS

17 5

DEPTH	AREA	VOLUME	STORMQ	INFILTRQ ***
0.000	1.300	0.000	0.00	0.00
0.010	1.310	0.010	0.001	0.15
1.000	1.320	1.320	0.007	0.15
2.000	1.342	2.650	0.010	0.15
3.000	1.363	4.000	0.012	0.15
4.000	1.385	5.370	0.013	0.15
5.000	2.672	8.000	0.015	0.15
6.000	2.739	10.700	0.017	0.15
7.000	2.807	13.470	0.018	0.15
8.000	2.876	16.300	0.019	0.15
8.300	2.896	17.176	0.031	0.15
9.000	2.945	19.210	0.041	0.15
10.000	3.014	22.180	0.051	0.15
11.000	3.084	25.228	0.058	0.15
11.100	3.092	25.540	0.675	0.15
11.300	3.106	26.162	3.260	0.15
12.000	3.155	28.340	15.190	0.15

END FTABLE247

\*\*\* SDN3A: 3RD RUNWAY VAULT TO MILLER CREEK (LEVEL 2): \*\*\*

FTABLE 37

\*\*\* PROJECT C SDN3A EFFECTIVE DEPTH=11.0FT RISER DIA=24 INCHES  
 ROWS COLS \*\*\* VAULT BASED ON IMPERVIOUS TOP SURO

14 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.644	0.000	0.000	
0.010	0.644	0.006	0.001	
1.000	0.644	0.643	0.016	
3.980	0.644	2.558	0.033	
6.030	0.644	3.876	0.041	
9.010	0.644	5.792	0.050	
10.00	0.644	6.428	0.052	
10.46	0.644	6.724	0.072	
11.00	0.644	7.071	0.082	
11.10	0.644	7.135	0.699	
11.20	0.644	7.199	1.830	
11.30	0.644	7.264	3.290	
11.40	0.644	7.328	5.020	
11.60	0.644	7.456	9.140	

END FTABLE 37

\*\*\* SDN3A: 3RD RUNWAY POND C TO MILLER CREEK (LEVEL 2): \*\*\*

FTABLE 237

\*\*\* PROJECT C SDN3A EFFECTIVE DEPTH= 9.0FT RISER DIA=24 INCHES  
ROWS COLS \*\*\* POND BASED ON INTERFLOW AND PERVIOUS TOP SURO

19 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	1.3090	0.000	0.00	
0.020	1.3120	0.026	0.009	
1.020	1.3550	1.358	0.070	
2.070	1.4030	2.806	0.100	
3.130	1.4530	4.320	0.123	
4.020	1.4980	5.632	0.139	
5.070	1.5460	7.229	0.156	
7.750	1.6720	11.549	0.193	
7.800	1.6800	11.633	0.199	
7.850	1.6840	11.718	0.213	
8.250	1.7050	12.395	0.249	
8.340	1.7090	12.549	0.270	
8.570	1.7210	12.944	0.313	
8.950	1.7410	13.601	0.354	
9.500	1.7690	14.567	0.399	
9.600	1.7740	14.744	0.714	
9.800	1.7850	15.100	2.020	
10.300	1.8110	15.999	3.840	
10.900	1.8430	17.095	4.960	

END FTABLE237

\*\*\* SDN-3X: 3RD RUNWAY NORTH VAULT (LEVEL 2): \*\*\*

FTABLE 43

ROWS COLS \*\*\* EFFECTIVE DEPTH=20 FT RISER DIA=24 INCHES

21 4

DEPTH	AREA	VOLUME	FLOW	***
(FT)	(ACRES)	(ACRE-FT)	(FT3/S)	***
0.00	1.288	0.00	0.00	
0.14	1.288	0.180	0.067	
1.39	1.288	1.790	0.216	
3.35	1.288	4.314	0.336	
5.31	1.288	6.839	0.423	
8.06	1.288	10.380	0.521	
8.84	1.288	11.385	0.545	
10.02	1.288	12.905	0.580	
11.98	1.288	15.429	0.635	
12.37	1.288	15.931	0.645	
14.00	1.288	18.030	0.686	
14.10	1.288	18.159	0.705	
14.91	1.288	19.202	0.757	
16.09	1.288	20.722	0.810	
18.00	1.288	23.182	0.881	
18.32	1.288	23.594	1.150	
18.76	1.288	24.161	1.360	
20.00	1.288	25.758	1.680	
20.10	1.288	25.886	2.320	
20.50	1.288	26.402	8.620	
20.80	1.288	26.788	15.370	

END FTABLE 43

\*\*\* SDN-4X/2X: 3RD RUNWAY NORTH VAULT

(COMBINED FACILITY)

AR 011444

FTABLE 51  
 ROWS COLS \*\*\* EFFECTIVE DEPTH=19FT RISER DIA=24 INCHES

DEPTH (FT)	AREA (ACRES)	VOLUME (ACRE-FT)	OUTFLOW (FT3/S)	***
0.00	0.789	0.000	0.000	***
0.16	0.789	0.126	0.056	***
1.51	0.789	1.192	0.169	
3.28	0.789	2.588	0.249	
5.49	0.789	4.332	0.322	
7.26	0.789	5.729	0.370	
10.35	0.789	8.168	0.442	
12.12	0.789	9.564	0.478	
13.44	0.789	10.606	0.503	
14.33	0.789	11.308	0.520	
15.57	0.789	12.287	0.654	
16.72	0.789	13.194	0.828	
17.19	0.789	13.565	0.950	
17.63	0.789	13.913	1.030	
18.00	0.789	14.205	1.080	
19.00	0.789	14.994	1.960	
19.10	0.789	15.073	2.580	
19.40	0.789	15.309	6.930	
19.60	0.789	15.467	11.080	
20.00	0.789	15.783	17.190	

END FTABLE 51

\*\*\* SDW-1B:3RD RUNWAY CENTRAL SOUTH POND D TO MILLER CREEK (LEVEL 2): \*\*\*

FTABLE 57  
 ROWS COLS \*\*\* EFFECTIVE DEPTH = 14.0 FT

DEPTH (FT)	AREA (ACRES)	VOLUME (ACRE-FT)	STORMQ (FT3/S)	***
0.00	2.430	0.000	0.000	***
0.01	2.430	0.041	0.010	***
1.00	2.680	2.411	0.183	
2.00	2.760	4.860	0.257	
3.00	2.818	7.370	0.319	
4.00	3.079	9.945	0.366	
5.00	5.832	15.320	0.411	
6.00	5.927	20.742	0.450	
7.00	6.022	26.264	0.481	
8.00	6.118	31.888	0.518	
9.00	6.210	37.613	0.550	
10.00	6.311	43.441	0.583	
11.00	6.408	49.372	0.609	
12.00	6.607	55.406	0.634	
13.00	6.405	61.543	0.764	
14.00	6.504	67.786	1.320	
15.00	7.000	70.000	16.600	

END FTABLE 57

\*\*\* SDW-1B:3RD RUNWAY CENTRAL SOUTH POND D TO MILLER CREEK (LEVEL 2): \*\*\*

FTABLE 257  
 \*\*\* PROJECT SDW1B EFFECTIVE DIAMETER=3.0 FT  
 ROWS COLS \*\*\* INFILTRATION TANK TO OBTAIN 0.2 CFS

15 5

DEPTH	AREA	VOLUME	STORMQ	INFILTRQ ***
0.000	0.001	0.000	0.000	0.000
0.010	0.001	0.001	0.000	0.002
0.250	0.002	0.002	0.000	0.017
0.500	0.004	0.004	0.000	0.035
1.000	0.012	0.012	0.000	0.071
1.500	0.020	0.020	0.000	0.106
2.000	0.029	0.029	0.000	0.142
2.500	0.036	0.036	0.000	0.178
3.000	0.041	0.0406	0.000	0.213
3.100	0.041	0.0420	0.596	0.220
3.200	0.041	0.0421	1.685	0.227
3.300	0.041	0.0422	3.096	0.233
3.400	0.041	0.0423	4.766	0.241
3.500	0.041	0.0424	6.661	0.248
3.750	0.041	0.0425	12.237	0.266

END FTABLE257

FTABLE 570  
 \*\*\* PROJECT SDW1B FLOW SPLITTER (to 257 and 57)  
 ROWS COLS \*\*\*  
 15 5

DEPTH	AREA	VOLUME	STORMQ	INFILTRQ ***
0.000	0.00	0.000	0.000	0.000
0.100	0.01	0.0002	0.000	0.050
0.400	0.01	0.0009	0.000	0.110
0.600	0.01	0.0014	0.000	0.130
0.750	0.01	0.0017	0.000	0.150
0.800	0.01	0.0018	0.720	0.150
1.000	0.01	0.0023	8.050	0.170
1.100	0.01	0.0025	13.330	0.180
1.200	0.01	0.0027	19.440	0.190
1.300	0.01	0.0030	26.270	0.190
1.400	0.01	0.0032	33.750	0.200
1.420	0.01	0.0033	35.320	0.200
1.440	0.01	0.0033	36.910	0.200
1.450	0.01	0.0034	37.920	0.200
1.460	0.01	0.0035	38.530	0.200

END FTABLE570

FTABLE 61  
 ROWS COLS \*\*\*  
 \*\*\* SDN-2X: DETAIN OVERFLOW FROM NCPS AND NSMPS-  
 17 4

DEPTH	AREA	VOLUME	OUTFLOW ***
0.000	0.5740	0.0000	0.00
1.200	0.5740	0.7710	0.151
2.220	0.5740	1.4270	0.205
3.240	0.5740	2.0830	0.247
3.650	0.5740	2.3460	0.262
4.260	0.5740	2.7380	0.283
4.660	0.5740	2.9950	0.296
5.680	0.5740	3.6510	0.327
6.640	0.5740	4.2680	0.517
7.650	0.5740	4.9170	0.644
8.670	0.5740	5.9710	0.739
9.810	0.5740	6.3570	0.836



10.700	0.5740	6.8780	0.894
12.000	0.5740	7.7130	0.978
12.100	0.5740	7.7780	1.600
12.300	0.5740	7.9060	4.200
12.800	0.5740	8.2280	14.560

END FTABLE 61

PRE AMBAUM DETENTION \*\*\*

FTABLE 111

ROWS COLS \*\*\*

15 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.500	0.2160	0.0750	5.30	
1.000	0.2730	0.1990	21.10	
1.500	0.2890	0.3410	43.90	
2.000	0.2900	0.4830	68.80	
2.500	0.2910	0.6070	89.10	
3.000	0.2950	0.6820	90.00	
3.500	0.3000	2.1000	100.00	
4.000	0.3050	2.5000	105.00	
4.500	0.3100	3.0000	110.00	
5.000	0.3200	3.5000	120.00	
5.500	0.3300	4.0000	130.00	
6.000	0.3800	5.0530	166.48	
6.500	0.3980	5.9430	225.31	
7.000	0.4150	6.9040	320.10	

END FTABLE111

FTABLE 135

ROWS COLS \*\*\* VACA FARM

6 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.00	0.10	0.00	0.00	
1.00	0.10	0.10	4.00	
2.00	0.11	0.21	8.00	
2.50	1.00	0.48	13.00	
3.50	6.50	4.23	86.00	
4.50	13.00	13.98	235.00	

END FTABLE135

FTABLE 240

\*\*\* NORTH SNOWMELT PUMP STATION (SDN-2) (INSTALLED IN LATE 1997/1998)

ROWS COLS

14 5

DEPTH	AREA	VOLUME	(IWS)	(SDS)	***
(FT)	(ACRES)	(ACRE-FT)	(CFS)	(CFS)	***
0.0	0.002	0.00	0.00	0.00	***
1.00	0.002	0.0023	0.00	0.00	
2.00	0.002	0.0046	1.67	0.00	
3.00	0.002	0.0069	1.67	0.00	
4.00	0.002	0.0092	1.67	0.00	
5.00	0.002	0.0115	1.67	0.00	
5.25	0.002	0.0121	1.67	1.53	
5.50	0.002	0.0126	1.67	6.06	
5.75	0.002	0.0132	1.67	12.65	
6.00	0.002	0.0138	1.67	19.83	

6.25	0.002	0.0144	1.67	25.66
6.50	0.002	0.0149	1.67	25.70
6.75	0.002	0.0155	1.67	26.70
7.00	0.002	0.0161	1.67	50.00

END FTABLE240

FTABLE 242

\*\*\* NORTH CARGO PUMP STATION (SDN-2) (INSTALLED IN OCTOBER 1997)

ROWS COLS

14 5

DEPTH (FT)	AREA (ACRES)	VOLUME (ACRE-FT)	(IWS) (CFS)	(SDS) (CFS)
0.0	0.002	0.00	0.00	0.00
1.00	0.002	0.0023	0.00	0.00
2.00	0.002	0.0046	6.13	0.00
3.00	0.002	0.0069	6.13	0.00
4.00	0.002	0.0092	6.13	0.00
5.00	0.002	0.0115	6.13	0.00
5.25	0.002	0.0121	6.13	0.28
5.50	0.002	0.0126	6.13	1.16
5.75	0.002	0.0132	6.13	2.53
6.00	0.002	0.0138	6.13	4.23
6.25	0.002	0.0144	6.13	6.05
6.50	0.002	0.0149	6.13	7.72
6.75	0.002	0.0155	6.13	8.50
7.00	0.002	0.0161	6.13	20.0

END FTABLE242

END FTABLES

MASS-LINK

<Volume> <Name>	<-Grp>	<-Member->	<--Mult-->	<Target> <Name>	<-Grp>	<-Member->	**** <Name> # #****
conversion from acre-inches to acre-ft (1/12)		1					***
PERLND PWATER PERO			0.0833333	RCHRES			INFLOW IVOL
END MASS-LINK		1					
MASS-LINK		2					
IMPLND IWATER SURO			0.0833333	RCHRES			INFLOW IVOL
END MASS-LINK		2					
MASS-LINK		3					
RCHRES ROFLOW				RCHRES			INFLOW
END MASS-LINK		3					
MASS-LINK		4					
RCHRES OFLOW OVOL		1		RCHRES			INFLOW IVOL
END MASS-LINK		4					
MASS-LINK		5					
RCHRES OFLOW OVOL		2		RCHRES			INFLOW IVOL
END MASS-LINK		5					
MASS-LINK		6					
PERLND PWATER SURO			0.0833333	RCHRES			INFLOW IVOL
PERLND PWATER IFWO			0.0833333	RCHRES			INFLOW IVOL

AR 011448

```

END MASS-LINK      6

MASS-LINK          7
PERLND    PWATER AGWO      0.0833333  RCHRES      INFLOW IVOL
END MASS-LINK      7

MASS-LINK          10
COPY      OUTPUT MEAN      COPY      INPUT  MEAN
END MASS-LINK      10

MASS-LINK          11
RCHRES    ROFLOW      COPY      INPUT  MEAN
END MASS-LINK      11

MASS-LINK          12
COPY      OUTPUT MEAN      RCHRES      INFLOW IVOL
END MASS-LINK      12

MASS-LINK          14
RCHRES    OFLOW  OVOL      1      COPY      INPUT  MEAN
END MASS-LINK      14

MASS-LINK          15
RCHRES    OFLOW  OVOL      2      COPY      INPUT  MEAN
END MASS-LINK      15

MASS-LINK          21
PERLND    PWATER PERO      0.0833333  COPY      INPUT  MEAN
END MASS-LINK      21

MASS-LINK          22
IMPLND    IWATER SURO      0.0833333  COPY      INPUT  MEAN
END MASS-LINK      22

MASS-LINK          26
PERLND    PWATER SURO      0.0833333  COPY      INPUT  MEAN
PERLND    PWATER IFWO      0.0833333  COPY      INPUT  MEAN
END MASS-LINK      26

MASS-LINK          27
PERLND    PWATER AGWO      0.0833333  COPY      INPUT  MEAN
END MASS-LINK      27

```

END MASS-LINK

COPY

TIMESERIES

Copy-opn

#	-	#	NPT	NMN
37		71		1
240		242		1
357		357		1
645		645		1

END TIMESERIES

END COPY

END RUN

\*\*\*  
\*\*\*

RUN

GLOBAL

\*\*\* FILE: mill65F.inp REVISED Aug 2000 Joe Brascher(atc)  
\*\*\* for parameterix  
\*\*\* SEATAC AIRPORT HSPF BASIN MODEL OF MILLER CREEK  
\*\*\* - POST-MILLER CK DETENTION FACIITY 10/92-6/93  
\*\*\* m23 AND M24 new area west of m2. Flows to rdf  
\*\*\* Calibration file run for full length of record  
MILLER CREEK BASIN HSPF MODEL  
START 1948 10 1 0 0 END 1996 9 30 24 0  
RUN INTERP OUTPUT LEVEL 3  
RESUME 0 RUN 1

END GLOBAL

FILES

<type> <fun>\*\*\*<-----fname----->  
MESSU 24 D:\PARA\SEATAC\MILLER\lowflow\MILL.MES  
WDM 25 D:\PARA\SEATAC\MILLER\lowflow\Mlowflow.wdm  
61 D:\PARA\SEATAC\MILLER\lowflow\PER.L61  
62 D:\PARA\SEATAC\MILLER\lowflow\RCH.L62

END FILES

OPN SEQUENCE

INGRP INDELT 01:00  
PERLND 14  
PERLND 16  
PERLND 18  
PERLND 24  
PERLND 26  
PERLND 28  
PERLND 34  
PERLND 44  
PERLND 54  
IMPLND 14  
RCHRES 1  
RCHRES 23  
RCHRES 24  
RCHRES 2  
RCHRES 3  
RCHRES 33  
RCHRES 4  
RCHRES 5  
RCHRES 50  
RCHRES 52  
RCHRES 53  
RCHRES 54  
RCHRES 34  
RCHRES 135  
RCHRES 35  
RCHRES 10  
RCHRES 16  
RCHRES 11  
RCHRES 13  
RCHRES 12  
RCHRES 15  
RCHRES 14  
RCHRES 17

AR 011450

END INGRP  
END OPN SEQUENCE

\*\*\*

COPY  
TIMESERIES  
Copy-opn  
# - # NPT NMN  
1 5 1  
END TIMESERIES  
END COPY

\*\*\*  
\*\*\*

PERLND  
GEN-INFO  
<PLS > Name NBLKS Unit-systems Printer  
# - # User t-series Engl Metr  
in out  
14 TFF- TILL FOR FLT 1 1 1 1 61 0  
16 TFM- TILL FOR MOD 1 1 1 1 61 0  
18 TFS- TILL FOR STP 1 1 1 1 61 0  
24 TGF- TILL GR FLT 1 1 1 1 61 0  
26 TGM- TILL GR MOD 1 1 1 1 61 0  
28 TGS- TILL GR STP 1 1 1 1 61 0  
34 OF - OUTWASH FOR 1 1 1 1 61 0  
44 OG - OUTWASH GR 1 1 1 1 61 0  
\*\*\*PERLND FOR NEW AIRPORT FILL; NONE IN CALIBRATION  
45 AIRPORT FILL 1 1 1 1 61 0  
54 SA - WETLANDS 1 1 1 1 61 0  
64 RES- GROUNDWATER 1 1 1 1 61 0

\*\*\*  
\*\*\*  
\*\*\*

END GEN-INFO

ACTIVITY  
<PLS > \*\*\*\*\* Active Sections \*\*\*\*\*  
# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC  
14 200 0 0 1 0 0 0 0 0 0 0 0 0 0

\*\*\*

END ACTIVITY

PRINT-INFO  
<PLS > \*\*\*\*\* Print-flags \*\*\*\*\* PIVL PYR  
# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC \*\*\*\*\*  
14 200 0 0 5 0 0 0 0 0 0 0 0 0 1 9

END PRINT-INFO

PWAT-PARM1  
<PLS > \*\*\*\*\* Flags \*\*\*\*\*  
# - # CSNO RTOP UZFG VCS VUZ VNN VIFW VIRC VLE  
14 0 0 0 0 0 0 0 0 0  
16 0 0 0 0 0 0 0 0 0  
18 0 0 0 0 0 0 0 0 0  
24 0 0 0 0 0 0 0 0 0  
26 0 0 0 0 0 0 0 0 0  
28 0 0 0 0 0 0 0 0 0  
34 0 0 0 0 0 0 0 0 0  
44 0 0 0 0 0 0 0 0 0  
45 0 0 0 0 0 0 0 0 0  
54 0 0 0 0 0 0 0 0 0  
64 0 0 0 0 0 0 0 0 0

\*\*\*

END PWAT-PARM1

PWAT-PARM2  
<PLS > \*\*\*  
# - # \*\*\*FOREST LZSN INFILT LSUR SLSUR KVARV AGWRC

AR 011451

14	9.0000	0.3200	400.00	0.0500	0.5000	0.9960
16	9.0000	0.3200	400.00	0.1000	0.5000	0.9960
18	9.0000	0.3200	200.00	0.2000	0.5000	0.9960
24	9.0000	0.1200	400.00	0.0500	0.5000	0.9960
26	9.0000	0.1200	400.00	0.1000	0.5000	0.9960
28	9.0000	0.1200	200.00	0.2000	0.5000	0.9960
34	10.0000	2.0000	400.00	0.0500	0.3000	0.9960
44	10.0000	0.8000	400.00	0.0500	0.3000	0.9960
45	7.5000	0.0200	300.00	0.0700	0.0000	0.9000
54	8.0000	2.0000	100.00	0.0010	0.5000	0.9960

END PWAT-PARM2

PWAT-PARM3

<PLS >\*\*\*

# - #***	PETMAX	PETMIN	INFEXP	INFILD	DEEPFR	BASETP	AGWETP
14			2.0000	2.0000	0.33	0.00	0.0
16			2.0000	2.0000	0.33	0.00	0.0
18			2.0000	2.0000	0.33	0.00	0.0
24			2.0000	2.0000	0.33	0.00	0.
26			2.0000	2.0000	0.33	0.	0.
28			2.0000	2.0000	0.33	0.	0.
34			2.0000	2.0000	0.33	0.00	0.0
44			2.0000	2.0000	0.33	0.	0.
54			10.000	2.0000	0.33	0.	0.7

END PWAT-PARM3

PWAT-PARM4

<PLS >

# - #	CEPSC	UZSN	NSUR	INTFW	IRC	LZETP***
14	0.2000	1.5000	0.3500	9.000	0.7000	0.7000
16	0.2000	0.7500	0.3500	9.000	0.7000	0.7000
18	0.2000	0.4500	0.3500	9.000	0.3000	0.7000
24	0.1000	0.7500	0.2500	9.000	0.7000	0.2500
26	0.1000	0.3750	0.2500	9.000	0.7000	0.2500
28	0.1000	0.2250	0.2500	9.000	0.3000	0.2500
34	0.2000	0.7500	0.3500	0.000	0.7000	0.7000
44	0.1000	0.7500	0.2500	0.000	0.7000	0.2500
54	0.1000	2.2500	0.5000	1.000	0.7000	0.8000

END PWAT-PARM4

PWAT-STATE1

<PLS > PWATER state variables\*\*\*

# - #***	CEPS	SURS	UZS	IFWS	LZS	AGWS	GWVS
14	0.078	0.	0.2500	0.10	2.000	2.000	0.000
16	0.078	0.	0.2500	0.10	2.000	2.000	0.000
18	0.078	0.	0.2500	0.10	2.000	2.000	0.000
24	0.051	0.	0.2500	0.10	2.000	2.000	0.000
26	0.051	0.	0.2500	0.10	2.000	2.000	0.000
28	0.051	0.	0.2500	0.10	2.000	2.000	0.000
34	0.078	0.	0.2500	0.10	2.000	2.000	0.000
44	0.051	0.	0.2500	0.10	2.000	2.000	0.000
45	0.051	0.	0.2500	0.10	2.000	2.000	0.000
54	0.051	0.	0.2500	0.10	2.000	2.000	0.000
64	0.051	0.	0.2500	0.10	2.000	2.000	0.000

END PWAT-STATE1

END PERLND

IMPLND

GEN-INFO

<ILS >	Name	Unit-systems	Printer	***
# - #		User t-series	Engl Metr	***

```

                                in out
14      IMPERVIOUS              1   1   1  60   0
END GEN-INFO
ACTIVITY
<ILS > ***** Active Sections ****
# - # ATMP SNOW IWAT  SLD  IWG IQAL  ***
14      0   0   1   0   0   0
END ACTIVITY
PRINT-INFO
<ILS > ***** Print-flags ***** PIVL  PYR
# - # ATMP SNOW IWAT  SLD  IWG IQAL *****
14      0   0   6   0   0   0   1   9
END PRINT-INFO
IWAT-PARM1
<ILS >              Flags              ***
# - # CSNO RTOP  VRS  VNN RTLI  ***
14      0   0   0   0   0
END IWAT-PARM1
IWAT-PARM2
<ILS >              ***
# - #          LSUR      SLSUR      NSUR      RETSC      ***
14      100.00   0.0100   0.1000   0.1000
END IWAT-PARM2
IWAT-PARM3
<ILS >              ***
# - #          PETMAX    PETMIN
14
END IWAT-PARM3
IWAT-STATE1
<ILS >  IWATER state variables      ***
# - #          RETS      SURS      ***
14      1.0000E-3  1.0000E-3
END IWAT-STATE1
END IMPLND
EXT SOURCES
*** NOTE: The only RCHRES that precip and PET are applied to are lakes.
<-Volume-> <Member> SsysSgap<--Mult-->Tran <-Target vols> <-Grp> <-Member->
<Name>   # <Name> # tem strg<-factor->strg <Name>   #   #   <Name> # #
*** PRECIP/EVAP TO PERVIOUS/IMPERV SURFACES
WDM  1002 PREC      ENGLZERO  1.00      PERLND  14 200 EXTNL  PREC
WDM  1002 PREC      ENGLZERO  1.00      IMPLND  14      EXTNL  PREC
WDM   1  EVAP      ENGLZERO  0.8       PERLND  14 18  EXTNL  PETINP
WDM   1  EVAP      ENGLZERO  0.8       PERLND  24 28  EXTNL  PETINP
WDM   1  EVAP      ENGLZERO  0.8       PERLND  34 54  EXTNL  PETINP
WDM   1  EVAP      ENGLZERO  0.8       PERLND  64      EXTNL  PETINP
WDM   1  EVAP      ENGLZERO  0.8       IMPLND  14      EXTNL  PETINP
*** PRECIP/EVAP TO LAKES
WDM  1002 PREC      ENGLZERO      RCHRES  1      EXTNL  PREC
WDM   1  EVAP      ENGLZERO  0.8      RCHRES  1      EXTNL  POTEV
WDM  1002 PREC      ENGLZERO      RCHRES  4      EXTNL  PREC
WDM   1  EVAP      ENGLZERO  0.8      RCHRES  4      EXTNL  POTEV
WDM   1  EVAP      ENGLZERO  0.8      RCHRES  1      EXTNL  POTEV
WDM  1002 PREC      ENGLZERO      RCHRES  1      EXTNL  PREC
WDM   1  EVAP      ENGLZERO  0.8      RCHRES  4      EXTNL  POTEV

```

WDM	1002	PREC	ENGLZERO		RCHRES	4	EXTNL	PREC
WDM	1002	PREC	ENGLZERO		RCHRES	11	EXTNL	PREC
WDM	1	EVAP	ENGLZERO	0.8	RCHRES	11	EXTNL	POTEV
WDM	1002	PREC	ENGLZERO		RCHRES	13	EXTNL	PREC
WDM	1	EVAP	ENGLZERO	0.8	RCHRES	13	EXTNL	POTEV
WDM	1002	PREC	ENGLZERO		RCHRES	23	EXTNL	PREC
WDM	1	EVAP	ENGLZERO	0.8	RCHRES	23	EXTNL	POTEV
WDM	1002	PREC	ENGLZERO		RCHRES	34	EXTNL	PREC
WDM	1	EVAP	ENGLZERO	0.8	RCHRES	34	EXTNL	POTEV
WDM	1002	PREC	ENGLZERO		RCHRES	53	EXTNL	PREC
WDM	1	EVAP	ENGLZERO	0.8	RCHRES	53	EXTNL	POTEV
WDM	1002	PREC	ENGLZERO		RCHRES	54	EXTNL	PREC
WDM	1	EVAP	ENGLZERO	0.8	RCHRES	54	EXTNL	POTEV

END EXT SOURCES

EXT TARGETS

```

<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Volume-> <Member> Tsys Tgap Amd ***
<Name> # <Name> # <-factor->strg <Name> # <Name> tem strg strg***
*** UPPER MILLER CREEK GROUNDWATER PUMPING
COPY *** 1 OUTPUT MEAN 1 12.1 WDM 18 FLOW ENGL REPL
*** GAUGE POINTS (17=MOOUTH, 54=MILLER RDF, 50=SR 518, 18=WALKER CK)
RCHRES 35 HYDR RO WDM 7035 FLOW ENGL REPL
RCHRES 17 HYDR RO WDM 33 FLOW ENGL REPL
RCHRES 54 HYDR RO WDM 34 FLOW ENGL REPL
RCHRES 50 HYDR RO *** WDM 35 FLOW ENGL REPL
RCHRES 18 HYDR RO *** WDM 36 FLOW ENGL REPL
*** MISC (20=WALKER WETLAND, 55=SR509, 56=1ST AVE, 1=ARBOR LAKE)
RCHRES 23 HYDR STAGE *** WDM 91 STAG ENGL REPL
RCHRES 20 HYDR RO *** WDM 37 FLOW ENGL REPL
RCHRES 55 HYDR RO *** WDM 38 FLOW ENGL REPL
RCHRES 62 HYDR RO *** WDM 39 FLOW ENGL REPL
RCHRES 1 HYDR RO *** WDM 80 FLOW ENGL REPL
<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Volume-> <Member> Tsys Tgap Amd ***
<Name> # <Name> # <-factor->strg <Name> # <Name> tem strg strg***
***MOOUTH
RCHRES 54 HYDR RO 1 1 0.000419 *** WDM 60 SIMQ ENGL REPL
RCHRES 17 HYDR RO 1 1 0.000213 *** WDM 70 SIMQ ENGL REPL
END EXT TARGETS

```

\*\*\*

SCHEMATIC

```

<-Source-> <--Area--> <-Target-> MBLK ***
<Name> # <-factor-> <Name> # Tbl# ***
*** SUB-CATCHMENT 1 all agwo goes to sound
PERLND 16 3.41 RCHRES 1 6
PERLND 26 232.36 RCHRES 1 6
PERLND 34 3.07 RCHRES 1 6
PERLND 44 38.03 RCHRES 1 6
PERLND 54 3.87 RCHRES 1 6
IMPLND 14 56.14 RCHRES 1 2
*** SUB-CATCHMENT 2 10% of area GW goes to vaca 90% goes to sound
PERLND 16 5.56 RCHRES 2 6
PERLND 26 200.05 RCHRES 2 6
PERLND 34 0.46 RCHRES 2 6
PERLND 44 38.71 RCHRES 2 6
PERLND 16 0.56 RCHRES 135 7
PERLND 26 20.01 RCHRES 135 7

```



PERLND	34	0.05	RCHRES	135	7
PERLND	44	3.87	RCHRES	135	7
IMPLND	14	42.22	RCHRES	2	2
*** SUB-CATCHMENT 23 New subbasin 15 % OF GW GOES TO VACCA 85% TO SOUND					
PERLND	16	3.09	RCHRES	23	6
PERLND	26	156.15	RCHRES	23	6
PERLND	34	2.25	RCHRES	23	6
PERLND	44	45.84	RCHRES	23	6
PERLND	16	0.46	RCHRES	135	7
PERLND	26	23.42	RCHRES	135	7
PERLND	34	0.34	RCHRES	135	7
PERLND	44	6.88	RCHRES	135	7
IMPLND	14	58.44	RCHRES	23	2
*** SUB-CATCHMENT 24 New subbasin 60 % OF GW GOES TO 11 40% TO SOUND					
PERLND	26	135.43	RCHRES	24	6
PERLND	34	2.02	RCHRES	24	6
PERLND	44	69.29	RCHRES	24	6
PERLND	26	81.26	RCHRES	11	7
PERLND	34	1.21	RCHRES	11	7
PERLND	44	41.57	RCHRES	11	7
IMPLND	14	79.98	RCHRES	24	2
*** SUB-CATCHMENT 3 agwo goes to vaca(135)					
PERLND	16	8.26	RCHRES	3	6
PERLND	26	108.38	RCHRES	3	6
PERLND	34	16.02	RCHRES	3	6
PERLND	44	102.89	RCHRES	3	6
PERLND	54	0.04	RCHRES	3	6
PERLND	16	8.26	RCHRES	135	7
PERLND	26	108.38	RCHRES	135	7
PERLND	34	16.02	RCHRES	135	7
PERLND	44	102.89	RCHRES	135	7
PERLND	54	0.04	RCHRES	135	7
IMPLND	14	27.30	RCHRES	3	2
*** SUB-CATCHMENT 4 10% of agwo goes to rchres 90% goes to sound					
PERLND	16	2.95	RCHRES	4	6
PERLND	26	85.95	RCHRES	4	6
PERLND	34	3.75	RCHRES	4	6
PERLND	44	92.06	RCHRES	4	6
PERLND	16	0.30	RCHRES	4	7
PERLND	26	8.60	RCHRES	4	7
PERLND	34	0.38	RCHRES	4	7
PERLND	44	9.21	RCHRES	4	7
IMPLND	14	18.43	RCHRES	4	2
*** SUB-CATCHMENT 4a 70% of agwo goes to rchres 30% goes to sound					
PERLND	16	8.66	RCHRES	4	6
PERLND	26	61.64	RCHRES	4	6
PERLND	34	22.06	RCHRES	4	6
PERLND	44	78.09	RCHRES	4	6
PERLND	54	12.50	RCHRES	4	6
PERLND	16	6.06	RCHRES	4	7
PERLND	26	43.15	RCHRES	4	7
PERLND	34	15.44	RCHRES	4	7
PERLND	44	54.66	RCHRES	4	7
PERLND	54	8.75	RCHRES	4	7
IMPLND	14	29.14	RCHRES	4	2
*** SUB-CATCHMENT 5					
PERLND	26	10.29	RCHRES	5	1

PERLND	44	50.04	RCHRES	5	1
PERLND	54	10.74	RCHRES	5	1
IMPLND	14	16.31	RCHRES	5	2
*** SUB-CATCHMENT 6					
PERLND	16	10.66	RCHRES	53	1
PERLND	26	41.08	RCHRES	53	1
PERLND	34	21.75	RCHRES	53	1
PERLND	44	13.39	RCHRES	53	1
PERLND	54	0.82	RCHRES	53	1
IMPLND	14	7.14	RCHRES	53	2
*** SUB-CATCHMENT 8					
PERLND	44	22.21	RCHRES	35	1
IMPLND	14	6.60	RCHRES	35	2
*** SUB-CATCHMENT 9					
PERLND	16	4.94	RCHRES	34	1
PERLND	26	14.32	RCHRES	34	1
PERLND	34	0.05	RCHRES	34	1
PERLND	44	56.70	RCHRES	34	1
PERLND	54	0.01	RCHRES	34	1
IMPLND	14	22.46	RCHRES	34	2
*** SUB-CATCHMENT 10					
PERLND	16	4.15	RCHRES	10	1
PERLND	26	31.94	RCHRES	10	1
PERLND	44	95.23	RCHRES	10	1
IMPLND	14	71.97	RCHRES	10	2
*** SUB-CATCHMENT 11 25% OF AGWO GOES TO 15					
PERLND	16	0.89	RCHRES	11	6
PERLND	26	217.92	RCHRES	11	6
PERLND	34	1.32	RCHRES	11	6
PERLND	44	65.65	RCHRES	11	6
PERLND	16	0.67	RCHRES	11	7
PERLND	26	163.44	RCHRES	11	7
PERLND	34	0.99	RCHRES	11	7
PERLND	44	49.24	RCHRES	11	7
PERLND	16	0.22	RCHRES	15	7
PERLND	26	54.48	RCHRES	15	7
PERLND	34	0.33	RCHRES	15	7
PERLND	44	16.41	RCHRES	15	7
IMPLND	14	230.80	RCHRES	11	2
*** SUB-CATCHMENT 12					
PERLND	16	0.39	RCHRES	12	1
PERLND	26	101.18	RCHRES	12	1
PERLND	34	5.64	RCHRES	12	1
PERLND	44	54.98	RCHRES	12	1
PERLND	54	0.64	RCHRES	12	1
IMPLND	14	79.83	RCHRES	12	2
*** SUB-CATCHMENT 13					
PERLND	16	0.79	RCHRES	13	1
PERLND	26	197.68	RCHRES	13	1
IMPLND	14	27.66	RCHRES	13	2
*** SUB-CATCHMENT 14 50% OF AGWO GOES TO SOUND					
PERLND	16	0.24	RCHRES	14	6
PERLND	26	118.67	RCHRES	14	6
PERLND	34	13.46	RCHRES	14	6
PERLND	44	41.91	RCHRES	14	6
PERLND	16	0.12	RCHRES	14	7
PERLND	26	59.34	RCHRES	14	7

PERLND	34	6.73	RCHRES	14	7
PERLND	44	20.95	RCHRES	14	7
IMPLND	14	20.66	RCHRES	14	2
*** SUB-CATCHMENT 15					
PERLND	16	6.59	RCHRES	15	1
PERLND	26	49.55	RCHRES	15	1
PERLND	34	50.09	RCHRES	15	1
PERLND	44	86.52	RCHRES	15	1
IMPLND	14	19.47	RCHRES	15	2
*** SUB-CATCHMENT 16					
PERLND	16	10.93	RCHRES	16	1
PERLND	26	30.30	RCHRES	16	1
PERLND	34	20.03	RCHRES	16	1
PERLND	44	31.42	RCHRES	16	1
IMPLND	14	15.54	RCHRES	16	2
*** SUB-CATCHMENT 17 AGWO GOES TO SOUND					
PERLND	16	0.90	RCHRES	17	6
PERLND	26	16.31	RCHRES	17	6
PERLND	34	34.82	RCHRES	17	6
PERLND	44	82.11	RCHRES	17	6
PERLND	54	2.19	RCHRES	17	6
IMPLND	14	10.49	RCHRES	17	2
*** SUB-CATCHMENT MC-1					
PERLND	26	0.17	RCHRES	52	1
PERLND	44	8.21	RCHRES	52	1
PERLND	54	0.27	RCHRES	52	1
IMPLND	14	0.09	RCHRES	52	2
*** SUB-CATCHMENT MC-2					
PERLND	16	0.08	RCHRES	53	1
PERLND	26	0.64	RCHRES	53	1
PERLND	34	6.72	RCHRES	53	1
PERLND	44	10.43	RCHRES	53	1
PERLND	54	15.25	RCHRES	53	1
IMPLND	14	0.27	RCHRES	53	2
*** SUB-CATCHMENT MC-3					
PERLND	34	5.44	RCHRES	54	1
PERLND	44	5.03	RCHRES	54	1
PERLND	54	2.28	RCHRES	54	1
IMPLND	14	0.11	RCHRES	54	2
*** SUB-CATCHMENT MC-4					
PERLND	44	17.32	RCHRES	135	1
PERLND	54	14.41	RCHRES	135	1
IMPLND	14	1.77	RCHRES	135	2
*** SUB-CATCHMENT MC-5					
PERLND	26	13.49	RCHRES	35	1
PERLND	44	31.06	RCHRES	35	1
PERLND	54	5.95	RCHRES	35	1
IMPLND	14	2.50	RCHRES	35	2
*** SUB-CATCHMENT MC-6					
PERLND	44	17.75	RCHRES	35	1
PERLND	54	6.54	RCHRES	35	1
IMPLND	14	0.95	RCHRES	35	2
*** SUB-CATCHMENT MC-6B					
PERLND	26	34.94	RCHRES	35	1
PERLND	34	7.81	RCHRES	35	1
PERLND	44	52.91	RCHRES	35	1
PERLND	54	4.61	RCHRES	35	1

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IMPLND	14	3.14	RCHRES	35	2
*** SUB-CATCHMENT MC-7					
PERLND	26	12.66	RCHRES	16	1
PERLND	44	33.53	RCHRES	16	1
PERLND	54	4.16	RCHRES	16	1
IMPLND	14	3.88	RCHRES	16	2
*** SUB-CATCHMENT MC-7B					
PERLND	26	36.16	RCHRES	16	1
PERLND	44	8.46	RCHRES	16	1
PERLND	54	1.92	RCHRES	16	1
IMPLND	14	2.12	RCHRES	16	2
***all sdn basin agwo goes to 35					
*** SUB-CATCHMENT SDN-1					
PERLND	26	3.23	RCHRES	52	6
PERLND	44	2.11	RCHRES	52	6
PERLND	54	0.20	RCHRES	52	6
PERLND	26	3.23	RCHRES	135	7
PERLND	44	2.11	RCHRES	135	7
PERLND	54	0.20	RCHRES	135	7
IMPLND	14	8.29	RCHRES	52	2
*** SUB-CATCHMENT SDN-1-LWR					
PERLND	44	4.97	RCHRES	52	6
PERLND	54	0.07	RCHRES	52	6
PERLND	44	4.97	RCHRES	135	7
PERLND	54	0.07	RCHRES	135	7
IMPLND	14	0.38	RCHRES	52	2
*** SUB-CATCHMENT SDN-1-OFF					
PERLND	26	29.12	RCHRES	52	6
PERLND	44	3.62	RCHRES	52	6
PERLND	54	1.67	RCHRES	52	6
PERLND	26	29.12	RCHRES	135	7
PERLND	44	3.62	RCHRES	135	7
PERLND	54	1.67	RCHRES	135	7
IMPLND	14	11.50	RCHRES	52	2
*** SUB-CATCHMENT SDN-2					
PERLND	26	10.41	RCHRES	52	6
PERLND	44	3.04	RCHRES	52	6
PERLND	26	10.41	RCHRES	135	7
PERLND	44	3.04	RCHRES	135	7
IMPLND	14	33.22	RCHRES	52	2
*** SUB-CATCHMENT SDN-2X					
PERLND	26	1.37	RCHRES	52	6
PERLND	44	5.84	RCHRES	52	6
PERLND	26	1.37	RCHRES	135	7
PERLND	44	5.84	RCHRES	135	7
IMPLND	14	0.28	RCHRES	52	2
*** SUB-CATCHMENT SDN-3					
PERLND	26	49.79	RCHRES	54	6
PERLND	26	49.79	RCHRES	135	7
IMPLND	14	15.82	RCHRES	54	2
*** SUB-CATCHMENT SDN-3X					

PERLND	16	0.65	RCHRES	54	6
PERLND	26	5.17	RCHRES	54	6
PERLND	34	13.64	RCHRES	54	6
PERLND	44	5.34	RCHRES	54	6
PERLND	54	0.57	RCHRES	54	6
PERLND	16	0.65	RCHRES	135	7
PERLND	26	5.17	RCHRES	135	7
PERLND	34	13.64	RCHRES	135	7
PERLND	44	5.34	RCHRES	135	7
PERLND	54	0.57	RCHRES	135	7

\*\*\* SUB-CATCHMENT SDN-4

PERLND	26	24.43	RCHRES	52	6
PERLND	44	3.19	RCHRES	52	6
PERLND	26	24.43	RCHRES	135	7
PERLND	44	3.19	RCHRES	135	7
IMPLND	14	2.61	RCHRES	52	2

\*\*\* SUB-CATCHMENT SDN-4X

PERLND	26	1.57	RCHRES	52	6
PERLND	34	1.16	RCHRES	52	6
PERLND	44	10.01	RCHRES	52	6
PERLND	26	1.57	RCHRES	135	7
PERLND	34	1.16	RCHRES	135	7
PERLND	44	10.01	RCHRES	135	7

\*\*\*ROUTING FOR MILLER CREEK

\*\*\* M1 TO M2 TO M3 TO STORAGE 50. M4 TO M5 TO STORAGE 50

RCHRES	1		RCHRES	2	4
RCHRES	23		RCHRES	24	4
RCHRES	24		RCHRES	3	3
RCHRES	2		RCHRES	3	3
RCHRES	3		RCHRES	33	3
RCHRES	33		RCHRES	50	3
RCHRES	4		RCHRES	5	4
RCHRES	5		RCHRES	50	3

\*\*\* NEW STREAM REACH 52 TO LAKE REBA 53 TO RDF 54

RCHRES	52		RCHRES	53	3
RCHRES	53		RCHRES	54	3
RCHRES	50		RCHRES	54	3

\*\*\* RDF 54 TO 35

RCHRES	54		RCHRES	135	3
RCHRES	34		RCHRES	135	4
RCHRES	34		RCHRES	135	5
RCHRES	135		RCHRES	35	3
RCHRES	10		RCHRES	16	3
RCHRES	35		RCHRES	16	3
RCHRES	11		RCHRES	15	3
RCHRES	13		RCHRES	12	4
RCHRES	13		RCHRES	12	5
RCHRES	12		RCHRES	15	3
RCHRES	16		RCHRES	15	3
RCHRES	14		RCHRES	17	3
RCHRES	15		RCHRES	17	3

END SCHEMATIC

NETWORK

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***
***      <MEMBER> SSSYSSGAP<--MULT-->TRAN <-TARGET VOLLS>      <-MEMBER->
<NAME>  # <NAME>  TEM STRG<-FACTOR->STRG <NAME>  #  # <-GRP> <NAME> # # ***
***

```

END NETWORK

RCHRES

GEN-INFO

RCHRES	Name	Nexits	Unit	Systems	Printer					
#	#<-----><----->	User	T-series	Engl	Metr	LKFG				
		in	out							
1	Arbor Lake M 1	2	1	1	1	62	0	0		
2	Arbor Ck -03710 M 2	1	1	1	1	62	0	0		
3	Arbor Ck M 3	1	1	1	1	62	0	0		
4	Tub Lake M 4	2	1	1	1	62	0	0		
5	Miller Ck SR518 M5	1	1	1	1	62	0	0		
10	Trib (0371G) M 10	1	1	1	1	62	0	0		
11	M11 Ambaum Detention	1	1	1	1	62	0	0		
12	Trib(0354) M 12	1	1	1	1	62	0	0		
13	Burien Lake M 13	2	1	1	1	62	0	0		
14	Trib (0353) M 14	1	1	1	1	62	0	0		
15	M/S U/S OF 17	1	1	1	1	62	0	0		
16	U/S OF 15 M/S	1	1	1	1	62	0	0		
17	GAGE	1	1	1	1	62	0	0		
23	BASIN M23	2	1	1	1	62	0	0		
24	BASIN M24	1	1	1	1	62	0	0		
33	detention m3	1	1	1	1	62	0	0		
34	LORA LAKE	2	1	1	1	62	0	0		
35	D/S OF VACA FARM	1	1	1	1	62	0	0		
38	MC basins	1	1	1	1	62	0	0		
50	sr 518	1	1	1	1	62	0	0		
52	U/S OF LAKE REBA	1	1	1	1	62	0	0		
53	Reba outflow	1	1	1	1	62	0	0		
54	Miller RDF outflow	1	1	1	1	62	0	0		
135	VACA FARMS	1	1	1	1	62	0	0		

END GEN-INFO

ACTIVITY

RCHRES	***** Active Sections *****										
#	#	HYFG	ADFG	CNFG	HTFG	SDFG	GQFG	OXFG	NUFG	PKFG	PHFG
1	999	1	0	0	0	0	0	0	0	0	0

END ACTIVITY

PRINT-INFO

RCHRES	***** Printout Flags *****											PIVL	PYR
#	#	HYDR	ADCA	CONS	HEAT	SED	GQL	OXRX	NUTR	PLNK	PHCB	*****	
1	999	5	0	0	0	0	0	0	0	0	0	1	9

END PRINT-INFO

HYDR-PARM1

RCHRES	Flags for each HYDR Section																			
#	#	VC	A1	A2	A3	ODFVFG for each possible exit				ODGTFG for each possible exit				FUNCT for each possible exit						
		FG	FG	FG	FG	*	*	*	*	*	*	*	*	*	*	*	*			
1		0	1	0	0	4	5	0	0	0	0	0	0	0	0	0	2	2	2	2
2		0	0	0	0	4	0	0	0	0	0	0	0	0	0	0	2	2	2	2
3		0	0	0	0	4	0	0	0	0	0	0	0	0	0	0	2	2	2	2
4		0	1	0	0	4	5	0	0	0	0	0	0	0	0	0	2	2	2	2
5	12	0	0	0	0	4	0	0	0	0	0	0	0	0	0	0	2	2	2	2
13		0	1	0	0	4	5	0	0	0	0	0	0	0	0	0	2	2	2	2

14	22	0	0	0	0	4	0	0	0	0	0	0	0	0	2	2	2	2	2
23		0	1	0	0	4	5	0	0	0	0	0	0	0	2	2	2	2	2
24	33	0	0	0	0	4	0	0	0	0	0	0	0	0	2	2	2	2	2
34		0	1	0	0	4	5	0	0	0	0	0	0	0	2	2	2	2	2
35	999	0	0	0	0	4	0	0	0	0	0	0	0	0	2	2	2	2	2

END HYDR-PARM1

HYDR-PARM2

RCHRES															***					
#	- #	FTABNO	LEN	DELTH	STCOR	KS	DB50													***
----->																	***			
1		1	0.010			0.3														
2		2	0.776			0.3														
3		3	0.980			0.3														
4		4	0.010			0.3														
5		5	0.380			0.3														
10		10	0.380			0.3														
11		11	0.010			0.3														
12		12	1.000			0.3														
13		13	0.015			0.3														
14		14	0.450			0.3														
15		15	0.735			0.3														
16		16	0.587			0.3														
17		17	0.379			0.3														
23		23	0.379		300.0	0.3														
24		24	0.379			0.3														
33		33	0.200			0.3														
34		34	0.852			0.3														
35		35	0.663			0.3														
38		38	0.010			0.3														
50		50	0.010			0.3														
52		52	0.010			0.3														
53		53	0.010			0.3														
54		54	0.010			0.3														
135		135	0.350			0.3														

END HYDR-PARM2

HYDR-INIT

RCHRES		Initial conditions for each HYDR section													***	
#	- #	***	VOL	Initial value of COLIND										Initial value of OUTDGT		
		***	ac-ft	for each possible exit										for each possible exit		
----->																
1		2.0		4.0	5.0											
2		0.0		4.0												
3		0.0		4.0	5.0											
4		2.0		4.0												
5		0.0		4.0												
10		0.0		4.0												
11		0.0		4.0												
12		0.0		4.0												
13		10.0		4.0	5.0											
14		0.0		4.0												
15		0.0		4.0												
16		0.0		4.0												
17		0.0		4.0												
23		6.0		4.0	5.0											
24		0.0		4.0												
33		0.0		4.0												
34		9.0		4.0	5.0											

35	0.1	4.0
38	0.1	4.0
50	0.0	4.0
52	0.0	4.0
53	0.1	4.0
54	2.25	4.0
135	0.00	4.0

END HYDR-INIT  
END RCHRES

FTABLES  
\*\*\*UPPER BASIN  
\*\*\*=====

FTABLE 1  
ROWS COLS \*\*\*  
11 5

DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2***
0.00	3.00	0.00	0.00	0.00
2.50	3.00	7.50	0.00	0.11
3.00	3.00	9.00	1.80	0.11
3.50	3.30	10.58	5.00	0.11
4.00	3.60	12.30	10.90	0.11
4.50	3.90	14.18	17.50	0.11
5.00	4.10	16.18	26.20	0.11
5.50	4.30	18.28	32.50	0.11
6.00	4.50	20.48	35.90	0.11
7.00	5.00	25.23	38.10	0.11
8.00	5.50	30.48	46.40	0.11

END FTABLE 1

FTABLE 2  
ROWS COLS \*\*\*  
9 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.2571	0.0129	0.16	
0.500	0.3873	0.1417	6.53	
1.000	0.5501	0.3761	25.95	
1.500	0.7128	0.6918	59.86	
2.000	0.8756	1.0889	110.67	
3.000	1.2011	2.1273	272.24	
3.500	1.3639	2.7685	387.38	
4.000	1.5266	3.4912	528.19	

END FTABLE 2

FTABLE 3  
ROWS COLS \*\*\*  
12 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.9669	0.0483	0.13	
0.500	1.0637	0.4545	4.92	
1.000	1.1846	1.0165	17.12	
1.500	1.3055	1.6390	34.92	
2.000	1.4264	2.3220	57.95	
2.500	1.5473	3.0654	86.14	



3.000	1.6682	3.8693	119.53
3.500	1.7891	4.7336	158.24
4.000	1.9100	5.6584	202.41
4.500	2.0294	6.6310	251.52
5.000	2.1488	7.6624	306.28

END FTABLE 3

FTABLE 4  
ROWS COLS \*\*\*  
7 5

DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2***
0.00	3.00	0.00	0.00	0.00
2.50	4.50	9.38	0.00	0.11
3.00	6.00	12.00	6.00	0.11
4.00	10.00	20.00	13.00	0.11
5.00	15.00	32.50	20.00	0.11
6.00	20.00	50.00	26.00	0.11
7.00	25.00	72.50	168.00	0.11

END FTABLE 4

FTABLE 5  
ROWS COLS \*\*\*  
10 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.1010	0.0051	0.03	
0.500	0.1754	0.0603	1.46	
1.000	0.2684	0.1713	6.16	
1.500	0.3614	0.3288	14.89	
2.000	0.4544	0.5327	28.48	
2.500	0.5474	0.7832	47.70	
3.000	0.6404	1.0801	73.29	
3.500	0.7334	1.4236	105.94	
4.000	0.8264	1.8136	146.33	

END FTABLE 5

FTABLE 10  
ROWS COLS \*\*\*  
9 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.1010	0.0051	0.06	
0.500	0.1660	0.0585	2.27	
1.000	0.2472	0.1618	9.32	
1.500	0.3285	0.3057	22.08	
2.000	0.4097	0.4902	41.66	
2.500	0.4909	0.7154	69.09	
3.000	0.5722	0.9811	105.37	
4.000	0.6887	1.6116	209.70	

END FTABLE 10

POST AMBAUM DETENTION \*\*\*  
FTABLE 11  
ROWS COLS \*\*\*  
11 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	

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1.000	0.1000	0.2300	3.90
2.000	0.2000	0.6000	6.30
3.000	0.3000	0.9700	8.10
4.000	0.4000	1.3400	11.10
5.000	0.5000	1.8200	16.00
6.000	0.6000	2.2700	19.10
7.000	0.7000	2.8300	21.60
8.000	0.8000	3.3700	30.80
9.000	0.9000	4.0000	38.10
10.000	1.0000	4.6500	74.10
10.500	1.1000	5.2000	133.00
11.000	1.1500	5.3000	500.00

END FTABLE 11

FTABLE 12  
ROWS COLS \*\*\*  
6 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.6327	0.0316	0.15	
0.500	0.7960	0.3174	5.87	
1.000	1.0002	0.7664	21.53	
1.500	1.2043	1.3176	46.43	
2.000	1.4085	1.9708	81.20	
3.000	1.8168	3.5834	183.79	
4.000	2.2251	5.6044	336.22	
5.000	2.6335	8.0337	545.30	
6.000	3.0418	10.8713	817.51	

END FTABLE 12

FTABLE 13  
ROWS COLS \*\*\*  
7 5

DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2***
0.000	40.000	0.0000	0.00	0.00
1.000	41.400	40.000	0.00	0.11
1.500	42.000	60.000	10.00	0.11
2.000	42.700	80.000	16.00	0.11
2.500	43.300	100.00	20.00	0.11
3.000	44.000	120.00	28.00	0.11
5.000	45.000	210.00	45.00	0.11

END FTABLE 13

FTABLE 14  
ROWS COLS \*\*\*  
6 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.3361	0.0168	0.24	
0.500	0.3809	0.1602	9.04	
1.000	0.4370	0.3647	31.61	
1.500	0.4930	0.5972	65.00	
2.000	0.5491	0.8577	108.85	
2.500	0.6051	1.1462	163.33	
3.000	0.6612	1.4628	228.78	

END FTABLE 14

FTABLE 15  
 ROWS COLS \*\*\*  
 4 4  
 DEPTH AREA VOLUME OUTFLOW \*\*\*  
 0.00 0.10 0.00 0.00  
 1.00 1.00 0.55 91.00  
 2.00 1.10 1.60 268.00  
 3.00 1.20 2.75 493.00  
 END FTABLE 15

FTABLE 16  
 ROWS COLS \*\*\*  
 4 4  
 DEPTH AREA VOLUME OUTFLOW \*\*\*  
 0.00 0.10 0.00 0.00  
 1.00 1.00 0.55 74.00  
 2.00 1.10 1.60 219.00  
 3.00 1.20 2.75 403.00  
 END FTABLE 16

FTABLE 17  
 ROWS COLS \*\*\*  
 5 4  
 DEPTH AREA VOLUME OUTFLOW \*\*\*  
 0.00 0.10 0.00 0.00  
 1.00 1.00 0.55 59.00  
 2.00 1.10 1.60 173.00  
 3.00 1.20 2.75 318.00  
 4.00 1.30 4.00 484.00  
 END FTABLE 17

FTABLE 23  
 ROWS COLS \*\*\* HERMES  
 9 5  
 DEPTH AREA VOLUME OUTFLOW OUTFLOW \*\*\*  
 0.00 0.00 0.00 0.00 0.00 0.00  
 5.00 0.50 1.91 0.00 0.00 305.00  
 11.00 0.79 5.79 0.00 0.00 311.00  
 15.00 1.13 9.64 0.50 0.01 315.00  
 19.00 1.72 15.34 0.50 0.05 319.00  
 29.00 2.86 38.25 0.50 0.10 329.00  
 39.00 4.40 74.55 0.50 0.20 339.00  
 50.00 6.22 132.98 0.50 0.30 350.00  
 60.00 10.00 1212.98 0.50 0.40 360.00  
 END FTABLE 23

FTABLE 24  
 ROWS COLS \*\*\*  
 9 4  
 DEPTH AREA VOLUME OUTFLOW \*\*\*  
 0.000 0.0000 0.0000 0.00  
 0.100 0.2571 0.0129 0.16  
 0.500 0.3873 0.1417 6.53  
 1.000 0.5501 0.3761 25.95  
 1.500 0.7128 0.6918 59.86  
 2.000 0.8756 1.0889 110.67  
 3.000 1.2011 2.1273 272.24

3.500	1.3639	2.7685	387.38
4.000	1.5266	3.4912	528.19

END FTABLE 24

FTABLE 33  
ROWS COLS \*\*\*  
11 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.00	1.00	0.00	0.00	
0.50	1.20	0.55	2.00	
1.00	1.40	1.20	6.00	
1.50	1.60	1.95	9.00	
2.00	1.80	2.80	13.00	
2.50	2.00	3.75	16.50	
3.00	2.20	4.80	20.00	
3.50	2.40	5.95	23.00	
4.00	2.60	7.20	26.00	
5.00	2.80	9.90	104.00	
6.00	3.00	12.80	246.00	

END FTABLE 33

FTABLE 34  
ROWS COLS \*\*\* REVISED 11/19/97 BASED ON HEC-RAS MODEL  
6 5

DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2***
0.00	3.00	0.00	0.00	0.00
3.00	3.05	9.08	0.00	0.11
4.00	3.10	12.15	0.00	0.11
5.00	3.15	15.28	0.00	0.11
6.00	3.20	18.45	72.0	0.11
7.00	3.25	21.68	225.0	0.11

END FTABLE 34

FTABLE 35  
ROWS COLS \*\*\* REVISED 11/19/97 BASED ON HECRAS MODEL  
5 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.00	0.10	0.00	0.00	
1.00	1.10	0.60	38.00	
2.00	1.20	1.75	108.00	
3.00	1.30	3.00	194.00	
4.00	1.40	4.35	290.00	

END FTABLE 35

FTABLE 38  
ROWS COLS \*\*\*  
7 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
1.000	0.4000	0.4000	2.00	
1.500	0.5000	1.0000	4.00	
2.000	0.9000	1.3000	11.00	
2.500	1.3000	1.6000	15.00	
3.000	1.6000	2.0000	18.00	
3.500	1.9000	2.5000	20.80	

END FTABLE 38

FTABLE 45  
 ROWS COLS \*\*\*  
 NORTH EMPLOYEE PARKING LOT VAULT (AS-BUILT)\*\*\*

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.2200	0.0000	0.00	
2.000	0.2200	0.4500	1.20	
4.000	0.2200	0.9000	1.70	
6.000	0.2200	1.3400	2.10	
8.000	0.2200	1.7900	2.40	
10.000	0.2200	2.2400	2.70	
12.240	0.2200	2.7400	3.00	
14.000	0.2200	3.1400	6.90	
15.440	0.2200	3.4600	8.30	
16.000	0.2200	3.5800	10.30	
18.000	0.2200	4.0300	13.60	
20.000	0.2200	4.4800	30.79	

END FTABLE 45

FTABLE 50  
 ROWS COLS \*\*\*

DEPTH	AREA	VOLUME	OUTFLOW	***
0.00	1.00	0.00	0.00	
0.50	1.10	0.53	5.00	
1.00	1.20	1.10	15.00	
1.50	1.30	1.73	25.00	
2.00	1.40	2.40	35.00	
2.50	1.50	3.13	52.00	
3.00	1.60	3.90	70.00	
3.50	1.70	4.73	87.00	
4.00	1.80	5.60	105.00	
6.00	1.90	9.30	165.00	

END FTABLE 50

FTABLE 52  
 ROWS COLS \*\*\*

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.3680	0.0184	0.25	
0.500	0.3717	0.1664	9.39	
1.000	0.3763	0.3534	31.06	
2.000	0.3819	0.7325	94.37	
3.000	0.3874	1.1171	174.33	

END FTABLE 52

FTABLE 53  
 OLD LAKE REBA \*\*\*  
 MAX DEPTH = 4.9 FEET \*\*\*  
 30" CMP, 40 CFS DISCHARGE AT MAX DEPTH \*\*\*  
 ROWS COLS \*\*\*

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	2.4000	0.0000	0.00	
1.000	2.5800	2.5000	18.00	
2.000	2.9400	5.3000	26.00	

3.000	3.4100	8.4000	31.00
4.000	3.8800	12.100	36.00
4.900	4.3000	15.800	40.00
6.000	4.3000	15.810	500.00

END FTABLE 53

FTABLE 54  
 EXISTING MILLER CREEK DETENTION FACILITY\*\*\* REVISED STORAGE/Q DATA  
 GATE SETTING: 2.0 FEET\*\*\*  
 ROWS COLS \*\*\*

12	4			
DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.00	0.00	0.00	
1.300	0.01	0.01	10.00	
2.000	0.01	0.02	20.00	
2.900	0.70	0.40	30.00	
4.000	1.50	1.50	40.00	
5.400	3.50	4.90	50.00	
7.000	8.60	13.30	60.00	
8.800	15.60	34.80	70.00	
10.000	19.90	57.30	76.00	
10.500	21.50	68.00	92.00	
11.000	23.10	78.80	179.00	
11.500	24.70	88.60	303.00	

END FTABLE 54

PRE AMBAUM DETENTION \*\*\*  
 FTABLE 111  
 ROWS COLS \*\*\*

12	4			
DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.500	0.2160	0.0750	5.30	
1.000	0.2730	0.1990	21.10	
1.500	0.2890	0.3410	43.90	
2.000	0.2900	0.4830	68.80	
2.500	0.2910	0.6070	89.10	
3.000	0.2950	0.6820	90.00	
3.500	0.3000	2.1000	100.00	
4.000	0.3050	2.5000	105.00	
4.500	0.3100	3.0000	110.00	
5.000	0.3200	3.5000	120.00	
5.500	0.3300	4.0000	130.00	

END FTABLE111

FTABLE 135  
 ROWS COLS \*\*\* VACA FARM

6	4			
DEPTH	AREA	VOLUME	OUTFLOW	***
0.00	0.10	0.00	0.00	
1.00	0.10	0.10	4.00	
2.00	0.11	0.21	8.00	
2.50	1.00	0.48	13.00	
3.50	6.50	4.23	86.00	
4.50	13.00	13.98	235.00	

END FTABLE135

\*\*\*

END FTABLES

```
MASS-LINK
<Volume>  <-Grp> <-Member-><--Mult-->  <Target>  <-Grp> <-Member->***
<Name>     <Name> # #<-factor->  <Name>     <Name> # #***
  MASS-LINK      1
  conversion from acre-inches to acre-ft (1/12)      ***
PERLND  PWATER  PERO      0.0833333  RCHRES      INFLOW IVOL
  END MASS-LINK      1

  MASS-LINK      2
IMPLND  IWATER  SURO      0.0833333  RCHRES      INFLOW IVOL
  END MASS-LINK      2

  MASS-LINK      3
RCHRES  ROFLOW      RCHRES      INFLOW
  END MASS-LINK      3

  MASS-LINK      4
RCHRES  OFLOW  OVOL  1  RCHRES      INFLOW IVOL
  END MASS-LINK      4

  MASS-LINK      5
RCHRES  OFLOW  OVOL  2  RCHRES      INFLOW IVOL
  END MASS-LINK      5

  MASS-LINK      6
PERLND  PWATER  SURO      0.0833333  RCHRES      INFLOW IVOL
PERLND  PWATER  IFWO      0.0833333  RCHRES      INFLOW IVOL
  END MASS-LINK      6

  MASS-LINK      7
PERLND  PWATER  AGWO      0.0833333  RCHRES      INFLOW IVOL
  END MASS-LINK      7

  MASS-LINK      8
PERLND  PWATER  PERO      0.0833333  COPY        INPUT  MEAN
  END MASS-LINK      8
  MASS-LINK      12
PERLND  PWATER  AGWO      0.0833333  COPY        INPUT  MEAN
  END MASS-LINK      12

  MASS-LINK      9
IMPLND  IWATER  SURO      0.0833333  COPY        INPUT  MEAN
  END MASS-LINK      9

  MASS-LINK      10
COPY    OUTPUT  MEAN      RCHRES      INFLOW IVOL
  END MASS-LINK      10

END MASS-LINK
END RUN
```

AR 011469

RUN

GLOBAL

```
*** FILE: mill65.inp REVISED Aug 2000 Joe Brascher(atc)
*** for parameterix
*** SEATAC AIRPORT HSPF BASIN MODEL OF MILLER CREEK
*** - POST-MILLER CK DETENTION FACIITY 10/92-6/93
*** m23 AND M24 new area west of m2. Flows to rdf
*** Calibration file run for four years using full length calibration run 1990
*** data for initial conditions
MILLER CREEK BASIN HSPF MODEL
START          1990 10  1  0  0  END      1994  9 30 24  0
RUN INTERP OUTPUT LEVEL      3
RESUME         0 RUN         1
```

END GLOBAL

FILES

```
<type> <fun>***<-----fname----->
MESSU      24  D:\PARA\SEATAC\MILLER\lowflow\MILL.MES
WDM         25  D:\PARA\SEATAC\MILLER\lowflow\Mlowflow.wdm
            61  D:\PARA\SEATAC\MILLER\lowflow\PER.L61
            62  D:\PARA\SEATAC\MILLER\lowflow\RCH.L62
```

END FILES

OPN SEQUENCE

```
INGRP          INDELT 01:00
  PERLND        14
  PERLND        16
  PERLND        18
  PERLND        24
  PERLND        26
  PERLND        28
  PERLND        34
  PERLND        44
  PERLND        54
  IMPLND        14
  RCHRES         1
  RCHRES        23
  RCHRES        24
  RCHRES         2
  RCHRES         3
  RCHRES        33
  RCHRES         4
  RCHRES         5
  RCHRES        50
  RCHRES        52
  RCHRES        53
  RCHRES        54
  RCHRES        34
  RCHRES       135
  RCHRES        35
  RCHRES        10
  RCHRES        16
  RCHRES        11
  RCHRES        13
  RCHRES        12
  RCHRES        15
  RCHRES        14
```

AR 011470



RCHRES 17  
 END INGRP  
 END OPN SEQUENCE

\*\*\*

COPY  
 TIMESERIES  
 Copy-opn  
 # - # NPT NMN  
 1 5 1  
 END TIMESERIES  
 END COPY

\*\*\*  
 \*\*\*

PERLND  
 GEN-INFO  
 <PLS > Name NBLKS Unit-systems Printer  
 # - # User t-series Engl Metr  
 in out  
 14 TFF- TILL FOR FLT 1 1 1 1 61 0  
 16 TFM- TILL FOR MOD 1 1 1 1 61 0  
 18 TFS- TILL FOR STP 1 1 1 1 61 0  
 24 TGF- TILL GR FLT 1 1 1 1 61 0  
 26 TGM- TILL GR MOD 1 1 1 1 61 0  
 28 TGS- TILL GR STP 1 1 1 1 61 0  
 34 OF - OUTWASH FOR 1 1 1 1 61 0  
 44 OG - OUTWASH GR 1 1 1 1 61 0  
 \*\*\*PERLND FOR NEW AIRPORT FILL; NONE IN CALIBRATION  
 45 AIRPORT FILL 1 1 1 1 61 0  
 54 SA - WETLANDS 1 1 1 1 61 0  
 64 RES- GROUNDWATER 1 1 1 1 61 0

\*\*\*  
 \*\*\*  
 \*\*\*

END GEN-INFO

ACTIVITY  
 <PLS > \*\*\*\*\* Active Sections \*\*\*\*\*  
 # - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC  
 14 200 0 0 1 0 0 0 0 0 0 0 0 0 0

\*\*\*

END ACTIVITY

PRINT-INFO  
 <PLS > \*\*\*\*\* Print-flags \*\*\*\*\* PIVL PYR  
 # - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC \*\*\*\*\*  
 14 200 0 0 5 0 0 0 0 0 0 0 0 0 1 9

END PRINT-INFO

PWAT-PARM1  
 <PLS > \*\*\*\*\* Flags \*\*\*\*\*  
 # - # CSNO RTOP UZFG VCS VUZ VNN VIFW VIRC VLE  
 14 0 0 0 0 0 0 0 0 0  
 16 0 0 0 0 0 0 0 0 0  
 18 0 0 0 0 0 0 0 0 0  
 24 0 0 0 0 0 0 0 0 0  
 26 0 0 0 0 0 0 0 0 0  
 28 0 0 0 0 0 0 0 0 0  
 34 0 0 0 0 0 0 0 0 0  
 44 0 0 0 0 0 0 0 0 0  
 45 0 0 0 0 0 0 0 0 0  
 54 0 0 0 0 0 0 0 0 0  
 64 0 0 0 0 0 0 0 0 0

\*\*\*

END PWAT-PARM1

PWAT-PARM2

<PLS > \*\*\*

#	#	***FOREST	LZSN	INFILT	LSUR	SLSUR	KVARY	AGWRC
14			9.0000	0.3200	400.00	0.0500	0.5000	0.9960
16			9.0000	0.3200	400.00	0.1000	0.5000	0.9960
18			9.0000	0.3200	200.00	0.2000	0.5000	0.9960
24			9.0000	0.1200	400.00	0.0500	0.5000	0.9960
26			9.0000	0.1200	400.00	0.1000	0.5000	0.9960
28			9.0000	0.1200	200.00	0.2000	0.5000	0.9960
34			10.0000	2.0000	400.00	0.0500	0.3000	0.9960
44			10.0000	0.8000	400.00	0.0500	0.3000	0.9960
45			7.5000	0.0200	300.00	0.0700	0.0000	0.9000
54			8.0000	2.0000	100.00	0.0010	0.5000	0.9960

END PWAT-PARM2

PWAT-PARM3

<PLS >\*\*\*

#	#	*** PETMAX	PETMIN	INFEXP	INFILD	DEEPFR	BASETP	AGWETP
14				2.0000	2.0000	0.33	0.00	0.0
16				2.0000	2.0000	0.33	0.00	0.0
18				2.0000	2.0000	0.33	0.00	0.0
24				2.0000	2.0000	0.33	0.00	0.
26				2.0000	2.0000	0.33	0.	0.
28				2.0000	2.0000	0.33	0.	0.
34				2.0000	2.0000	0.33	0.00	0.0
44				2.0000	2.0000	0.33	0.	0.
54				10.000	2.0000	0.33	0.	0.7

END PWAT-PARM3

PWAT-PARM4

<PLS >

#	#	CEPSC	UZSN	NSUR	INTFW	IRC	LZETP***
14		0.2000	1.5000	0.3500	9.000	0.7000	0.7000
16		0.2000	0.7500	0.3500	9.000	0.7000	0.7000
18		0.2000	0.4500	0.3500	9.000	0.3000	0.7000
24		0.1000	0.7500	0.2500	9.000	0.7000	0.2500
26		0.1000	0.3750	0.2500	9.000	0.7000	0.2500
28		0.1000	0.2250	0.2500	9.000	0.3000	0.2500
34		0.2000	0.7500	0.3500	0.000	0.7000	0.7000
44		0.1000	0.7500	0.2500	0.000	0.7000	0.2500
54		0.1000	2.2500	0.5000	1.000	0.7000	0.8000

END PWAT-PARM4

PWAT-STATE1

<PLS > PWATER state variables\*\*\*

#	#	***	CEPS	SURS	UZS	IFWS	LZS	AGWS	GWVS
16			0.000	0.	0.0010	0.00	0.941	3.108	0.048
26			0.000	0.	0.0010	0.00	7.672	3.341	0.071
34			0.000	0.	0.0010	0.00	1.187	3.776	0.052
44			0.000	0.	0.0040	0.00	9.402	4.905	0.104
45			0.000	0.	0.0000	0.00	2.000	2.000	0.000
54			0.000	0.	0.0960	0.00	3.211	0.000	0.000
14			0.078	0.	0.2500	0.10	2.000	2.000	0.000
18			0.078	0.	0.2500	0.10	2.000	2.000	0.000
24			0.051	0.	0.2500	0.10	2.000	2.000	0.000
28			0.051	0.	0.2500	0.10	2.000	2.000	0.000
64			0.051	0.	0.2500	0.10	2.000	2.000	0.000

END PWAT-STATE1

END PERLND

IMPLND

GEN-INFO

<ILS > Name Unit-systems Printer \*\*\*

```

# - # User t-series Engl Metr
in out
14 IMPERVIOUS 1 1 1 60 0
END GEN-INFO
ACTIVITY
<ILS > ***** Active Sections ****
# - # ATMP SNOW IWAT SLD IWG IQAL ***
14 0 0 1 0 0 0
END ACTIVITY
PRINT-INFO
<ILS > ***** Print-flags ***** PIVL PYR
# - # ATMP SNOW IWAT SLD IWG IQAL *****
14 0 0 6 0 0 0 1 9
END PRINT-INFO
IWAT-PARM1
<ILS > Flags ***
# - # CSNO RTOP VRS VNN RTLI ***
14 0 0 0 0 0
END IWAT-PARM1
IWAT-PARM2
<ILS > ***
# - # LSUR SLSUR NSUR RETSC ***
14 100.00 0.0100 0.1000 0.1000
END IWAT-PARM2
IWAT-PARM3
<ILS > ***
# - # PETMAX PETMIN ***
14
END IWAT-PARM3
IWAT-STATE1
<ILS > IWATER state variables ***
# - # RETS SURS ***
14 1.0000E-3 1.0000E-3
END IWAT-STATE1
END IMPLND
EXT SOURCES
*** NOTE: The only RCHRES that precip and PET are applied to are lakes.
***
<-Volume-> <Member> SsysSgap<--Mult-->Tran <-Target vols> <-Grp> <-Member->
<Name> # <Name> # tem strg<-factor->strg <Name> # # <Name> # #
*** PRECIP/EVAP TO PERVIOUS/IMPERV SURFACES
WDM 1002 PREC ENGLZERO 1.00 PERLND 14 200 EXTNL PREC
WDM 1002 PREC ENGLZERO 1.00 IMPLND 14 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 PERLND 14 18 EXTNL PETINP
WDM 1 EVAP ENGLZERO 0.8 PERLND 24 28 EXTNL PETINP
WDM 1 EVAP ENGLZERO 0.8 PERLND 34 54 EXTNL PETINP
WDM 1 EVAP ENGLZERO 0.8 PERLND 64 EXTNL PETINP
WDM 1 EVAP ENGLZERO 0.8 IMPLND 14 EXTNL PETINP
*** PRECIP/EVAP TO LAKES
WDM 1002 PREC ENGLZERO RCHRES 1 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 1 EXTNL POTEV
WDM 1002 PREC ENGLZERO RCHRES 4 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 4 EXTNL POTEV
WDM 1 EVAP ENGLZERO 0.8 RCHRES 1 EXTNL POTEV
WDM 1002 PREC ENGLZERO RCHRES 1 EXTNL PREC

```

WDM	1	EVAP	ENGLZERO	0.8	RCHRES	4	EXTNL	POTEV
WDM	1002	PREC	ENGLZERO		RCHRES	4	EXTNL	PREC
WDM	1002	PREC	ENGLZERO		RCHRES	11	EXTNL	PREC
WDM	1	EVAP	ENGLZERO	0.8	RCHRES	11	EXTNL	POTEV
WDM	1002	PREC	ENGLZERO		RCHRES	13	EXTNL	PREC
WDM	1	EVAP	ENGLZERO	0.8	RCHRES	13	EXTNL	POTEV
WDM	1002	PREC	ENGLZERO		RCHRES	23	EXTNL	PREC
WDM	1	EVAP	ENGLZERO	0.8	RCHRES	23	EXTNL	POTEV
WDM	1002	PREC	ENGLZERO		RCHRES	34	EXTNL	PREC
WDM	1	EVAP	ENGLZERO	0.8	RCHRES	34	EXTNL	POTEV
WDM	1002	PREC	ENGLZERO		RCHRES	53	EXTNL	PREC
WDM	1	EVAP	ENGLZERO	0.8	RCHRES	53	EXTNL	POTEV
WDM	1002	PREC	ENGLZERO		RCHRES	54	EXTNL	PREC
WDM	1	EVAP	ENGLZERO	0.8	RCHRES	54	EXTNL	POTEV

END EXT SOURCES

EXT TARGETS

```

<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Volume-> <Member> Tsys Tgap Amd ***
<Name> # <Name> # <-factor->strg <Name> # <Name> tem strg strg***
*** UPPER MILLER CREEK GROUNDWATER PUMPING
COPY *** 1 OUTPUT MEAN 1 12.1 WDM 18 FLOW ENGL REPL
*** GAUGE POINTS (17=MOUTH, 54=MILLER RDF, 50=SR 518, 18=WALKER CK)
RCHRES 35 HYDR RO WDM 8035 FLOW ENGL REPL
RCHRES 17 HYDR RO *** WDM 33 FLOW ENGL REPL
RCHRES 54 HYDR RO *** WDM 34 FLOW ENGL REPL
RCHRES 50 HYDR RO *** WDM 35 FLOW ENGL REPL
RCHRES 18 HYDR RO *** WDM 36 FLOW ENGL REPL
*** MISC (20=WALKER WETLAND, 55=SR509, 56=1ST AVE, 1=ARBOR LAKE)
RCHRES 23 HYDR STAGE *** WDM 91 STAG ENGL REPL
RCHRES 20 HYDR RO *** WDM 37 FLOW ENGL REPL
RCHRES 55 HYDR RO *** WDM 38 FLOW ENGL REPL
RCHRES 62 HYDR RO *** WDM 39 FLOW ENGL REPL
RCHRES 1 HYDR RO *** WDM 80 FLOW ENGL REPL
<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Volume-> <Member> Tsys Tgap Amd ***
<Name> # <Name> # <-factor->strg <Name> # <Name> tem strg strg***
***MOUTH
RCHRES 54 HYDR RO 1 1 0.000419 *** WDM 60 SIMQ ENGL REPL
RCHRES 17 HYDR RO 1 1 0.000213 *** WDM 70 SIMQ ENGL REPL

```

SCHEMATIC

```

<-Source-> <--Area--> <-Target-> MBLK ***
<Name> # <-factor-> <Name> # Tbl# ***
*** SUB-CATCHMENT 1 all agwo goes to sound
PERLND 16 3.41 RCHRES 1 6
PERLND 26 232.36 RCHRES 1 6
PERLND 34 3.07 RCHRES 1 6
PERLND 44 38.03 RCHRES 1 6
PERLND 54 3.87 RCHRES 1 6
IMPLND 14 56.14 RCHRES 1 2
*** SUB-CATCHMENT 2 10% of area GW goes to vaca 90% goes to sound
PERLND 16 5.56 RCHRES 2 6
PERLND 26 200.05 RCHRES 2 6
PERLND 34 0.46 RCHRES 2 6
PERLND 44 38.71 RCHRES 2 6
PERLND 16 0.56 RCHRES 135 7

```

PERLND	26	20.01	RCHRES	135	7
PERLND	34	0.05	RCHRES	135	7
PERLND	44	3.87	RCHRES	135	7
IMPLND	14	42.22	RCHRES	2	2
*** SUB-CATCHMENT 23 New subbasin 15 % OF GW GOES TO VACCA 85% TO SOUND					
PERLND	16	3.09	RCHRES	23	6
PERLND	26	156.15	RCHRES	23	6
PERLND	34	2.25	RCHRES	23	6
PERLND	44	45.84	RCHRES	23	6
PERLND	16	0.46	RCHRES	135	7
PERLND	26	23.42	RCHRES	135	7
PERLND	34	0.34	RCHRES	135	7
PERLND	44	6.88	RCHRES	135	7
IMPLND	14	58.44	RCHRES	23	2
*** SUB-CATCHMENT 24 New subbasin 60 % OF GW GOES TO 11 40% TO SOUND					
PERLND	26	135.43	RCHRES	24	6
PERLND	34	2.02	RCHRES	24	6
PERLND	44	69.29	RCHRES	24	6
PERLND	26	81.26	RCHRES	11	7
PERLND	34	1.21	RCHRES	11	7
PERLND	44	41.57	RCHRES	11	7
IMPLND	14	79.98	RCHRES	24	2
*** SUB-CATCHMENT 3 agwo goes to vaca(135)					
PERLND	16	8.26	RCHRES	3	6
PERLND	26	108.38	RCHRES	3	6
PERLND	34	16.02	RCHRES	3	6
PERLND	44	102.89	RCHRES	3	6
PERLND	54	0.04	RCHRES	3	6
PERLND	16	8.26	RCHRES	135	7
PERLND	26	108.38	RCHRES	135	7
PERLND	34	16.02	RCHRES	135	7
PERLND	44	102.89	RCHRES	135	7
PERLND	54	0.04	RCHRES	135	7
IMPLND	14	27.30	RCHRES	3	2
*** SUB-CATCHMENT 4 10% of agwo goes to rchres 90% goes to sound					
PERLND	16	2.95	RCHRES	4	6
PERLND	26	85.95	RCHRES	4	6
PERLND	34	3.75	RCHRES	4	6
PERLND	44	92.06	RCHRES	4	6
PERLND	16	0.30	RCHRES	4	7
PERLND	26	8.60	RCHRES	4	7
PERLND	34	0.38	RCHRES	4	7
PERLND	44	9.21	RCHRES	4	7
IMPLND	14	18.43	RCHRES	4	2
*** SUB-CATCHMENT 4a 70% of agwo goes to rchres 30% goes to sound					
PERLND	16	8.66	RCHRES	4	6
PERLND	26	61.64	RCHRES	4	6
PERLND	34	22.06	RCHRES	4	6
PERLND	44	78.09	RCHRES	4	6
PERLND	54	12.50	RCHRES	4	6
PERLND	16	6.06	RCHRES	4	7
PERLND	26	43.15	RCHRES	4	7
PERLND	34	15.44	RCHRES	4	7
PERLND	44	54.66	RCHRES	4	7
PERLND	54	8.75	RCHRES	4	7
IMPLND	14	29.14	RCHRES	4	2
*** SUB-CATCHMENT 5					

PERLND	26	10.29	RCHRES	5	1
PERLND	44	50.04	RCHRES	5	1
PERLND	54	10.74	RCHRES	5	1
IMPLND	14	16.31	RCHRES	5	2
*** SUB-CATCHMENT 6					
PERLND	16	10.66	RCHRES	53	1
PERLND	26	41.08	RCHRES	53	1
PERLND	34	21.75	RCHRES	53	1
PERLND	44	13.39	RCHRES	53	1
PERLND	54	0.82	RCHRES	53	1
IMPLND	14	7.14	RCHRES	53	2
*** SUB-CATCHMENT 8					
PERLND	44	22.21	RCHRES	35	1
IMPLND	14	6.60	RCHRES	35	2
*** SUB-CATCHMENT 9					
PERLND	16	4.94	RCHRES	34	1
PERLND	26	14.32	RCHRES	34	1
PERLND	34	0.05	RCHRES	34	1
PERLND	44	56.70	RCHRES	34	1
PERLND	54	0.01	RCHRES	34	1
IMPLND	14	22.46	RCHRES	34	2
*** SUB-CATCHMENT 10					
PERLND	16	4.15	RCHRES	10	1
PERLND	26	31.94	RCHRES	10	1
PERLND	44	95.23	RCHRES	10	1
IMPLND	14	71.97	RCHRES	10	2
*** SUB-CATCHMENT 11 25% OF AGWO GOES TO 15					
PERLND	16	0.89	RCHRES	11	6
PERLND	26	217.92	RCHRES	11	6
PERLND	34	1.32	RCHRES	11	6
PERLND	44	65.65	RCHRES	11	6
PERLND	16	0.67	RCHRES	11	7
PERLND	26	163.44	RCHRES	11	7
PERLND	34	0.99	RCHRES	11	7
PERLND	44	49.24	RCHRES	11	7
PERLND	16	0.22	RCHRES	15	7
PERLND	26	54.48	RCHRES	15	7
PERLND	34	0.33	RCHRES	15	7
PERLND	44	16.41	RCHRES	15	7
IMPLND	14	230.80	RCHRES	11	2
*** SUB-CATCHMENT 12					
PERLND	16	0.39	RCHRES	12	1
PERLND	26	101.18	RCHRES	12	1
PERLND	34	5.64	RCHRES	12	1
PERLND	44	54.98	RCHRES	12	1
PERLND	54	0.64	RCHRES	12	1
IMPLND	14	79.83	RCHRES	12	2
*** SUB-CATCHMENT 13					
PERLND	16	0.79	RCHRES	13	1
PERLND	26	197.68	RCHRES	13	1
IMPLND	14	27.66	RCHRES	13	2
*** SUB-CATCHMENT 14 50% OF AGWO GOES TO SOUND					
PERLND	16	0.24	RCHRES	14	6
PERLND	26	118.67	RCHRES	14	6
PERLND	34	13.46	RCHRES	14	6
PERLND	44	41.91	RCHRES	14	6
PERLND	16	0.12	RCHRES	14	7

PERLND	26	59.34	RCHRES	14	7
PERLND	34	6.73	RCHRES	14	7
PERLND	44	20.95	RCHRES	14	7
IMPLND	14	20.66	RCHRES	14	2
*** SUB-CATCHMENT 15					
PERLND	16	6.59	RCHRES	15	1
PERLND	26	49.55	RCHRES	15	1
PERLND	34	50.09	RCHRES	15	1
PERLND	44	86.52	RCHRES	15	1
IMPLND	14	19.47	RCHRES	15	2
*** SUB-CATCHMENT 16					
PERLND	16	10.93	RCHRES	16	1
PERLND	26	30.30	RCHRES	16	1
PERLND	34	20.03	RCHRES	16	1
PERLND	44	31.42	RCHRES	16	1
IMPLND	14	15.54	RCHRES	16	2
*** SUB-CATCHMENT 17 AGWO GOES TO SOUND					
PERLND	16	0.90	RCHRES	17	6
PERLND	26	16.31	RCHRES	17	6
PERLND	34	34.82	RCHRES	17	6
PERLND	44	82.11	RCHRES	17	6
PERLND	54	2.19	RCHRES	17	6
IMPLND	14	10.49	RCHRES	17	2
*** SUB-CATCHMENT MC-1					
PERLND	26	0.17	RCHRES	52	1
PERLND	44	8.21	RCHRES	52	1
PERLND	54	0.27	RCHRES	52	1
IMPLND	14	0.09	RCHRES	52	2
*** SUB-CATCHMENT MC-2					
PERLND	16	0.08	RCHRES	53	1
PERLND	26	0.64	RCHRES	53	1
PERLND	34	6.72	RCHRES	53	1
PERLND	44	10.43	RCHRES	53	1
PERLND	54	15.25	RCHRES	53	1
IMPLND	14	0.27	RCHRES	53	2
*** SUB-CATCHMENT MC-3					
PERLND	34	5.44	RCHRES	54	1
PERLND	44	5.03	RCHRES	54	1
PERLND	54	2.28	RCHRES	54	1
IMPLND	14	0.11	RCHRES	54	2
*** SUB-CATCHMENT MC-4					
PERLND	44	17.32	RCHRES	135	1
PERLND	54	14.41	RCHRES	135	1
IMPLND	14	1.77	RCHRES	135	2
*** SUB-CATCHMENT MC-5					
PERLND	26	13.49	RCHRES	35	1
PERLND	44	31.06	RCHRES	35	1
PERLND	54	5.95	RCHRES	35	1
IMPLND	14	2.50	RCHRES	35	2
*** SUB-CATCHMENT MC-6					
PERLND	44	17.75	RCHRES	35	1
PERLND	54	6.54	RCHRES	35	1
IMPLND	14	0.95	RCHRES	35	2
*** SUB-CATCHMENT MC-6B					
PERLND	26	34.94	RCHRES	35	1
PERLND	34	7.81	RCHRES	35	1
PERLND	44	52.91	RCHRES	35	1

PERLND	54	4.61	RCHRES	35	1
IMPLND	14	3.14	RCHRES	35	2
*** SUB-CATCHMENT MC-7					
PERLND	26	12.66	RCHRES	16	1
PERLND	44	33.53	RCHRES	16	1
PERLND	54	4.16	RCHRES	16	1
IMPLND	14	3.88	RCHRES	16	2
*** SUB-CATCHMENT MC-7B					
PERLND	26	36.16	RCHRES	16	1
PERLND	44	8.46	RCHRES	16	1
PERLND	54	1.92	RCHRES	16	1
IMPLND	14	2.12	RCHRES	16	2
***all sdn basin agwo goes to 35					
*** SUB-CATCHMENT SDN-1					
PERLND	26	3.23	RCHRES	52	6
PERLND	44	2.11	RCHRES	52	6
PERLND	54	0.20	RCHRES	52	6
PERLND	26	3.23	RCHRES	135	7
PERLND	44	2.11	RCHRES	135	7
PERLND	54	0.20	RCHRES	135	7
IMPLND	14	8.29	RCHRES	52	2
*** SUB-CATCHMENT SDN-1-LWR					
PERLND	44	4.97	RCHRES	52	6
PERLND	54	0.07	RCHRES	52	6
PERLND	44	4.97	RCHRES	135	7
PERLND	54	0.07	RCHRES	135	7
IMPLND	14	0.38	RCHRES	52	2
*** SUB-CATCHMENT SDN-1-OFF					
PERLND	26	29.12	RCHRES	52	6
PERLND	44	3.62	RCHRES	52	6
PERLND	54	1.67	RCHRES	52	6
PERLND	26	29.12	RCHRES	135	7
PERLND	44	3.62	RCHRES	135	7
PERLND	54	1.67	RCHRES	135	7
IMPLND	14	11.50	RCHRES	52	2
*** SUB-CATCHMENT SDN-2					
PERLND	26	10.41	RCHRES	52	6
PERLND	44	3.04	RCHRES	52	6
PERLND	26	10.41	RCHRES	135	7
PERLND	44	3.04	RCHRES	135	7
IMPLND	14	33.22	RCHRES	52	2
*** SUB-CATCHMENT SDN-2X					
PERLND	26	1.37	RCHRES	52	6
PERLND	44	5.84	RCHRES	52	6
PERLND	26	1.37	RCHRES	135	7
PERLND	44	5.84	RCHRES	135	7
IMPLND	14	0.28	RCHRES	52	2
*** SUB-CATCHMENT SDN-3					
PERLND	26	49.79	RCHRES	54	6
PERLND	26	49.79	RCHRES	135	7
IMPLND	14	15.82	RCHRES	54	2



\*\*\* SUB-CATCHMENT SDN-3X

PERLND	16	0.65	RCHRES	54	6
PERLND	26	5.17	RCHRES	54	6
PERLND	34	13.64	RCHRES	54	6
PERLND	44	5.34	RCHRES	54	6
PERLND	54	0.57	RCHRES	54	6
PERLND	16	0.65	RCHRES	135	7
PERLND	26	5.17	RCHRES	135	7
PERLND	34	13.64	RCHRES	135	7
PERLND	44	5.34	RCHRES	135	7
PERLND	54	0.57	RCHRES	135	7

\*\*\* SUB-CATCHMENT SDN-4

PERLND	26	24.43	RCHRES	52	6
PERLND	44	3.19	RCHRES	52	6
PERLND	26	24.43	RCHRES	135	7
PERLND	44	3.19	RCHRES	135	7
IMPLND	14	2.61	RCHRES	52	2

\*\*\* SUB-CATCHMENT SDN-4X

PERLND	26	1.57	RCHRES	52	6
PERLND	34	1.16	RCHRES	52	6
PERLND	44	10.01	RCHRES	52	6
PERLND	26	1.57	RCHRES	135	7
PERLND	34	1.16	RCHRES	135	7
PERLND	44	10.01	RCHRES	135	7

\*\*\*ROUTING FOR MILLER CREEK

\*\*\* M1 TO M2 TO M3 TO STORAGE 50. M4 TO M5 TO STORAGE 50

RCHRES	1		RCHRES	2	4
RCHRES	23		RCHRES	24	4
RCHRES	24		RCHRES	3	3
RCHRES	2		RCHRES	3	3
RCHRES	3		RCHRES	33	3
RCHRES	33		RCHRES	50	3
RCHRES	4		RCHRES	5	4
RCHRES	5		RCHRES	50	3

\*\*\* NEW STREAM REACH 52 TO LAKE REBA 53 TO RDF 54

RCHRES	52		RCHRES	53	3
RCHRES	53		RCHRES	54	3
RCHRES	50		RCHRES	54	3

\*\*\* RDF 54 TO 35

RCHRES	54		RCHRES	135	3
RCHRES	34		RCHRES	135	4
RCHRES	34		RCHRES	135	5
RCHRES	135		RCHRES	35	3
RCHRES	10		RCHRES	16	3
RCHRES	35		RCHRES	16	3
RCHRES	11		RCHRES	15	3
RCHRES	13		RCHRES	12	4
RCHRES	13		RCHRES	12	5
RCHRES	12		RCHRES	15	3
RCHRES	16		RCHRES	15	3
RCHRES	14		RCHRES	17	3
RCHRES	15		RCHRES	17	3

END SCHEMATIC

NETWORK

\*\*\* <MEMBER> SSSSSGAP<--MULT-->TRAN <-TARGET VOLS> <-MEMBER->
<NAME> # <NAME> TEM STRG<-FACTOR->STRG <NAME> # # <-GRP> <NAME> # # \*\*\*

END NETWORK

RCHRES

GEN-INFO

Table with columns: RCHRES, Name, Nexits, Unit Systems (in, out), Printer (Engl, Metr, LKFG). Lists various water treatment units like Arbor Lake, Miller Ck, etc.

END GEN-INFO

ACTIVITY

Table with columns: RCHRES, HYFR, ADFG, CNFG, HTFG, SDFG, GQFG, OXFG, NUFG, PKFG, PHFG. Shows activity levels for various sections.

END ACTIVITY

PRINT-INFO

Table with columns: RCHRES, HYDR, ADCA, CONS, HEAT, SED, GQL, OXRX, NUTR, PLNK, PHCB, PIVL, PYR. Shows printout flags for various parameters.

END PRINT-INFO

HYDR-PARM1

Table with columns: RCHRES, Flags for each HYDR Section, ODFVFG for each possible exit, ODGTFG for each possible exit, FUNCT for each possible exit. Shows flags and functional parameters for hydrant sections.

13		0	1	0	0	4	5	0	0	0	0	0	0	0	2	2	2	2	2
14	22	0	0	0	0	4	0	0	0	0	0	0	0	0	2	2	2	2	2
23		0	1	0	0	4	5	0	0	0	0	0	0	0	2	2	2	2	2
24	33	0	0	0	0	4	0	0	0	0	0	0	0	0	2	2	2	2	2
34		0	1	0	0	4	5	0	0	0	0	0	0	0	2	2	2	2	2
35	999	0	0	0	0	4	0	0	0	0	0	0	0	0	2	2	2	2	2

END HYDR-PARM1

HYDR-PARM2

RCHRES		FTABNO	LEN	DELTH	STCOR	KS	DB50	***
#	- #							***
1		1	0.010			0.3		***
2		2	0.776			0.3		***
3		3	0.980			0.3		***
4		4	0.010			0.3		***
5		5	0.380			0.3		***
10		10	0.380			0.3		***
11		11	0.010			0.3		***
12		12	1.000			0.3		***
13		13	0.015			0.3		***
14		14	0.450			0.3		***
15		15	0.735			0.3		***
16		16	0.587			0.3		***
17		17	0.379			0.3		***
23		23	0.379		300.0	0.3		***
24		24	0.379			0.3		***
33		33	0.200			0.3		***
34		34	0.852			0.3		***
35		35	0.663			0.3		***
38		38	0.010			0.3		***
50		50	0.010			0.3		***
52		52	0.010			0.3		***
53		53	0.010			0.3		***
54		54	0.010			0.3		***
135		135	0.350			0.3		***

END HYDR-PARM2

HYDR-INIT

RCHRES		VOL	Initial conditions for each HYDR section		***
#	- #	*** ac-ft	Initial value of COLIND	Initial value of OUTDGT	***
			for each possible exit	for each possible exit	
1		2.0	4.0	5.0	
2		0.0	4.0		
3		0.0	4.0	5.0	
4		2.0	4.0		
5		0.0	4.0		
10		0.0	4.0		
11		0.0	4.0		
12		0.0	4.0		
13		10.0	4.0	5.0	
14		0.0	4.0		
15		0.0	4.0		
16		0.0	4.0		
17		0.0	4.0		
23		6.0	4.0	5.0	
24		0.0	4.0		
33		0.0	4.0		

34	9.0	4.0	5.0
35	0.1	4.0	
38	0.1	4.0	
50	0.0	4.0	
52	0.0	4.0	
53	0.1	4.0	
54	2.25	4.0	
135	0.00	4.0	

END HYDR-INIT  
END RCHRES

FTABLES  
\*\*\*UPPER BASIN  
\*\*\*=====

FTABLE 1  
ROWS COLS \*\*\*  
11 5

DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2***
0.00	3.00	0.00	0.00	0.00
2.50	3.00	7.50	0.00	0.11
3.00	3.00	9.00	1.80	0.11
3.50	3.30	10.58	5.00	0.11
4.00	3.60	12.30	10.90	0.11
4.50	3.90	14.18	17.50	0.11
5.00	4.10	16.18	26.20	0.11
5.50	4.30	18.28	32.50	0.11
6.00	4.50	20.48	35.90	0.11
7.00	5.00	25.23	38.10	0.11
8.00	5.50	30.48	46.40	0.11

END FTABLE 1

FTABLE 2  
ROWS COLS \*\*\*  
9 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.2571	0.0129	0.16	
0.500	0.3873	0.1417	6.53	
1.000	0.5501	0.3761	25.95	
1.500	0.7128	0.6918	59.86	
2.000	0.8756	1.0889	110.67	
3.000	1.2011	2.1273	272.24	
3.500	1.3639	2.7685	387.38	
4.000	1.5266	3.4912	528.19	

END FTABLE 2

FTABLE 3  
ROWS COLS \*\*\*  
12 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.9669	0.0483	0.13	
0.500	1.0637	0.4545	4.92	
1.000	1.1846	1.0165	17.12	
1.500	1.3055	1.6390	34.92	
2.000	1.4264	2.3220	57.95	

2.500	1.5473	3.0654	86.14
3.000	1.6682	3.8693	119.53
3.500	1.7891	4.7336	158.24
4.000	1.9100	5.6584	202.41
4.500	2.0294	6.6310	251.52
5.000	2.1488	7.6624	306.28

END FTABLE 3

FTABLE 4

ROWS COLS \*\*\*

7 5

DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2***
0.00	3.00	0.00	0.00	0.00
2.50	4.50	9.38	0.00	0.11
3.00	6.00	12.00	6.00	0.11
4.00	10.00	20.00	13.00	0.11
5.00	15.00	32.50	20.00	0.11
6.00	20.00	50.00	26.00	0.11
7.00	25.00	72.50	168.00	0.11

END FTABLE 4

FTABLE 5

ROWS COLS \*\*\*

10 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.1010	0.0051	0.03	
0.500	0.1754	0.0603	1.46	
1.000	0.2684	0.1713	6.16	
1.500	0.3614	0.3288	14.89	
2.000	0.4544	0.5327	28.48	
2.500	0.5474	0.7832	47.70	
3.000	0.6404	1.0801	73.29	
3.500	0.7334	1.4236	105.94	
4.000	0.8264	1.8136	146.33	

END FTABLE 5

FTABLE 10

ROWS COLS \*\*\*

9 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.1010	0.0051	0.06	
0.500	0.1660	0.0585	2.27	
1.000	0.2472	0.1618	9.32	
1.500	0.3285	0.3057	22.08	
2.000	0.4097	0.4902	41.66	
2.500	0.4909	0.7154	69.09	
3.000	0.5722	0.9811	105.37	
4.000	0.6887	1.6116	209.70	

END FTABLE 10

POST AMBAUM DETENTION \*\*\*

FTABLE 11

ROWS COLS \*\*\*

11 4

DEPTH	AREA	VOLUME	OUTFLOW	***
-------	------	--------	---------	-----

AR 011483

0.000	0.0000	0.0000	0.00
1.000	0.1000	0.2300	3.90
2.000	0.2000	0.6000	6.30
3.000	0.3000	0.9700	8.10
4.000	0.4000	1.3400	11.10
5.000	0.5000	1.8200	16.00
6.000	0.6000	2.2700	19.10
7.000	0.7000	2.8300	21.60
8.000	0.8000	3.3700	30.80
9.000	0.9000	4.0000	38.10
10.000	1.0000	4.6500	74.10
10.500	1.1000	5.2000	133.00
11.000	1.1500	5.3000	500.00

END FTABLE 11

FTABLE 12  
ROWS COLS \*\*\*  
6 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.6327	0.0316	0.15	
0.500	0.7960	0.3174	5.87	
1.000	1.0002	0.7664	21.53	
1.500	1.2043	1.3176	46.43	
2.000	1.4085	1.9708	81.20	
3.000	1.8168	3.5834	183.79	
4.000	2.2251	5.6044	336.22	
5.000	2.6335	8.0337	545.30	
6.000	3.0418	10.8713	817.51	

END FTABLE 12

FTABLE 13  
ROWS COLS \*\*\*  
7 5

DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2***
0.000	40.000	0.0000	0.00	0.00
1.000	41.400	40.000	0.00	0.11
1.500	42.000	60.000	10.00	0.11
2.000	42.700	80.000	16.00	0.11
2.500	43.300	100.00	20.00	0.11
3.000	44.000	120.00	28.00	0.11
5.000	45.000	210.00	45.00	0.11

END FTABLE 13

FTABLE 14  
ROWS COLS \*\*\*  
6 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.3361	0.0168	0.24	
0.500	0.3809	0.1602	9.04	
1.000	0.4370	0.3647	31.61	
1.500	0.4930	0.5972	65.00	
2.000	0.5491	0.8577	108.85	
2.500	0.6051	1.1462	163.33	
3.000	0.6612	1.4628	228.78	

END FTABLE 14

FTABLE 15  
 ROWS COLS \*\*\*  
 4 4  
 DEPTH AREA VOLUME OUTFLOW \*\*\*  
 0.00 0.10 0.00 0.00  
 1.00 1.00 0.55 91.00  
 2.00 1.10 1.60 268.00  
 3.00 1.20 2.75 493.00  
 END FTABLE 15

FTABLE 16  
 ROWS COLS \*\*\*  
 4 4  
 DEPTH AREA VOLUME OUTFLOW \*\*\*  
 0.00 0.10 0.00 0.00  
 1.00 1.00 0.55 74.00  
 2.00 1.10 1.60 219.00  
 3.00 1.20 2.75 403.00  
 END FTABLE 16

FTABLE 17  
 ROWS COLS \*\*\*  
 5 4  
 DEPTH AREA VOLUME OUTFLOW \*\*\*  
 0.00 0.10 0.00 0.00  
 1.00 1.00 0.55 59.00  
 2.00 1.10 1.60 173.00  
 3.00 1.20 2.75 318.00  
 4.00 1.30 4.00 484.00  
 END FTABLE 17

FTABLE 23  
 ROWS COLS \*\*\* HERMES  
 9 5  
 DEPTH AREA VOLUME OUTFLOW OUTFLOW \*\*\*  
 0.00 0.00 0.00 0.00 0.00 0.00  
 5.00 0.50 1.91 0.00 0.00 305.00  
 11.00 0.79 5.79 0.00 0.00 311.00  
 15.00 1.13 9.64 0.50 0.01 315.00  
 19.00 1.72 15.34 0.50 0.05 319.00  
 29.00 2.86 38.25 0.50 0.10 329.00  
 39.00 4.40 74.55 0.50 0.20 339.00  
 50.00 6.22 132.98 0.50 0.30 350.00  
 60.00 10.00 1212.98 0.50 0.40 360.00  
 END FTABLE 23

FTABLE 24  
 ROWS COLS \*\*\*  
 9 4  
 DEPTH AREA VOLUME OUTFLOW \*\*\*  
 0.000 0.0000 0.0000 0.00  
 0.100 0.2571 0.0129 0.16  
 0.500 0.3873 0.1417 6.53  
 1.000 0.5501 0.3761 25.95  
 1.500 0.7128 0.6918 59.86  
 2.000 0.8756 1.0889 110.67

3.000	1.2011	2.1273	272.24
3.500	1.3639	2.7685	387.38
4.000	1.5266	3.4912	528.19

END FTABLE 24

FTABLE 33  
ROWS COLS \*\*\*  
11 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.00	1.00	0.00	0.00	
0.50	1.20	0.55	2.00	
1.00	1.40	1.20	6.00	
1.50	1.60	1.95	9.00	
2.00	1.80	2.80	13.00	
2.50	2.00	3.75	16.50	
3.00	2.20	4.80	20.00	
3.50	2.40	5.95	23.00	
4.00	2.60	7.20	26.00	
5.00	2.80	9.90	104.00	
6.00	3.00	12.80	246.00	

END FTABLE 33

FTABLE 34  
ROWS COLS \*\*\* REVISED 11/19/97 BASED ON HEC-RAS MODEL  
6 5

DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2***
0.00	3.00	0.00	0.00	0.00
3.00	3.05	9.08	0.00	0.11
4.00	3.10	12.15	0.00	0.11
5.00	3.15	15.28	0.00	0.11
6.00	3.20	18.45	72.0	0.11
7.00	3.25	21.68	225.0	0.11

END FTABLE 34

FTABLE 35  
ROWS COLS \*\*\* REVISED 11/19/97 BASED ON HECRAS MODEL  
5 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.00	0.10	0.00	0.00	
1.00	1.10	0.60	38.00	
2.00	1.20	1.75	108.00	
3.00	1.30	3.00	194.00	
4.00	1.40	4.35	290.00	

END FTABLE 35

FTABLE 38  
ROWS COLS \*\*\*  
7 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
1.000	0.4000	0.4000	2.00	
1.500	0.5000	1.0000	4.00	
2.000	0.9000	1.3000	11.00	
2.500	1.3000	1.6000	15.00	
3.000	1.6000	2.0000	18.00	
3.500	1.9000	2.5000	20.80	

END FTABLE 38



FTABLE 45  
 ROWS COLS \*\*\*  
 NORTH EMPLOYEE PARKING LOT VAULT (AS-BUILT)\*\*\*

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.2200	0.0000	0.00	
2.000	0.2200	0.4500	1.20	
4.000	0.2200	0.9000	1.70	
6.000	0.2200	1.3400	2.10	
8.000	0.2200	1.7900	2.40	
10.000	0.2200	2.2400	2.70	
12.240	0.2200	2.7400	3.00	
14.000	0.2200	3.1400	6.90	
15.440	0.2200	3.4600	8.30	
16.000	0.2200	3.5800	10.30	
18.000	0.2200	4.0300	13.60	
20.000	0.2200	4.4800	30.79	

END FTABLE 45

FTABLE 50  
 ROWS COLS \*\*\*

DEPTH	AREA	VOLUME	OUTFLOW	***
0.00	1.00	0.00	0.00	
0.50	1.10	0.53	5.00	
1.00	1.20	1.10	15.00	
1.50	1.30	1.73	25.00	
2.00	1.40	2.40	35.00	
2.50	1.50	3.13	52.00	
3.00	1.60	3.90	70.00	
3.50	1.70	4.73	87.00	
4.00	1.80	5.60	105.00	
6.00	1.90	9.30	165.00	

END FTABLE 50

FTABLE 52  
 ROWS COLS \*\*\*

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.3680	0.0184	0.25	
0.500	0.3717	0.1664	9.39	
1.000	0.3763	0.3534	31.06	
2.000	0.3819	0.7325	94.37	
3.000	0.3874	1.1171	174.33	

END FTABLE 52

FTABLE 53  
 OLD LAKE REBA \*\*\*  
 MAX DEPTH = 4.9 FEET \*\*\*  
 30" CMP, 40 CFS DISCHARGE AT MAX DEPTH \*\*\*  
 ROWS COLS \*\*\*

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	2.4000	0.0000	0.00	
1.000	2.5800	2.5000	18.00	

2.000	2.9400	5.3000	26.00
3.000	3.4100	8.4000	31.00
4.000	3.8800	12.100	36.00
4.900	4.3000	15.800	40.00
6.000	4.3000	15.810	500.00

END FTABLE 53

FTABLE 54

EXISTING MILLER CREEK DETENTION FACILITY\*\*\* REVISED STORAGE/Q DATA

GATE SETTING: 2.0 FEET\*\*\*

ROWS COLS \*\*\*

12 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.00	0.00	0.00	
1.300	0.01	0.01	10.00	
2.000	0.01	0.02	20.00	
2.900	0.70	0.40	30.00	
4.000	1.50	1.50	40.00	
5.400	3.50	4.90	50.00	
7.000	8.60	13.30	60.00	
8.800	15.60	34.80	70.00	
10.000	19.90	57.30	76.00	
10.500	21.50	68.00	92.00	
11.000	23.10	78.80	179.00	
11.500	24.70	88.60	303.00	

END FTABLE 54

PRE AMBAUM DETENTION \*\*\*

FTABLE 111

ROWS COLS \*\*\*

12 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.500	0.2160	0.0750	5.30	
1.000	0.2730	0.1990	21.10	
1.500	0.2890	0.3410	43.90	
2.000	0.2900	0.4830	68.80	
2.500	0.2910	0.6070	89.10	
3.000	0.2950	0.6820	90.00	
3.500	0.3000	2.1000	100.00	
4.000	0.3050	2.5000	105.00	
4.500	0.3100	3.0000	110.00	
5.000	0.3200	3.5000	120.00	
5.500	0.3300	4.0000	130.00	

END FTABLE111

FTABLE 135

ROWS COLS \*\*\* VACA FARM

6 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.00	0.10	0.00	0.00	
1.00	0.10	0.10	4.00	
2.00	0.11	0.21	8.00	
2.50	1.00	0.48	13.00	
3.50	6.50	4.23	86.00	
4.50	13.00	13.98	235.00	

END FTABLE135

\*\*\*

END FTABLES

```

MASS-LINK
<Volume>  <-Grp> <-Member-><--Mult-->  <Target>  <-Grp> <-Member->***
<Name>     <Name> # #<-factor->         <Name>     <Name> # #***
  MASS-LINK      1
  conversion from acre-inches to acre-ft (1/12)      ***
PERLND  PWATER  PERO      0.0833333  RCHRES      INFLOW IVOL
  END MASS-LINK      1

  MASS-LINK      2
IMPLND  IWATER  SURO      0.0833333  RCHRES      INFLOW IVOL
  END MASS-LINK      2

  MASS-LINK      3
RCHRES  ROFLOW      RCHRES      INFLOW
  END MASS-LINK      3

  MASS-LINK      4
RCHRES  OFLOW  OVOL  1      RCHRES      INFLOW IVOL
  END MASS-LINK      4

  MASS-LINK      5
RCHRES  OFLOW  OVOL  2      RCHRES      INFLOW IVOL
  END MASS-LINK      5

  MASS-LINK      6
PERLND  PWATER  SURO      0.0833333  RCHRES      INFLOW IVOL
PERLND  PWATER  IFWO      0.0833333  RCHRES      INFLOW IVOL
  END MASS-LINK      6

  MASS-LINK      7
PERLND  PWATER  AGWO      0.0833333  RCHRES      INFLOW IVOL
  END MASS-LINK      7

  MASS-LINK      8
PERLND  PWATER  PERO      0.0833333  COPY        INPUT  MEAN
  END MASS-LINK      8
  MASS-LINK      12
PERLND  PWATER  AGWO      0.0833333  COPY        INPUT  MEAN
  END MASS-LINK      12

  MASS-LINK      9
IMPLND  IWATER  SURO      0.0833333  COPY        INPUT  MEAN
  END MASS-LINK      9

  MASS-LINK      10
COPY    OUTPUT  MEAN      RCHRES      INFLOW IVOL
  END MASS-LINK      10

```

END MASS-LINK  
END RUN

AR 011489

RUN

GLOBAL

\*\*\* SEATAC AIRPORT HSPF BASIN MODEL OF MILLER CREEK  
\*\*\* FILE: MLOWFLOW2.INP - 2006 future condition  
\*\*\* LOW FLOW ANALYSIS  
\*\*\* BASED ON MILL65.INP FILE FROM AQUA TERRA  
\*\*\* REMOVED FILL AREAS  
\*\*\* PERLND 80 is the groundwater PERLND for the Fill area.  
\*\*\* FOLLOWING STATEMENTS MAY NOT APPLY  
\*\*\* ADDED PERLND 47,57,  
\*\*\* ADDED GROUND WATER INFILTRATION TO WDM FOR USE WITH MCAGWO.INP  
\*\*\* FK revised SDW1A and SDW1B with flow splitters, storages at SDN3/3X,  
SDN2X/4X;  
\*\*\* FK revised MC-1 and SDN-2X land uses, added POC at Lake Reba, removed  
run-of-river tables

\*\*\* FOUR YEAR RUN USING CALIBRATION INITIAL CONDITIONS

MILLER CREEK BASIN HSPF MODEL

\*\*\* START 1994 1 1 0 0 END 1996 8 30 24 0  
START 1990 10 1 0 0 END 1994 9 30 24 0  
RUN INTERP OUTPUT LEVEL 3  
RESUME 0 RUN 1

END GLOBAL

FILES

<type> <fun>\*\*\*<-----fname----->  
MESSU 24 D:\PARA\SEATAC\MILLER\LOWFLOW\MLOWFLOW2.MES  
WDM 25 D:\PARA\SEATAC\MILLER\LOWFLOW\MLOWFLOW.WDM  
61 D:\PARA\SEATAC\MILLER\LOWFLOW\PER.L61  
62 D:\PARA\SEATAC\MILLER\LOWFLOW\RCH.L62

END FILES

OPN SEQUENCE

INGRP INDELT 01:00  
PERLND 16  
PERLND 26  
PERLND 34  
PERLND 44  
PERLND 45  
\*\*\* special PERLND for infiltration SDW1A  
PERLND 47  
PERLND 54  
\*\*\* special PERLND for infiltration SDW1B  
PERLND 57  
\*\*\* PERLND FOR INFLOW OF LOW FLOW FROM FILL PGG  
PERLND 80  
IMPLND 14  
RCHRES 1  
RCHRES 23  
RCHRES 24  
RCHRES 2  
RCHRES 3  
RCHRES 33  
RCHRES 4  
RCHRES 5  
RCHRES 50  
RCHRES 242  
RCHRES 240

AR 011490

COPY 61  
 COPY 44  
 RCHRES 51  
 RCHRES 43  
 RCHRES 451  
 RCHRES 452  
 COPY 45  
 COPY 645  
 RCHRES 46  
 RCHRES 552  
 RCHRES 52  
 RCHRES 53  
 COPY 53  
 RCHRES 54  
 RCHRES 37  
 RCHRES 237  
 COPY 37  
 RCHRES 147  
 RCHRES 247  
 COPY 66  
 COPY 69  
 RCHRES 47  
 COPY 62  
 COPY 63  
 COPY 67  
 COPY 68

\*\*\* output special PERLND outflow to check

COPY 47  
 COPY 70  
 RCHRES 34  
 RCHRES 135  
 RCHRES 570  
 RCHRES 57  
 RCHRES 257  
 COPY 64  
 COPY 65  
 COPY 357  
 COPY 56

\*\*\* output special PERLND outflow to check

COPY 57  
 COPY 71  
 RCHRES 35  
 COPY 55  
 RCHRES 10  
 RCHRES 16  
 RCHRES 11  
 RCHRES 13  
 RCHRES 12  
 RCHRES 15  
 RCHRES 14  
 RCHRES 17

END INGRP

END OPN SEQUENCE

\*\*\*

PERLND

GEN-INFO

<PLS >

Name

NBLKS

Unit-systems

Printer

\*\*\*

AR 011491

```

# - # User t-series Engl Metr
in out
16 TFM- TILL FOR MOD 1 1 1 1 61 0
26 TGM- TILL GR MOD 1 1 1 1 61 0
34 OF - OUTWASH FOR 1 1 1 1 61 0
44 OG - OUTWASH GR 1 1 1 1 61 0
***PERLND FOR NEW AIRPORT FILL; NONE IN CALIBRATION
45 AIRPORT FILL 1 1 1 1 61 0
47 OG - INFILTRATION 1 1 1 1 61 0
54 SA - WETLANDS 1 1 1 1 61 0
57 OG - INFILTRATION 3 1 1 1 1 61 0
80 LOW FLOW 1 1 1 1 61 0

```

END GEN-INFO

ACTIVITY

```

<PLS > ***** Active Sections *****
# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC
14 200 0 0 1 0 0 0 0 0 0 0 0 0 0

```

END ACTIVITY

PRINT-INFO

```

<PLS > ***** Print-flags ***** PIVL PYR
# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC *****
14 200 0 0 5 0 0 0 0 0 0 0 0 0 1 9

```

END PRINT-INFO

PWAT-PARM1

```

<PLS > ***** Flags *****
# - # CSNO RTOP UZFG VCS VUZ VNN VIFW VIRC VLE
14 200 0 0 0 0 0 0 0 0 0

```

END PWAT-PARM1

PWAT-PARM2

```

<PLS > ***
# - # ***FOREST LZSN INFILT LSUR SLSUR KVARY AGWRC
16 9.0000 0.3200 400.00 0.1000 0.5000 0.9960
26 9.0000 0.1200 400.00 0.1000 0.5000 0.9960
34 10.0000 2.0000 400.00 0.0500 0.3000 0.9960
44 10.0000 0.8000 400.00 0.0500 0.3000 0.9960
45 7.5000 0.0200 300.00 0.0700 0.0000 0.9960
47 10.0000 0.8000 400.00 0.0500 0.3000 0.9960
54 8.0000 2.0000 100.00 0.0010 0.5000 0.9960
57 10.0000 0.8000 400.00 0.0500 0.3000 0.9960
80 9.0000 0.1200 400.00 0.1000 0.5000 0.9960

```

END PWAT-PARM2

PWAT-PARM3

```

<PLS > ***
# - # *** PETMAX PETMIN INFEXP INFILD DEEPFR BASETP AGWETP
16 2.0000 2.0000 0.33 0.00 0.0
26 2.0000 2.0000 0.33 0. 0.
34 2.0000 2.0000 0.33 0.00 0.0
44 2.0000 2.0000 0.33 0. 0.
47 2.0000 2.0000 0.33 0. 0.
45 2.0000 2.0000 0.33 0. 0.
54 10.000 2.0000 0.33 0. 0.7
57 2.0000 2.0000 0.33 0. 0.
80 2.0000 2.0000 0.33 0. 0.

```

END PWAT-PARM3

PWAT-PARM4

```

<PLS >
# - # CEPSC UZSN NSUR INTFW IRC LZETP***

```

16	0.2000	0.7500	0.3500	9.000	0.7000	0.7000
26	0.1000	0.3750	0.2500	9.000	0.7000	0.2500
34	0.2000	0.7500	0.3500	0.000	0.7000	0.7000
44	0.1000	0.7500	0.2500	0.000	0.7000	0.2500
47	0.1000	0.7500	0.2500	0.000	0.7000	0.2500
45	0.1000	0.2800	0.2500	6.000	0.1500	0.6000
54	0.1000	2.2500	0.5000	1.000	0.7000	0.8000
57	0.1000	0.7500	0.2500	0.000	0.7000	0.2500
80	0.1000	0.3750	0.2500	9.000	0.7000	0.2500

END PWAT-PARM4

PWAT-STATE1

<PLS > PWATER state variables\*\*\*

#	#***	CEPS	SURS	UZS	IFWS	LZS	AGWS	GWVS
16		0.078	0.	0.2500	0.10	2.000	2.000	0.000
26		0.051	0.	0.2500	0.10	2.000	2.000	0.000
34		0.078	0.	0.2500	0.10	2.000	2.000	0.000
44		0.051	0.	0.2500	0.10	2.000	2.000	0.000
47		0.051	0.	0.2500	0.10	2.000	2.000	0.000
45		0.051	0.	0.2500	0.10	2.000	2.000	0.000
54		0.051	0.	0.2500	0.10	2.000	2.000	0.000
57		0.051	0.	0.2500	0.10	2.000	2.000	0.000
80		0.051	0.	0.2500	0.10	2.000	2.000	0.000

END PWAT-STATE1

END PERLND

IMPLND

GEN-INFO

<ILS >	Name	Unit-systems			Printer		***
# - #		User	t-series	Engl	Metr		***
		in	out				***
14	IMPERVIOUS	1	1	1	60	0	

END GEN-INFO

ACTIVITY

<ILS >	***** Active Sections ****						***
# - #	ATMP	SNOW	IWAT	SLD	IWG	IQAL	***
14	0	0	1	0	0	0	

END ACTIVITY

PRINT-INFO

<ILS >	***** Print-flags *****						PIVL	PYR	***
# - #	ATMP	SNOW	IWAT	SLD	IWG	IQAL	*****		***
14	0	0	6	0	0	0	1	9	

END PRINT-INFO

IWAT-PARM1

<ILS >	Flags						***
# - #	CSNO	RTOP	VRS	VNN	RTLI	***	***
14	0	0	0	0	0		

END IWAT-PARM1

IWAT-PARM2

<ILS >					***
# - #	LSUR	SLSUR	NSUR	RETSC	***
14	100.00	0.0100	0.1000	0.1000	

END IWAT-PARM2

IWAT-PARM3

<ILS >			***
# - #	PETMAX	PETMIN	***
14			

```

END IWAT-PARM3
IWAT-STATE1
  <ILS > IWATER state variables
  # - #      RETS      SURS
  14      1.0000E-3  1.0000E-3
END IWAT-STATE1
END IMPLND

```

EXT SOURCES

\*\*\* NOTE: The only RCHRES that precip and PET are applied to are lakes and ponds  
 \*\*\* FOLLOWING RCHRES ARE PONDS: 57, 247, 237

```

<-Volume-> <Member> SsysSgap<--Mult-->Tran <-Target vols> <-Grp> <-Member->
<Name> # <Name> # tem strg<-factor->strg <Name> # # <Name> # #
*** PRECIP/EVAP TO PERVIOUS/IMPERV SURFACES
WDM 1002 PREC ENGLZERO 1.00 PERLND 14 65 EXTNL PREC
WDM 1002 PREC ENGLZERO 0.0 PERLND 80 EXTNL PREC
WDM 1002 PREC ENGLZERO 1.00 IMPLND 14 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 PERLND 14 65 EXTNL PETINP
WDM 1 EVAP ENGLZERO 0.0 PERLND 80 EXTNL PETINP
WDM 1 EVAP ENGLZERO 0.8 IMPLND 14 EXTNL PETINP
*** --> lateral inflow from reinfiltration chamber for SDW1A
*** WDM 5 FLOW ENGLZERO 1.0 PERLND 47 EXTNL AGWLI
*** --> lateral inflow from reinfiltration chamber for SDW1B
*** WDM 6 FLOW ENGLZERO 1.0 PERLND 57 EXTNL AGWLI
*** PRECIP/EVAP TO LAKES
WDM 1002 PREC ENGLZERO RCHRES 1 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 1 EXTNL POTEV
WDM 1002 PREC ENGLZERO RCHRES 4 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 4 EXTNL POTEV
WDM 1002 PREC ENGLZERO RCHRES 11 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 11 EXTNL POTEV
WDM 1002 PREC ENGLZERO RCHRES 13 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 13 EXTNL POTEV
WDM 1002 PREC ENGLZERO RCHRES 23 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 23 EXTNL POTEV
WDM 1002 PREC ENGLZERO RCHRES 34 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 34 EXTNL POTEV
WDM 1002 PREC ENGLZERO RCHRES 53 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 53 EXTNL POTEV
WDM 1002 PREC ENGLZERO RCHRES 54 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 54 EXTNL POTEV
WDM 1002 PREC ENGLZERO RCHRES 237 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 237 EXTNL POTEV
WDM 1002 PREC ENGLZERO RCHRES 247 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 247 EXTNL POTEV
WDM 1002 PREC ENGLZERO RCHRES 57 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 57 EXTNL POTEV
*** till seepage groundwater flow from Fill area. PGG time series
WDM 7001 FLOW ENGLZERO.000000099 PERLND 80 EXTNL AGWLI
*** Fill flow directly to stream
WDM 7000 FLOW ENGL .000000957 RCHRES 35 INFLOW IVOL

```

END EXT SOURCES

EXT TARGETS



```

<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Volume-> <Member> Tsys Tgap Amd ***
<Name> # <Name> # #<-factor->strg <Name> # <Name> tem strg strg***
***PROJECT CONDITION FLOWS
*** RCHRES=LOCATION:
*** 54=MCDF 47=SDW1A INFILTRATION TANK 43=SDN3X 247=SDW1A POND G
*** 17=MOUTH 49=SDW2 44=SDN4X 52=SDN1 451= EXISTING NEPL
*** 61=SDN2X 57=SDW1B 51=SDN2X+SDN4X 53=Lake Reba 452=NEW NEPL
*** 45=NEPL POC 55=SR509 39=SDN3A/SDW1A POC
*** 46=CARGO 37=SDN3AI VAULT 237=SDN3AO POND
*** GAUGE POINTS (17=MOUTH, 54=MILLER RDF, 55=SR509)
RCHRES 17 HYDR RO 1 1 WDM 7017 FLOW ENGL REPL
RCHRES 35 HYDR RO 1 1 WDM 7036 FLOW ENGL REPL
***COPY 55 OUTPUT MEAN 1 1 12.1 WDM 118 FLOW ENGL REPL
RCHRES 54 HYDR RO 1 1 WDM 7054 FLOW ENGL REPL
*** DETENTION POND FLOWS
***COPY 61 OUTPUT MEAN 1 1 12.1 WDM 101 FLOW ENGL REPL
***RCHRES 552 HYDR RO 1 1 WDM 102 FLOW ENGL REPL
***RCHRES 451 HYDR RO 1 1 WDM 105 FLOW ENGL REPL
***RCHRES 452 HYDR RO 1 1 WDM 119 FLOW ENGL REPL
***RCHRES 46 HYDR RO 1 1 WDM 106 FLOW ENGL REPL
*** write RCHRES 47 (Inf. Area # 1)outlet 1 and 2 to WDM 107 and 108 like so:
***COPY 62 OUTPUT MEAN 1 1 12.1 WDM 107 FLOW ENGL REPL
***COPY 63 OUTPUT MEAN 1 1 12.1 WDM 108 FLOW ENGL REPL
***COPY 66 OUTPUT MEAN 1 1 12.1 WDM 112 FLOW ENGL REPL
***COPY 69 OUTPUT MEAN 1 1 12.1 WDM 1120 FLOW ENGL REPL
*** write SDW1a vault flows to WDM:
***COPY 67 OUTPUT MEAN 1 1 12.1 WDM 109 FLOW ENGL REPL
***COPY 68 OUTPUT MEAN 1 1 12.1 WDM 1090 FLOW ENGL REPL
*** write RCHRES 570 outlet 1 and 2 to WDM 110 and 115 like so:
***RCHRES 570 HYDR RO 1 1 WDM 210 FLOW ENGL REPL
***COPY 64 OUTPUT MEAN 1 1 12.1 WDM 110 FLOW ENGL REPL
***COPY 65 OUTPUT MEAN 1 1 12.1 WDM 115 FLOW ENGL REPL
***COPY 357 OUTPUT MEAN 1 1 12.1 WDM 211 FLOW ENGL REPL
***COPY 56 OUTPUT MEAN 1 1 12.1 WDM 121 FLOW ENGL REPL
*** write RCHRES 37 vault to WDM 111
***RCHRES 37 HYDR RO 1 1 WDM 111 FLOW ENGL REPL
***RCHRES 237 HYDR RO 1 1 WDM 122 FLOW ENGL REPL
***RCHRES 43 HYDR RO 1 1 WDM 103 FLOW ENGL REPL
***COPY 44 OUTPUT MEAN 1 1 12.1 WDM 104 FLOW ENGL REPL
***RCHRES 51 HYDR RO 1 1 WDM 139 FLOW ENGL REPL
*** DETENTION STAGES
***RCHRES 47 HYDR STAGE WDM 652 STAG ENGL REPL
***RCHRES 147 HYDR STAGE WDM 657 STAG ENGL REPL
***RCHRES 247 HYDR STAGE WDM 654 STAG ENGL REPL
***RCHRES 552 HYDR STAGE WDM 601 STAG ENGL REPL
***RCHRES 57 HYDR STAGE WDM 651 STAG ENGL REPL
***RCHRES 257 HYDR STAGE WDM 655 STAG ENGL REPL
***RCHRES 237 HYDR STAGE WDM 656 STAG ENGL REPL
***RCHRES 37 HYDR STAGE WDM 650 STAG ENGL REPL
***RCHRES 54 HYDR STAGE WDM 61 STAG ENGL REPL
***RCHRES 451 HYDR STAGE WDM 662 STAG ENGL REPL
***RCHRES 452 HYDR STAGE WDM 667 STAG ENGL REPL
***RCHRES 46 HYDR STAGE WDM 663 STAG ENGL REPL
***RCHRES 43 HYDR STAGE WDM 664 STAG ENGL REPL
***RCHRES 44 HYDR STAGE WDM 665 STAG ENGL REPL
***RCHRES 51 HYDR STAGE WDM 666 STAG ENGL REPL
*** DETENTION VOLUMES

```

```

***RCHRES 47 HYDR VOL WDM 752 VOL ENGL REPL
***RCHRES 147 HYDR VOL WDM 757 VOL ENGL REPL
***RCHRES 247 HYDR VOL WDM 754 VOL ENGL REPL
***RCHRES 552 HYDR VOL WDM 602 VOL ENGL REPL
***RCHRES 57 HYDR VOL WDM 751 VOL ENGL REPL
***RCHRES 257 HYDR VOL WDM 755 VOL ENGL REPL
***RCHRES 237 HYDR VOL WDM 756 VOL ENGL REPL
***RCHRES 37 HYDR VOL WDM 750 VOL ENGL REPL
***RCHRES 54 HYDR VOL WDM 62 VOL ENGL REPL
***RCHRES 451 HYDR VOL WDM 762 VOL ENGL REPL
***RCHRES 452 HYDR VOL WDM 767 VOL ENGL REPL
***RCHRES 46 HYDR VOL WDM 763 VOL ENGL REPL
***RCHRES 43 HYDR VOL WDM 764 VOL ENGL REPL
***RCHRES 44 HYDR VOL WDM 765 VOL ENGL REPL
***RCHRES 51 HYDR VOL WDM 766 VOL ENGL REPL

```

\*\*\* POINT OF COMPLIANCE (POC) FLOWS

```

***COPY 37 OUTPUT MEAN 1 1 12.1 WDM 125 FLOW ENGL REPL
***COPY 45 OUTPUT MEAN 1 1 12.1 WDM 199 FLOW ENGL REPL
***COPY 53 OUTPUT MEAN 1 1 12.1 WDM 399 FLOW ENGL REPL
***COPY 70 OUTPUT MEAN 1 1 12.1 WDM 7000 FLOW ENGL REPL
***COPY 71 OUTPUT MEAN 1 1 12.1 WDM 7001 FLOW ENGL REPL

```

\*\*\* SPECIAL PERLND REINFILTRATION RESULTS

```

*** --> output special PERLND parameters to check operations:
*** --> PERLND 47 active ground water storage depth (in)
*** PERLND 47 PWATER AGWS WDM 471 AGWS ENGL
REPL
*** --> PERLND 47 active ground water outflow (acft/2ac -> in/acre)
***COPY 47 OUTPUT MEAN 1 1 12 WDM 472 FLOW ENGL REPL
*** --> PERLND 57 active ground water storage depth (in)
***PERLND 57 PWATER AGWS WDM 571 AGWS ENGL REPL
*** --> PERLND 57 active ground water outflow (acft/2ac -> in/acre)
***COPY 57 OUTPUT MEAN 1 1 12 WDM 572 FLOW ENGL REPL

```

END EXT TARGETS

SCHEMATIC

```

<-Source-> <--Area--> <-Target-> MBLK ***
<Name> # <-factor-> <Name> # Tbl# ***
*** SUB-CATCHMENT 1 all agwo goes to sound
PERLND 16 3.41 RCHRES 1 6
PERLND 26 232.36 RCHRES 1 6
PERLND 34 3.07 RCHRES 1 6
PERLND 44 38.03 RCHRES 1 6
PERLND 54 3.87 RCHRES 1 6
IMPLND 14 56.14 RCHRES 1 2
*** SUB-CATCHMENT 2 10% of area GW goes to vaca 90% goes to sound
PERLND 16 5.56 RCHRES 2 6
PERLND 26 200.05 RCHRES 2 6
PERLND 34 0.46 RCHRES 2 6
PERLND 44 38.71 RCHRES 2 6
PERLND 16 0.56 RCHRES 135 7
PERLND 26 20.00 RCHRES 135 7
PERLND 34 0.05 RCHRES 135 7
PERLND 44 3.87 RCHRES 135 7
IMPLND 14 42.22 RCHRES 2 2
*** SUB-CATCHMENT 23 New subbasin 15 % OF GW GOES TO VACCA 85% TO SOUND
PERLND 16 3.09 RCHRES 23 6

```

PERLND	26	156.15	RCHRES	23	6
PERLND	34	2.25	RCHRES	23	6
PERLND	44	45.84	RCHRES	23	6
PERLND	16	0.46	RCHRES	135	7
PERLND	26	23.42	RCHRES	135	7
PERLND	34	0.34	RCHRES	135	7
PERLND	44	6.88	RCHRES	135	7
IMPLND	14	58.44	RCHRES	23	2
*** SUB-CATCHMENT 24 New subbasin 60 % OF GW GOES TO 11 40% TO SOUND					
PERLND	26	135.43	RCHRES	24	6
PERLND	34	2.02	RCHRES	24	6
PERLND	44	69.29	RCHRES	24	6
PERLND	26	81.26	RCHRES	11	7
PERLND	34	1.21	RCHRES	11	7
PERLND	44	41.57	RCHRES	11	7
IMPLND	14	79.98	RCHRES	24	2
*** SUB-CATCHMENT 3 agwo goes to vaca(135)					
PERLND	16	8.26	RCHRES	3	6
PERLND	26	108.38	RCHRES	3	6
PERLND	34	16.02	RCHRES	3	6
PERLND	44	102.89	RCHRES	3	6
PERLND	54	0.04	RCHRES	3	6
PERLND	16	8.26	RCHRES	135	7
PERLND	26	108.38	RCHRES	135	7
PERLND	34	16.02	RCHRES	135	7
PERLND	44	102.89	RCHRES	135	7
PERLND	54	0.04	RCHRES	135	7
IMPLND	14	27.30	RCHRES	3	2
*** SUB-CATCHMENT 4 10% of agwo goes to rchres 90% goes to sound					
PERLND	16	2.95	RCHRES	4	6
PERLND	26	85.95	RCHRES	4	6
PERLND	34	3.75	RCHRES	4	6
PERLND	44	92.06	RCHRES	4	6
PERLND	16	0.30	RCHRES	4	7
PERLND	26	8.59	RCHRES	4	7
PERLND	34	0.38	RCHRES	4	7
PERLND	44	9.21	RCHRES	4	7
IMPLND	14	18.43	RCHRES	4	2
*** SUB-CATCHMENT 4a 70% of agwo goes to rchres 30% goes to sound					
PERLND	16	8.66	RCHRES	4	6
PERLND	26	61.64	RCHRES	4	6
PERLND	34	22.06	RCHRES	4	6
PERLND	44	78.09	RCHRES	4	6
PERLND	54	12.50	RCHRES	4	6
PERLND	16	6.06	RCHRES	4	7
PERLND	26	43.15	RCHRES	4	7
PERLND	34	15.44	RCHRES	4	7
PERLND	44	54.66	RCHRES	4	7
PERLND	54	8.75	RCHRES	4	7
IMPLND	14	29.14	RCHRES	4	2
*** SUB-CATCHMENT 5					
PERLND	26	10.29	RCHRES	5	1
PERLND	44	50.05	RCHRES	5	1
PERLND	54	10.74	RCHRES	5	1
IMPLND	14	16.31	RCHRES	5	2
*** SUB-CATCHMENT 6					
PERLND	16	1.42	COPY	645	26

PERLND	26	20.38	COPY	645	26
PERLND	34	13.44	COPY	645	26
PERLND	44	11.79	COPY	645	26
PERLND	54	0.82	COPY	645	26
PERLND	16	1.42	RCHRES	53	7
PERLND	26	20.38	RCHRES	53	7
PERLND	34	13.44	RCHRES	53	7
PERLND	44	11.79	RCHRES	53	7
PERLND	54	0.82	RCHRES	53	7
IMPLND	14	6.23	COPY	645	22
*** SUB-CATCHMENT 8					
PERLND	44	22.21	RCHRES	35	1
IMPLND	14	6.60	RCHRES	35	2
*** SUB-CATCHMENT 9					
PERLND	16	4.98	RCHRES	34	1
PERLND	26	14.38	RCHRES	34	1
PERLND	34	0.05	RCHRES	34	1
PERLND	44	56.71	RCHRES	34	1
PERLND	54	0.01	RCHRES	34	1
IMPLND	14	22.47	RCHRES	34	2
*** SUB-CATCHMENT 10					
PERLND	16	4.15	RCHRES	10	1
PERLND	26	31.94	RCHRES	10	1
PERLND	44	95.22	RCHRES	10	1
IMPLND	14	71.98	RCHRES	10	2
*** SUB-CATCHMENT 11 25% OF AGWO GOES TO 15					
PERLND	16	0.89	RCHRES	11	6
PERLND	26	217.92	RCHRES	11	6
PERLND	34	1.32	RCHRES	11	6
PERLND	44	65.65	RCHRES	11	6
PERLND	16	0.67	RCHRES	11	7
PERLND	26	163.44	RCHRES	11	7
PERLND	34	0.99	RCHRES	11	7
PERLND	44	49.24	RCHRES	11	7
PERLND	16	0.22	RCHRES	15	7
PERLND	26	54.48	RCHRES	15	7
PERLND	34	0.33	RCHRES	15	7
PERLND	44	16.41	RCHRES	15	7
IMPLND	14	230.80	RCHRES	11	2
*** SUB-CATCHMENT 12					
PERLND	16	0.39	RCHRES	12	1
PERLND	26	101.18	RCHRES	12	1
PERLND	34	5.64	RCHRES	12	1
PERLND	44	54.98	RCHRES	12	1
PERLND	54	0.64	RCHRES	12	1
IMPLND	14	79.83	RCHRES	12	2
*** SUB-CATCHMENT 13					
PERLND	16	0.79	RCHRES	13	1
PERLND	26	197.68	RCHRES	13	1
IMPLND	14	27.66	RCHRES	13	2
*** SUB-CATCHMENT 14 50% OF AGWO GOES TO SOUND					
PERLND	16	0.24	RCHRES	14	6
PERLND	26	118.67	RCHRES	14	6
PERLND	34	13.46	RCHRES	14	6
PERLND	44	41.91	RCHRES	14	6
PERLND	16	0.12	RCHRES	14	7
PERLND	26	59.34	RCHRES	14	7

PERLND 34	6.73	RCHRES 14	7
PERLND 44	20.95	RCHRES 14	7
IMPLND 14	20.66	RCHRES 14	2
*** SUB-CATCHMENT 15			
PERLND 16	6.59	RCHRES 15	1
PERLND 26	49.55	RCHRES 15	1
PERLND 34	50.09	RCHRES 15	1
PERLND 44	86.52	RCHRES 15	1
IMPLND 14	19.47	RCHRES 15	2
*** SUB-CATCHMENT 16			
PERLND 16	10.93	RCHRES 16	1
PERLND 26	29.93	RCHRES 16	1
PERLND 34	20.03	RCHRES 16	1
PERLND 44	31.83	RCHRES 16	1
IMPLND 14	15.58	RCHRES 16	2
*** SUB-CATCHMENT 17 AGWO GOES TO SOUND			
PERLND 16	0.90	RCHRES 17	6
PERLND 26	16.31	RCHRES 17	6
PERLND 34	34.82	RCHRES 17	6
PERLND 44	82.11	RCHRES 17	6
PERLND 54	2.19	RCHRES 17	6
IMPLND 14	10.49	RCHRES 17	2
*** SUB-CATCHMENT MC-1			
PERLND 26	0.14	RCHRES 52	1
PERLND 44	9.44	RCHRES 52	1
PERLND 45	0.14	RCHRES 52	1
PERLND 54	0.27	RCHRES 52	1
IMPLND 14	1.98	RCHRES 52	2
*** SUB-CATCHMENT MC-2			
PERLND 16	0.08	RCHRES 53	1
PERLND 26	0.53	RCHRES 53	1
PERLND 34	3.60	RCHRES 53	1
PERLND 44	9.20	RCHRES 53	1
PERLND 45	2.22	RCHRES 53	1
PERLND 54	15.14	RCHRES 53	1
IMPLND 14	2.54	RCHRES 53	2
*** SUB-CATCHMENT MC-3			
PERLND 34	3.70	RCHRES 54	1
PERLND 44	4.91	RCHRES 54	1
PERLND 45	1.07	RCHRES 54	1
PERLND 54	1.84	RCHRES 54	1
IMPLND 14	1.42	RCHRES 54	2
*** SUB-CATCHMENT MC-4			
PERLND 34	0.27	RCHRES 135	1
PERLND 44	16.51	RCHRES 135	1
PERLND 45	4.23	RCHRES 135	1
PERLND 54	11.98	RCHRES 135	1
IMPLND 14	3.31	RCHRES 135	2
*** SUB-CATCHMENT MC-5			
PERLND 26	13.43	RCHRES 35	1
PERLND 44	33.84	RCHRES 35	1
PERLND 54	7.44	RCHRES 35	1
IMPLND 14	0.02	RCHRES 35	2
*** SUB-CATCHMENT MC-6			
*** --> reduce by 2 acres to make special PERLND 47 for SDW1A			
***PERLND 44	14.10	RCHRES 35	1
PERLND 44	12.10	RCHRES 35	1

PERLND	45	0.09	RCHRES	35	1
PERLND	54	0.90	RCHRES	35	1
IMPLND	14	0.26	RCHRES	35	2
***	--> add 2 acres from special PERLND 47 for SDW1A				
PERLND	47	2.00	RCHRES	35	1
***	--> output outflow from special PERLND 47 (acft/ac)				
PERLND	47	1.00	COPY	47	21

\*\*\* SUB-CATCHMENT MC-7

***	--> reduce by 2 acres to make special PERLND 57 for SDW1B				
PERLND	26	11.26	COPY	55	21
***	--> reduce by 2 acres to make special PERLND 57 for SDW1B				
***PERLND	44	31.80	COPY	55	21
***	--> add 2 acres from special PERLND 57 for SDW1B				
PERLND	57	2.00	COPY	55	21
***	--> output outflow from special PERLND 57 (acft/ac)				
PERLND	57	1.00	COPY	57	21
PERLND	44	29.80	COPY	55	21
PERLND	54	3.20	COPY	55	21
IMPLND	14	0.03	COPY	55	22

\*\*\*note: SDN AGWO TO VACCA FARMS (135)NOT TO PONDS

\*\*\* SUB-CATCHMENT SDN-1

PERLND	26	1.97	RCHRES	552	6
PERLND	44	1.29	RCHRES	552	6
PERLND	54	0.20	RCHRES	552	6
PERLND	26	1.97	RCHRES	135	7
PERLND	44	1.29	RCHRES	135	7
PERLND	54	0.20	RCHRES	135	7
IMPLND	14	12.68	RCHRES	552	2

\*\*\* SUB-CATCHMENT SDN-1-LWR

PERLND	44	4.79	RCHRES	552	6
PERLND	54	0.07	RCHRES	552	6
PERLND	44	4.79	RCHRES	135	7
PERLND	54	0.07	RCHRES	135	7
IMPLND	14	0.56	RCHRES	552	2

\*\*\* SUB-CATCHMENT SDN-1-OFF

PERLND	26	23.01	RCHRES	52	6
PERLND	44	3.58	RCHRES	52	6
PERLND	54	1.67	RCHRES	52	6
PERLND	26	23.01	RCHRES	135	7
PERLND	44	3.58	RCHRES	135	7
PERLND	54	1.67	RCHRES	135	7
IMPLND	14	8.00	RCHRES	52	2

\*\*\* SUB-CATCHMENT SDN-2X (TO POND)

PERLND	26	0.63	COPY	61	26
PERLND	44	2.40	COPY	61	26
PERLND	45	0.86	COPY	61	26
PERLND	26	0.63	RCHRES	135	7
PERLND	44	2.40	RCHRES	135	7
PERLND	45	0.86	RCHRES	135	7
IMPLND	14	0.36	COPY	61	22

```

*** SUB-CATCHMENT SDN-3 (TO POND)
PERLND 26          23.56      RCHRES 43      6
PERLND 26          23.56      RCHRES 135     7
IMPLND 14          24.30      RCHRES 43      2

*** SUB-CATCHMENT SDN-3X (TO POND)
PERLND 26          1.61        RCHRES 43      6
***Original PERLND area
***PERLND 45          23.77      RCHRES 43      6
***PERLND AREA TO BE REMOVED = 0.29 AC
PERLND 45          23.48      RCHRES 43      6
PERLND 80          0.29        RCHRES 135     7
PERLND 26          1.61        RCHRES 135     7
PERLND 45          23.48      RCHRES 135     7

*** SUB-CATCHMENT SDN-4 (TO POND)
PERLND 26          15.75      COPY 44      26
PERLND 44          1.31        COPY 44      26
PERLND 45          0.99        COPY 44      26
PERLND 26          15.75      RCHRES 135     7
PERLND 44          1.31        RCHRES 135     7
PERLND 45          0.99        RCHRES 135     7
IMPLND 14          12.26      COPY 44      22

*** SUB-CATCHMENT SDN-4X (TO POND)
PERLND 26          1.92        COPY 44      26
PERLND 44          0.75        COPY 44      26
PERLND 45          8.31        COPY 44      26
PERLND 26          1.92        RCHRES 135     7
PERLND 44          0.75        RCHRES 135     7
PERLND 45          8.31        RCHRES 135     7
IMPLND 14          4.21        COPY 44      22

*** SUB-CATCHMENT IWS-NCPS (TO POND)
PERLND 26          4.78        RCHRES 242     6
PERLND 26          4.78        RCHRES 135     7
IMPLND 14          30.93      RCHRES 242     2

*** SUB-CATCHMENT IWS-NSMPS (TO POND)
PERLND 26          2.69        RCHRES 240     6
PERLND 44          1.97        RCHRES 240     6
PERLND 45          0.01        RCHRES 240     6
PERLND 26          2.69        RCHRES 135     7
PERLND 44          1.97        RCHRES 135     7
PERLND 45          0.01        RCHRES 135     7
IMPLND 14          1.95        RCHRES 240     2

*** SUB-CATCHMENT NEPL (TO POND)
PERLND 26          10.00      RCHRES 452     6
PERLND 26          10.00      RCHRES 135     7
IMPLND 14          6.00        RCHRES 451     2
IMPLND 14          26.29      RCHRES 452     2

*** SUB-CATCHMENT CARGO (TO POND)
IMPLND 14          8.12        RCHRES 46      2

*** SUB-CATCHMENT SDN3AI (TO VAULT)

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***original IMPLND area
***IMPLND 14 5.87 RCHRES 37 2
***IMPLND AREA TO BE REMOVED = 5.69 AC
IMPLND 14 0.18 RCHRES 37 2
PERLND 80 5.69 RCHRES 135 7

*** SUB-CATCHMENT SDN3AO (TO POND)
PERLND 26 0.08 RCHRES 237 6
PERLND 44 0.03 RCHRES 237 6
***original PERLND area
***PERLND 45 22.12 RCHRES 237 6
***PERLND AREA TO BE REMOVED = 11.16 AC
***PERLND AREA TO BE REMOVED = 4.56 AC
PERLND 45 6.40 RCHRES 237 6
PERLND 80 11.16 RCHRES 135 7
PERLND 80 4.56 RCHRES 135 7
PERLND 26 0.08 RCHRES 135 7
PERLND 44 0.03 RCHRES 135 7
PERLND 45 6.40 RCHRES 135 7
***original IMPLND area
***IMPLND 14 2.35 RCHRES 237 2
***IMPLND AREA TO BE REMOVED = 2.19 AC
IMPLND 14 0.16 RCHRES 237 2
PERLND 80 2.19 RCHRES 135 7

*** SUB-CATCHMENT SDW1AO (TO POND)
***original PERLND area
***PERLND 26 4.28 RCHRES 247 6
***PERLND AREA TO BE REMOVED = 0.67 AC
PERLND 26 3.61 RCHRES 247 6
PERLND 80 0.67 RCHRES 135 7
PERLND 44 0.69 RCHRES 247 6
***original PERLND area
***PERLND 45 32.44 RCHRES 247 6
***PERLND AREA TO BE REMOVED = 18.06 AC
***PERLND AREA TO BE REMOVED = 0.60 AC
PERLND 45 13.78 RCHRES 247 6
PERLND 80 18.06 RCHRES 135 7
PERLND 80 0.60 RCHRES 135 7
PERLND 26 3.61 RCHRES 135 7
PERLND 44 0.69 RCHRES 135 7
PERLND 45 13.78 RCHRES 135 7
***original IMPLND area
***IMPLND 14 1.64 RCHRES 247 2
***IMPLND AREA TO BE REMOVED = 0.93 AC
IMPLND 14 0.71 RCHRES 247 2
PERLND 80 0.93 RCHRES 135 7
*** PERVIOUS AREA FOR 1AI IS IN 1AO
*** SUB-CATCHMENT SDN1AI (TO VAULT)
***original IMPLND area
***IMPLND 14 13.78 RCHRES 147 2
***IMPLND AREA TO BE REMOVED = 13.07 AC
IMPLND 14 0.71 RCHRES 147 2
PERLND 80 13.07 RCHRES 35 7
*** CONTAINS BOTH I AND O
*** SUB-CATCHMENT SDW1B (TO POND)
*** AGWO TO 35, AS 57 IS D/S OF VACCA FARMS (135)

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***original PERLND area
***PERLND 26 21.25 RCHRES 570 6
***PERLND AREA TO BE REMOVED = 0.54 AC
PERLND 26 20.71 RCHRES 570 6
PERLND 80 0.54 RCHRES 35 7
PERLND 44 2.39 RCHRES 570 6
***original PERLND area
***PERLND 45 46.26 RCHRES 570 6
***PERLND AREA TO BE REMOVED = 34.71 AC
***PERLND AREA TO BE REMOVED = 1.34 AC
PERLND 45 10.21 RCHRES 570 6
PERLND 80 34.71 RCHRES 35 7
PERLND 80 1.34 RCHRES 35 7
PERLND 26 20.71 RCHRES 35 7
PERLND 44 2.39 RCHRES 35 7
PERLND 45 10.21 RCHRES 35 7
***original IMPLND area
***IMPLND 14 26.95 RCHRES 570 2
***IMPLND AREA TO BE REMOVED = 20.79 AC
***IMPLND AREA TO BE REMOVED = 1.62 AC
IMPLND 14 4.54 RCHRES 570 2
PERLND 80 20.79 RCHRES 35 7
PERLND 80 1.62 RCHRES 35 7

```

\*\*\* ADD SUB-CATCHMENT IWS-PRIMARY TO PREDEVELOPEMENT ONLY

\*\*\*ROUTING FOR MILLER CREEK

\*\*\* M1 TO M2 TO M3 TO STORAGE 50. M4 TO M5 TO STORAGE 50

```

RCHRES 1 RCHRES 2 4
RCHRES 23 RCHRES 24 4
RCHRES 24 RCHRES 3 3
RCHRES 2 RCHRES 3 3
RCHRES 3 RCHRES 33 3
RCHRES 33 RCHRES 50 3
RCHRES 4 RCHRES 5 4
RCHRES 5 RCHRES 50 3

```

\*\*\* PONDS TO 52, 53 & 54

```

RCHRES 242 RCHRES 240 5

```

\*\*\* OVERFLOW ONLY TO 61

```

RCHRES 240 RCHRES 51 5
COPY 61 RCHRES 51 12
COPY 44 RCHRES 51 12
RCHRES 51 RCHRES 52 3
RCHRES 43 RCHRES 54 3

```

\*\*\* 2 NEPL VAULTS\* (FK-Changed to eliminate run-of-river tables)

```

RCHRES 451 COPY 45 11
RCHRES 452 COPY 45 11
COPY 45 COPY 645 10
COPY 645 RCHRES 53 12
RCHRES 46 RCHRES 53 3

```

\*\*\* NEW STREAM REACH 52 TO LAKE REBA 53 TO RDF 54 (FK-changed to insert new POC at Lake Reba)

```

RCHRES 552 RCHRES 52 3
RCHRES 52 RCHRES 53 3
RCHRES 53 COPY 53 11
COPY 53 RCHRES 54 12
RCHRES 50 RCHRES 54 3

```

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\*\*\* RDF 54 TO 35

RCHRES 54		RCHRES 135	3	
<-Source->	<--Area-->	<-Target->	MBLK	***
<Name> #	<-factor->	<Name> #	Tbl#	***

\*\*\* PONDS TO 34

RCHRES 37		COPY 37	11	
RCHRES 237		COPY 37	11	
COPY 37		RCHRES 135	12	

\*\*\* SDW1A flow to bypass added (FK, June 2001)

SDW1AI VAULT FLOW TO INFILTRATION 1 \*\*\*

RCHRES 147		RCHRES 47	4	
------------	--	-----------	---	--

SDW1AI VAULT FLOW TO BYPASS \*\*\*

RCHRES 147		COPY 70	15	
------------	--	---------	----	--

STORMWATER Q 1ST EXIT AT POND G (Bypass) \*\*\*

RCHRES 247		COPY 70	14	
------------	--	---------	----	--

RCHRES 247		COPY 66	14	
------------	--	---------	----	--

RCHRES 247		COPY 69	15	
------------	--	---------	----	--

2ND EXIT TO INFILTRATION TANK-MILLER CREEK \*\*\*

RCHRES 247		RCHRES 47	5	
------------	--	-----------	---	--

STORMWATER Q 1ST EXIT TO BYPASS \*\*\*

RCHRES 47		COPY 70	14	
-----------	--	---------	----	--

\*\*\* 2ND EXIT TO SOIL AND MILLER CREEK (2nd exit intr. as AGWLI) \*\*\*

*** RCHRES 47		COPY 70	15	
---------------	--	---------	----	--

COPY BLOCK FOR OUTPUT PURPOSES \*\*\*

RCHRES 47		COPY 62	14	
-----------	--	---------	----	--

RCHRES 47		COPY 63	15	
-----------	--	---------	----	--

RCHRES 147		COPY 67	14	
------------	--	---------	----	--

RCHRES 147		COPY 68	15	
------------	--	---------	----	--

COPY 70		RCHRES 135	12	
---------	--	------------	----	--

RCHRES 34		RCHRES 135	4	
-----------	--	------------	---	--

RCHRES 34		RCHRES 135	5	
-----------	--	------------	---	--

RCHRES 135		RCHRES 35	3	
------------	--	-----------	---	--

RCHRES 10		RCHRES 16	3	
-----------	--	-----------	---	--

\*\*\* PONDS TO 35

\*\*\* Configuration changed to flow splitter to Pond D and Infiltration Basin 3 (FK, June 2001)

STORM Q - 1ST EXIT OF FLOW SPLITTER TO POND D \*\*\*

RCHRES 570		RCHRES 57	4	
------------	--	-----------	---	--

\*\*\*INFILTRATION Q - 2ND EXIT OF FLOW SPLITTER TO SOIL \*\*\*

*** RCHRES 570		RCHRES 257	5	
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STORM Q EXIT OF POND D TO MILLER CREEK \*\*\*

RCHRES 57		COPY 71	11	
-----------	--	---------	----	--

COPY BLOCK FOR OUTPUT PURPOSES \*\*\*

RCHRES 570		COPY 64	14	
------------	--	---------	----	--

RCHRES 570		COPY 65	15	
------------	--	---------	----	--

RCHRES 57		COPY 357	11	
-----------	--	----------	----	--

RCHRES 257		COPY 56	14	
------------	--	---------	----	--

RCHRES 257		COPY 71	14	
------------	--	---------	----	--

RCHRES 257		COPY 71	15	
------------	--	---------	----	--

COPY 71		RCHRES 35	12	
---------	--	-----------	----	--

RCHRES 35		COPY 55	11	
-----------	--	---------	----	--

COPY 55		RCHRES 16	12	
---------	--	-----------	----	--

RCHRES 11		RCHRES 15	3	
-----------	--	-----------	---	--

RCHRES 13		RCHRES 12	4	
-----------	--	-----------	---	--

RCHRES 13		RCHRES 12	5	
-----------	--	-----------	---	--

RCHRES 12		RCHRES 15	3	
-----------	--	-----------	---	--

RCHRES 16		RCHRES 15	3	
-----------	--	-----------	---	--





23	23	0.379	0.0	0.3
24	24	0.379		0.3
33	33	0.200		0.3
34	34	0.852		0.3
35	35	0.663		0.3
37	37	0.010	0.0	0.3
38	38	0.010		0.3
43	43	0.010		0.3
46	46	0.010		0.3
47	47	0.010	0.0	0.3
50	50	0.010		0.3
51	51	0.010		0.3
52	52	0.010		0.3
53	53	0.010		0.3
54	54	0.010	0.0	0.3
57	57	0.010	0.0	0.3
135	135	0.350		0.3
147	147	0.010		0.3
237	237	0.010	0.0	0.3
240	240	0.010		0.3
242	242	0.010		0.3
247	247	0.010	0.0	0.3
257	257	0.010	0.0	0.3
451	451	0.010	0.0	0.3
452	452	0.010	0.0	0.3
552	552	0.010	0.0	0.3
570	570	0.010	0.0	0.3

END HYDR-PARM2

HYDR-INIT

RCHRES		Initial conditions for each HYDR section		***	
# - #	*** VOL	Initial value of COLIND		Initial value of OUTDGT	
	*** ac-ft	for each possible exit		for each possible exit	
<----->	<----->	<----->	<----->	***	<----->
1	2.0	4.0	5.0		
2	0.0	4.0			
3	0.0	4.0			
4	2.0	4.0	5.0		
5	0.0	4.0			
10	0.0	4.0			
11	0.0	4.0			
12	0.0	4.0			
13	10.0	4.0	5.0		
14	0.0	4.0			
15	0.0	4.0			
16	0.0	4.0			
17	0.0	4.0			
23	6.0	4.0	5.0		
24	0.0	4.0			
33	0.0	4.0			
34	9.0	4.0	5.0		
35	0.1	4.0			
37	0.0	4.0			
38	0.1	4.0			
43	0.0	4.0			
46	0.0	4.0			
47	0.0	4.0	5.0		

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50	0.0	4.0	
51	0.0	4.0	
52	0.0	4.0	
53	0.1	4.0	
54	2.25	4.0	
57	0.0	4.0	
237	0.00	4.0	
147	0.00	4.0	5.0
135	0.00	4.0	
240	0.0	4.0	5.0
242	0.0	4.0	5.0
247	0.0	4.0	5.0
257	0.0	4.0	5.0
451	0.0	4.0	
452	0.0	4.0	
552	0.0	4.0	
570	0.0	4.0	5.0

END HYDR-INIT  
 END RCHRES

FTABLES  
 \*\*\*UPPER BASIN  
 \*\*\*=====

FTABLE 1  
 \*\*\* REVISED 8/16/00 ADDED 2ND OUTFLOW  
 ROWS COLS \*\*\*  
 11 5  

DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2***
0.00	3.00	0.00	0.00	0.00
2.50	3.00	7.50	0.00	0.11
3.00	3.00	9.00	1.80	0.11
3.50	3.30	10.58	5.00	0.11
4.00	3.60	12.30	10.90	0.11
4.50	3.90	14.18	17.50	0.11
5.00	4.10	16.18	26.20	0.11
5.50	4.30	18.28	32.50	0.11
6.00	4.50	20.48	35.90	0.11
7.00	5.00	25.23	38.10	0.11
8.00	5.50	30.48	46.40	0.11

 END FTABLE 1

FTABLE 2  
 ROWS COLS \*\*\*  
 9 4  

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.2571	0.0129	0.16	
0.500	0.3873	0.1417	6.53	
1.000	0.5501	0.3761	25.95	
1.500	0.7128	0.6918	59.86	
2.000	0.8756	1.0889	110.67	
3.000	1.2011	2.1273	272.24	
3.500	1.3639	2.7685	387.38	
4.000	1.5266	3.4912	528.19	

 END FTABLE 2

FTABLE 3  
 ROWS COLS \*\*\*  
 12 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.9669	0.0483	0.13	
0.500	1.0637	0.4545	4.92	
1.000	1.1846	1.0165	17.12	
1.500	1.3055	1.6390	34.92	
2.000	1.4264	2.3220	57.95	
2.500	1.5473	3.0654	86.14	
3.000	1.6682	3.8693	119.53	
3.500	1.7891	4.7336	158.24	
4.000	1.9100	5.6584	202.41	
4.500	2.0294	6.6310	251.52	
5.000	2.1488	7.6624	306.28	

END FTABLE 3

FTABLE 4  
 \*\*\* REVISED 8/16/00 ADDED 2ND OUTFLOW  
 ROWS COLS \*\*\*  
 7 5

DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2***
0.00	3.00	0.00	0.00	0.00
2.50	4.50	9.38	0.00	0.11
3.00	6.00	12.00	6.00	0.11
4.00	10.00	20.00	13.00	0.11
5.00	15.00	32.50	20.00	0.11
6.00	20.00	50.00	26.00	0.11
7.00	25.00	72.50	168.00	0.11

END FTABLE 4

FTABLE 5  
 ROWS COLS \*\*\*  
 10 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.1010	0.0051	0.03	
0.500	0.1754	0.0603	1.46	
1.000	0.2684	0.1713	6.16	
1.500	0.3614	0.3288	14.89	
2.000	0.4544	0.5327	28.48	
2.500	0.5474	0.7832	47.70	
3.000	0.6404	1.0801	73.29	
3.500	0.7334	1.4236	105.94	
4.000	0.8264	1.8136	146.33	

END FTABLE 5

FTABLE 10  
 ROWS COLS \*\*\*  
 9 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.1010	0.0051	0.06	
0.500	0.1660	0.0585	2.27	
1.000	0.2472	0.1618	9.32	
1.500	0.3285	0.3057	22.08	

2.000	0.4097	0.4902	41.66
2.500	0.4909	0.7154	69.09
3.000	0.5722	0.9811	105.37
4.000	0.6887	1.6116	209.70

END FTABLE 10

POST AMBAUM DETENTION \*\*\*  
 FTABLE 11  
 ROWS COLS \*\*\*  
 12 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
1.000	0.1000	0.2300	3.90	
2.000	0.2000	0.6000	6.30	
3.000	0.3000	0.9700	8.10	
4.000	0.4000	1.3400	11.10	
5.000	0.5000	1.8200	16.00	
6.000	0.6000	2.2700	19.10	
7.000	0.7000	2.8300	21.60	
8.000	0.8000	3.3700	30.80	
9.000	0.9000	4.0000	38.10	
10.000	1.0000	4.6500	74.10	
10.500	1.1000	5.2000	133.00	
11.000	1.1500	6.0000	500.00	
11.500	1.3000	11.000	1300.00	

END FTABLE 11

FTABLE 12  
 ROWS COLS \*\*\*  
 6 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.6327	0.0316	0.15	
0.500	0.7960	0.3174	5.87	
1.000	1.0002	0.7664	21.53	
1.500	1.2043	1.3176	46.43	
2.000	1.4085	1.9708	81.20	
3.000	1.8168	3.5834	183.79	
4.000	2.2251	5.6044	336.22	
5.000	2.6335	8.0337	545.30	
6.000	3.0418	10.8713	817.51	

END FTABLE 12

FTABLE 13  
 \*\*\* REVISED 8/16/00 ADDED 2ND OUTFLOW  
 ROWS COLS \*\*\*  
 7 5

DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2***
0.000	40.000	0.0000	0.00	0.00
1.000	41.400	40.000	0.00	0.11
1.500	42.000	60.000	10.00	0.11
2.000	42.700	80.000	16.00	0.11
2.500	43.300	100.00	20.00	0.11
3.000	44.000	120.00	28.00	0.11
5.000	45.000	210.00	45.00	0.11

END FTABLE 13



FTABLE 14  
 ROWS COLS \*\*\*  
 6 4  
 DEPTH AREA VOLUME OUTFLOW \*\*\*  
 0.000 0.0000 0.0000 0.00  
 0.100 0.3361 0.0168 0.24  
 0.500 0.3809 0.1602 9.04  
 1.000 0.4370 0.3647 31.61  
 1.500 0.4930 0.5972 65.00  
 2.000 0.5491 0.8577 108.85  
 2.500 0.6051 1.1462 163.33  
 3.000 0.6612 1.4628 228.78  
 END FTABLE 14

FTABLE 15  
 ROWS COLS \*\*\*  
 4 4  
 DEPTH AREA VOLUME OUTFLOW \*\*\*  
 0.00 0.10 0.00 0.00  
 1.00 1.00 0.55 91.00  
 2.00 1.10 1.60 268.00  
 3.00 1.20 2.75 493.00  
 END FTABLE 15

FTABLE 16  
 ROWS COLS \*\*\*  
 4 4  
 DEPTH AREA VOLUME OUTFLOW \*\*\*  
 0.00 0.10 0.00 0.00  
 1.00 1.00 0.55 74.00  
 2.00 1.10 1.60 219.00  
 3.00 1.20 2.75 403.00  
 END FTABLE 16

FTABLE 17  
 ROWS COLS \*\*\*  
 5 4  
 DEPTH AREA VOLUME OUTFLOW \*\*\*  
 0.00 0.10 0.00 0.00  
 1.00 1.00 0.55 59.00  
 2.00 1.10 1.60 173.00  
 3.00 1.20 2.75 318.00  
 4.00 1.30 4.00 484.00  
 END FTABLE 17

FTABLE 23  
 ROWS COLS \*\*\* HERMES  
 9 5  
 DEPTH AREA VOLUME OUTFLOW OUTFLOW \*\*\*  
 0.00 0.00 0.00 0.00 0.00 0.00  
 5.00 0.50 1.91 0.00 0.00 305.00  
 11.00 0.79 5.79 0.00 0.00 311.00  
 15.00 1.13 9.64 0.50 0.01 315.00  
 19.00 1.72 15.34 0.50 0.05 319.00  
 29.00 2.86 38.25 0.50 0.10 329.00  
 39.00 4.40 74.55 0.50 0.20 339.00  
 50.00 6.22 132.98 0.50 0.30 350.00

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60.00 10.00 1212.98 0.50 0.40 360.00  
 END FTABLE 23

FTABLE 24  
 ROWS COLS \*\*\*  
 9 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.2571	0.0129	0.16	
0.500	0.3873	0.1417	6.53	
1.000	0.5501	0.3761	25.95	
1.500	0.7128	0.6918	59.86	
2.000	0.8756	1.0889	110.67	
3.000	1.2011	2.1273	272.24	
3.500	1.3639	2.7685	387.38	
4.000	1.5266	3.4912	528.19	

END FTABLE 24

FTABLE 33  
 ROWS COLS \*\*\*  
 11 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.00	1.00	0.00	0.00	
0.50	1.20	0.55	2.00	
1.00	1.40	1.20	6.00	
1.50	1.60	1.95	9.00	
2.00	1.80	2.80	13.00	
2.50	2.00	3.75	16.50	
3.00	2.20	4.80	20.00	
3.50	2.40	5.95	23.00	
4.00	2.60	7.20	26.00	
5.00	2.80	9.90	104.00	
6.00	3.00	12.80	246.00	

END FTABLE 33

FTABLE 34  
 ROWS COLS \*\*\* REVISED 11/19/97 BASED ON HEC-RAS MODEL  
 \*\*\* REVISED 8/16/00 ADDED 2ND OUTFLOW

DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW2***
0.00	3.00	0.00	0.00	0.00
3.00	3.05	9.08	0.00	0.11
4.00	3.10	12.15	0.00	0.11
5.00	3.15	15.28	0.00	0.11
6.00	3.20	18.45	72.0	0.11
7.00	3.25	21.68	225.0	0.11

END FTABLE 34

FTABLE 35  
 ROWS COLS \*\*\* REVISED 11/19/97 BASED ON HECRAS MODEL  
 5 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.00	0.10	0.00	0.00	
1.00	1.10	0.60	38.00	
2.00	1.20	1.75	108.00	
3.00	1.30	3.00	194.00	
4.00	1.40	4.35	290.00	

END FTABLE 35

FTABLE 38

ROWS COLS \*\*\*

7 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
1.000	0.4000	0.4000	2.00	
1.500	0.5000	1.0000	4.00	
2.000	0.9000	1.3000	11.00	
2.500	1.3000	1.6000	15.00	
3.000	1.6000	2.0000	18.00	
3.500	1.9000	2.5000	20.80	

END FTABLE 38

FTABLE 45

ROWS COLS \*\*\*

4 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0010	0.0000	0.00	
0.000	0.0100	0.0100	10.00	
0.100	0.1000	0.1000	100.00	
1.000	1.0000	1.0000	1000.00	
10.000	10.0000	10.0000	10000.00	

END FTABLE 45

FTABLE 645

ROWS COLS \*\*\*

4 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0010	0.0000	0.00	
0.000	0.0100	0.0100	10.00	
0.100	0.1000	0.1000	100.00	
1.000	1.0000	1.0000	1000.00	
10.000	10.0000	10.0000	10000.00	

END FTABLE645

FTABLE 50

ROWS COLS \*\*\*

10 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.00	1.00	0.00	0.00	
0.50	1.10	0.53	5.00	
1.00	1.20	1.10	15.00	
1.50	1.30	1.73	25.00	
2.00	1.40	2.40	35.00	
2.50	1.50	3.13	52.00	
3.00	1.60	3.90	70.00	
3.50	1.70	4.73	87.00	
4.00	1.80	5.60	105.00	
6.00	1.90	9.30	165.00	

END FTABLE 50

FTABLE 52

ROWS COLS \*\*\*

6 4

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DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.3680	0.0184	0.25	
0.500	0.3717	0.1664	9.39	
1.000	0.3763	0.3534	31.06	
2.000	0.3819	0.7325	94.37	
3.000	0.3874	1.1171	174.33	

END FTABLE 52

FTABLE 552  
ROWS COLS \*\*\* SDN1 VAULT EFFECTIVE DEPTH=12 FT RISER=24 INCHES  
15 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.4308	0.0000	0.00	
1.290	0.4308	0.6520	0.111	
2.130	0.4308	1.0760	0.143	
3.530	0.4308	1.7830	0.184	
4.640	0.4308	2.3430	0.211	
5.200	0.4308	2.6260	0.223	
6.320	0.4308	3.1920	0.246	
7.430	0.4308	3.7530	0.267	
8.200	0.4308	4.1410	0.280	
9.220	0.4308	4.6570	0.407	
10.190	0.4308	5.1460	0.567	
11.250	0.4308	5.6820	0.954	
12.100	0.4308	6.1110	2.130	
12.300	0.4308	6.2120	4.730	
13.700	0.4308	6.9190	21.360	

END FTABLE552

FTABLE 53  
OLD LAKE REBA \*\*\*  
MAX DEPTH = 4.9 FEET \*\*\*  
30" CMP, 40 CFS DISCHARGE AT MAX DEPTH \*\*\*  
ROWS COLS \*\*\*  
7 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	2.4000	0.0000	0.00	
1.000	2.5800	2.5000	18.00	
2.000	2.9400	5.3000	26.00	
3.000	3.4100	8.4000	31.00	
4.000	3.8800	12.100	36.00	
4.900	4.3000	15.800	40.00	
6.000	4.3000	15.810	500.00	

END FTABLE 53

FTABLE 54  
EXISTING MILLER CREEK DETENTION FACILITY\*\*\* REVISED STORAGE/Q DATA  
GATE SETTING: 2.0 FEET\*\*\* BASED ON CALIBRATION FILE  
ROWS COLS \*\*\*  
12 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.00	0.00	0.00	
1.300	0.01	0.01	10.00	
2.000	0.01	0.02	20.00	
2.900	0.70	0.40	30.00	
4.000	1.50	1.50	40.00	

5.400	3.50	4.90	50.00
7.000	8.60	13.30	60.00
8.800	15.60	34.80	70.00
10.000	19.90	57.30	76.00
10.500	21.50	68.00	92.00
11.000	23.10	78.80	179.00
11.500	24.70	88.60	303.00

END FTABLE 54

FTABLE 104  
 MILLER CREEK DETENTION FACILITY\*\*\* WITH ADD'L AREA 1+AREA 2 55.5 ACFT @ 10FT  
 GATE SETTING: 2.0 FEET\*\*\* EXISTING OUTLET NO LOW FLOW CONTROL  
 ROWS COLS \*\*\*

17	4			
DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.500	0.0100	0.0100	2.50	
1.500	0.0300	0.2800	14.29	
2.500	1.1100	1.3900	24.88	
3.500	2.6100	4.0000	34.51	
4.500	4.6100	9.1400	43.20	
5.500	7.1200	19.600	50.98	
6.000	8.3600	21.180	54.53	
6.500	11.870	30.060	57.87	
7.000	15.370	38.930	61.00	
7.500	18.870	47.800	63.91	
8.000	21.860	59.160	66.62	
8.500	24.850	70.510	69.12	
9.000	27.340	84.160	71.42	
9.500	29.820	97.820	73.53	
10.000	32.050	112.83	75.44	
10.500	34.275	127.84	90.74	
11.500	38.220	161.54	320.00	

END FTABLE104

FTABLE 69  
 PRE-MILLER CREEK DETENTION FACILITY\*\*\*  
 ROWS COLS \*\*\*

12	4			
DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.1860	0.0093	0.12	
0.500	0.2552	0.0975	4.84	
1.000	0.3417	0.2467	18.49	
1.500	0.4282	0.4392	41.30	
2.000	0.5148	0.6750	74.40	
2.500	0.6013	0.9540	119.01	
3.000	0.6878	1.2763	176.30	
3.500	0.7744	1.6418	247.41	
4.000	0.8609	2.0506	333.43	
4.500	0.9470	2.4992	434.59	
5.000	1.0331	2.9905	552.33	

END FTABLE 69

\*\*\* PROJECT CONDITION PONDS/VAULTS

FTABLE 452  
 ROWS COLS \*\*\*

AR 011515

\*\*\* NEW NORTH EMPLOYEE PARKING LOT VAULT (NEPL)  
 \*\*\* PARALLEL VAULT BASED ON KCRTS EFFECTIVE DEPTH=20 FT

DEPTH	AREA	VOLUME	OUTFLOW ***
0.00	3.214	0.000	0.000
1.11	3.214	0.826	0.129
1.57	3.214	1.168	0.154
3.43	3.214	2.551	0.227
4.83	3.214	3.593	0.269
8.08	3.214	6.010	0.348
10.41	3.214	7.743	0.395
12.74	3.214	9.476	0.437
14.00	3.214	10.413	0.458
14.65	3.214	10.897	0.557
16.09	3.214	11.968	0.665
16.23	3.214	12.072	0.754
17.92	3.214	13.329	1.140
18.22	3.214	13.552	1.310
18.81	3.214	13.991	1.860
19.11	3.214	14.214	2.190
20.00	3.214	14.876	3.350
20.20	3.214	15.025	5.110
20.70	32.14	15.397	14.820
21.00	32.14	15.620	18.560

END FTABLE452

FTABLE 451  
 ROWS COLS \*\*\*  
 \*\*\* NORTH EMPLOYEE PARKING LOT VAULT (NEPL)  
 \*\*\* EXISTING VAULT W/MODIFIED OUTLET EFFECTIVE DEPTH= 18.0 FT

DEPTH	AREA	VOLUME	OUTFLOW ***
0.000	0.2240	0.0000	0.00
2.170	0.2240	0.4860	0.031
4.260	0.2240	0.9550	0.043
5.930	0.2240	1.3290	0.051
8.030	0.2240	1.8000	0.059
10.120	0.2240	2.2680	0.066
12.210	0.2240	2.7360	0.073
14.040	0.2240	3.1460	0.109
15.510	0.2240	3.4760	0.166
16.220	0.2240	3.6350	0.295
18.000	0.2240	4.0340	1.080
18.400	0.2240	4.1240	5.400
19.000	0.2240	4.2580	12.680
19.900	0.2240	4.4600	17.080

END FTABLE451

FTABLE 46  
 ROWS COLS \*\*\*  
 SDN-6: 24TH STREET CARGO VAULT \*\*\* EFFECTIVE DEPTH=14 FT RISER DIA=12 IN

DEPTH	AREA	VOLUME	OUTFLOW ***
0.00	0.35	0.000	0.000
0.37	0.35	0.131	0.021
1.19	0.35	0.421	0.037

3.39	0.35	1.198	0.063
5.03	0.35	1.778	0.077
7.23	0.35	2.556	0.092
9.15	0.35	3.235	0.104
10.25	0.35	3.624	0.110
10.53	0.35	3.723	0.111
10.92	0.35	3.861	0.128
12.00	0.35	4.242	0.165
12.13	0.35	4.288	0.190
12.95	0.35	4.578	0.245
13.77	0.35	4.868	0.282
14.00	0.35	4.949	0.291
14.10	0.35	4.985	0.910
14.20	0.35	5.020	2.040
14.30	0.35	5.056	3.500
14.50	0.35	5.126	7.200
14.70	0.35	5.197	11.720

END FTABLE 46

\*\*\* SDW-1A: 3RD RUNWAY POND G TO MILLER CREEK (LEVEL 2): \*\*\*

FTABLE 47

\*\*\* PROJECT SDW1A EFFECTIVE DIAMETER=3.0 FT  
 ROWS COLS \*\*\* INFILTRATION TANK TO OBTAIN 0.3 CFS

14	5				
DEPTH	AREA	VOLUME	STORMQ	INFILTRQ	***
0.000	0.000	0.000	0.000	0.000	
0.250	0.002	0.002	0.000	0.027	
0.500	0.004	0.004	0.000	0.054	
1.000	0.012	0.012	0.000	0.109	
1.500	0.020	0.020	0.000	0.164	
2.000	0.029	0.029	0.000	0.218	
2.500	0.036	0.036	0.000	0.272	
3.000	0.041	0.0406	0.000	0.327	
3.100	0.041	0.0419	0.596	0.338	
3.200	0.041	0.0420	1.685	0.349	
3.300	0.041	0.0421	3.096	0.360	
3.400	0.041	0.0422	4.766	0.371	
3.500	0.041	0.0423	6.661	0.382	
3.750	0.041	0.0424	12.237	0.409	

END FTABLE 47

\*\*\* SDW-1A: 3RD RUNWAY NORTH POND G TO MILLER CREEK (LEVEL 2): \*\*\*

FTABLE 147

\*\*\* PROJECT SDW1A EFFECTIVE DEPTH=14.0 FT RISER DIA 24 INCHES  
 ROWS COLS \*\*\* VAULT BASED ON INFILTRATION=0.15CFS

17	5				
DEPTH	AREA	VOLUME	INFILTRQ	BYPASS Q	***
0.000	0.689	0.000	0.0000	0.0000	
0.010	0.689	0.007	0.1400	0.0000	
1.000	0.689	0.689	0.1408	0.0000	
2.000	0.689	1.377	0.1417	0.0000	
4.000	0.689	2.755	0.1432	0.0000	
6.000	0.689	4.132	0.1446	0.0000	
8.000	0.689	5.510	0.1461	0.0000	
10.000	0.689	6.887	0.1475	0.0000	
12.000	0.689	8.264	0.1489	0.0000	

14.000	0.689	9.642	0.1503	0.0000
16.000	0.689	11.019	0.1517	0.0000
16.750	0.689	11.536	0.1517	10.7600
16.900	0.689	11.639	0.1517	13.9600
17.000	0.689	11.708	0.1517	16.1000
17.100	0.689	11.777	0.1517	18.5700
17.300	0.689	11.915	0.1517	23.8600
18.000	0.689	12.397	0.1517	45.5400

END FTABLE147

\*\*\* SDW-1A: 3RD RUNWAY NORTH POND G TO MILLER CREEK (LEVEL 2): \*\*\*

FTABLE 247

\*\*\* PROJECT SDW1A EFFECTIVE DEPTH=12.0 FT RISER DIA 12 INCHES  
 ROWS COLS \*\*\* POND BASED ON INFILTRATION=0.15CFS

17	5				
DEPTH	AREA	VOLUME	STORMQ	INFILTRQ	***
0.000	1.300	0.000	0.00	0.00	
0.010	1.310	0.010	0.001	0.15	
1.000	1.320	1.320	0.007	0.15	
2.000	1.342	2.650	0.010	0.15	
3.000	1.363	4.000	0.012	0.15	
4.000	1.385	5.370	0.013	0.15	
5.000	2.672	8.000	0.015	0.15	
6.000	2.739	10.700	0.017	0.15	
7.000	2.807	13.470	0.018	0.15	
8.000	2.876	16.300	0.019	0.15	
8.300	2.896	17.176	0.031	0.15	
9.000	2.945	19.210	0.041	0.15	
10.000	3.014	22.180	0.051	0.15	
11.000	3.084	25.228	0.058	0.15	
11.100	3.092	25.540	0.675	0.15	
11.300	3.106	26.162	3.260	0.15	
12.000	3.155	28.340	15.190	0.15	

END FTABLE247

\*\*\* SDN3A: 3RD RUNWAY VAULT TO MILLER CREEK (LEVEL 2): \*\*\*

FTABLE 37

\*\*\* PROJECT C SDN3A EFFECTIVE DEPTH=11.0FT RISER DIA=24 INCHES  
 ROWS COLS \*\*\* VAULT BASED ON IMPERVIOUS TOP SURO

14	4				
DEPTH	AREA	VOLUME	OUTFLOW	***	
0.000	0.644	0.000	0.000		
0.010	0.644	0.006	0.001		
1.000	0.644	0.643	0.016		
3.980	0.644	2.558	0.033		
6.030	0.644	3.876	0.041		
9.010	0.644	5.792	0.050		
10.00	0.644	6.428	0.052		
10.46	0.644	6.724	0.072		
11.00	0.644	7.071	0.082		
11.10	0.644	7.135	0.699		
11.20	0.644	7.199	1.830		
11.30	0.644	7.264	3.290		
11.40	0.644	7.328	5.020		
11.60	0.644	7.456	9.140		

END FTABLE 37



\*\*\* SDN3A: 3RD RUNWAY POND C TO MILLER CREEK (LEVEL 2): \*\*\*

FTABLE 237

\*\*\* PROJECT C SDN3A EFFECTIVE DEPTH= 9.0FT RISER DIA=24 INCHES  
 ROWS COLS \*\*\* POND BASED ON INTERFLOW AND PERVIOUS TOP SURO

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	1.3090	0.000	0.00	
0.020	1.3120	0.026	0.009	
1.020	1.3550	1.358	0.070	
2.070	1.4030	2.806	0.100	
3.130	1.4530	4.320	0.123	
4.020	1.4980	5.632	0.139	
5.070	1.5460	7.229	0.156	
7.750	1.6720	11.549	0.193	
7.800	1.6800	11.633	0.199	
7.850	1.6840	11.718	0.213	
8.250	1.7050	12.395	0.249	
8.340	1.7090	12.549	0.270	
8.570	1.7210	12.944	0.313	
8.950	1.7410	13.601	0.354	
9.500	1.7690	14.567	0.399	
9.600	1.7740	14.744	0.714	
9.800	1.7850	15.100	2.020	
10.300	1.8110	15.999	3.840	
10.900	1.8430	17.095	4.960	

END FTABLE237

\*\*\* SDN-3X: 3RD RUNWAY NORTH VAULT (LEVEL 2): \*\*\*

FTABLE 43

ROWS COLS \*\*\* EFFECTIVE DEPTH=20 FT RISER DIA=24 INCHES

DEPTH	AREA	VOLUME	FLOW	***
(FT)	(ACRES)	(ACRE-FT)	(FT3/S)	***
0.00	1.288	0.00	0.00	
0.14	1.288	0.180	0.067	
1.39	1.288	1.790	0.216	
3.35	1.288	4.314	0.336	
5.31	1.288	6.839	0.423	
8.06	1.288	10.380	0.521	
8.84	1.288	11.385	0.545	
10.02	1.288	12.905	0.580	
11.98	1.288	15.429	0.635	
12.37	1.288	15.931	0.645	
14.00	1.288	18.030	0.686	
14.10	1.288	18.159	0.705	
14.91	1.288	19.202	0.757	
16.09	1.288	20.722	0.810	
18.00	1.288	23.182	0.881	
18.32	1.288	23.594	1.150	
18.76	1.288	24.161	1.360	
20.00	1.288	25.758	1.680	
20.10	1.288	25.886	2.320	
20.50	1.288	26.402	8.620	
20.80	1.288	26.788	15.370	

END FTABLE 43

\*\*\* SDN-4X/2X: 3RD RUNWAY NORTH VAULT (COMBINED FACILITY)

FTABLE 51  
 ROWS COLS \*\*\* EFFECTIVE DEPTH=19FT RISER DIA=24 INCHES

DEPTH (FT)	AREA (ACRES)	VOLUME (ACRE-FT)	OUTFLOW (FT3/S)	***
0.00	0.789	0.000	0.000	***
0.16	0.789	0.126	0.056	***
1.51	0.789	1.192	0.169	
3.28	0.789	2.588	0.249	
5.49	0.789	4.332	0.322	
7.26	0.789	5.729	0.370	
10.35	0.789	8.168	0.442	
12.12	0.789	9.564	0.478	
13.44	0.789	10.606	0.503	
14.33	0.789	11.308	0.520	
15.57	0.789	12.287	0.654	
16.72	0.789	13.194	0.828	
17.19	0.789	13.565	0.950	
17.63	0.789	13.913	1.030	
18.00	0.789	14.205	1.080	
19.00	0.789	14.994	1.960	
19.10	0.789	15.073	2.580	
19.40	0.789	15.309	6.930	
19.60	0.789	15.467	11.080	
20.00	0.789	15.783	17.190	

END FTABLE 51

\*\*\* SDW-1B:3RD RUNWAY CENTRAL SOUTH POND D TO MILLER CREEK (LEVEL 2): \*\*\*

FTABLE 57  
 ROWS COLS \*\*\* EFFECTIVE DEPTH = 14.0 FT

DEPTH (FT)	AREA (ACRES)	VOLUME (ACRE-FT)	STORMQ (FT3/S)	***
0.00	2.430	0.000	0.000	***
0.01	2.430	0.041	0.010	
1.00	2.680	2.411	0.183	
2.00	2.760	4.860	0.257	
3.00	2.818	7.370	0.319	
4.00	3.079	9.945	0.366	
5.00	5.832	15.320	0.411	
6.00	5.927	20.742	0.450	
7.00	6.022	26.264	0.481	
8.00	6.118	31.888	0.518	
9.00	6.210	37.613	0.550	
10.00	6.311	43.441	0.583	
11.00	6.408	49.372	0.609	
12.00	6.607	55.406	0.634	
13.00	6.405	61.543	0.764	
14.00	6.504	67.786	1.320	
15.00	7.000	70.000	16.600	

END FTABLE 57

\*\*\* SDW-1B:3RD RUNWAY CENTRAL SOUTH POND D TO MILLER CREEK (LEVEL 2): \*\*\*

FTABLE 257  
 \*\*\* PROJECT SDW1B EFFECTIVE DIAMETER=3.0 FT  
 ROWS COLS \*\*\* INFILTRATION TANK TO OBTAIN 0.2 CFS

15 5

DEPTH	AREA	VOLUME	STORMQ	INFILTRQ ***
0.000	0.001	0.000	0.000	0.000
0.010	0.001	0.001	0.000	0.002
0.250	0.002	0.002	0.000	0.017
0.500	0.004	0.004	0.000	0.035
1.000	0.012	0.012	0.000	0.071
1.500	0.020	0.020	0.000	0.106
2.000	0.029	0.029	0.000	0.142
2.500	0.036	0.036	0.000	0.178
3.000	0.041	0.0406	0.000	0.213
3.100	0.041	0.0420	0.596	0.220
3.200	0.041	0.0421	1.685	0.227
3.300	0.041	0.0422	3.096	0.233
3.400	0.041	0.0423	4.766	0.241
3.500	0.041	0.0424	6.661	0.248
3.750	0.041	0.0425	12.237	0.266

END FTABLE257

FTABLE 570  
 \*\*\* PROJECT SDW1B FLOW SPLITTER (to 257 and 57)  
 ROWS COLS \*\*\*  
 15 5

DEPTH	AREA	VOLUME	STORMQ	INFILTRQ ***
0.000	0.00	0.000	0.000	0.000
0.100	0.01	0.0002	0.000	0.050
0.400	0.01	0.0009	0.000	0.110
0.600	0.01	0.0014	0.000	0.130
0.750	0.01	0.0017	0.000	0.150
0.800	0.01	0.0018	0.720	0.150
1.000	0.01	0.0023	8.050	0.170
1.100	0.01	0.0025	13.330	0.180
1.200	0.01	0.0027	19.440	0.190
1.300	0.01	0.0030	26.270	0.190
1.400	0.01	0.0032	33.750	0.200
1.420	0.01	0.0033	35.320	0.200
1.440	0.01	0.0033	36.910	0.200
1.450	0.01	0.0034	37.920	0.200
1.460	0.01	0.0035	38.530	0.200

END FTABLE570

FTABLE 61  
 ROWS COLS \*\*\*  
 \*\*\* SDN-2X: DETAIN OVERFLOW FROM NCPS AND NSMPS-  
 17 4

DEPTH	AREA	VOLUME	OUTFLOW ***
0.000	0.5740	0.0000	0.00
1.200	0.5740	0.7710	0.151
2.220	0.5740	1.4270	0.205
3.240	0.5740	2.0830	0.247
3.650	0.5740	2.3460	0.262
4.260	0.5740	2.7380	0.283
4.660	0.5740	2.9950	0.296
5.680	0.5740	3.6510	0.327
6.640	0.5740	4.2680	0.517
7.650	0.5740	4.9170	0.644
8.670	0.5740	5.9710	0.739
9.810	0.5740	6.3570	0.836

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10.700	0.5740	6.8780	0.894
12.000	0.5740	7.7130	0.978
12.100	0.5740	7.7780	1.600
12.300	0.5740	7.9060	4.200
12.800	0.5740	8.2280	14.560

END FTABLE 61

PRE AMBAUM DETENTION \*\*\*  
 FTABLE 111  
 ROWS COLS \*\*\*

15	4				***
DEPTH	AREA	VOLUME	OUTFLOW		
0.000	0.0000	0.0000	0.00		
0.500	0.2160	0.0750	5.30		
1.000	0.2730	0.1990	21.10		
1.500	0.2890	0.3410	43.90		
2.000	0.2900	0.4830	68.80		
2.500	0.2910	0.6070	89.10		
3.000	0.2950	0.6820	90.00		
3.500	0.3000	2.1000	100.00		
4.000	0.3050	2.5000	105.00		
4.500	0.3100	3.0000	110.00		
5.000	0.3200	3.5000	120.00		
5.500	0.3300	4.0000	130.00		
6.000	0.3800	5.0530	166.48		
6.500	0.3980	5.9430	225.31		
7.000	0.4150	6.9040	320.10		

END FTABLE111

FTABLE 135  
 ROWS COLS \*\*\* VACA FARM  
 6 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.00	0.10	0.00	0.00	
1.00	0.10	0.10	4.00	
2.00	0.11	0.21	8.00	
2.50	1.00	0.48	13.00	
3.50	6.50	4.23	86.00	
4.50	13.00	13.98	235.00	

END FTABLE135

FTABLE 240  
 \*\*\* NORTH SNOWMELT PUMP STATION (SDN-2) (INSTALLED IN LATE 1997/1998) \*\*\*

14	5				***
DEPTH	AREA	VOLUME	(IWS)	(SDS)	***
(FT)	(ACRES)	(ACRE-FT)	(CFS)	(CFS)	***
0.0	0.002	0.00	0.00	0.00	
1.00	0.002	0.0023	0.00	0.00	
2.00	0.002	0.0046	1.67	0.00	
3.00	0.002	0.0069	1.67	0.00	
4.00	0.002	0.0092	1.67	0.00	
5.00	0.002	0.0115	1.67	0.00	
5.25	0.002	0.0121	1.67	1.53	
5.50	0.002	0.0126	1.67	6.06	
5.75	0.002	0.0132	1.67	12.65	
6.00	0.002	0.0138	1.67	19.83	

6.25	0.002	0.0144	1.67	25.66
6.50	0.002	0.0149	1.67	25.70
6.75	0.002	0.0155	1.67	26.70
7.00	0.002	0.0161	1.67	50.00

END FTABLE240

FTABLE 242

\*\*\* NORTH CARGO PUMP STATION (SDN-2) (INSTALLED IN OCTOBER 1997)

ROWS COLS

14 5

DEPTH (FT)	AREA (ACRES)	VOLUME (ACRE-FT)	(IWS) (CFS)	(SDS) (CFS)
0.0	0.002	0.00	0.00	0.00
1.00	0.002	0.0023	0.00	0.00
2.00	0.002	0.0046	6.13	0.00
3.00	0.002	0.0069	6.13	0.00
4.00	0.002	0.0092	6.13	0.00
5.00	0.002	0.0115	6.13	0.00
5.25	0.002	0.0121	6.13	0.28
5.50	0.002	0.0126	6.13	1.16
5.75	0.002	0.0132	6.13	2.53
6.00	0.002	0.0138	6.13	4.23
6.25	0.002	0.0144	6.13	6.05
6.50	0.002	0.0149	6.13	7.72
6.75	0.002	0.0155	6.13	8.50
7.00	0.002	0.0161	6.13	20.0

END FTABLE242

END FTABLES

MASS-LINK

<Volume>	<-Grp>	<-Member->	<--Mult-->	<Target>	<-Grp>	<-Member->
<Name>	<Name>	#	#<-factor->	<Name>	<Name>	# #
MASS-LINK		1				
conversion from acre-inches to acre-ft (1/12)					***	
PERLND	PWATER	PERO	0.0833333	RCHRES	INFLOW	IVOL
END MASS-LINK		1				
MASS-LINK		2				
IMPLND	IWATER	SURO	0.0833333	RCHRES	INFLOW	IVOL
END MASS-LINK		2				
MASS-LINK		3				
RCHRES	ROFLOW			RCHRES	INFLOW	
END MASS-LINK		3				
MASS-LINK		4				
RCHRES	OFLOW	OVOL	1	RCHRES	INFLOW	IVOL
END MASS-LINK		4				
MASS-LINK		5				
RCHRES	OFLOW	OVOL	2	RCHRES	INFLOW	IVOL
END MASS-LINK		5				
MASS-LINK		6				
PERLND	PWATER	SURO	0.0833333	RCHRES	INFLOW	IVOL
PERLND	PWATER	IFWO	0.0833333	RCHRES	INFLOW	IVOL

```

END MASS-LINK      6

MASS-LINK          7
PERLND    PWATER AGWO      0.0833333  RCHRES      INFLOW IVOL
END MASS-LINK      7

MASS-LINK          10
COPY      OUTPUT MEAN      COPY      INPUT  MEAN
END MASS-LINK      10

MASS-LINK          11
RCHRES    ROFLOW      COPY      INPUT  MEAN
END MASS-LINK      11

MASS-LINK          12
COPY      OUTPUT MEAN      RCHRES      INFLOW IVOL
END MASS-LINK      12

MASS-LINK          14
RCHRES    OFLOW  OVOL      1      COPY      INPUT  MEAN
END MASS-LINK      14

MASS-LINK          15
RCHRES    OFLOW  OVOL      2      COPY      INPUT  MEAN
END MASS-LINK      15

MASS-LINK          21
PERLND    PWATER PERO      0.0833333  COPY      INPUT  MEAN
END MASS-LINK      21

MASS-LINK          22
IMPLND    IWATER SURO      0.0833333  COPY      INPUT  MEAN
END MASS-LINK      22

MASS-LINK          26
PERLND    PWATER SURO      0.0833333  COPY      INPUT  MEAN
PERLND    PWATER IFWO      0.0833333  COPY      INPUT  MEAN
END MASS-LINK      26

MASS-LINK          27
PERLND    PWATER AGWO      0.0833333  COPY      INPUT  MEAN
END MASS-LINK      27

```

END MASS-LINK

COPY

TIMESERIES

Copy-opn

#	-	#	NPT	NMN
37		71		1
240		242		1
357		357		1
645		645		1

\*\*\*  
\*\*\*

END TIMESERIES

END COPY

END RUN

RUN

GLOBAL

```

*** FILE: WClowflo.inp REVISED July 2001. ATC
*** SEATAC AIRPORT HSPF BASIN MODEL OF WALKER CREEK
*** 2006 FUTURE PROJECT CONDITION SIZING BASED
*** CHANGED PREC DSN 2 TO NEW PREC DSN 1002
*** WALKER CREEK FOUR YEAR RUN USING CALIBRATION INITIAL CONDITIONS
WALKER CREEK BASIN HSPF MODEL
START      1990 10  1  0  0  END      1994  9 30 24  0

```

```

RUN INTERP OUTPUT LEVEL      3
RESUME      0 RUN      1

```

END GLOBAL

FILES

```

<type> <fun>***<-----fname----->
MESSU      24  D:\PARA\SEATAC\MILLER\LOWFLOW\WALKER.MES
WDM        25  D:\PARA\SEATAC\MILLER\LOWFLOW\m_lowflow.WDM
           61  D:\PARA\SEATAC\MILLER\LOWFLOW\wPER.L61
           62  D:\PARA\SEATAC\MILLER\LOWFLOW\WRCH.L62

```

END FILES

OPN SEQUENCE

```

INGRP              INDELT 01:00
  PERLND           14
  PERLND           16
  PERLND           18
  PERLND           24
  PERLND           26
  PERLND           28
  PERLND           34
  PERLND           44
  PERLND           45
  PERLND           54
  PERLND           64
  PERLND           65
  PERLND           80
  IMPLND           14
  RCHRES           49
  RCHRES           20
  RCHRES           19
  RCHRES           18

```

END INGRP

END OPN SEQUENCE

\*\*\*

COPY

TIMESERIES

Copy-opn

```

# - # NPT NMN
1   5   1

```

END TIMESERIES

END COPY

\*\*\*

\*\*\*

PERLND

GEN-INFO

```

<PLS >      Name      NBLKS  Unit-systems  Printer
# - #              User  t-series Engl Metr

```

\*\*\*

\*\*\*

```

                                     in out
14   TFF- TILL FOR FLT             1 1 1 1 61 0
16   TFM- TILL FOR MOD             1 1 1 1 61 0
18   TFS- TILL FOR STP             1 1 1 1 61 0
24   TGF- TILL GR FLT              1 1 1 1 61 0
26   TGM- TILL GR MOD              1 1 1 1 61 0
28   TGS- TILL GR STP              1 1 1 1 61 0
34   OF - OUTWASH FOR              1 1 1 1 61 0
44   OG - OUTWASH GR               1 1 1 1 61 0
***PERLND FOR NEW AIRPORT FILL; NONE IN CALIBRATION
45   AIRPORT FILL                  1 1 1 1 61 0
54   SA - WETLANDS                 1 1 1 1 61 0
64   TGM DES MOINES                1 1 1 1 61 0
65   OG DES MOINES                 1 1 1 1 61 0
80   Fill AGWO                     1 1 1 1 61 0

```

END GEN-INFO

ACTIVITY

```

<PLS > ***** Active Sections *****
# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC
14 200 0 0 1 0 0 0 0 0 0 0 0 0

```

END ACTIVITY

PRINT-INFO

```

<PLS > ***** Print-flags ***** PIVL PYR
# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC *****
14 200 0 0 5 0 0 0 0 0 0 0 0 0 1 9

```

END PRINT-INFO

PWAT-PARM1

```

<PLS > ***** Flags *****
# - # CSNO RTOP UZFG VCS VUZ VNN VIFW VIRC VLE
14 0 0 0 0 0 0 0 0 0
16 0 0 0 0 0 0 0 0 0
18 0 0 0 0 0 0 0 0 0
24 0 0 0 0 0 0 0 0 0
26 0 0 0 0 0 0 0 0 0
28 0 0 0 0 0 0 0 0 0
34 0 0 0 0 0 0 0 0 0
44 0 0 0 0 0 0 0 0 0
45 0 0 0 0 0 0 0 0 0
54 0 0 0 0 0 0 0 0 0
64 0 0 0 0 0 0 0 0 0
80 0 0 0 0 0 0 0 0 0

```

END PWAT-PARM1

PWAT-PARM2

```

<PLS > ***
# - # ***FOREST LZSN INFILT LSUR SLSUR KVARY AGWRC
14 4.5000 0.0800 400.00 0.0500 0.5000 0.9960
16 4.5000 0.0800 400.00 0.1000 0.5000 0.9960
18 4.5000 0.0800 200.00 0.2000 0.5000 0.9960
24 4.5000 0.0300 400.00 0.0500 0.5000 0.9960
26 4.5000 0.0300 400.00 0.1000 0.5000 0.9960
28 4.5000 0.0300 200.00 0.2000 0.5000 0.9960
34 5.0000 2.0000 400.00 0.0500 0.3000 0.9960
44 5.0000 0.8000 400.00 0.0500 0.3000 0.9960
45 7.5000 0.0200 300.00 0.0700 0.0000 0.9960
54 4.0000 2.0000 100.00 0.0010 0.5000 0.9960
64 4.5000 0.1200 400.00 0.1000 0.5000 0.9990
65 5.0000 0.8000 400.00 0.0500 0.5000 0.9960

```



WDM 7100 FLOW ENGL .000000957 RCHRES 20 INFLOW IVOL  
 END EXT SOURCES

\*\*\*

EXT TARGETS

<-Volume->	<-Grp>	<-Member->	<-Mult-->	Tran	<-Volume->	<Member>	Tsys	Tgap	Amd	***	
<Name>	#	<Name>	#	<-factor->	strg	<Name>	#	<Name>	tem	strg	strg***
RCHRES	18	HYDR	RO		WDM	7196	FLOW	ENGL		REPL	
RCHRES	20	HYDR	RO		WDM	7197	FLOW	ENGL		REPL	
***PROJECT CONDITION FLOWS											
RCHRES	49	HYDR	RO	1 1	***	WDM	109	FLOW	ENGL	REPL	
*** DETENTION STAGES											
RCHRES	49	HYDR	STAGE		***	WDM	649	STAG	ENGL	REPL	
*** DETENTION VOLUME											
RCHRES	49	HYDR	VOL		***	WDM	749	VOL	ENGL	REPL	
*** 39=SR509 37=SDW2											
END EXT TARGETS											

\*\*\*

SCHEMATIC

<-Source->	<-Area-->	<-Target->	MBLK	***		
<Name>	#	<-factor->	<Name>	#	Tbl#	***

\*\*\*WALKER CREEK

*** SUB-CATCHMENT MC 8						
PERLND	26	3.93	RCHRES	20	1	
PERLND	44	18.73	RCHRES	20	1	
PERLND	54	2.70	RCHRES	20	1	
IMPLND	14	0.01	RCHRES	20	2	
*** *** SUB-CATCHMENT SDW-2						
***PERLND	26	26.82	RCHRES	49	6	
***PERLND	44	1.42	RCHRES	49	6	
***PERLND	45	6.70	RCHRES	49	6	
***PERLND	26	26.82	RCHRES	20	7	
***PERLND	44	1.42	RCHRES	20	7	
***PERLND	45	6.70	RCHRES	20	7	
***IMPLND	14	9.51	RCHRES	49	2	
***REPLACE SUBCATCHMENT SDW-2 WITH NEW SDW-2 SUBBASIN						
***SUBBASIN SDW-2 ROUTING AS OF 4/19/01						
*** old fill area						
***PERLND	26	26.88	RCHRES	49	6	
*** area removed = 0.04 ac						
PERLND	26	26.84	RCHRES	49	6	
PERLND	44	1.51	RCHRES	49	6	
*** old fill area						
***PERLND	45	6.700	RCHRES	49	6	
*** area removed = 4.66 ac						
PERLND	45	2.040	RCHRES	49	6	
PERLND	26	26.84	RCHRES	20	7	
PERLND	44	1.51	RCHRES	20	7	
PERLND	45	2.040	RCHRES	20	7	
*** old fill area						
***IMPLND	14	9.51	RCHRES	49	2	
*** area removed = 3.35 ac						
IMPLND	14	6.16	RCHRES	49	2	
*** FILL AGWO PERLND 80 ALL TO GROUNDWATER						
PERLND	80	8.05	RCHRES	20	7	
*** SUB-CATCHMENT MC 9						
PERLND	26	9.28	RCHRES	20	1	

PERLND	44	0.76	RCHRES	20	1
IMPLND	14	0.40	RCHRES	20	2

\*\*\* SUB-CATCHMENT 18

PERLND	16	0.76	RCHRES	18	1
PERLND	26	16.08	RCHRES	18	1
PERLND	34	20.95	RCHRES	18	1
PERLND	44	49.22	RCHRES	18	1
IMPLND	14	3.30	RCHRES	18	2

\*\*\* SUB-CATCHMENT 19

PERLND	16	12.72	RCHRES	19	1
PERLND	26	92.07	RCHRES	19	1
PERLND	34	8.39	RCHRES	19	1
PERLND	44	95.55	RCHRES	19	1
IMPLND	14	30.53	RCHRES	19	2

\*\*\* SUB-CATCHMENT 20

PERLND	26	12.54	RCHRES	20	1
PERLND	44	53.42	RCHRES	20	1
PERLND	54	33.43	RCHRES	20	1
IMPLND	14	52.83	RCHRES	20	2

\*\*\* DOWN STREAM OF WALKER CREEK GAGE

\*\*\* SUB-CATCHMENT 21 33% OF GW GOES TO GAGE REST GOES TO SOUND

PERLND	16	2.54	RCHRES	18	7
PERLND	26	44.30	RCHRES	18	7
PERLND	34	2.03	RCHRES	18	7
PERLND	44	41.13	RCHRES	18	7
PERLND	16	2.54	RCHRES	21	6
PERLND	26	44.30	RCHRES	21	6
PERLND	34	2.03	RCHRES	21	6
PERLND	44	41.13	RCHRES	21	6
IMPLND	14	16.54	RCHRES	21	2
PERLND	16	5.07	RCHRES	21	1
PERLND	26	88.61	RCHRES	21	1
PERLND	34	4.06	RCHRES	21	1
PERLND	44	82.26	RCHRES	21	1
IMPLND	14	33.09	RCHRES	21	2

\*\*\* SUB-CATCHMENT 22

PERLND	34	4.30	RCHRES	22	1
PERLND	44	19.49	RCHRES	22	1
PERLND	54	3.21	RCHRES	22	1
IMPLND	14	3.95	RCHRES	22	2

\*\*\*GROUNDWATER FROM OUTSIDE OF WALKER CREEK

PERLND	64	***	630.00	RCHRES	20	7
*** reduced area because of new impervious						
PERLND	64		570.66	RCHRES	20	7
PERLND	65	***	130.00	RCHRES	20	7

\*\*\*STREAM ROUTING

RCHRES	49		RCHRES	20	3
RCHRES	20		RCHRES	19	3
RCHRES	19		RCHRES	18	3

END SCHEMATIC

80 4.5000 0.0300 400.00 0.1000 0.5000 0.9960  
 END PWAT-PARM2  
 PWAT-PARM3

<PLS >\*\*\*  
 # - #\*\*\* PETMAX PETMIN INFEXP INFILD DEEPFR BASETP AGWETP  
 14 2.0000 2.0000 0.00 0.00 0.0  
 16 2.0000 2.0000 0.00 0.00 0.0  
 18 2.0000 2.0000 0.00 0.00 0.0  
 24 2.0000 2.0000 0.00 0.00 0.  
 26 2.0000 2.0000 0.00 0. 0.  
 28 2.0000 2.0000 0.00 0. 0.  
 34 2.0000 2.0000 0.00 0.00 0.0  
 44 2.0000 2.0000 0.00 0. 0.  
 45 2.0000 2.0000 0.00 0. 0.  
 54 10.000 2.0000 0.00 0. 0.7  
 64 2.0000 2.0000 0.00 0. 0.0  
 80 2.0000 2.0000 0.00 0. 0.

END PWAT-PARM3  
 PWAT-PARM4

<PLS > \*\*\*  
 # - # CEPSC UZSN NSUR INTFW IRC LZETP\*\*\*  
 14 0.2000 1.0000 0.3500 2.000 0.1500 0.7000  
 16 0.2000 0.5000 0.3500 2.000 0.1500 0.7000  
 18 0.2000 0.3000 0.3500 2.000 0.1500 0.7000  
 24 0.1000 0.5000 0.2500 2.000 0.1500 0.2500  
 26 0.1000 0.2500 0.2500 2.000 0.1500 0.2500  
 28 0.1000 0.1500 0.2500 2.000 0.1500 0.2500  
 34 0.2000 0.5000 0.3500 0.000 0.5000 0.7000  
 44 0.1000 0.5000 0.2500 0.000 0.5000 0.2500  
 45 0.1000 0.2800 0.2500 6.000 0.1500 0.6000  
 54 0.1000 3.0000 0.5000 1.000 0.7000 0.8000  
 64 0.1000 0.2500 0.2500 3.000 0.5000 0.2500  
 65 0.1000 0.5000 0.2500 0.000 0.5000 0.2500  
 80 0.1000 0.2500 0.2500 2.000 0.1500 0.2500

END PWAT-PARM4  
 PWAT-STATE1

<PLS > PWATER state variables\*\*\*  
 # - #\*\*\* CEPS SURS UZS IFWS LZS AGWS GWVS  
 14 0.078 0. 0.2500 0.10 2.500 2.00 0.000  
 16 0.078 0. 0.2500 0.10 2.500 2.00 0.000  
 18 0.078 0. 0.2500 0.10 2.500 2.00 0.000  
 24 0.051 0. 0.2500 0.10 2.500 2.00 0.000  
 26 0.051 0. 0.2500 0.10 2.500 2.00 0.000  
 28 0.051 0. 0.2500 0.10 2.500 2.00 0.000  
 34 0.078 0. 0.2500 0.10 0.000 2.00 0.000  
 44 0.051 0. 0.2500 0.10 0.000 2.00 0.000  
 45 0.051 0. 0.2500 0.10 0.000 2.00 0.000  
 54 0.051 0. 0.2500 0.10 2.000 2.00 0.000  
 64 0.051 0. 0.2500 0.10 2.000 20.00 0.000  
 65 0.051 0. 0.2500 0.10 0.000 20.00 0.000  
 80 0.051 0. 0.2500 0.10 2.500 2.00 0.000

END PWAT-STATE1  
 END PERLND  
 IMPLND

GEN-INFO

<ILS > Name Unit-systems Printer \*\*\*  
 # - # User t-series Engl Metr \*\*\*

```

                                in out
14      IMPERVIOUS              1   1   1  60   0
END GEN-INFO
ACTIVITY
<ILS > ***** Active Sections ****
# - # ATMP SNOW IWAT  SLD  IWG IQAL  ***
14      0      0      1      0      0      0
END ACTIVITY
PRINT-INFO
<ILS > ***** Print-flags ***** PIVL  PYR
# - # ATMP SNOW IWAT  SLD  IWG IQAL *****
14      0      0      6      0      0      0      1      9
END PRINT-INFO
IWAT-PARM1
<ILS >          Flags          ***
# - # CSNO RTOP  VRS  VNN RTLI  ***
14      0      0      0      0      0
END IWAT-PARM1
IWAT-PARM2
<ILS >          ***
# - #          LSUR          SLSUR          NSUR          RETSC          ***
14      100.00      0.0100      0.1000      0.1000
END IWAT-PARM2
IWAT-PARM3
<ILS >          ***
# - #          PETMAX      PETMIN          ***
14
END IWAT-PARM3
IWAT-STATE1
<ILS >  IWATER state variables          ***
# - #          RETS          SURS          ***
14      1.0000E-3  1.0000E-3
END IWAT-STATE1
END IMPLND
EXT SOURCES
*** NOTE: The only RCHRES that precip and PET are applied to are lakes.
<-Volume-> <Member> SsysSgap<--Mult-->Tran <-Target vols> <-Grp> <-Member->
<Name> # <Name> # tem strg<-factor->strg <Name> # # <Name> # #
*** PRECIP/EVAP TO PERVIOUS/IMPERV SURFACES
WDM  1002 PREC      ENGLZERO  1.00      PERLND  14  65  EXTNL  PREC
WDM  1002 PREC      ENGLZERO  0.00      PERLND  80      EXTNL  PREC
WDM  1002 PREC      ENGLZERO  1.00      IMPLND  14      EXTNL  PREC
WDM   1  EVAP      ENGLZERO  0.8       PERLND  14  65  EXTNL  PETINP
WDM   1  EVAP      ENGLZERO  0.0       PERLND  80      EXTNL  PETINP
WDM   1  EVAP      ENGLZERO  0.8       IMPLND  14      EXTNL  PETINP
*** PRECIP/EVAP TO LAKES
WDM  1002 PREC      ENGLZERO      RCHRES  20      EXTNL  PREC
WDM   1  EVAP      ENGLZERO  0.8       RCHRES  20      EXTNL  POTEV
WDM  1002 PREC      ENGLZERO      RCHRES  49      EXTNL  PREC
WDM   1  EVAP      ENGLZERO  0.8       RCHRES  49      EXTNL  POTEV
*** till seepage groundwater flow from Fill area.  PGG time series
WDM  7101 FLOW      ENGLZERO.000001426  PERLND  80      EXTNL  AGWLI
*** Fill flow directly to stream

```

NETWORK

\*\*\* <MEMBER> SSYSSGAP<--MULT-->TRAN <-TARGET VOLS> <-MEMBER->
<NAME> # <NAME> TEM STRG<-FACTOR->STRG <NAME> # # <-GRP> <NAME> # # \*\*\*

END NETWORK

RCHRES

GEN-INFO

RCHRES Name Nexits Unit Systems Printer
# - #<-----><-----> User T-series Engr Metr LKFG
in out
18 Trib (0371A) M 18 1 1 1 1 62 0 0
19 Trib (0371A) M 19 1 1 1 1 62 0 0
20 Trib M 20 1 1 1 1 62 0 1
21 Trib (0371H) M 21 1 1 1 1 62 0 0
22 Trib (0371A) M 22 1 1 1 1 62 0 0
39 SR509 1 1 1 1 62 0 0
49 SDW2 POND 1 1 1 1 62 0 0

END GEN-INFO

ACTIVITY

RCHRES \*\*\*\*\* Active Sections \*\*\*\*\*
# - # HYFG ADFG CNFG HTFG SDFG GQFG OXFG NUFG PKFG PHFG
1 63 1 0 0 0 0 0 0 0 0 0 0

END ACTIVITY

PRINT-INFO

RCHRES \*\*\*\*\* Printout Flags \*\*\*\*\* PIVL PYR
# - # HYDR ADCA CONS HEAT SED GOL OXRX NUTR PLNK PHCB \*\*\*\*\*
1 63 5 0 0 0 0 0 0 0 0 0 1 9

END PRINT-INFO

HYDR-PARM1

RCHRES Flags for each HYDR Section
# - # VC A1 A2 A3 ODFVFG for each \*\*\* ODGTFG for each FUNCT for each
FG FG FG FG possible exit \*\*\* possible exit possible exit
\* \* \* \* \* \* \* \* \* \* \* \* \* \* \* \*
1 99 0 1 0 0 4 0 0 0 0 0 0 0 0 2 2 2 2 2

END HYDR-PARM1

HYDR-PARM2

RCHRES
# - # FTABNO LEN DELTH STCOR KS DB50
18 18 0.800 0.3
19 19 0.568 0.3
20 20 0.379 0.3
21 21 0.450 0.3
22 22 0.300 0.3
49 49 0.010 0.0 0.3

END HYDR-PARM2

HYDR-INIT

RCHRES Initial conditions for each HYDR section
# - # \*\*\* VOL Initial value of COLIND Initial value of OUTDGT
\*\*\* ac-ft for each possible exit for each possible exit
18 0.1 4.0
19 0.1 4.0

20	10.0	4.0
21	0.1	4.0
22	0.1	4.0
49	0.0	4.0

END HYDR-INIT  
 END RCHRES

FTABLES

FTABLE 18  
 ROWS COLS \*\*\*  
 3 4  
 DEPTH AREA VOLUME OUTFLOW \*\*\*  
 0.00 1.30 0.00 0.00  
 1.00 1.30 1.30 166.00  
 2.00 1.40 2.65 490.00  
 END FTABLE 18

FTABLE 19  
 ROWS COLS \*\*\*  
 3 4  
 DEPTH AREA VOLUME OUTFLOW \*\*\*  
 0.00 1.10 0.00 0.00  
 1.00 1.10 1.10 65.00  
 2.00 1.20 2.25 223.00  
 END FTABLE 19

FTABLE 49  
 \*\*\* PROJECT POND F SDW2  
 ROWS COLS \*\*\*  
 12 4  
 DEPTH AREA VOLUME OUTFLOW \*\*\*  
 0.000 0.8880 0.0000 0.00  
 1.000 0.9270 1.0823 0.25  
 2.000 0.9690 2.2096 0.35  
 3.000 1.0450 3.3828 0.42  
 4.000 1.0450 4.6027 0.49  
 5.000 1.0860 5.8726 0.55  
 6.000 1.1260 7.1935 0.60  
 7.000 1.1670 8.5645 1.20  
 8.000 1.2130 9.9861 2.69  
 8.300 1.2560 10.454 3.09  
 9.000 1.2560 11.459 7.57  
 10.000 1.3000 12.987 16.88  
 END FTABLE 49

FTABLE 20  
 \*\*\* WALKER CREEK WETLAND  
 ROWS COLS \*\*\*  
 10 4  
 DEPTH AREA VOLUME OUTFLOW OUTFLOW \*\*\*  
 0.00 0.00 0.00 0.00  
 1.00 2.50 1.25 7.04  
 2.00 5.00 5.00 17.84  
 3.00 12.00 13.50 32.17  
 4.00 19.00 29.00 45.13

5.00	22.00	49.50	54.95
6.00	23.00	72.00	61.62
6.10	23.00	74.30	62.15
7.00	23.50	95.25	67.00
7.24	24.10	101.10	100.00

END FTABLE 20

FTABLE 21  
ROWS COLS \*\*\*  
8 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.2259	0.0113	0.11	
0.500	0.2707	0.1106	4.27	
1.000	0.3268	0.2600	15.13	
1.500	0.3828	0.4374	31.67	
2.000	0.4389	0.6428	54.02	
2.500	0.4949	0.8763	82.52	
3.000	0.5510	1.1377	117.55	

END FTABLE 21

FTABLE 22  
ROWS COLS \*\*\*  
9 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.3680	0.0184	0.25	
0.500	0.3717	0.1664	9.39	
1.000	0.3763	0.3534	31.06	
2.000	0.3819	0.7325	94.37	
3.000	0.3874	1.1171	174.33	
4.000	0.3930	1.5073	265.38	
5.000	0.3985	1.9030	364.68	
6.000	0.4040	2.3043	470.60	

END FTABLE 22

END FTABLES

MASS-LINK

<Volume>	<-Grp>	<-Member-><--Mult-->	<Target>	<-Grp>	<-Member->***
<Name>		<Name> # #<-factor->	<Name>		<Name> # #***

MASS-LINK	1				
		conversion from acre-inches to acre-ft (1/12)		***	
PERLND	PWATER	PERO	0.0833333	RCHRES	INFLOW IVOL
END MASS-LINK	1				

MASS-LINK	2				
IMPLND	IWATER	SURO	0.0833333	RCHRES	INFLOW IVOL
END MASS-LINK	2				

MASS-LINK	3				
RCHRES	ROFLOW			RCHRES	INFLOW
END MASS-LINK	3				

MASS-LINK	4				
RCHRES	OFLOW	OVOL	1	RCHRES	INFLOW IVOL
END MASS-LINK	4				

MASS-LINK 5

RCHRES	OFLOW	OVOL	2		RCHRES	INFLOW	IVOL
END MASS-LINK		5					
MASS-LINK		6					
PERLND	PWATER	SURO	0.0833333		RCHRES	INFLOW	IVOL
PERLND	PWATER	IFWO	0.0833333		RCHRES	INFLOW	IVOL
END MASS-LINK		6					
MASS-LINK		7					
PERLND	PWATER	AGWO	0.0833333		RCHRES	INFLOW	IVOL
END MASS-LINK		7					
MASS-LINK		8					
PERLND	PWATER	PERO	0.0833333		COPY	INPUT	MEAN
END MASS-LINK		8					
MASS-LINK		12					
PERLND	PWATER	AGWO	0.0833333		COPY	INPUT	MEAN
END MASS-LINK		12					
MASS-LINK		9					
IMPLND	IWATER	SURO	0.0833333		COPY	INPUT	MEAN
END MASS-LINK		9					
MASS-LINK		10					
COPY	OUTPUT	MEAN			RCHRES	INFLOW	IVOL
END MASS-LINK		10					
END MASS-LINK							
END RUN							



RUN

GLOBAL

\*\*\* FILE: WClowfT.inp REVISED July 2001. ATC  
\*\*\* SEATAC AIRPORT HSPF BASIN MODEL OF WALKER CREEK  
\*\*\* 2006 FUTURE PROJECT CONDITION SIZING BASED  
\*\*\* CHANGED PREC DSN 2 TO NEW PREC DSN 1002  
\*\*\* WALKER CREEK FOUR YEAR RUN USING FULL LENGTH 1990 DATA FOR  
\*\*\* INITIAL CONDITIONS  
WALKER CREEK BASIN HSPF MODEL  
START 1990 10 1 0 0 END 1994 9 30 24 0

RUN INTERP OUTPUT LEVEL 3  
RESUME 0 RUN 1

END GLOBAL

FILES

<type> <fun>\*\*\*<-----fname----->  
MESSU 24 D:\PARA\SEATAC\MILLER\LOWFLOW\WALKER.MES  
WDM 25 D:\PARA\SEATAC\MILLER\LOWFLOW\mlowflow.WDM  
61 D:\PARA\SEATAC\MILLER\LOWFLOW\wPER.L61  
62 D:\PARA\SEATAC\MILLER\LOWFLOW\wrch.L62

END FILES

OPN SEQUENCE

INGRP INDELT 01:00  
PERLND 14  
PERLND 16  
PERLND 18  
PERLND 24  
PERLND 26  
PERLND 28  
PERLND 34  
PERLND 44  
PERLND 45  
PERLND 54  
PERLND 64  
PERLND 65  
PERLND 80  
IMPLND 14  
RCHRES 49  
RCHRES 20  
RCHRES 19  
RCHRES 18

END INGRP

END OPN SEQUENCE

\*\*\*

COPY

TIMESERIES

Copy-opn

# - # NPT NMN  
1 5 1

END TIMESERIES

END COPY

\*\*\*

\*\*\*

PERLND

GEN-INFO

<PLS > Name NBLKS Unit-systems Printer

\*\*\*

```

# - # User t-series Engl Metr
in out
14 TFF- TILL FOR FLT 1 1 1 1 61 0
16 TFM- TILL FOR MOD 1 1 1 1 61 0
18 TFS- TILL FOR STP 1 1 1 1 61 0
24 TGF- TILL GR FLT 1 1 1 1 61 0
26 TGM- TILL GR MOD 1 1 1 1 61 0
28 TGS- TILL GR STP 1 1 1 1 61 0
34 OF - OUTWASH FOR 1 1 1 1 61 0
44 OG - OUTWASH GR 1 1 1 1 61 0
***PERLND FOR NEW AIRPORT FILL; NONE IN CALIBRATION
45 AIRPORT FILL 1 1 1 1 61 0
54 SA - WETLANDS 1 1 1 1 61 0
64 TGM DES MOINES 1 1 1 1 61 0
65 OG DES MOINES 1 1 1 1 61 0
80 Fill AGWO 1 1 1 1 61 0

```

END GEN-INFO

ACTIVITY

```

<PLS > ***** Active Sections *****
# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC
14 200 0 0 1 0 0 0 0 0 0 0 0 0

```

END ACTIVITY

PRINT-INFO

```

<PLS > ***** Print-flags ***** PIVL PYR
# - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC *****
14 200 0 0 5 0 0 0 0 0 0 0 0 0 1 9

```

END PRINT-INFO

PWAT-PARM1

```

<PLS > ***** Flags *****
# - # CSNO RTOP UZFG VCS VUZ VNN VIFW VIRC VLE
14 0 0 0 0 0 0 0 0 0
16 0 0 0 0 0 0 0 0 0
18 0 0 0 0 0 0 0 0 0
24 0 0 0 0 0 0 0 0 0
26 0 0 0 0 0 0 0 0 0
28 0 0 0 0 0 0 0 0 0
34 0 0 0 0 0 0 0 0 0
44 0 0 0 0 0 0 0 0 0
45 0 0 0 0 0 0 0 0 0
54 0 0 0 0 0 0 0 0 0
64 0 0 0 0 0 0 0 0 0
80 0 0 0 0 0 0 0 0 0

```

END PWAT-PARM1

PWAT-PARM2

```

<PLS > ***
# - # ***FOREST LZSN INFILT LSUR SLSUR KVARY AGWRC
14 4.5000 0.0800 400.00 0.0500 0.5000 0.9960
16 4.5000 0.0800 400.00 0.1000 0.5000 0.9960
18 4.5000 0.0800 200.00 0.2000 0.5000 0.9960
24 4.5000 0.0300 400.00 0.0500 0.5000 0.9960
26 4.5000 0.0300 400.00 0.1000 0.5000 0.9960
28 4.5000 0.0300 200.00 0.2000 0.5000 0.9960
34 5.0000 2.0000 400.00 0.0500 0.3000 0.9960
44 5.0000 0.8000 400.00 0.0500 0.3000 0.9960
45 7.5000 0.0200 300.00 0.0700 0.0000 0.9960
54 4.0000 2.0000 100.00 0.0010 0.5000 0.9960
64 4.5000 0.1200 400.00 0.1000 0.5000 0.9990

```

65	5.0000	0.8000	400.00	0.0500	0.5000	0.9960
80	4.5000	0.0300	400.00	0.1000	0.5000	0.9960

END PWAT-PARM2

PWAT-PARM3

<PLS >\*\*\*

#	-	****	PETMAX	PETMIN	INFEXP	INFILD	DEEPPFR	BASETP	AGWETP
14					2.0000	2.0000	0.00	0.00	0.0
16					2.0000	2.0000	0.00	0.00	0.0
18					2.0000	2.0000	0.00	0.00	0.0
24					2.0000	2.0000	0.00	0.00	0.
26					2.0000	2.0000	0.00	0.	0.
28					2.0000	2.0000	0.00	0.	0.
34					2.0000	2.0000	0.00	0.00	0.0
44					2.0000	2.0000	0.00	0.	0.
45					2.0000	2.0000	0.00	0.	0.
54					10.000	2.0000	0.00	0.	0.7
64					2.0000	2.0000	0.00	0.	0.0
80					2.0000	2.0000	0.00	0.	0.

END PWAT-PARM3

PWAT-PARM4

<PLS >

#	-	#	CEPSC	UZSN	NSUR	INTFW	IRC	LZETP***
14			0.2000	1.0000	0.3500	2.000	0.1500	0.7000
16			0.2000	0.5000	0.3500	2.000	0.1500	0.7000
18			0.2000	0.3000	0.3500	2.000	0.1500	0.7000
24			0.1000	0.5000	0.2500	2.000	0.1500	0.2500
26			0.1000	0.2500	0.2500	2.000	0.1500	0.2500
28			0.1000	0.1500	0.2500	2.000	0.1500	0.2500
34			0.2000	0.5000	0.3500	0.000	0.5000	0.7000
44			0.1000	0.5000	0.2500	0.000	0.5000	0.2500
45			0.1000	0.2800	0.2500	6.000	0.1500	0.6000
54			0.1000	3.0000	0.5000	1.000	0.7000	0.8000
64			0.1000	0.2500	0.2500	3.000	0.5000	0.2500
65			0.1000	0.5000	0.2500	0.000	0.5000	0.2500
80			0.1000	0.2500	0.2500	2.000	0.1500	0.2500

END PWAT-PARM4

PWAT-STATE1

<PLS > PWATER state variables\*\*\*

#	-	****	CEPS	SURS	UZS	IFWS	LZS	AGWS	GWVS
14			0.000	0.	0.0000	0.00	2.500	2.00	0.000
16			0.000	0.	0.0010	0.00	0.020	3.51	0.047
18			0.000	0.	0.0000	0.00	2.500	2.00	0.000
24			0.000	0.	0.0000	0.00	2.500	2.00	0.000
26			0.000	0.	0.0000	0.00	1.598	2.81	0.041
28			0.000	0.	0.0000	0.00	2.500	2.00	0.000
34			0.000	0.	0.0000	0.00	0.023	5.66	0.084
44			0.000	0.	0.0000	0.00	2.756	6.25	0.134
45			0.000	0.	0.0000	0.00	0.373	3.01	0.000
54			0.000	0.	0.3650	0.00	0.561	0.00	0.000
64			0.000	0.	0.0000	0.00	1.978	22.28	0.000
65			0.000	0.	0.0000	0.00	0.000	20.00	0.000
80			0.000	0.	0.0000	0.00	1.598	2.81	0.041

END PWAT-STATE1

END PERLND

IMPLND

GEN-INFO

<ILS >

Name

Unit-systems

Printer

\*\*\*

```

# - # User t-series Engl Metr ***
in out ***
14 IMPERVIOUS 1 1 1 60 0
END GEN-INFO
ACTIVITY
<ILS > ***** Active Sections ****
# - # ATMP SNOW IWAT SLD IWG IQAL ***
14 0 0 1 0 0 0
END ACTIVITY
PRINT-INFO
<ILS > ***** Print-flags ***** PIVL PYR
# - # ATMP SNOW IWAT SLD IWG IQAL *****
14 0 0 6 0 0 0 1 9
END PRINT-INFO
IWAT-PARM1
<ILS > Flags ***
# - # CSNO RTOP VRS VNN RTLI ***
14 0 0 0 0 0
END IWAT-PARM1
IWAT-PARM2
<ILS > ***
# - # LSUR SLSUR NSUR RETSC ***
14 100.00 0.0100 0.1000 0.1000
END IWAT-PARM2
IWAT-PARM3
<ILS > ***
# - # PETMAX PETMIN ***
14
END IWAT-PARM3
IWAT-STATE1
<ILS > IWATER state variables ***
# - # RETS SURS ***
14 1.0000E-3 1.0000E-3
END IWAT-STATE1
END IMPLND ***
EXT SOURCES ***
*** NOTE: The only RCHRES that precip and PET are applied to are lakes. ***
<-Volume-> <Member> SsysSgap<--Mult-->Tran <-Target vols> <-Grp> <-Member-> ***
<Name> # <Name> # tem strg<-factor->strg <Name> # # <Name> # # ***
*** PRECIP/EVAP TO PERVIOUS/IMPERV SURFACES
WDM 1002 PREC ENGLZERO 1.00 PERLND 14 65 EXTNL PREC
WDM 1002 PREC ENGLZERO 0.00 PERLND 80 EXTNL PREC
WDM 1002 PREC ENGLZERO 1.00 IMPLND 14 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 PERLND 14 65 EXTNL PETINP
WDM 1 EVAP ENGLZERO 0.0 PERLND 80 EXTNL PETINP
WDM 1 EVAP ENGLZERO 0.8 IMPLND 14 EXTNL PETINP
*** PRECIP/EVAP TO LAKES
WDM 1002 PREC ENGLZERO RCHRES 20 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 20 EXTNL POTEV
WDM 1002 PREC ENGLZERO RCHRES 49 EXTNL PREC
WDM 1 EVAP ENGLZERO 0.8 RCHRES 49 EXTNL POTEV
*** till seepage groundwater flow from Fill area. PGG time series
WDM 7101 FLOW ENGLZERO.000001426 PERLND 80 EXTNL AGWLI

```

\*\*\* Fill flow directly to stream  
WDM 7100 FLOW ENGL .000000957 RCHRES 20 INFLOW IVOL  
END EXT SOURCES

\*\*\*

EXT TARGETS

<-Volume->	<-Grp>	<-Member->	<--Mult-->	Tran	<-Volume->	<Member>	Tsys	Tgap	Amd	***	
<Name>	#	<Name>	#	<-factor->	strg	<Name>	#	<Name>	tem	strg	strg***
RCHRES	18	HYDR	RO			WDM	7196	FLOW	ENGL		REPL
RCHRES	20	HYDR	RO			WDM	7197	FLOW	ENGL		REPL
***PROJECT CONDITION FLOWS											
RCHRES	49	HYDR	RO	1	1	***	WDM	109	FLOW	ENGL	REPL
*** DETENTION STAGES											
RCHRES	49	HYDR	STAGE			***	WDM	649	STAG	ENGL	REPL
*** DETENTION VOLUME											
RCHRES	49	HYDR	VOL			***	WDM	749	VOL	ENGL	REPL
*** 39=SR509 37=SDW2											
END EXT TARGETS											

\*\*\*

SCHEMATIC

<-Source->	<-Area-->	<-Target->	MBLK	***		
<Name>	#	<-factor->	<Name>	#	Tbl#	***

\*\*\*WALKER CREEK

\*\*\* SUB-CATCHMENT MC 8

PERLND	26	3.93	RCHRES	20	1
PERLND	44	18.73	RCHRES	20	1
PERLND	54	2.70	RCHRES	20	1
IMPLND	14	0.01	RCHRES	20	2

\*\*\* SUB-CATCHMENT SDW-2

***PERLND	26	26.82	RCHRES	49	6
***PERLND	44	1.42	RCHRES	49	6
***PERLND	45	6.70	RCHRES	49	6
***PERLND	26	26.82	RCHRES	20	7
***PERLND	44	1.42	RCHRES	20	7
***PERLND	45	6.70	RCHRES	20	7
***IMPLND	14	9.51	RCHRES	49	2

\*\*\*REPLACE SUBCATCHMENT SDW-2 WITH NEW SDW-2 SUBBASIN

\*\*\*SUBBASIN SDW-2 ROUTING AS OF 4/19/01

\*\*\* old fill area

***PERLND	26	26.88	RCHRES	49	6
-----------	----	-------	--------	----	---

\*\*\* area removed = 0.04 ac

PERLND	26	26.84	RCHRES	49	6
PERLND	44	1.51	RCHRES	49	6

\*\*\* old fill area

***PERLND	45	6.700	RCHRES	49	6
-----------	----	-------	--------	----	---

\*\*\* area removed = 4.66 ac

PERLND	45	2.040	RCHRES	49	6
PERLND	26	26.84	RCHRES	20	7
PERLND	44	1.51	RCHRES	20	7
PERLND	45	2.040	RCHRES	20	7

\*\*\* old fill area

***IMPLND	14	9.51	RCHRES	49	2
-----------	----	------	--------	----	---

\*\*\* area removed = 3.35 ac

IMPLND	14	6.16	RCHRES	49	2
--------	----	------	--------	----	---

\*\*\* FILL AGWO PERLND 80 ALL TO GROUNDWATER RCHRES 20

PERLND	80	8.05	RCHRES	20	7
--------	----	------	--------	----	---

\*\*\* SUB-CATCHMENT MC 9

AR 011539

PERLND	26	9.28	RCHRES	20	1	
PERLND	44	0.76	RCHRES	20	1	
IMPLND	14	0.40	RCHRES	20	2	
*** SUB-CATCHMENT 18						
PERLND	16	0.76	RCHRES	18	1	
PERLND	26	16.08	RCHRES	18	1	
PERLND	34	20.95	RCHRES	18	1	
PERLND	44	49.22	RCHRES	18	1	
IMPLND	14	3.30	RCHRES	18	2	
*** SUB-CATCHMENT 19						
PERLND	16	12.72	RCHRES	19	1	
PERLND	26	92.07	RCHRES	19	1	
PERLND	34	8.39	RCHRES	19	1	
PERLND	44	95.55	RCHRES	19	1	
IMPLND	14	30.53	RCHRES	19	2	
*** SUB-CATCHMENT 20						
PERLND	26	12.54	RCHRES	20	1	
PERLND	44	53.42	RCHRES	20	1	
PERLND	54	33.43	RCHRES	20	1	
IMPLND	14	52.83	RCHRES	20	2	
*** DOWN STREAM OF WALKER CREEK GAGE						
*** SUB-CATCHMENT 21 33% OF GW GOES TO GAGE REST GOES TO SOUND						
PERLND	16	2.54	RCHRES	18	7	
PERLND	26	44.30	RCHRES	18	7	
PERLND	34	2.03	RCHRES	18	7	
PERLND	44	41.13	RCHRES	18	7	
PERLND	16	2.54	RCHRES	21	6	
PERLND	26	44.30	RCHRES	21	6	
PERLND	34	2.03	RCHRES	21	6	
PERLND	44	41.13	RCHRES	21	6	
IMPLND	14	16.54	RCHRES	21	2	
PERLND	16	5.07	RCHRES	21	1	
PERLND	26	88.61	RCHRES	21	1	
PERLND	34	4.06	RCHRES	21	1	
PERLND	44	82.26	RCHRES	21	1	
IMPLND	14	33.09	RCHRES	21	2	
*** SUB-CATCHMENT 22						
PERLND	34	4.30	RCHRES	22	1	
PERLND	44	19.49	RCHRES	22	1	
PERLND	54	3.21	RCHRES	22	1	
IMPLND	14	3.95	RCHRES	22	2	
***GROUNDWATER FROM OUTSIDE OF WALKER CREEK						
PERLND	64	***	630.00	RCHRES	20	7
*** reduced area because of new impervious						
PERLND	64		570.66	RCHRES	20	7
PERLND	65	***	130.00	RCHRES	20	7
***STREAM ROUTING						
RCHRES	49			RCHRES	20	3
RCHRES	20			RCHRES	19	3
RCHRES	19			RCHRES	18	3
END SCHEMATIC						

NETWORK

\*\*\* <MEMBER> SSSYSSGAP<--MULT-->TRAN <-TARGET VOLS> <-MEMBER->
<NAME> # <NAME> TEM STRG<-FACTOR->STRG <NAME> # # <-GRP> <NAME> # # \*\*\*

END NETWORK

RCHRES

GEN-INFO

RCHRES Name Nexits Unit Systems Printer
# - #<-----><----> User T-series Engl Metr LKFG
in out
18 Trib (0371A) M 18 1 1 1 1 62 0 0
19 Trib (0371A) M 19 1 1 1 1 62 0 0
20 Trib M 20 1 1 1 1 62 0 1
21 Trib (0371H) M 21 1 1 1 1 62 0 0
22 Trib (0371A) M 22 1 1 1 1 62 0 0
39 SR509 1 1 1 1 62 0 0
49 SDW2 POND 1 1 1 1 62 0 0

END GEN-INFO

ACTIVITY

RCHRES \*\*\*\*\* Active Sections \*\*\*\*\*
# - # HYFG ADFG CNFG HTFG SDFG GQFG OXFG NUGF PKFG PHFG
1 63 1 0 0 0 0 0 0 0 0 0 0

END ACTIVITY

PRINT-INFO

RCHRES \*\*\*\*\* Printout Flags \*\*\*\*\* PIVL PYR
# - # HYDR ADCA CONS HEAT SED GQL OXRX NUTR PLNK PHCB \*\*\*\*\*
1 63 5 0 0 0 0 0 0 0 0 0 0 1 9

END PRINT-INFO

HYDR-PARM1

RCHRES Flags for each HYDR Section
# - # VC A1 A2 A3 ODFVFG for each \*\*\* ODGTFG for each FUNCT for each
FG FG FG FG possible exit \*\*\* possible exit
\* \* \* \* \* \* \* \* \* \* \* \* \* \* \* \*
1 99 0 1 0 0 4 0 0 0 0 0 0 0 0 0 2 2 2 2 2

END HYDR-PARM1

HYDR-PARM2

RCHRES
# - # FTABNO LEN DELTH STCOR KS DB50
18 18 0.800 0.3
19 19 0.568 0.3
20 20 0.379 0.3
21 21 0.450 0.3
22 22 0.300 0.3
49 49 0.010 0.0 0.3

END HYDR-PARM2

HYDR-INIT

RCHRES Initial conditions for each HYDR section
# - # \*\*\* VOL Initial value of COLIND Initial value of OUTDGT
\*\*\* ac-ft for each possible exit for each possible exit
18 0.1 4.0

19	0.1	4.0
20	10.0	4.0
21	0.1	4.0
22	0.1	4.0
49	0.0	4.0

END HYDR-INIT  
END RCHRES

FTABLES

FTABLE 18  
ROWS COLS \*\*\*  
3 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.00	1.30	0.00	0.00	
1.00	1.30	1.30	166.00	
2.00	1.40	2.65	490.00	

END FTABLE 18

FTABLE 19  
ROWS COLS \*\*\*  
3 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.00	1.10	0.00	0.00	
1.00	1.10	1.10	65.00	
2.00	1.20	2.25	223.00	

END FTABLE 19

FTABLE 49  
\*\*\* PROJECT POND F SDW2  
ROWS COLS \*\*\*  
12 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.8880	0.0000	0.00	
1.000	0.9270	1.0823	0.25	
2.000	0.9690	2.2096	0.35	
3.000	1.0450	3.3828	0.42	
4.000	1.0450	4.6027	0.49	
5.000	1.0860	5.8726	0.55	
6.000	1.1260	7.1935	0.60	
7.000	1.1670	8.5645	1.20	
8.000	1.2130	9.9861	2.69	
8.300	1.2560	10.454	3.09	
9.000	1.2560	11.459	7.57	
10.000	1.3000	12.987	16.88	

END FTABLE 49

FTABLE 20  
\*\*\* WALKER CREEK WETLAND  
ROWS COLS \*\*\*  
10 4

DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW	***
0.00	0.00	0.00	0.00		
1.00	2.50	1.25	7.04		
2.00	5.00	5.00	17.84		
3.00	12.00	13.50	32.17		



4.00	19.00	29.00	45.13
5.00	22.00	49.50	54.95
6.00	23.00	72.00	61.62
6.10	23.00	74.30	62.15
7.00	23.50	95.25	67.00
7.24	24.10	101.10	100.00

END FTABLE 20

FTABLE 21  
ROWS COLS \*\*\*  
8 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.2259	0.0113	0.11	
0.500	0.2707	0.1106	4.27	
1.000	0.3268	0.2600	15.13	
1.500	0.3828	0.4374	31.67	
2.000	0.4389	0.6428	54.02	
2.500	0.4949	0.8763	82.52	
3.000	0.5510	1.1377	117.55	

END FTABLE 21

FTABLE 22  
ROWS COLS \*\*\*  
9 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.3680	0.0184	0.25	
0.500	0.3717	0.1664	9.39	
1.000	0.3763	0.3534	31.06	
2.000	0.3819	0.7325	94.37	
3.000	0.3874	1.1171	174.33	
4.000	0.3930	1.5073	265.38	
5.000	0.3985	1.9030	364.68	
6.000	0.4040	2.3043	470.60	

END FTABLE 22

END FTABLES

MASS-LINK

<Volume>	<-Grp>	<-Member->	<--Mult-->	<Target>	<-Grp>	<-Member->	***
<Name>		<Name> #	<-factor->	<Name>		<Name> #	***
MASS-LINK		1					***
conversion from acre-inches to acre-ft (1/12)							
PERLND	PWATER	PERO	0.0833333	RCHRES			INFLOW IVOL
END MASS-LINK 1							
MASS-LINK		2					***
IMPLND	IWATER	SURO	0.0833333	RCHRES			INFLOW IVOL
END MASS-LINK 2							
MASS-LINK		3					***
RCHRES	ROFLOW			RCHRES			INFLOW
END MASS-LINK 3							
MASS-LINK		4					***
RCHRES	OFLOW	OVOL	1	RCHRES			INFLOW IVOL
END MASS-LINK 4							

MASS-LINK		5				
RCHRES	OFLOW	OVOL	2		RCHRES	INFLOW IVOL
END MASS-LINK		5				
MASS-LINK		6				
PERLND	PWATER	SURO	0.0833333		RCHRES	INFLOW IVOL
PERLND	PWATER	IFWO	0.0833333		RCHRES	INFLOW IVOL
END MASS-LINK		6				
MASS-LINK		7				
PERLND	PWATER	AGWO	0.0833333		RCHRES	INFLOW IVOL
END MASS-LINK		7				
MASS-LINK		8				
PERLND	PWATER	PERO	0.0833333		COPY	INPUT MEAN
END MASS-LINK		8				
MASS-LINK		12				
PERLND	PWATER	AGWO	0.0833333		COPY	INPUT MEAN
END MASS-LINK		12				
MASS-LINK		9				
IMPLND	IWATER	SURO	0.0833333		COPY	INPUT MEAN
END MASS-LINK		9				
MASS-LINK		10				
COPY	OUTPUT	MEAN			RCHRES	INFLOW IVOL
END MASS-LINK		10				
END MASS-LINK						
END RUN						

RUN

GLOBAL

```

*** COPY COMMAND ADDED
*** FILE: WCPREDF.inp REVISED OCTOBER 2000
*** SEATAC AIRPORT HSPF BASIN MODEL OF WALKER CREEK
*** CALIBRATION FILE RUN FOR FULL LENGTH OF RECORD
WALKER CREEK BASIN HSPF MODEL
START      1948 10 1 0 0  END      1996  9 30 24  0
RUN INTERP OUTPUT LEVEL      3
RESUME     0 RUN      1

```

END GLOBAL

FILES

```

<type> <fun>***<-----fname----->
MESSU      24  D:\PARA\SEATAC\MILLER\LOWFLOW\WALKER.MES
WDM        25  D:\PARA\SEATAC\MILLER\LOWFLOW\m_lowflow.WDM
           61  D:\PARA\SEATAC\MILLER\LOWFLOW\wPER.L61
           62  D:\PARA\SEATAC\MILLER\LOWFLOW\WRCH.L62

```

END FILES

OPN SEQUENCE

```

INGRP      INDELT 01:00
  PERLND    14
  PERLND    16
  PERLND    18
  PERLND    24
  PERLND    26
  PERLND    28
  PERLND    34
  PERLND    44
  PERLND    45
  PERLND    54
  PERLND    64
  PERLND    65
  IMPLND    14
  COPY       2
  COPY       1
  COPY       3
  RCHRES    20
  RCHRES    19
  RCHRES    18

```

END INGRP

END OPN SEQUENCE

\*\*\*

COPY

TIMESERIES

Copy-opn

```

# - # NPT NMN
1 5 1

```

END TIMESERIES

END COPY

\*\*\*

\*\*\*

PERLND

GEN-INFO

```

<PLS >      Name      NBLKS  Unit-systems  Printer
# - #      User  t-series  Engr Metr
              in  out

```

\*\*\*

\*\*\*

\*\*\*

14	TFF- TILL FOR FLT	1	1	1	1	61	0
16	TFM- TILL FOR MOD	1	1	1	1	61	0
18	TFS- TILL FOR STP	1	1	1	1	61	0
24	TGF- TILL GR FLT	1	1	1	1	61	0
26	TGM- TILL GR MOD	1	1	1	1	61	0
28	TGS- TILL GR STP	1	1	1	1	61	0
34	OF - OUTWASH FOR	1	1	1	1	61	0
44	OG - OUTWASH GR	1	1	1	1	61	0

\*\*\*PERLND FOR NEW AIRPORT FILL; NONE IN CALIBRATION

45	AIRPORT FILL	1	1	1	1	61	0
54	SA - WETLANDS	1	1	1	1	61	0
64	TGM DES MOINES	1	1	1	1	61	0
65	OG DES MOINES	1	1	1	1	61	0

END GEN-INFO

ACTIVITY

<PLS > \*\*\*\*\* Active Sections \*\*\*\*\*

#	-	#	ATMP	SNOW	PWAT	SED	PST	PWG	PQAL	MSTL	PEST	NITR	PHOS	TRAC	***
14		200	0	0	1	0	0	0	0	0	0	0	0	0	

END ACTIVITY

PRINT-INFO

<PLS > \*\*\*\*\* Print-flags \*\*\*\*\* PIVL PYR

#	-	#	ATMP	SNOW	PWAT	SED	PST	PWG	PQAL	MSTL	PEST	NITR	PHOS	TRAC	*****	PIVL	PYR
14		200	0	0	5	0	0	0	0	0	0	0	0	0		1	9

END PRINT-INFO

PWAT-PARM1

<PLS > \*\*\*\*\* Flags \*\*\*\*\*

#	-	#	CSNO	RTOP	UZFG	VCS	VUZ	VNN	VIFW	VIRC	VLE	***
14			0	0	0	0	0	0	0	0	0	
16			0	0	0	0	0	0	0	0	0	
18			0	0	0	0	0	0	0	0	0	
24			0	0	0	0	0	0	0	0	0	
26			0	0	0	0	0	0	0	0	0	
28			0	0	0	0	0	0	0	0	0	
34			0	0	0	0	0	0	0	0	0	
44			0	0	0	0	0	0	0	0	0	
45			0	0	0	0	0	0	0	0	0	
54			0	0	0	0	0	0	0	0	0	
64			0	0	0	0	0	0	0	0	0	

END PWAT-PARM1

PWAT-PARM2

<PLS > \*\*\*

#	-	#	***FOREST	LZSN	INFILT	LSUR	SLSUR	KVARY	AGWRC
14				4.5000	0.0800	400.00	0.0500	0.5000	0.9960
16				4.5000	0.0800	400.00	0.1000	0.5000	0.9960
18				4.5000	0.0800	200.00	0.2000	0.5000	0.9960
24				4.5000	0.0300	400.00	0.0500	0.5000	0.9960
26				4.5000	0.0300	400.00	0.1000	0.5000	0.9960
28				4.5000	0.0300	200.00	0.2000	0.5000	0.9960
34				5.0000	2.0000	400.00	0.0500	0.3000	0.9960
44				5.0000	0.8000	400.00	0.0500	0.3000	0.9960
45				7.5000	0.0200	300.00	0.0700	0.0000	0.9960
54				4.0000	2.0000	100.00	0.0010	0.5000	0.9960
64				4.5000	0.1200	400.00	0.1000	0.5000	0.9990
65				5.0000	0.8000	400.00	0.0500	0.5000	0.9960

END PWAT-PARM2

PWAT-PARM3

<PLS >\*\*\*

#	PETMAX	PETMIN	INFEXP	INFILD	DEEPFR	BASETP	AGWETP
14			2.0000	2.0000	0.00	0.00	0.0
16			2.0000	2.0000	0.00	0.00	0.0
18			2.0000	2.0000	0.00	0.00	0.0
24			2.0000	2.0000	0.00	0.00	0.
26			2.0000	2.0000	0.00	0.	0.
28			2.0000	2.0000	0.00	0.	0.
34			2.0000	2.0000	0.00	0.00	0.0
44			2.0000	2.0000	0.00	0.	0.
45			2.0000	2.0000	0.00	0.	0.
54			10.000	2.0000	0.00	0.	0.7
64			2.0000	2.0000	0.00	0.	0.0

END PWAT-PARM3

PWAT-PARM4

<PLS >

\*\*\*

#	#	CEPSC	UZSN	NSUR	INTFW	IRC	LZETP***
14		0.2000	1.0000	0.3500	2.000	0.1500	0.7000
16		0.2000	0.5000	0.3500	2.000	0.1500	0.7000
18		0.2000	0.3000	0.3500	2.000	0.1500	0.7000
24		0.1000	0.5000	0.2500	2.000	0.1500	0.2500
26		0.1000	0.2500	0.2500	2.000	0.1500	0.2500
28		0.1000	0.1500	0.2500	2.000	0.1500	0.2500
34		0.2000	0.5000	0.3500	0.000	0.5000	0.7000
44		0.1000	0.5000	0.2500	0.000	0.5000	0.2500
45		0.1000	0.2800	0.2500	6.000	0.1500	0.6000
54		0.1000	3.0000	0.5000	1.000	0.7000	0.8000
64		0.1000	0.2500	0.2500	3.000	0.5000	0.2500
65		0.1000	0.5000	0.2500	0.000	0.5000	0.2500

END PWAT-PARM4

PWAT-STATE1

<PLS > PWATER state variables\*\*\*

#	#***	CEPS	SURS	UZS	IFWS	LZS	AGWS	GWVS
14		0.078	0.	0.2500	0.10	2.500	2.00	0.000
16		0.078	0.	0.2500	0.10	2.500	2.00	0.000
18		0.078	0.	0.2500	0.10	2.500	2.00	0.000
24		0.051	0.	0.2500	0.10	2.500	2.00	0.000
26		0.051	0.	0.2500	0.10	2.500	2.00	0.000
28		0.051	0.	0.2500	0.10	2.500	2.00	0.000
34		0.078	0.	0.2500	0.10	0.000	2.00	0.000
44		0.051	0.	0.2500	0.10	0.000	2.00	0.000
45		0.051	0.	0.2500	0.10	0.000	2.00	0.000
54		0.051	0.	0.2500	0.10	2.000	2.00	0.000
64		0.051	0.	0.2500	0.10	2.000	20.00	0.000
65		0.051	0.	0.2500	0.10	0.000	20.00	0.000

END PWAT-STATE1

END PERLND

IMPLND

GEN-INFO

<ILS >

# - #

Name

Unit-systems

Printer

\*\*\*

User t-series Engl Metr

\*\*\*

in out

\*\*\*

14	IMPERVIOUS	1	1	1	60	0
----	------------	---	---	---	----	---

END GEN-INFO

ACTIVITY

<ILS > \*\*\*\*\* Active Sections \*\*\*\*

# - # ATMP SNOW IWAT SLD IWG IQAL \*\*\*

14	0	0	1	0	0	0
----	---	---	---	---	---	---

```

END ACTIVITY
PRINT-INFO
<ILS > ***** Print-flags ***** PIVL  PYR
# - # ATMP SNOW IWAT  SLD  IWG IQAL *****
14      0      0      6      0      0      0      1      9
END PRINT-INFO
IWAT-PARM1
<ILS >          Flags          ***
# - # CSNO RTOP  VRS  VNN RTLI  ***
14      0      0      0      0      0
END IWAT-PARM1
IWAT-PARM2
<ILS >          ***
# - #          LSUR          SLSUR          NSUR          RETSC          ***
14          100.00          0.0100          0.1000          0.1000
END IWAT-PARM2
IWAT-PARM3
<ILS >          ***
# - #          PETMAX          PETMIN          ***
14
END IWAT-PARM3
IWAT-STATE1
<ILS >  IWATER state variables          ***
# - #          RETS          SURS          ***
14          1.0000E-3  1.0000E-3
END IWAT-STATE1
END IMPLND          ***
EXT SOURCES          ***
*** NOTE: The only RCHRES that precip and PET are applied to are lakes.
***
<-Volume-> <Member> SsysSgap<--Mult-->Tran <-Target vols> <-Grp> <-Member->          ***
<Name> # <Name> # tem strg<-factor->strg <Name> # #          <Name> # #          ***
*** PRECIP/EVAP TO PERVIOUS/IMPERV SURFACES
WDM  1002  PREC          ENGLZERO  1.00          PERLND  14  200  EXTNL  PREC
WDM  1002  PREC          ENGLZERO  1.00          IMPLND  14          EXTNL  PREC
WDM    1  EVAP          ENGLZERO  0.8          PERLND  14  65  EXTNL  PETINP
WDM    1  EVAP          ENGLZERO  0.8          IMPLND  14          EXTNL  PETINP
*** PRECIP/EVAP TO LAKES
WDM  1002  PREC          ENGLZERO          RCHRES  20          EXTNL  PREC
WDM    1  EVAP          ENGLZERO  0.8          RCHRES  20          EXTNL  POTEV
END EXT SOURCES          ***
EXT TARGETS
<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Volume-> <Member> Tsys Tgap Amd ***
<Name> #          <Name> # #<-factor->strg <Name> # <Name>          tem strg strg***
***WALKER NR MTH
RCHRES  18  HYDR  RO          WDM  96  FLOW          ENGL          REPL
*** MISC (20=WALKER WETLAND, 55=SR509, 56=1ST AVE)
RCHRES  20  HYDR  RO          WDM  97  FLOW          ENGL          REPL
COPY *** 2  OUTPUT MEAN  1  1          12.1          WDM  89  FLOW          ENGL          REPL
END EXT TARGETS          ***
SCHEMATIC
<-Source->          <--Area-->          <-Target->          MBLK          ***

```

<Name>	#	<-factor->	<Name>	#	Tbl#	***
***WALKER CREEK						
*** SUB-CATCHMENT MC 8						
PERLND	26	4.10	COPY	1	8	
PERLND	44	18.57	COPY	1	8	
PERLND	54	2.72	COPY	1	8	
IMPLND	14	1.34	COPY	1	9	
*** ** SUB-CATCHMENT SDW2 10-75-15 PREDEVELOPMENT W/1994 IMP						
***PERLND	16	30.91	COPY	2	10	
***PERLND	26	7.16	COPY	2	10	
***PERLND	34	1.69	COPY	2	10	
***PERLND	44	0.39	COPY	2	10	
***PERLND	54	1.13	COPY	2	10	
***PERLND	16	30.91	COPY	1	11	
***PERLND	26	7.16	COPY	1	11	
***PERLND	34	1.69	COPY	1	11	
***PERLND	44	0.39	COPY	1	11	
***PERLND	54	1.13	COPY	1	11	
***IMPLND	14	3.31	COPY	2	9	
*** SUB-CATCHMENT SDW2 10-75-15 PREDEVELOPMENT W/1994 IMP						
PERLND	16	16.38	COPY	2	10	
PERLND	26	4.37	COPY	2	10	
PERLND	34	16.47	COPY	2	10	
PERLND	44	4.39	COPY	2	10	
PERLND	54	1.05	COPY	2	10	
PERLND	16	16.38	COPY	1	11	
PERLND	26	4.37	COPY	1	11	
PERLND	34	16.47	COPY	1	11	
PERLND	44	4.39	COPY	1	11	
PERLND	54	1.05	COPY	1	11	
IMPLND	14	2.20	COPY	2	9	
*** SUB-CATCHMENT MC 9						
PERLND	26	9.44	COPY	1	8	
PERLND	44	0.74	COPY	1	8	
PERLND	54	0.00	COPY	1	8	***
IMPLND	14	0.24	COPY	1	9	
*** SUB-CATCHMENT 18						
PERLND	16	0.76	RCHRES	18	1	
PERLND	26	16.08	RCHRES	18	1	
PERLND	34	20.95	RCHRES	18	1	
PERLND	44	49.22	RCHRES	18	1	
IMPLND	14	3.30	RCHRES	18	2	
*** SUB-CATCHMENT 19						
PERLND	16	12.72	RCHRES	19	1	
PERLND	26	92.07	RCHRES	19	1	
PERLND	34	8.39	RCHRES	19	1	
PERLND	44	95.55	RCHRES	19	1	
IMPLND	14	30.53	RCHRES	19	2	
*** SUB-CATCHMENT 20						
PERLND	26	12.53	RCHRES	20	1	
PERLND	44	53.43	RCHRES	20	1	
PERLND	54	33.43	RCHRES	20	1	
IMPLND	14	52.83	RCHRES	20	2	
*** DOWN STREAM OF WALKER CREEK GAGE						
*** SUB-CATCHMENT 21 33% OF GW GOES TO GAGE REST GOES TO SOUND						
PERLND	16	2.54	RCHRES	18	7	

PERLND	26	44.30	RCHRES	18	7
PERLND	34	2.03	RCHRES	18	7
PERLND	44	41.13	RCHRES	18	7
PERLND	16	2.54	RCHRES	21	6
PERLND	26	44.30	RCHRES	21	6
PERLND	34	2.03	RCHRES	21	6
PERLND	44	41.13	RCHRES	21	6
PERLND	16	5.07	RCHRES	21	1
PERLND	26	88.61	RCHRES	21	1
PERLND	34	4.06	RCHRES	21	1
PERLND	44	82.26	RCHRES	21	1
IMPLND	14	33.09	RCHRES	21	2
IMPLND	14	16.54	RCHRES	21	2

\*\*\* SUB-CATCHMENT 22

PERLND	34	4.30	RCHRES	22	1
PERLND	44	19.49	RCHRES	22	1
PERLND	54	3.21	RCHRES	22	1
IMPLND	14	3.95	RCHRES	22	2

\*\*\*GROUNDWATER FROM OUTSIDE OF WALKER CREEK

PERLND	64	630.00	RCHRES	20	7
PERLND	65	*** 130.00	RCHRES	20	7

\*\*\*STREAM ROUTING

COPY	2		COPY	3	14
COPY	1		COPY	3	14
COPY	3		RCHRES	20	13
RCHRES	20		RCHRES	19	3
RCHRES	19		RCHRES	18	3

END SCHEMATIC

NETWORK

```

***          <MEMBER> SSSYSSGAP<--MULT-->TRAN <-TARGET VOLS>          <-MEMBER-->
<NAME> # <NAME>   TEM STRG<-FACTOR-->STRG <NAME> # # <-GRP> <NAME> # #
***
***

```

END NETWORK

RCHRES

GEN-INFO

RCHRES	Name	Nexits	Unit	Systems	Printer					
# - #	<----->	<---->	User	T-series	Engl	Metr	LKFG			
			in	out						
18	Trib (0371A) M 18	1	1	1	1	62	0	0		
19	Trib (0371A) M 19	1	1	1	1	62	0	0		
20	Trib M 20	1	1	1	1	62	0	1		
21	Trib (0371H) M 21	1	1	1	1	62	0	0		
22	Trib (0371A) M 22	1	1	1	1	62	0	0		

END GEN-INFO

ACTIVITY

```

RCHRES ***** Active Sections *****
# - # HYFG ADFG CNFG HTFG SDFG GQFG OXFG NUFG PKFG PHFG
1 63 1 0 0 0 0 0 0 0 0 0 0

```



END ACTIVITY

PRINT-INFO

```

RCHRES ***** Printout Flags ***** PIVL  PYR
# - # HYDR ADCA CONS HEAT SED  GOL OXRX NUTR PLNK PHCB *****
1   63   5   0   0   0   0   0   0   0   0   0   0   1   9

```

END PRINT-INFO

HYDR-PARM1

```

RCHRES  Flags for each HYDR Section                                     ***
# - #   VC A1 A2 A3  ODFVFG for each *** ODGTFG for each  FUNCT for each
      FG FG FG FG  possible exit *** possible exit  possible exit
      * * * *   * * * *   * * * *   * * * *   * * * *
1   99   0  1  0  0   4  0  0  0  0   0  0  0  0  0   2  2  2  2  2

```

END HYDR-PARM1

HYDR-PARM2

```

RCHRES                                     ***
# - #   FTABNO      LEN      DELTH      STCOR      KS      DB50      ***
<-----><-----><-----><-----><-----><-----><----->
18          18   0.800                                0.3
19          19   0.568                                0.3
20          20   0.379                                0.3
21          21   0.450                                0.3
22          22   0.300                                0.3

```

END HYDR-PARM2

HYDR-INIT

```

RCHRES  Initial conditions for each HYDR section                       ***
# - #   *** VOL      Initial value of COLIND      Initial value of OUTDGT
      *** ac-ft      for each possible exit      for each possible exit
<-----><----->      <-----><-----><-----><-----> *** <-----><-----><-----><----->
18     0.1                                4.0
19     0.1                                4.0
20    10.0                                4.0
21     0.1                                4.0
22     0.1                                4.0

```

END HYDR-INIT

END RCHRES

FTABLES

```

FTABLE      18
ROWS COLS ***
  3      4
  DEPTH      AREA      VOLUME      OUTFLOW ***
  0.00      1.30      0.00      0.00
  1.00      1.30      1.30      166.00
  2.00      1.40      2.65      490.00
END FTABLE 18

```

```

FTABLE      19
ROWS COLS ***
  3      4
  DEPTH      AREA      VOLUME      OUTFLOW ***
  0.00      1.10      0.00      0.00
  1.00      1.10      1.10      65.00
  2.00      1.20      2.25      223.00
END FTABLE 19

```

FTABLE 20

\*\*\* WALKER CREEK WETLAND

ROWS COLS \*\*\*

10 4

DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW ***
0.00	0.00	0.00	0.00	
1.00	2.50	1.25	7.04	
2.00	5.00	5.00	17.84	
3.00	12.00	13.50	32.17	
4.00	19.00	29.00	45.13	
5.00	22.00	49.50	54.95	
6.00	23.00	72.00	61.62	
6.10	23.00	74.30	62.15	
7.00	23.50	95.25	67.00	
7.24	24.10	101.10	100.00	

END FTABLE 20

FTABLE 21

ROWS COLS \*\*\*

8 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.2259	0.0113	0.11	
0.500	0.2707	0.1106	4.27	
1.000	0.3268	0.2600	15.13	
1.500	0.3828	0.4374	31.67	
2.000	0.4389	0.6428	54.02	
2.500	0.4949	0.8763	82.52	
3.000	0.5510	1.1377	117.55	

END FTABLE 21

FTABLE 22

ROWS COLS \*\*\*

9 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.3680	0.0184	0.25	
0.500	0.3717	0.1664	9.39	
1.000	0.3763	0.3534	31.06	
2.000	0.3819	0.7325	94.37	
3.000	0.3874	1.1171	174.33	
4.000	0.3930	1.5073	265.38	
5.000	0.3985	1.9030	364.68	
6.000	0.4040	2.3043	470.60	

END FTABLE 22

END FTABLES

MASS-LINK

<Volume> <-Grp> <-Member-><--Mult--> <Target> <-Grp> <-Member->\*\*\*  
 <Name> <Name> # #<-factor-> <Name> <Name> # #\*\*\*

MASS-LINK 1

conversion from acre-inches to acre-ft (1/12)

\*\*\*

PERLND PWATER PERO 0.0833333 RCHRES INFLOW IVOL

END MASS-LINK 1

MASS-LINK 2

IMPLND IWATER SURO 0.0833333 RCHRES INFLOW IVOL

END MASS-LINK 2

MASS-LINK		3						
RCHRES	ROFLOW				RCHRES		INFLOW	
END MASS-LINK		3						
MASS-LINK		4						
RCHRES	OFLOW	OVOL	1		RCHRES		INFLOW	IVOL
END MASS-LINK		4						
MASS-LINK		5						
RCHRES	OFLOW	OVOL	2		RCHRES		INFLOW	IVOL
END MASS-LINK		5						
MASS-LINK		6						
PERLND	PWATER	SURO		0.0833333	RCHRES		INFLOW	IVOL
PERLND	PWATER	IFWO		0.0833333	RCHRES		INFLOW	IVOL
END MASS-LINK		6						
MASS-LINK		7						
PERLND	PWATER	AGWO		0.0833333	RCHRES		INFLOW	IVOL
END MASS-LINK		7						
MASS-LINK		8						
PERLND	PWATER	PERO		0.0833333	COPY		INPUT	MEAN
END MASS-LINK		8						
MASS-LINK		9						
IMPLND	IWATER	SURO		0.0833333	COPY		INPUT	MEAN
END MASS-LINK		9						
MASS-LINK		10						
PERLND	PWATER	SURO		0.0833333	COPY		INPUT	MEAN
PERLND	PWATER	IFWO		0.0833333	COPY		INPUT	MEAN
END MASS-LINK		10						
MASS-LINK		11						
PERLND	PWATER	AGWO		0.0833333	COPY		INPUT	MEAN
END MASS-LINK		11						
MASS-LINK		12						
PERLND	PWATER	AGWO		0.0833333	COPY		INPUT	MEAN
END MASS-LINK		12						
MASS-LINK		13						
COPY	OUTPUT	MEAN			RCHRES		INFLOW	IVOL
END MASS-LINK		13						
MASS-LINK		14						
COPY	OUTPUT	MEAN			COPY		INPUT	MEAN
END MASS-LINK		14						
END MASS-LINK								
END RUN								

```

RUN
GLOBAL
*** COPY COMMAND ADDED
*** FILE: WCPREDT.inp REVISED OCTOBER 2000
*** SEATAC AIRPORT HSPF BASIN MODEL OF WALKER CREEK
*** CALIBRATION FILE USING FULL LENGTH RUN 1990 DATA FOR INTITIAL CONDITIONS
WALKER CREEK BASIN HSPF MODEL
START      1990 10  1  0  0  END      1994  9 30 24  0
RUN INTERP OUTPUT LEVEL      3
RESUME     0 RUN      1
END GLOBAL

```

```

FILES
<type> <fun>***<-----fname----->
MESSU    24  D:\PARA\SEATAC\MILLER\LOWFLOW\WALKER.MES
WDM       25  D:\PARA\SEATAC\MILLER\LOWFLOW\mlowflow.WDM
          61  D:\PARA\SEATAC\MILLER\LOWFLOW\wPER.L61
          62  D:\PARA\SEATAC\MILLER\LOWFLOW\WRCH.L62
END FILES

```

```

OPN SEQUENCE
  INGRP                INDELT 01:00
    PERLND             14
    PERLND             16
    PERLND             18
    PERLND             24
    PERLND             26
    PERLND             28
    PERLND             34
    PERLND             44
    PERLND             45
    PERLND             54
    PERLND             64
    PERLND             65
    IMPLND             14
    COPY               2
    COPY               1
    COPY               3
    RCHRES             20
    RCHRES             19
    RCHRES             18
  END INGRP
END OPN SEQUENCE

```

```

COPY
TIMESERIES
Copy-opn
# - # NPT NMN
1  5  1
END TIMESERIES
END COPY

```

```

PERLND
GEN-INFO
<PLS >      Name      NBLKS  Unit-systems  Printer
# - #              User  t-series  Engl Metr
              in  out

```

14	TFF- TILL FOR FLT	1	1	1	1	61	0
16	TFM- TILL FOR MOD	1	1	1	1	61	0
18	TFS- TILL FOR STP	1	1	1	1	61	0
24	TGF- TILL GR FLT	1	1	1	1	61	0
26	TGM- TILL GR MOD	1	1	1	1	61	0
28	TGS- TILL GR STP	1	1	1	1	61	0
34	OF - OUTWASH FOR	1	1	1	1	61	0
44	OG - OUTWASH GR	1	1	1	1	61	0

\*\*\*PERLND FOR NEW AIRPORT FILL; NONE IN CALIBRATION

45	AIRPORT FILL	1	1	1	1	61	0
54	SA - WETLANDS	1	1	1	1	61	0
64	TGM DES MOINES	1	1	1	1	61	0
65	OG DES MOINES	1	1	1	1	61	0

END GEN-INFO

ACTIVITY

<PLS > \*\*\*\*\* Active Sections \*\*\*\*\*

#	-	#	ATMP	SNOW	PWAT	SED	PST	PWG	PQAL	MSTL	PEST	NITR	PHOS	TRAC	***
14	-	200	0	0	1	0	0	0	0	0	0	0	0	0	0

END ACTIVITY

PRINT-INFO

<PLS > \*\*\*\*\* Print-flags \*\*\*\*\* PIVL PYR

#	-	#	ATMP	SNOW	PWAT	SED	PST	PWG	PQAL	MSTL	PEST	NITR	PHOS	TRAC	*****	PIVL	PYR
14	-	200	0	0	5	0	0	0	0	0	0	0	0	0	1	9	

END PRINT-INFO

PWAT-PARM1

<PLS > \*\*\*\*\* Flags \*\*\*\*\*

#	-	#	CSNO	RTOP	UZFG	VCS	VUZ	VNN	VIFW	VIRC	VLE	***
14	-		0	0	0	0	0	0	0	0	0	
16	-		0	0	0	0	0	0	0	0	0	
18	-		0	0	0	0	0	0	0	0	0	
24	-		0	0	0	0	0	0	0	0	0	
26	-		0	0	0	0	0	0	0	0	0	
28	-		0	0	0	0	0	0	0	0	0	
34	-		0	0	0	0	0	0	0	0	0	
44	-		0	0	0	0	0	0	0	0	0	
45	-		0	0	0	0	0	0	0	0	0	
54	-		0	0	0	0	0	0	0	0	0	
64	-		0	0	0	0	0	0	0	0	0	

END PWAT-PARM1

PWAT-PARM2

<PLS > \*\*\*

#	-	#	***FOREST	LZSN	INFILT	LSUR	SLSUR	KVARY	AGWRC
14	-			4.5000	0.0800	400.00	0.0500	0.5000	0.9960
16	-			4.5000	0.0800	400.00	0.1000	0.5000	0.9960
18	-			4.5000	0.0800	200.00	0.2000	0.5000	0.9960
24	-			4.5000	0.0300	400.00	0.0500	0.5000	0.9960
26	-			4.5000	0.0300	400.00	0.1000	0.5000	0.9960
28	-			4.5000	0.0300	200.00	0.2000	0.5000	0.9960
34	-			5.0000	2.0000	400.00	0.0500	0.3000	0.9960
44	-			5.0000	0.8000	400.00	0.0500	0.3000	0.9960
45	-			7.5000	0.0200	300.00	0.0700	0.0000	0.9960
54	-			4.0000	2.0000	100.00	0.0010	0.5000	0.9960
64	-			4.5000	0.1200	400.00	0.1000	0.5000	0.9990
65	-			5.0000	0.8000	400.00	0.0500	0.5000	0.9960

END PWAT-PARM2

PWAT-PARM3

<PLS >\*\*\*

#	****	PETMAX	PETMIN	INFEXP	INFILD	DEEPPFR	BASETP	AGWETP
14				2.0000	2.0000	0.00	0.00	0.0
16				2.0000	2.0000	0.00	0.00	0.0
18				2.0000	2.0000	0.00	0.00	0.0
24				2.0000	2.0000	0.00	0.00	0.
26				2.0000	2.0000	0.00	0.	0.
28				2.0000	2.0000	0.00	0.	0.
34				2.0000	2.0000	0.00	0.00	0.0
44				2.0000	2.0000	0.00	0.	0.
45				2.0000	2.0000	0.00	0.	0.
54				10.000	2.0000	0.00	0.	0.7
64				2.0000	2.0000	0.00	0.	0.0

END PWAT-PARM3

PWAT-PARM4

<PLS > \*\*\*

#	#	CEPSC	UZSN	NSUR	INTFW	IRC	LZETP***
14		0.2000	1.0000	0.3500	2.000	0.1500	0.7000
16		0.2000	0.5000	0.3500	2.000	0.1500	0.7000
18		0.2000	0.3000	0.3500	2.000	0.1500	0.7000
24		0.1000	0.5000	0.2500	2.000	0.1500	0.2500
26		0.1000	0.2500	0.2500	2.000	0.1500	0.2500
28		0.1000	0.1500	0.2500	2.000	0.1500	0.2500
34		0.2000	0.5000	0.3500	0.000	0.5000	0.7000
44		0.1000	0.5000	0.2500	0.000	0.5000	0.2500
45		0.1000	0.2800	0.2500	6.000	0.1500	0.6000
54		0.1000	3.0000	0.5000	1.000	0.7000	0.8000
64		0.1000	0.2500	0.2500	3.000	0.5000	0.2500
65		0.1000	0.5000	0.2500	0.000	0.5000	0.2500

END PWAT-PARM4

PWAT-STATE1

<PLS > PWATER state variables\*\*\*

#	****	CEPS	SURS	UZS	IFWS	LZS	AGWS	GWVS
14		0.000	0.	0.0000	0.00	2.500	2.00	0.000
16		0.000	0.	0.0010	0.00	0.020	3.51	0.047
18		0.000	0.	0.0000	0.00	2.500	2.00	0.000
24		0.000	0.	0.0000	0.00	2.500	2.00	0.000
26		0.000	0.	0.0000	0.00	1.598	2.81	0.041
28		0.000	0.	0.0000	0.00	2.500	2.00	0.000
34		0.000	0.	0.0000	0.00	0.023	5.66	0.084
44		0.000	0.	0.0000	0.00	2.756	6.25	0.134
45		0.000	0.	0.0000	0.00	0.373	3.01	0.000
54		0.000	0.	0.3650	0.00	0.561	0.00	0.000
64		0.000	0.	0.0000	0.00	1.978	22.28	0.000
65		0.000	0.	0.0000	0.00	0.000	20.00	0.000

END PWAT-STATE1

END PERLND

IMPLND

GEN-INFO

<ILS > Name Unit-systems Printer \*\*\*

#	#	User	t-series	Engl	Metr	<span style="float:right">***</span>
		in out				<span style="float:right">***</span>
14	IMPERVIOUS	1	1 1	60	0	<span style="float:right">***</span>

END GEN-INFO

ACTIVITY

<ILS > \*\*\*\*\* Active Sections \*\*\*\*\*

#	#	ATMP	SNOW	IWAT	SLD	IWG	IQAL	***
14		0	0	1	0	0	0	

```

END ACTIVITY
PRINT-INFO
  <ILS > ***** Print-flags ***** PIVL  PYR
  # - # ATMP SNOW IWAT SLD IWG IQAL *****
  14  0  0  6  0  0  0  1  9
END PRINT-INFO
IWAT-PARM1
  <ILS >                Flags                ***
  # - # CSNO RTOP  VRS  VNN RTLI  ***
  14  0  0  0  0  0
END IWAT-PARM1
IWAT-PARM2
  <ILS >                ***
  # - #          LSUR      SLSUR      NSUR      RETSC      ***
  14  100.00  0.0100  0.1000  0.1000
END IWAT-PARM2
IWAT-PARM3
  <ILS >                ***
  # - #          PETMAX    PETMIN      ***
  14
END IWAT-PARM3
IWAT-STATE1
  <ILS >  IWATER state variables      ***
  # - #          RETS      SURS      ***
  14  1.0000E-3  1.0000E-3
END IWAT-STATE1
END IMPLND
EXT SOURCES
*** NOTE: The only RCHRES that precip and PET are applied to are lakes.
***
<-Volume-> <Member> SsysSgap<--Mult-->Tran <-Target vols> <-Grp> <-Member->
<Name> # <Name> # tem strg<-factor->strg <Name> # # <Name> # #
*** PRECIP/EVAP TO PERVIOUS/IMPERV SURFACES
WDM  1002 PREC      ENGLZERO  1.00      PERLND  14 200 EXTNL  PREC
WDM  1002 PREC      ENGLZERO  1.00      IMPLND  14      EXTNL  PREC
WDM  1 EVAP      ENGLZERO  0.8      PERLND  14 65 EXTNL  PETINP
WDM  1 EVAP      ENGLZERO  0.8      IMPLND  14      EXTNL  PETINP
*** PRECIP/EVAP TO LAKES
WDM  1002 PREC      ENGLZERO      RCHRES  20      EXTNL  PREC
WDM  1 EVAP      ENGLZERO  0.8      RCHRES  20      EXTNL  POTEV
END EXT SOURCES
EXT TARGETS
<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Volume-> <Member> Tsys Tgap Amd ***
<Name> # <Name> # #<-factor->strg <Name> # <Name> tem strg strg***
***WALKER NR MTH
RCHRES 18 HYDR RO      ***      WDM  96 FLOW      ENGL      REPL
*** MISC (20=WALKER WETLAND, 55=SR509, 56=1ST AVE)
RCHRES 20 HYDR RO      WDM  8097 FLOW      ENGL      REPL
COPY *** 2 OUTPUT MEAN 1 1 12.1 WDM  89 FLOW      ENGL      REPL
END EXT TARGETS
SCHEMATIC
<-Source->                <--Area-->      <-Target->      MBLK      ***

```

<Name>	#	<-factor->	<Name>	#	Tbl#	***
***WALKER CREEK						
*** SUB-CATCHMENT MC 8						
PERLND	26	4.10	COPY	1	8	
PERLND	44	18.57	COPY	1	8	
PERLND	54	2.72	COPY	1	8	
IMPLND	14	1.34	COPY	1	9	
*** *** SUB-CATCHMENT SDW2 10-75-15 PREDEVELOPMENT W/1994 IMP						
***PERLND	16	30.91	COPY	2	10	
***PERLND	26	7.16	COPY	2	10	
***PERLND	34	1.69	COPY	2	10	
***PERLND	44	0.39	COPY	2	10	
***PERLND	54	1.13	COPY	2	10	
***PERLND	16	30.91	COPY	1	11	
***PERLND	26	7.16	COPY	1	11	
***PERLND	34	1.69	COPY	1	11	
***PERLND	44	0.39	COPY	1	11	
***PERLND	54	1.13	COPY	1	11	
***IMPLND	14	3.31	COPY	2	9	
*** SUB-CATCHMENT SDW2 10-75-15 PREDEVELOPMENT W/1994 IMP						
PERLND	16	16.38	COPY	2	10	
PERLND	26	4.37	COPY	2	10	
PERLND	34	16.47	COPY	2	10	
PERLND	44	4.39	COPY	2	10	
PERLND	54	1.05	COPY	2	10	
PERLND	16	16.38	COPY	1	11	
PERLND	26	4.37	COPY	1	11	
PERLND	34	16.47	COPY	1	11	
PERLND	44	4.39	COPY	1	11	
PERLND	54	1.05	COPY	1	11	
IMPLND	14	2.20	COPY	2	9	
*** SUB-CATCHMENT MC 9						
PERLND	26	9.44	COPY	1	8	
PERLND	44	0.74	COPY	1	8	
PERLND	54	0.00	COPY	1	8	***
IMPLND	14	0.24	COPY	1	9	
*** SUB-CATCHMENT 18						
PERLND	16	0.76	RCHRES	18	1	
PERLND	26	16.08	RCHRES	18	1	
PERLND	34	20.95	RCHRES	18	1	
PERLND	44	49.22	RCHRES	18	1	
IMPLND	14	3.30	RCHRES	18	2	
*** SUB-CATCHMENT 19						
PERLND	16	12.72	RCHRES	19	1	
PERLND	26	92.07	RCHRES	19	1	
PERLND	34	8.39	RCHRES	19	1	
PERLND	44	95.55	RCHRES	19	1	
IMPLND	14	30.53	RCHRES	19	2	
*** SUB-CATCHMENT 20						
PERLND	26	12.53	RCHRES	20	1	
PERLND	44	53.43	RCHRES	20	1	
PERLND	54	33.43	RCHRES	20	1	
IMPLND	14	52.83	RCHRES	20	2	
*** DOWN STREAM OF WALKER CREEK GAGE						
*** SUB-CATCHMENT 21 33% OF GW GOES TO GAGE REST GOES TO SOUND						
PERLND	16	2.54	RCHRES	18	7	



```

PERLND 26          44.30      RCHRES 18      7
PERLND 34          2.03      RCHRES 18      7
PERLND 44          41.13      RCHRES 18      7
PERLND 16          2.54      RCHRES 21      6
PERLND 26          44.30      RCHRES 21      6
PERLND 34          2.03      RCHRES 21      6
PERLND 44          41.13      RCHRES 21      6
PERLND 16          5.07      RCHRES 21      1
PERLND 26          88.61      RCHRES 21      1
PERLND 34          4.06      RCHRES 21      1
PERLND 44          82.26      RCHRES 21      1
IMPLND 14          33.09      RCHRES 21      2
IMPLND 14          16.54      RCHRES 21      2

```

\*\*\* SUB-CATCHMENT 22

```

PERLND 34          4.30      RCHRES 22      1
PERLND 44          19.49     RCHRES 22      1
PERLND 54          3.21      RCHRES 22      1
IMPLND 14          3.95      RCHRES 22      2

```

\*\*\*GROUNDWATER FROM OUTSIDE OF WALKER CREEK

```

PERLND 64          630.00     RCHRES 20      7
PERLND 65          *** 130.00     RCHRES 20      7

```

\*\*\*STREAM ROUTING

```

COPY      2          COPY      3      14
COPY      1          COPY      3      14
COPY      3          RCHRES 20      13
RCHRES 20          RCHRES 19      3
RCHRES 19          RCHRES 18      3

```

END SCHEMATIC

NETWORK

```

***          <MEMBER> SSSYSSGAP<--MULT-->TRAN <-TARGET VOLLS>          <-MEMBER->
<NAME> # <NAME> TEM STRG<--FACTOR-->STRG <NAME> # # <-GRP> <NAME> # # ***
***

```

END NETWORK

RCHRES

GEN-INFO

```

RCHRES          Name          Nexits  Unit Systems  Printer          ***
# - #<-----><----> User T-series  Engr Metr LKFG          ***
          in out          ***
18  Trib (0371A) M 18          1  1  1  1  62  0  0
19  Trib (0371A) M 19          1  1  1  1  62  0  0
20  Trib M 20                  1  1  1  1  62  0  1
21  Trib (0371H) M 21          1  1  1  1  62  0  0
22  Trib (0371A) M 22          1  1  1  1  62  0  0

```

END GEN-INFO

ACTIVITY

```

RCHRES ***** Active Sections *****
# - # HYFG ADFG CNFG HTFG SDFG GQFG OXFG NUFQ PKFG PHFG          ***
1  63  1  0  0  0  0  0  0  0  0  0  0

```



\*\*\* WALKER CREEK WETLAND

ROWS COLS \*\*\*

10 4

DEPTH	AREA	VOLUME	OUTFLOW	OUTFLOW ***
0.00	0.00	0.00	0.00	
1.00	2.50	1.25	7.04	
2.00	5.00	5.00	17.84	
3.00	12.00	13.50	32.17	
4.00	19.00	29.00	45.13	
5.00	22.00	49.50	54.95	
6.00	23.00	72.00	61.62	
6.10	23.00	74.30	62.15	
7.00	23.50	95.25	67.00	
7.24	24.10	101.10	100.00	

END FTABLE 20

FTABLE 21

ROWS COLS \*\*\*

8 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.2259	0.0113	0.11	
0.500	0.2707	0.1106	4.27	
1.000	0.3268	0.2600	15.13	
1.500	0.3828	0.4374	31.67	
2.000	0.4389	0.6428	54.02	
2.500	0.4949	0.8763	82.52	
3.000	0.5510	1.1377	117.55	

END FTABLE 21

FTABLE 22

ROWS COLS \*\*\*

9 4

DEPTH	AREA	VOLUME	OUTFLOW	***
0.000	0.0000	0.0000	0.00	
0.100	0.3680	0.0184	0.25	
0.500	0.3717	0.1664	9.39	
1.000	0.3763	0.3534	31.06	
2.000	0.3819	0.7325	94.37	
3.000	0.3874	1.1171	174.33	
4.000	0.3930	1.5073	265.38	
5.000	0.3985	1.9030	364.68	
6.000	0.4040	2.3043	470.60	

END FTABLE 22

END FTABLES

MASS-LINK

<Volume> <-Grp> <-Member-><--Mult--> <Target> <-Grp> <-Member->\*\*\*  
 <Name> <Name> # #<-factor-> <Name> <Name> # #\*\*\*

MASS-LINK 1

conversion from acre-inches to acre-ft (1/12)

PERLND PWATER PERO 0.0833333 RCHRES

\*\*\*  
 INFLOW IVOL

END MASS-LINK 1

MASS-LINK 2

IMPLND IWATER SURO 0.0833333 RCHRES

INFLOW IVOL

END MASS-LINK 2

MASS-LINK	3					
RCHRES	ROFLOW			RCHRES		INFLOW
END MASS-LINK	3					
MASS-LINK	4					
RCHRES	OFLOW	OVOL	1	RCHRES		INFLOW IVOL
END MASS-LINK	4					
MASS-LINK	5					
RCHRES	OFLOW	OVOL	2	RCHRES		INFLOW IVOL
END MASS-LINK	5					
MASS-LINK	6					
PERLND	PWATER	SURO	0.0833333	RCHRES		INFLOW IVOL
PERLND	PWATER	IFWO	0.0833333	RCHRES		INFLOW IVOL
END MASS-LINK	6					
MASS-LINK	7					
PERLND	PWATER	AGWO	0.0833333	RCHRES		INFLOW IVOL
END MASS-LINK	7					
MASS-LINK	8					
PERLND	PWATER	PERO	0.0833333	COPY		INPUT MEAN
END MASS-LINK	8					
MASS-LINK	9					
IMPLND	IWATER	SURO	0.0833333	COPY		INPUT MEAN
END MASS-LINK	9					
MASS-LINK	10					
PERLND	PWATER	SURO	0.0833333	COPY		INPUT MEAN
PERLND	PWATER	IFWO	0.0833333	COPY		INPUT MEAN
END MASS-LINK	10					
MASS-LINK	11					
PERLND	PWATER	AGWO	0.0833333	COPY		INPUT MEAN
END MASS-LINK	11					
MASS-LINK	12					
PERLND	PWATER	AGWO	0.0833333	COPY		INPUT MEAN
END MASS-LINK	12					
MASS-LINK	13					
COPY	OUTPUT	MEAN		RCHRES		INFLOW IVOL
END MASS-LINK	13					
MASS-LINK	14					
COPY	OUTPUT	MEAN		COPY		INPUT MEAN
END MASS-LINK	14					
END MASS-LINK						
END RUN						